March 16, 2026 – 6:00 PM REGULAR BOARD MEETING

Lehigh Acres Municipal Services Improvement District
Barrett Room
601 East County Lane
Lehigh Acres, FL 33936

This meeting is open to the general public.

REGULAR MEETING AGENDA

1) Preliminaries
- **A.** Call to Order
- **B.** Roll Call
- **C.** Invocation & Pledge of Allegiance
- **D.** Move, Remove and Add Agenda Items
- **E.** Approval of Agenda
- **F.** Public Comment for Agenda Items and Non-agenda Items
- **G.** Action on Removed Consent Items
- **H.** Approval of Consent Agenda

2) Consent Agenda
- **A.** Invoices and Change Orders
- **B.** Approval of Minutes – January 23, 2026, Regular Meeting
- **C.** PER #23-07(M1) - Ibis Landing Minor Modification
- **D.** PER #25-40(M1) - Blackstone Corp Park Minor Modification
- **E.** PER #25-85 - Beth Stacey Blvd. Extension
- **F.** PER #25-94 - Riverhall 2nd Gated Canal Crossing
- **G.** PER #26-16 - Comcast Bore along S. Boundary Canal
- **H.** PER #26-17 - Comcast Bore under Dog Canal at NW 16th Ter
- **I.** PER #26-40 - Chick-Fil-A - 1110 Homestead Rd N
- **J.** PER #26-41 - Ray Ave. Gas Sta. - 602 Meadow Rd

3) Action Agenda
- **A.** Good Cause
- **B.** 2024/2025 Independent Financial Audit - Purvis, Gray & Company
- **C.** Joaquin Garcia, Right of Way Issue
- **D.** RES 26-05, Change to Investment Policy
- **E.** ROV Purchase
- **F.** Dump Truck Purchase

4) Discussion Agenda
- **A.** Old Business
  1. 2026 Legislative Appropriation
  2. Leeland Bottomless Lake Building
- **B.** New Business
- **C.** Commissioner's Request
- **D.** Engineer's Report
- **E.** Attorney's Report
- **F.** Staff and Financial Report
- **G.** Treasurer's Report
- **H.** Secretary's Report

5) Adjournment

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: Invoices and Change Orders

Meeting Date: March 16, 2026

Agenda Item # 2A

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

1 Recommended motion: Move to approve all invoices as stated on the Summary Sheet.

2 What the action accomplishes: Approves all invoices that are outside the approval limits of the Treasurer and Manager.

3 Fiscal Impact: As listed on the Summary Sheet.

4 Staff comments:

5 Staff review and recommended approval:

[x] **MANAGER** [x] **ASSISTANT DISTRICT MGR**

[x] **ATTORNEY** [x] **FIELD DIRECTOR**

[x] **ENGINEER** [x] **FINANCE DIRECTOR**

6 Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

Lehigh Acres Municipal Services Improvement District
INVOICES REQUIRING BOARD APPROVAL FOR PAYMENT
Monday March 16th, 2026

***Paid with Procurement Card**

Vendor	Funding Source	Description	Invoice Number	Amount	General	Other
Vendor AIM Engineering	General	**Description** General Engineering through 02/27/26	**Invoice Number** 252166-5	**Amount** $ 3,648.00	3648.00	
Vendor AIM Engineering	General	**Description** 25-2111 LAMSID/ROBUST Phase 3 through 02/27/26- P.O. 1277	**Invoice Number** 252111-7	**Amount** $ 10,737.50	10737.50	
Vendor AIM Engineering	General	**Description** Engineering Estimate for Frank Mann Preserve through 02/27/26 - P.O. #1272	**Invoice Number** 211364-51	**Amount** $ 8,165.00	8165.00	
Vendor AIM Engineering	General	**Description** Proj: 25-2161 LAMSID/Robust Phase 4 through 02/27/26 - P.O. #1283	**Invoice Number** 252161-6	**Amount** $ 3,341.00	3341.00	
Vendor AIM Engineering	General	**Description** Proj: 25-2112 LAMSID/Frank Mann Preserve (GS-10 Concept Plan PH 2) through 02/27/26 - P.O. #1276	**Invoice Number** 252112-2	**Amount** $ 3,488.75	3488.75	
			Total AIM Engineering:	**$ 29,380.25**		
Vendor Lewis Longman Walker	General	**Description** Legislative Representation	**Invoice Number** MCL-167660	**Amount** $ 4,383.01	4383.01	
Vendor Lewis Longman Walker	General	**Description** General Counsel through 02/25/26	**Invoice Number** SCB-167632	**Amount** $ 5,125.00	5125.00	
			Total Lewis Longman Walker:	**$ 9,508.01**		
Vendor Turnbull Environmental, Inc.	General	**Description** Crest Phase III Project 01/01/26 - 02/28/26	**Invoice Number** 9-Crest Phase III	**Amount** $ 241,424.26	241424.26	
			Total Turnbull Environmental, Inc.:	**$ 241,424.26**		
			TOTAL DISBURSEMENTS:	**$ 280,312.52**		
			General Fund Total	$ 280,312.52		
			Other Funding Total	-		
			General and Other Total	**$ 280,312.52**		

AIM Engineering & Surveying, Inc.

Corporate Office
2161 Fowler Street
Suite 100
Fort Myers, FL 33901

239-332-4569
800-226-4569
Fax: 855-731-7971
www.aimengr.com

Successfully providing our clients and the community with quality planning, engineering and surveying since 1980.

Lehigh Acres Municipal Services Improvement District
601 East County Lane
Lehigh Acres, FL 33936

Invoice number 252166-5
Date 03/03/2026

Professional Services through February 27, 2026

Project 241976 - General Engineering FY 2025/2026

Employee Type / Activity		Date	Hours	Rate	Amount
General Engineering					
Engineering Manager					
D. Schroeder Sunshine	Revisions to Alico/Sunshine Roadway extension project list document	02/10/26	1.00	$257.00	$257.00
	February engineering report	02/12/26	1.00	$257.00	$257.00
	Review of board packet & February board meeting	02/23/26	2.00	$257.00	$514.00
Engineer Intern					
M. Pfau	Water quality analysis for BMAP update	02/25/26	2.50	$131.00	$327.50
R. Rosado	February monthly report	02/10/26	0.50	$131.00	$65.50
	Water quality analysis for BMAP update	02/25/26	4.00	$131.00	$524.00
V. Ehrhardt	Water quality analysis for BMAP update	02/25/26	3.00	$131.00	$393.00
		Task Total			**$2,338.00**
Review Permits					
Engineer Intern					
M. Hebert	Permits 26-16 & 26-17	02/10/26	2.00	$131.00	$262.00
	Permit 26-40	02/24/26	3.00	$131.00	$393.00
	Permits 26-40 & 25-18	02/25/26	2.00	$131.00	$262.00
	Permit 26-18	02/26/26	1.00	$131.00	$131.00
	Permits 23-07 & 25-86	02/27/26	1.00	$131.00	$131.00
R. Hiebert	Permit review QC	02/24/26	1.00	$131.00	$131.00
		Task Total			**$1,310.00**

2025/2026 General Engineering Billing Totals:	
Contract Amount	$90,000.00
Total to Date	$45,001.75
Previous Billing	$41,353.75
Total this Period	$3,648.00

Invoice Total **$3,648.00**

INIT DATE
DEPT 1 [illegible] 3/4/26
CODE
DEPT 2 MKC 3/4/26
CODE
MANAGER

→ 01-3130 - 3341
0001-0000-220-6011 257 Alico/Sunshine

ENTERED

AIM Engineering & Surveying, Inc.

Corporate Office
2161 Fowler Street
Suite 100
Fort Myers, FL 33901

239-332-4569
800-226-4569
Fax: 855-731-7971
www.aimengr.com

Successfully providing our clients and the community with quality planning, engineering and surveying since 1980.

Lehigh Acres - Municipal Services Improvement District
601 East County Lane
Lehigh Acres, FL 33936

Invoice number 252111-7
Date 03/03/2026

Project **25-2111 LAMSID / ROBUST PHASE 3 / PO #1277**

Professional services through February 27, 2026

Description	Contract Amount	Percent Complete	Fee Earned to Date	Previous Billing	Current Billed
TASK 1 - DESIGN SERVICES (LS)	295,000.00	14.48	42,712.00	31,974.50	10,737.50
TASK 2 - POST DESIGN SERVICES (LS)	12,000.00	0.00	0.00	0.00	0.00
TASK 3 - CONSTRUCTION PHASE SERVICES (LS)	35,000.00	0.00	0.00	0.00	0.00
Total	342,000.00	12.49	42,712.00	31,974.50	10,737.50

Invoice total **10,737.50**

	INIT	DATE
DEPT 1	VB	3/4/26
CODE	P.O. # 1277 Line 1	
DEPT 2	MSC	3/4/26
CODE		
MANAGER		

ENTERED

AIM Engineering & Surveying, Inc.

Corporate Office
2161 Fowler Street
Suite 100
Fort Myers, FL 33901

239-332-4569
800-226-4569
Fax: 855-731-7971
www.aimengr.com

Successfully providing our clients and the community with quality planning, engineering and surveying since 1980.

Lehigh Acres Municipal Services Improvements District
601 East County Lane
Lehigh Acres, FL 33936

Invoice Number: 211364-51

Date: 03/03/2026

ENTERED

INIT DATE
DEPT 1 DS 3/4/26
CODE P.O. # 1272 line 3
DEPT 2 MSC 3/4/26
CODE
MANAGER

Professional services rendered thru February 27, 2026

Purchase Order # 1272

Engineering Estimate for Frank Mann Preserve

	Task Title	Amount of Compensation	Basis of Compensation	Percent Complete	Fee Earned To Date	Less Previous Billing	Amount Due This Billing
1	Topographic Survey	$11,319.00	Lump Sum	100.00%	$11,319.00	$11,319.00	$0.00
2	Design - electrical services	$12,550.00	Lump Sum	0.00%	$0.00	$0.00	$0.00
3	Design & Construction plans	$76,000.00	Lump Sum	49.82%	$37,863.75	$29,698.75	$8,165.00
		$99,869.00		**49.25%**	**$49,182.75**	**$41,017.75**	**$8,165.00**

Fee Earned To Date	$49,182.75
Less Previous Payments	$41,017.75
Amount Due This Billing	**$8,165.00**

Please remit payment to:
2161 Fowler Street, Suite 100, Fort Myers, FL 33901

Thank you

AIM Engineering & Surveying, Inc.

Corporate Office
2161 Fowler Street
Suite 100
Fort Myers, FL 33901

239-332-4569
800-226-4569
Fax: 855-731-7971
www.aimengr.com

Successfully providing our clients and the community with quality planning, engineering and surveying since 1980.

Lehigh Acres - Municipal Services Improvement District
601 East County Lane
Lehigh Acres, FL 33936

Invoice number 252161-36
Date 03/03/2026

Project **25-2161 LAMSID / ROBUST PHASE 4 / PO #1283**

Professional services through February 27, 2026

Description	Contract Amount	Percent Complete	Fee Earned to Date	Previous Billing	Current Billed
TASK 1 - DESIGN SERVICES (LS)	276,000.00	8.26	22,808.75	19,467.75	3,341.00
TASK 2 - POST DESIGN SERVICES (LS)	11,000.00	0.00	0.00	0.00	0.00
TASK 3 - CONSTRUCTION PHASE SERVICES (LS)	33,000.00	0.00	0.00	0.00	0.00
Total	320,000.00	7.13	22,808.75	19,467.75	3,341.00

Invoice total **3,341.00**

INIT DATE
DEPT 1 JB 3/4/26
CODE P.O. # 1283 Lmct
DEPT 2 MSC 3/4/26
CODE
MANAGER

ENTERED

AIM Engineering & Surveying, Inc.

Corporate Office
2161 Fowler Street
Suite 100
Fort Myers, FL 33901

239-332-4569
800-226-4569
Fax: 855-731-7971
www.aimengr.com

Successfully providing our clients and the community with quality planning, engineering and surveying since 1980.

Lehigh Acres - Municipal Services Improvement District
601 East County Lane
Lehigh Acres, FL 33936

Invoice number 252161-6
Date 03/03/2026

Project **25-2161 LAMSID / ROBUST PHASE 4 / PO #1283**

Professional services through February 27, 2026

Description	Contract Amount	Percent Complete	Fee Earned to Date	Previous Billing	Current Billed
TASK 1 - DESIGN SERVICES (LS)	276,000.00	8.26	22,808.75	19,467.75	3,341.00
TASK 2 - POST DESIGN SERVICES (LS)	11,000.00	0.00	0.00	0.00	0.00
TASK 3 - CONSTRUCTION PHASE SERVICES (LS)	33,000.00	0.00	0.00	0.00	0.00
Total	320,000.00	7.13	22,808.75	19,467.75	3,341.00

Invoice total **3,341.00**

AIM Engineering & Surveying, Inc.

Corporate Office
2161 Fowler Street
Suite 100
Fort Myers, FL 33901

239-332-4569
800-226-4569
Fax: 855-731-7971
www.aimengr.com

Successfully providing our clients and the community with quality planning, engineering and surveying since 1980.

Lehigh Acres - Municipal Services Improvement District
601 East County Lane
Lehigh Acres, FL 33936

Invoice number 252112-2
Date 03/03/2026

Project **25-2112 LAMSID / FRANK MANN PRESERVE (GS-10 CONCEPT PLAN PH 2) / PO 1276**

Professional services through February 27, 2026

Description	Contract Amount	Percent Complete	Fee Earned to Date	Previous Billing	Current Billed
DESIGN SERVICES -- CIVIL (LS)	695,300.00	1.15	7,976.25	4,487.50	3,488.75
POST-DESIGN SERVICES -- CIVIL (LS)	15,000.00	0.00	0.00	0.00	0.00
CONSTRUCTION PHASE SERVICES -- CIVIL (LS)	85,000.00	0.00	0.00	0.00	0.00
APEX (WATER SCIENCE) SUB (LS)	189,700.00	0.00	0.00	0.00	0.00
Total	985,000.00	0.81	7,976.25	4,487.50	3,488.75

Invoice total **3,488.75**

INIT DATE
DEPT 1 3/4/26
CODE P.O.# 1276 Line 1
DEPT 2 3/4/26
CODE
MANAGER

LLW | LEWIS LONGMAN WALKER

360 South Rosemary Avenue
Suite 1100
West Palm Beach, FL 33401
Tel 561-640-0820
Fax 561-640-8202
Tax ID No. 65-0500793

Lehigh Acres Municipal Services Improvement District
c/o Mr. David E. Lindsay
601 East County Lane
Lehigh Acres, FL 33936

March 10, 2026
Invoice No. MCL-167660

FOR FEBRUARY SERVICES

CLIENT: 1208 - Lehigh Acres Municipal Services Improvement District
Re: 006 Legislative Representation
VIA ELECTRONIC MAIL

Total for Services	**$4,000.00**

Date	**Expenses**	**Amount**
01/11/26	Session travel and expenses.	268.99
01/21/26	Session dinner with House Republican Leadership.	114.02
Total Expenses		**$383.01**

Total for Services and Expenses	**$4,383.01**
Previous Balance	4,148.28
Payments Since Last Bill	-4,148.28
Amount Due	**$4,383.01**

	INIT	DATE
DEPT 1	MB	3/10/26
CODE	01-3110	
DEPT 2		
CODE		
MANAGER		3/11/26

Invoice No.	MCL-167660
Invoice Date:	March 10, 2026
Client Code:	1208
Client Name:	Lehigh Acres Municipal Services Improvement District
Matter Code:	006
Matter Name:	Legislative Representation

Total for Services and Expenses	**$4,383.01**
Previous Balance	4,148.28
Payments Since Last Bill	-4,148.28
Amount Due	**$4,383.01**

Amount enclosed: _____

Please remit checks to:

Lewis, Longman & Walker, P.A.
PO Box 160278
Altamonte Springs, FL 32716-0278

For your convenience, we accept credit card and e-check payments online at:

http://www.llw-law.com/template/payment/

For billing questions, please contact our Billing Department at: (561) 640-0820.

LLW LEWIS LONGMAN WALKER

360 South Rosemary Avenue
Suite 1100
West Palm Beach, FL 33401
Tel 561-640-0820
Fax 561-640-8202
Tax ID No. 65-0500793

Lehigh Acres Municipal Services Improvement District
c/o Mr. David E. Lindsay
601 East County Lane
Lehigh Acres, FL 33936

March 9, 2026
Invoice No. SCB-167632

CLIENT: 1208 - Lehigh Acres Municipal Services Improvement District
Re: 007 General Counsel
VIA ELECTRONIC MAIL

Date		Services	Hours	
02/02/26	KWD	Review correspondence and materials and telephone conference with client regarding proposed release of easement; telephone conference with D. Boyette regarding same and follow up.	0.80	
02/04/26	SCB	Review updated Interlocal Agreement with Lee County; forward questions to D. Lindsay and M. Cook regarding same.	1.00	- Sunshine
02/05/26	SCB	Telephone conference with D. Lindsay and M. Cook to review County road-widening Interlocal; continue review and edits to agreement.	1.50	- Sunshine
02/06/26	SCB	Continue revisions to County agreement for Sunshine widening and forward same to D. Lindsay and M. Cook.	1.60	- Sunshine
02/09/26	JDR	Telephone conference with D. Lindsay regarding right of way permits for fiber optic cable installations; evaluate options for blanket permits.	0.80	
02/10/26	SCB	Email exchanges with R. Price attorney for Lee County, regarding agreement with District and timing of exchange.	0.50	- Sunshine
02/13/26	JDR	Review and revise investment policy statement for consistency with Florida law.	2.50	

INIT DATE
DEPT 1 MS 3/10/26
CODE
DEPT 2
CODE
MANAGER DEF 3/11/26

01-3100 $3,515
0001-0000-220-0011 $1610 Sunshine

Date		Services	Hours
02/19/26	JDR	Research Florida law regarding investment policies related to tax certificates; email correspondence with D. March regarding same.	1.00
02/20/26	JDR	Telephone conference with D. Lindsay regarding moveable encroachments; research Florida statutes regarding notice requirements for private vehicle towing; review and revise District right of way policy; email correspondence with D. Lindsay regarding same.	1.50
02/23/26	SCB	Review agenda; telephone conference with D. Lindsay regarding all outstanding issues and agenda; attend Board of Supervisors meeting via teleconference.	1.50
02/23/26	JDR	Telephone conference with D. March; revise tax certificate section of investment policy in response to comment.	1.00
02/25/26	AIG	Draft affidavit/notice of additional carriers for yearly affirmance from Cell Tower Tenants.	1.10

Summary of Services

		Rate	Hours	Amount
SCB	Behn, Seth C.	350.00	6.10	2,135.00
KWD	Dodge, Kenneth W.	350.00	0.80	280.00
AIG	Gitli, Ana I.	300.00	1.10	330.00
JDR	Rustin, Janice D.	350.00	6.80	2,380.00
Total for Services			**14.80**	**$5,125.00**

Total for Services and Expenses	**$5,125.00**
Previous Balance	4,893.46
Payments Since Last Invoice	-4,893.46
Amount Due	**$5,125.00**

Invoice No.	SCB-167632
Invoice Date:	March 9, 2026
Client Code:	1208
Client Name:	Lehigh Acres Municipal Services Improvement District
Matter Code:	007
Matter Name:	General Counsel

Total for Services and Expenses	**$5,125.00**
Previous Balance	4,893.46
Payments Since Last Invoice	-4,893.46
Amount Due	**$5,125.00**

Amount enclosed: _____

Please remit checks to:

Lewis, Longman & Walker, P.A.
PO Box 160278
Altamonte Springs, FL 32716-0278

For your convenience, we accept credit card and e-check payments online at:

http://www.llw-law.com/template/payment/

For billing questions, please contact our Billing Department at: (561) 640-0820.

Contractor's Application for Payment

Owner: Lehigh Acres Municipal Services Improvement District
Owner's Project No.: 4030
Engineer: AIM Engineering & Surveying, Inc.
Engineer's Project No.: 19-1026
Contractor: Turnbull Environmental, Inc.
Contractor's Project No.: N/A
Project: CREST Phase III Project
Contract: CREST Phase III Project
Application No.: 9
Application Date: 03/04/2026
Application Period: From 01/01/2026 to 02/28/2026

1. Original Contract Price	$	3,715,593.97
2. Net change by Change Orders	$	(1,258,934.96)
CO #1 - Increase plant densities & species	$	175,728.20
CO #2 - No Cost Time Extension	$	-
CO #3 - Nuisance & Exotic Treatment	$	13,650.00
CO #4 - Decrease Onsite Excavation	$	(1,805,813.16)
CO #5 - Skid Mounted Pump System	$	357,500.00
3. Current Contract Price (Line 1 + Line 2)	$	2,456,659.01
4. Total Work completed and materials stored to date (Sum of Column G Lump Sum Total and Column J Unit Price Total)	$	1,258,642.54
5. Retainage		
5% X $ 1,258,642.54 Work Completed	$	62,932.13
5% X $ - Stored Materials	$	-
c. Total Retainage (Line 5.a + Line 5.b)	$	62,932.13
6. Amount eligible to date (Line 4 - Line 5.c)	$	1,195,710.42
7. Less previous payments (Line 6 from prior application)	$	954,286.16
8. Amount due this application	$	241,424.26
9. Balance to finish, including retainage (Line 3 - Line 4)	$	1,198,016.47

Contractor's Certification

The undersigned Contractor certifies, to the best of its knowledge, the following:
(1) All previous progress payments received from Owner on account of Work done under the Contract have been applied on account to discharge Contractor's legitimate obligations incurred in connection with the Work covered by prior Applications for Payment;
(2) Title to all Work, materials and equipment incorporated in said Work, or otherwise listed in or covered by this Application for Payment, will pass to Owner at time of payment free and clear of all liens, security interests, and encumbrances (except such as are covered by a bond acceptable to Owner indemnifying Owner against any such liens, security interest, or encumbrances); and
(3) All the Work covered by this Application for Payment is in accordance with the Contract Documents and is not defective.

Contractor: Turnbull Environmental Inc
Signature: [signature]
Date: 3/4/2026

Recommended by Engineer
By: [signature]
Title: Daniel Schroeder, Engineering Manager
Date: 03/10/2026

Approved by Owner
By: [signature]
Title: Assist. Dist. Manager
Date: 3/10/26

Approved by Funding Agency
By:
Title:
Date:

By:
Title:
Date:

EJCDC C-620 Contractor's Application for Payment
(c) 2018 National Society of Professional Engineers for EJCDC. All rights reserved.

INIT DATE
DEPT 1 LA-MSID [initials] 3/10/26
CODE P.O. #1248 Line
DEPT 2
CODE
MANAGER

1 38005.80
2 12706.54
CO#5 216125.00

ENTERED

Owner:	**Lehigh Acres Municipal Serv. Improv. District**					**Owner's Project No.:**		4030
Engineer:	**AIM Engineering & Surveying, Inc.**					**Engineer's Project No.:**		19-1026
Contractor:	**Turnbull Environmental, Inc.**					**Contractor's Project No.:**		N/A
Project:	**CREST Phase III Project**							
Contract:	**CREST Phase III Project**							
App. No.:	**9**	**Application Period:**	**From:**	**1/1/2026**	**to**	**2/28/2026**	**Application Date:**	**3/4/2026**

A	B	C	D	E	F	G	H	I
			Work Completed					
Item No.	Description	Scheduled Value ($)	(D + E) From Previous Application ($)	This Period ($)	Materials Currently Stored (not in D or E) ($)	Work Completed and Materials Stored to Date (D = E + F) ($)	% of Scheduled Value (G / C) (%)	Balance to Finish (C - G) ($)
	Original Contract							
1a	Mobilization	$ 76,500.00	$ 76,500.00	$ -	$ -	$ 76,500.00	100%	$ -
1b	Site Restoration / Demobilization	$ 13,500.00	$ -	$ -	$ -	$ -	0%	$ 13,500.00
2	General Requirements	$ 40,000.00	$ 36,500.00	$ 2,000.00	$ -	$ 38,500.00	96%	$ 1,500.00
3	Surveying	$ 60,000.00	$ 56,000.00	$ -	$ -	$ 56,000.00	93%	$ 4,000.00
4	Silt Fencing	$ 8,710.00	$ 8,210.00	$ 250.00	$ -	$ 8,460.00	97%	$ 250.00
5	Floating Turbidity Barrier	$ 1,920.00	$ 1,820.00	$ 50.00	$ -	$ 1,870.00	97%	$ 50.00
6	Excavation / Embank. Option 1	$ 2,184,564.15	$ 336,577.45	$ -	$ -	$ 336,577.45	15%	$ 1,847,986.70
A-1	Excavation / Embank. Option 2	$ (273,728.52)	$ (231,554.98)	$ -	$ -	$ (231,554.98)	85%	$ (42,173.54)
7	Stabil. Soil Perimeter Berm Only	$ 34,515.25	$ -	$ -	$ -	$ -	0%	$ 34,515.25
8	Concrete Maintenance Pad	$ 6,750.00	$ -	$ -	$ -	$ -	0%	$ 6,750.00
9	Restoration Sodding (2.80 / SY)	$ 206,001.60	$ 167,131.58	$ -	$ -	$ 167,131.58	81%	$ 38,870.02
10	Electric Actuator, Complete	$ 91,250.00	$ 86,687.50	$ -		$ 86,687.50	95%	$ 4,562.50
11	Electrical Service, Complete	$ 90,000.00	$ 85,500.00	$ 2,000.00	$ -	$ 87,500.00	97%	$ 2,500.00
12	Telemetry Components, Complete	$ 78,000.00	$ 74,100.00	$ 1,600.00	$ -	$ 75,700.00	97%	$ 2,300.00
13	I - Beam with Staff Gauge	$ 1,531.67	$ -	$ -	$ -	$ -	0%	$ 1,531.67
	Base Bid Total	**$ 2,619,514.15**	**$ 697,471.55**	**$ 5,900.00**	**$ -**	**$ 703,371.55**	**26.9%**	**$ 1,916,142.60**

(c) 2018 National Society of Professional Engineers for EJCDC. All rights reserved.

Owner:	**Lehigh Acres Municipal Serv. Improv. District**			**Owner's Project No.:**	4030
Engineer:	**AIM Engineering & Surveying, Inc.**			**Engineer's Project No.:**	19-1026
Contractor:	**Turnbull Environmental, Inc.**			**Contractor's Project No.:**	N/A
Project:	**CREST Phase III Project**				
Contract:	**CREST Phase III Project**				

App. No.:	**9**	**Application Period:**	**From:**	**1/1/2026**	**to**	**2/28/2026**	**Application Date:**	**3/4/2026**

A	B	C	D	E	F	G	H	I
			Work Completed					
Item No.	**Description**	**Scheduled Value ($)**	**(D + E) From Previous Application ($)**	**This Period ($)**	**Materials Currently Stored (not in D or E) ($)**	**Work Completed and Materials Stored to Date (D = E + F) ($)**	**% of Scheduled Value (G / C) (%)**	**Balance to Finish (C - G) ($)**
	Original Contract							
	Alternate Bid Items							
A-2	Excav. of Buffer Area Option 1	$ 443,813.45	$ 144,233.29	$ 25,230.79	$ -	$ 169,464.08	38%	$ 274,349.37
A-3	Excav. of Buffer Area Option 2	$ (55,610.36)	$ (27,095.82)	$ (4,500.00)	$ -	$ (31,595.82)	57%	$ (24,014.54)
A-4	Wetland Ground Cover Plantings	$ 89,677.50	$ 35,871.00	$ -	$ -	$ 35,871.00	40%	$ 53,806.50
A-5	Wetland Tree Plantings	$ 66,948.00	$ 26,779.20	$ -	$ -	$ 26,779.20	40%	$ 40,168.80
A-6	Stabil. Soil - Maint. Access Path	$ 32,573.75	$ 32,573.75	$ -	$ -	$ 32,573.75	100%	$ -
A-7	Pea Gravel for Berm	$ 50,998.75	$ -	$ -	$ -	$ -	0%	$ 50,998.75
A-8	Seed and Mulch	$ 138,339.85	$ -	$ -	$ -	$ -	0%	$ 138,339.85
	Alternate Bid Items Total	**$ 766,740.94**	**$ 212,361.42**	**$ 20,730.79**	**$ -**	**$ 233,092.21**	**30.4%**	**$ 533,648.73**
	NTE Original Contract Total	**$ 3,386,255.09**	**$ 909,832.97**	**$ 26,630.79**		**$ 936,463.76**	**27.7%**	**$ 2,449,791.33**
CO #1	Increase Plant Density & Species; Add Palms	$ 175,728.20	$ 81,028.78	$ -	$ -	$ 81,028.78	46%	$ 94,699.42
CO #3	Nuisance & Exotic Treatment	$ 13,650.00	$ 13,650.00	$ -	$ -	$ 13,650.00	100%	$ -
CO#4	Deduct to capture project savings due to off-site sale of fill by contractor	$ (1,805,813.16)	$ (1,805,813.16)	$ -		$ (1,805,813.16)	100%	$ -
CO#5	Skid-Mounted Pump and appurtenances	$ 357,500.00	$ -	$ 16,001.65	$ 211,498.35	$ 227,500.00	64%	$ 130,000.00
	Max Contract Total with Bid Item 6, Alternate A-2, & CO #1 thru 5	**$ 2,456,659.01**	**$ (589,803.06)**	**$ 42,632.44**	**$ 211,498.35**	**$ 1,258,642.54**	**51%**	**$ 1,198,016.47**

(c) 2018 National Society of Professional Engineers for EJCDC. All rights reserved.

Lehigh Acres Municipal Services Improvement District
601 East County Lane
Lehigh Acres, FL 33936
239-368-0044

PURCHASE ORDER
1248

IMPORTANT NOTICE TO VENDORS: YOU ARE RESPONSIBLE FOR THE FOLLOWING

1. Mail invoices to 601 East County Lane, Lehigh Acres, FL 33936 2. The purchase order number above must appear on all invoices, shipping papers, packages and correspondence 3. Invoice each Purchase Order separately 4. Substitutions, changes, and prices other than specified below must be authorized in writing by LA-MSID. 5. Do not ship freight collect. 6. Cancel all back orders if not shipped within 60 days after initial shipment.

ISSUED TO: **Turnbull Environmental, Inc.**
50 Agnes Circle
St. Augustine, FL 32080

SHIP TO: **Lehigh Acres Municipal Services Improvement District**
601 East County Lane
Lehigh Acres, FL 33936

DATE	REQ. NO.	REQUESTED BY	DEPARTMENT:
5/01/2024			Stormwater Operations

ITEM	Description	Quantity	Unit Price	U/M	Amount
1	C.R.E.S.T. Phase III Per Contract with Turnbull Environmental Inc Per Contract	3,715,593.97	1.00	each	3,715,593.97
2	Withhold Retainage	-92,889.85	1.00	each	-92,889.85
3	Pay Retainage	92,889.85	1.00	each	92,889.85
					3,715,593.97
	Change Order #1				
	Additional Plantings				
1	Additional Plantings Per Attached Proposal	175,728.20	1.00	each	175,728.20
	Change Order #1 Total				**175,728.20**
	Change Order #2				
1	Time Extension of 180 days do to permitting time	0.00	0.00	<none>	0.00
	Change Order #2 Total				**0.00**
	Change Order #3				
1	Herbicidal treatment of nuisance and exotic plants and weeks prior to planting.	13,650.00	1.00	each	13,650.00
	Change Order #3 Total				**13,650.00**
	Change Order #4				
1	Decrease contract amount for volume decrease of dirt in the amount of 445,298cy that was removed off site.	-1,805,813.16	1.00	each	-1,805,813.16
	Change Order #4 Total				**-1,805,813.16**
	Change Order #5				
	Skid Mounted Pump				
1	Skid Mounted Pump and Associated Infrastructure	357,500.00	1.00	each	357,500.00
	Change Order #5 Total				**357,500.00**
	Purchase Order Total				**2,456,659.01**

Account Codes	Amounts
0306-0000-205-0005	0.00
0306-4030-538-6300	2456659.01

PURCHASE AUTHORIZED BY:	
Approver 1	Date
Approver 2	Date
Finance Director	Date

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: Approval of Minutes - January 23, 2026, Regular Meeting

Meeting Date: March 16, 2026

Agenda Item # 2B

[x] **CONSENT** [ ] **ACTION** [ ] [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve the minutes from the January 23, 2026, Regular Board Meeting.

What the action accomplishes: Memorializes the Board Meetings as per Florida Statute.

Fiscal Impact: 2 hrs staff time.

Staff comments:

Staff review and recommended approval:

[x] **MANAGER** [ ] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

LET IT BE KNOWN that the "Lehigh Acres Municipal Services Improvement District" held its regularly scheduled monthly Board meeting at 6:30 P.M. in the Barrett Room, 601 East County Lane, Lehigh Acres, FL 33936, with various members of the public and the following Commissioners and staff present:

Attendance

CHAIR:	Katy Hoover
VICE CHAIR:	Michael Bonacolta
TREASURER:	Julie Hollingsworth
SECRETARY:	Rebecca Thompson
COMMISSIONER:	David Deetscreek
STAFF:	Dave Lindsay, District Manager Michael S. Cook, Director of Operations Dana March, Finance Director Alexis Nielson, Resource & Relations Director
ATTORNEY:	Seth Behn, Lewis, Longman & Walker (Virtual)
ENGINEER:	Daniel Schroeder, AIM Engineering

REGULAR MEETING AGENDA

1) Preliminaries

- **A.** Call to Order
- **B.** Roll Call
- **C.** Invocation & Pledge of Allegiance
- **D.** Move, Remove and Add Agenda Items
- **E.** Approval of Agenda
- **F.** Public Comment for Agenda Items and Non-agenda Items
- **G.** Action on Removed Consent Items
- **H.** Approval of Consent Agenda

2) Consent Agenda

- **A.** Invoices and Change Orders
- **B.** Approval of Minutes – December 8, 2025 Regular Meeting
- **C.** PER 25-88, PER 25-89, PER 25-90, PER 25-91, and 25-92, Comcast Cable
- **D.** PER 26-02, PER 26-03, PER 26-04, PER-05, PER 26-06, PER 26-07, PER 26-08, PER 26-09, PER 26-10, PER 26-11, PER 26-12 PER 26-13, LUMOS Fiber Bores
- **E.** PER 25-25 M1, Comcast Bore Under Mike Canal @ Moore Av.
- **F.** PER 26-15, LUMEN Bore under Sailfish Canal @ 25 St. Bridge
- **G.** PER 26-18, Lumos Temp. Aerial Crossing Spur "A" Canal at Arthur Ave
- **H.** Bank Signature Documents
- **I.** 26/27 Budget Calendar

3) Action Agenda

- **A.** Good Cause
- **B.** RES 26-03, Change to Right of Way Policy
- **C.** Developer's Agreement – FPL
- **D.** Leeland Bottomless Lake Building
- **E.** RES 26-04, Change to Investment Policy

4) Discussion Agenda

- **A.** Old Business
- 1. 2026 Legislative Appropriation
- **B.** New Business
- **C.** Commissioner's Request
- **D.** Engineer's Report
- **E.** Attorney's Report
- **F.** Staff and Financial Report

G. Treasurer's Report
H. Secretary's Report

5) Adjournment

1. PRELIMINARIES

A. CALL TO ORDER

Chair Bonacolta called the meeting to order at 6:30 p.m.

B. ROLL CALL

Deputy Secretary took roll. All Commissioners are present. There is a quorum.

C. INVOCATION AND PLEDGE OF ALLEGIANCE

Commissioner Thompson did the invocation. Commissioner Deetscreek led the pledge.

D. MOVE, REMOVE AND ADD AGENDA ITEM

District Manager Dave Lindsay recommended removing 2D to act independently one, removing 3A completely, and moving 3E to 4A2 for discussion.

E. APPROVAL OF AGENDA

- *Move to approve the agenda as amended.*
  - *Commissioner Hoover moved*
  - *Commissioner Thompson seconded*
- *Motion was unanimously approved.*

F. PUBLIC COMMENT FOR AGENDA ITEMS AND NON-AGENDA ITEMS

Joaquin Garcia – has a shed in the District right-of-way and is requesting to move it back partially, but not completely out of the right-of-way

G. ACTION ON REMOVED CONSENT ITEMS

2D - PER 26-02, PER 26-03, PER 26-04, PER-05, PER 26-06, PER 26-07, PER 26-08, PER 26-09, PER 26-10, PER 26-11, PER 26-12 PER 26-13, LUMOS Fiber Bores. Director of Operations Mike Cook gave an overview of the situation. The Board gave feedback and developed a consensus.

- *Move to approve to the permits that have been paid for and approve no further permits until an agreement is reached with LUMOS to catch up on projects that have not been permitted, but have been started.*
  - *Commissioner Hoover moved*
  - *Commissioner Thompson seconded*
- *Motion was unanimously approved.*

H. APPROVAL OF CONSENT AGENDA

- *Move to approve the Consent Agenda as submitted.*
  - *Commissioner Hollingsworth moved*
  - *Commissioner Thompson seconded*
- *Motion was unanimously approved.*

2. CONSENT AGENDA

A. Invoices and Change Orders
Move to approve all invoices as stated on the Summary Sheet.

B. Approval of Minutes – January 26, 2026 Regular Meeting
Move to approve the minutes of the January 26, 2065 Regular Board Meeting.

C. PER 25-88, PER 25-89, PER 25-90, PER 25-91, and 25-92, Comcast Cable
Move to approve PER 25-88, PER 25-89, PER 25-90, PER 25-91, and 25-92, Comcast Cable, with any stipulations.

D. Moved to 1G for independent action.

E. PER 25-25 M1, Comcast Bore Under Mike Canal @ Moore Ave
Move to approve PER 25-25 (M1) minor modification to Comcast Bore Under Mike Canal @ Moore Ave, with any listed stipulations.

F. PER 26-15, LUMEN Bore under Sailfish Canal @ 25 St. Bridge
Move to approve PER 26-15, LUMEN Bore under Sailfish Canal @ 25 St. Bridge, with any listed stipulations.

G. PER 26-18, Lumos Temp. Aerial Crossing Spur A Canal @ Arthur Ave
Move to approve PER 26-18, Lumos Temp. Aerial Crossing Spur A Canal @ Arthur Ave, with any listed stipulations.

H. Bank Signature Documents
Move to approve the addition of qualified signers on LA-MSID's bank accounts.

I. 26/27 Budget Calendar
Move to approve the 2026-2027 Budget Calendar that includes a Budget Hearing for July and August.

3. ACTION AGENDA

A. Good Cause
None.

B. RES 26-03, Change to Right of Way Policy

District Manager Dave Lindsay gave an overview.

- *Move to approve Resolution 2026-03, Change to Right of Way policy.*
  - *Commissioner Hollingsworth moved*
  - *Commissioner Deetscreek seconded*
- *Motion was unanimously approved.*

C. Developer's Agreement - FPL

Director of Operations Mike Cook gave an overview. Discussion ensued.

- *Move to approve the Developers Agreement with Florida, Power & Light.*
  - *Commissioner Deetscreek moved*
  - *Commissioner Hollingsworth seconded*
- *Motion was unanimously approved.*

D. Leeland Bottomless Lake Building

District Manager Dave Lindsay gave an overview along with updates from Chris Lyon. The Board gave consensus to move forward with the process.

4. DISCUSSION AGENDA

A. Old Business

1. **2026 Legislative Appropriation**
   District Manager Dave Lindsay gave an overview of the updates. The appropriation made it in to the Senate budget, but not the House budget so far.

2. **RES 26-04, Change to Section 6.3 Cash Management/Investment Financial Policy**
   Finance Director Dana March gave an overview of the updates to the investment policy. Commissioner Hollingsworth inquired about the section regarding the purchase of tax certificates by the District. Finance Director March explained the protections put in place by the policy.

B. New Business

None.

C. Commissioner's Request

Chair Bonacolta read and presented Director of Operations Mike Cook with a resolution and award for his dedication and service related to the LUMOS permits and the amount of work they have caused. Commissioner Hoover reflected on the Swamp Cabbage Festival parade and updates needed to the otter mascot suit.

D. Engineer's Report

Report is in the packet. Daniel Schroeder gave project updates.

E. Attorney's Report

Report is in the packet. District Attorney Seth Behn gave updates on the legislative session and Sunshine widening negotiation.

F. Staff and Financial Report

Report is in the packet.

G. Treasurer's Report

Report is in the packet.

H. Secretary's Report

None.

6. ADJOURNMENT

Motion to adjourn was made by Commissioner Hoover. Seconded by Commissioner Thompson. All in favor. Meeting adjourned at 7:22 P.M.

Reviewed by: _____
Secretary, David Deetscreek

ADOPTED ON: March 16, 2026 _____
Chair, Michael Bonacolta

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: PER #23-07 (M1) Ibis Landing Minor Modification

Meeting Date: March 16, 2026

Agenda Item # 2C

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve PER #23-07 (M1) Ibis Landing Minor Modification, with any listed stipulations.

What the action accomplishes: Allows minor changes to the existing permit #23-07 approved February 26, 2024.

Fiscal Impact: Permit Fees: $500.00

Staff comments: Revised the southeast corner of the proposed development.

Staff review and recommended approval:

[x] **MANAGER** [x] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

LEHIGH ACRES - MUNICIPAL SERVICES IMPROVEMENT DISTRICT PERMIT

THIS PERMIT is granted this 16th day of March, 2026, by Lehigh Acres Municipal Services Improvement District (hereinafter referred to as "DISTRICT"), 601 East County Lane, Lehigh Acres, Florida 33936, to Aquabella Development Group, LLC, 10481 Six Mile Cypress Parkway, Fort Myers, FL 33966, hereinafter referred to as the "Permittee") as a non-exclusive permit for a residential development on 298.86 +/- acres off of Beth Stacey Blvd. in Lehigh Acres, Florida, with drainage connections to Yellowtail Canal (56-1-2), Copperhead Canal (57-7-1), and Yellowtail Extension Canal (57-6-5).

W I T N E S S E T H:

1. In the event the DISTRICT is not the fee title owner of the real property (the "Property") to which this Permit is applicable, the Permittee agrees, prior to commencement, to obtain all necessary consents from the fee title owners of the Property.

2. This Permit does not constitute a waiver of the necessity by the Permittee to obtain such other necessary and appropriate permits required by other governmental bodies or agencies; therefore the Permittee, prior to commencement, is required to obtain any and all other applicable federal, state, and local permits required in connection with Permittee's use of the Property and agrees that at all times it will comply with the requirements of all federal, state and local laws, ordinances, rules and regulations applicable or pertaining to the use of the Property by the Permittee.

3. Permittee understands and agrees that the use of the Property pursuant to this Permit is subordinate to the rights and interests of the DISTRICT and to the extent applicable that of the fee title owner of the Property. Further, Permittee does hereby stipulate that the Permittee is not relying upon any representations whatsoever by the DISTRICT regarding the DISTRICT'S right, title, interest, or ownership as to the Property for which this Permit is applicable.

4. The Permittee, by accepting this permit, specifically agrees to allow authorized District Personnel, upon presentation of credentials or other documents as may be required by law, access to the premises, at reasonable times, where the permitted activity is located or conducted for the purpose of:
    a. Having access to and copying any records that must be kept under the conditions of the permit:
    b. Reviewing the facility, equipment, practices, or operations regulated or required under this permit; and
    c. Sampling or monitoring any substances or parameters at any location reasonably necessary to assure compliance with this permit or District rules.

5. The DISTRICT specifically reserves the right to maintain its facilities located over, under or upon the Property; to make improvements; add additional facilities; maintain, construct or alter its roads; maintain any facilities, devices, or improvements on the Property which aid in or are necessary to the DISTRICT'S operation; and the right to enter upon the Property at all times for such purposes. Permittee understands that in the exercise of such rights and interest, the DISTRICT may require Permittee, at no cost to DISTRICT, to relocate, alter, or remove the Permittee's facilities and equipment or other improvements made by Permittee pursuant to this Permit which interfere with or prevent the DISTRICT, in the DISTRICT'S opinion, from properly and faithfully constructing, improving and maintaining its facilities. If the Permittee should fail to relocate, alter, or remove the Permittee's facilities, equipment or other improvements made by the Permittee pursuant to this Permit within a reasonable time period following receipt of notice from the DISTRICT requesting same, then in that event the DISTRICT shall have the right to enter upon the lands and make such relocation, alterations or removal of the Permittee's facilities, equipment/or other improvements at no cost to DISTRICT.

6. Permittee agrees that it will not use the Property in any manner which interferes with the DISTRICT'S use of the Property or causes a hazardous condition to exist.

7. The DISTRICT assumes no responsibility for the ownership, operation and/or maintenance of the Permittee's facilities as permitted herein.

8. The Permittee agrees to backfill any excavation it makes within the Property and to repair or replace (i) any below-grade authorized facilities and features located therein, including but not limited to: water, sewer, stormwater or irrigation facilities, structures or appurtenances and (ii) as to at or above-grade facilities and features, this requirement shall only be applicable to grasses and sod, sidewalks, and roadway improvements (the term roadway improvements shall include but not be limited to curbing, roadway base, sub grade and asphaltic or concrete surfaces) which are removed or damaged as a direct result of said excavation. Such repair or replacement shall substantially restore these features to their condition as it existed immediately prior to the event precipitating the excavation.

9. All below-grade Permittee installations, including water and sewer lines, whether sub aqueous or not, together with appurtenant facilities, shall be located underground and have a minimum ground cover of thirty inches (30") below original design profile grade, including District waterways and roadways.

10. All Permittee activity related to sub aqueous utility, water and/or sewer lines, including appurtenant facilities, shall be conducted in such fashion that the primary functional purpose of the canal or lake body facility, specifically flood control, within which the activity is taking place is not adversely affected. More specifically, sub aqueous construction shall conform to the criteria given in the Lehigh Acres Municipal Services Improvement District (Lehigh Acres – MSID) Administrative Policy and Procedures.

11. Permittee shall, at no expense to DISTRICT and within a reasonable time following notice, adjust the positions and elevations of its facilities as may be required in connection with future improvements to, or construction of, works of the DISTRICT.

12. Permittee does hereby indemnify and hold harmless the DISTRICT, its Board of Commissioners, officers, personnel, and it's Engineers against any claims, losses, damages (including consequential), expenses, or legal fees that might arise out or result from the implementation of the proposed project by the Permittee or that may be caused by a failure of DISTRICT facilities or property.

13. The Permittee shall be obligated at the time of submission of its Permit application, to provide a detail of any other existing improvements located over, under, upon or through the Property and the Permittee's accompanying plans and specifications shall clearly indicate how the Permittee will comply with required clearances between the requested permitted activity and existing improvements located over, under and upon the Property, including compliance with other permitted users duly adopted and published standards.

14. Permittee shall reimburse the DISTRICT for its legal, engineering and other expenses incurred as a result of the implementation of the proposed project.

15. If Permittee should violate any of the terms or conditions of the Permit and shall not correct or remedy same within ten (10) days of receiving written notice of said violation from the DISTRICT, then in that event, the DISTRICT may, at its option, revoke, cancel and terminate this Permit.

16. This Permit may not be assigned by the Permittee without the prior written approval of the DISTRICT.

17. Unless otherwise adjusted, modified or terminated as set forth hereinabove, including the Conditions attached hereto, this Permit shall continue in full force and effect so long as Permittee complies with the terms of this Permit.

18. In any litigation, including breach, enforcement or interpretation arising out of this Permit, the prevailing party to this Permit shall be entitled to recover reasonable attorney's fees, costs and expenses related to said action from the non-prevailing party.

19. That attached hereto are Supplemental Conditions to this Permit which are incorporated herein and made a part hereof.

LEHIGH ACRES – MUNICIPAL SERVICES IMPROVEMENT DISTRICT

BY: _____

NAME TYPED: David E. Lindsay

TITLE: District Manager

SUPPLEMENTAL CONDITIONS TO PERMIT NO. 23-07 (M1)

A. GENERAL CONDITIONS

1. Notification shall be given by the Permittee to the District Engineer, AIM Engineering, at 239-332-4569 forty-eight (48) hours prior to commencement of the permitted activity. The District Engineer shall establish points of construction, if applicable, which will require inspection by the District Engineer prior to construction of the permitted activity. When the Permittee considers the permitted activity complete, the Permittee shall notify the District Engineer and schedule an on-site final inspection which shall be held in the presence of a representative of the Permittee and the District Engineer.

2. The permitted activity shall be implemented, constructed and/or installed in full accordance with the approved plans and specifications within six (6) full calendar years from the date of permit approval. Deviations from the approved plans or specifications shall be coordinated with and approved by the District Engineer prior to implementation of any such deviation.

3. When working in a District road right-of-way, not more than one-half (½) of the road or street shall be closed and traffic shall be controlled so as to provide for the safety and minimum hindrance to the public. All traffic control operations shall conform to the most current issue of the Florida Department of Transportation publication "Manual on Traffic Controls and safe Practices for Street and Highway Construction, Maintenance and Utility Operations."

4. The District Engineer shall be the final authority as to the quality and quantity of the material and works required to satisfy the terms and conditions of this Permit as they relate to District improvements and facilities.

5. Upon completion of the permitted activity and after its final review and acceptance by the District Engineer THE PERMITTEE SHALL DELIVER TO THE DISTRICT OFFICE ONE COMPLETE SET OF THE FINAL "RECORD DRAWINGS." FAILURE TO PROVIDE THE FINAL RECORD DRAWINGS MAY RESULT IN THE REVOCATION, CANCELLATION AND TERMINATION OF THIS PERMIT.

6. The above Conditions shall be continuing obligations of the Permittee and shall be complied with in the event of any subsequent maintenance, repair, replacement or modification of the permitted improvement.

Stipulations:

1). Permit Modification #23-07 (M1) is approved according to the revised plans and calculations signed/sealed February 23rd, 2026.

2). Please submit a copy of your SFWMD ERP Permit Modification prior to starting improvements.

3). Maintenance of the entire on-site storm water management system and cart paths, including the storm sewer pipes, catch basins, lakes, control structure and outfall pipes into LAMSID canals are the responsibility of the Permittee

4). An "As-Built" survey showing the completed improvements (including perimeter berm, all pipe inverts and outfall structure elevations) must be submitted to Lehigh Acres - MSID within 30 days following completion of the improvements.

PART I

DATE SUBMITTED: 10 - 20 - 25

PERMIT #: 23 - 07 (M1)

APPLICATION FOR A PERMIT

*Note: The review of permit applications will be conducted per the Lehigh Acres Municipal Services Improvement District's Administrative Policy and Procedures

AGENT NAME: Atwell, LLC **PHONE #:** (239)939-5490

PROJECT NAME: Ibis Landing - Phase 3 & Beth Stacey Boulevard

TOTAL ACREAGE: 34.87 (Basins 3 & 4) Only

Property Address (STRAP associated with this address will be used as the primary STRAP for this project): Beth Stacey Boulevard & Yellowtail Canal					
Mailing Address:	**10481 Six Mile Cypress Parkway**				
City:	**Fort Myers**	**State:**	**FL**	**Zip:**	**33966**

STRAP NUMBERS (*attach additional sheet if necessary*):

0	6	-	4	5	-	2	7	-	L	4	-	1	4	G	C	4	.	0	0	0	0
0	6	-	4	5	-	2	7	-	L	3	-	1	4	0	C	1	.	0	0	C	E
0	6	-	4	5	-	2	7	-	1	0	-	0	0	0	0	F	.	0	0	0	0
0	6	-	4	5	-	2	7	-	0	0	-	0	1	0	L	3	.	0	0	0	0
0	6	-	4	5	-	2	7	-	1	0	-	0	0	0	0	H	.	0	0	0	0
0	6	-	4	5	-	2	7	-	L	3	-	1	4	0	A	9	.	0	0	C	E
0	6	-	4	5	-	2	7	-	L	3	-	1	4	0	L	4	.	0	0	0	0
0	6	-	4	5	-	2	7	-	L	4	-	1	4	F	D	6	.	0	0	0	0

Brief description of proposed project:

Application for: The proposed application is for the modification of the southeast residential area and of Ibis Landing southern portion of Beth Stacey Blvd. including modification of the stormwater system. Updated surface water management calculations are provided. No modification of the previously approved control structures or outfall pipes is proposed.

Is the project proposed to be developed in phases? ☐ **Yes** ☒ **No**

Estimated # of Phases:	1
***Permit Fee:**	$ 500.00 **(to be verified by staff)**

* See Administrative Policy and Procedures Sections 4.15 - 4.17 for current Permit Fee Amount.

PART II

OWNER/APPLICANT/ DEVELOPER INFORMATION

A.

Name of Applicant	Aquabella Development Group, LLC						
Mailing Address:	Street:	10481 Six Mile Cypress Parkway					
City:	Fort Myers	State:	FL		Zip:	33966	
Phone No.	Area Code:	(239)	Number:	278-1177	Ext.		
Fax No.	Area Code:		Number:	-			
E-mail Address:	scott.edwards@lennar.com						

B. Relationship of applicant to property*:

☒ Owner* ☐ Trustee* ☐ Option holder* ☐ Lessee*

☐ Contract Purchaser* ☐ Other (indicate)* _____

C.

Name of Developer:	Aquabella Development Group, LLC					
Mailing Address:	Street:	10481 Six Mile Cypress Parkway				
City:	Fort Myers	State:	FL		Zip:	33966
Phone No.	Area Code:	(239)	Number:	278-1177	Ext.	
Fax No.	Area Code:	( )	Number:	-		
E-mail Address:	scott.edwards@lennar.com					

D.

Name of Authorized Agent(s):	Atwell, LLC					
Mailing Address:	Street:	10511 Six Mile Cypress Parkway				
City:	Fort Myers	State:	FL		Zip:	33966
Phone No.	Area Code:	(239)	Number:	939-5490	Ext.	
Fax No.	Area Code:	(239)	Number:			
E-mail Address:	jsheppard@atwell.com & dave.underhill@atwell.com					

E.

Name of Property Owner:	Aquabella Development Group, LLC					
Mailing Address:	Street:	10481 Six Mile Cypress Parkway				
City:	Fort Myers	State:	FL		Zip:	33966
Phone No.	Area Code:	(239)	Number:	278-1177	Ext.	
Fax No.	Area Code:	( )	Number:	-		
E-mail Address:	scott.edwards@lennar.com					

Name of Property Owner:	Millrose Properties Florida, LLC					
Mailing Address:	Street:	5505 Waterford District Drive				
City:	Miami	State:	FL		Zip:	33126
Phone No.	Area Code:	(239)	Number:	278-1177	Ext.	
Fax No.	Area Code:	( )	Number:	-		
E-mail Address:	scott.edwards@lennar.com					

*A disclosure of the name of all persons or entities having an ownership interest in the property is required (attached).

PART III

DISCLOSURE OF OWNERSHIP INTEREST FORM:

PRIMARY STRAP NUMBER: 06 - 45 - 27 - L4 - 14GC4 . 0000

1. If the property is owned in the fee simple by an **INDIVIDUAL**, tenancy by the entirety, tenancy in common, or joint tenancy, list all parties with an ownership interest as well as the percentage of such interest.

Name and Address	Percentage of Ownership

2. If the property is owned by a **CORPORATION**, list the officers and stockholders and the percentage of stock owned by each.

Name, Address, Office	Percentage of Stock

3. If the property is in the name of a **TRUSTEE**, list the beneficiaries of the trust with percentages of interest.

Name and Address	Percentage of Interest

4. If the property is in the name of a **GENERAL PARTNERSHIP OR LIMITED LIABILITY COMPANY**, list the names of general and limited partners.

Name and Address	Percentage of Interest
Lennar Homes, LLC	100%
10481 Six Mile Cypress Parkway	
Fort Myers, FL 33966	

5. If there is a **CONTACT FOR PURCHASE**, whether contingent on this application Or not, and whether a Corporation, Trustee, or Partnership, list the names of the contract purchasers below, including the officers, stockholders, beneficiaries, or partners.

Name and Address	Percentage of Interest

Date of Contract (mm/dd/yy): / /

6. If any contingency clause or contract terms involve additional parties, list all individuals or officers, if a corporation, partnership, or trust.

Name and Address	Percentage of Interest

For any changes of ownership of changes in contracts for purchase subsequent to the date of the application, but prior to the date of final certificate for compliance, a supplemental disclosure of interest must be filed.

The above is a full disclosure of all parties of interest in this application, to the best of my knowledge and belief.

Aquabella Development Group, LLC
By: Lennar Homes, LLC, its Manager

SIGNATURE OF APPLICANT
Scott Edwards - Vice President - Lennar Homes, LLC
PRINTED OR TYPED NAME OF APPLICANT

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this 26th day of Sept. (m), 2025 (y), by Scott Edwards who is personally know to me or has produced _____ as identification.

JENNIFER M. SHEPPARD
MY COMMISSION # HH 699789
EXPIRES: November 16, 2029

SIGNATURE OF NOTARY PUBLIC
Jennifer M. Sheppard
PRINTED NAME OF NOTARY PUBLIC

PART III

DISCLOSURE OF OWNERSHIP INTEREST FORM FORM:

PRIMARY STRAP NUMBER: 06 - 45 - 27 - 10 - 0000H . 0000

2. If the property is owned in the fee simple by an **INDIVIDUAL**, tenancy by the entirety, tenancy in common, or joint tenancy, list all parties with an ownership interest as well as the percentage of such interest.

Name and Address	Percentage of Ownership

2. If the property is owned by a **CORPORATION**, list the officers and stockholders and the percentage of stock owned by each.

Name, Address, Office	Percentage of Stock

3. If the property is in the name of a **TRUSTEE**, list the beneficiaries of the trust with percentages of interest.

Name and Address	Percentage of Interest

7. If the property is in the name of a **GENERAL PARTNERSHIP OR LIMITED LIABILITY COMPANY**, list the names of general and limited partners.

Name and Address	Percentage of Interest
Millrose Properties Florida, LLC	100%
5505 Waterford District Drive	
Miami, FL 33126	

8. If there is a **CONTACT FOR PURCHASE**, whether contingent on this application Or not, and whether a Corporation, Trustee, or Partnership, list the names of the contract purchasers below, including the officers, stockholders, beneficiaries, or partners.

Name and Address	Percentage of Interest

Date of Contract (mm/dd/yy): / /

9. If any contingency clause or contract terms involve additional parties, list all individuals or officers, if a corporation, partnership, or trust.

Name and Address	Percentage of Interest

For any changes of ownership of changes in contracts for purchase subsequent to the date of the application, but prior to the date of final certificate for compliance, a supplemental disclosure of interest must be filed.

The above is a full disclosure of all parties of interest in this application, to the best of my knowledge and belief.

Millrose Properties Florida, LLC
By: U.S. Home, LLC, its Sole Manager

SIGNATURE OF APPLICANT

Scott Edwards - Vice President - U.S. Home, LLC
PRINTED OR TYPED NAME OF APPLICANT

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this 26th day of Sept. (m), 2025 (y), by Scott Edwards who is personally know to me or has produced _____ as identification.

JENNIFER M. SHEPPARD
MY COMMISSION # HH 699789
EXPIRES: November 16, 2029

SIGNATURE OF NOTARY PUBLIC

Jennifer M. Sheppard
PRINTED NAME OF NOTARY PUBLIC

PART IV

EXPLANATION

All Permit Application sections must be completed and submitted.

Permit Application fees and submittal items (see Part VI) must be submitted with the application.

Any and all work involving the lands and/or facilities of the District shall not commence without a Board approved permit.

All set policies, procedures, technical requirements, and fee schedules can be found and reviewed in the 'Administrative Policy and Procedures Guide'. The Guide is available at the District Office located at 601 East County Lane, Lehigh Acres, FL or online at la-msid.com/.

PART V

PROFESSIONAL CONSULTANT FORM

ATTORNEY

Name:	Charles Mann - Pavese Law Firm						
Mailing Address:	1833 Hendry Street						
City:	Fort Myers	State:	FL			Zip:	33901
Business No.	Area Code:	(239)	Number:	334-2195		Ext.	
Other Contact:							

ENGINEER

Name:	David R. Underhill, Jr. - Atwell, LLC						
Mailing Address:	10511 Six Mile Cypress Parkway						
City:	Fort Myers	State:	FL			Zip:	33966
Business No.	Area Code:	(239)	Number:	939-5490		Ext.	
Other Contact:	Jennifer Sheppard - jsheppard@atwell.com						

OTHER REPRESENTATIVE/PROFESSIONAL

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

PART VI

SUBMITTALS

NO. OF COPIES SUB'D	NO. OF COPIES REQ'D	SUBMITTAL DOCUMENT
		ITEMS REQUIRED FOR INITIAL SUBMITTAL
✓	2	**Construction/Development Plans** signed and sealed by a Licensed Florida Professional Engineer including: 1. Location map showing the location of the proposed project in relation to adjacent streets and LA-MSID facilities. 2. Details of proposed works that fully communicate the work and/or use desired (i.e. Existing Conditions Survey, Proposed Site Plan, Paving Grading & Drainage Plan, Cross Sections, Element Details, and Erosion Control Plan sheets). 3. Sufficient survey data to determine elevations, property and ROW lines, limits of existing and proposed conditions, and notation of utilized vertical datum (NAVD/NGVD).
✓	2	**Stormwater Drainage Calculations** signed and sealed by a Licensed Florida Professional Engineer including, but not limited to, computer stormwater routing modeling data (ICPR4, Cascade, etc.) to verify compliance with the Design Criteria (input summary data below): **Water Quality Volume (Required):** **37.41** (ACRE-FT) **Water Quality Volume (Provided):** **42.18** (ACRE-FT) **Control Elevation:** **24 - 25.5** (FT-NGVD) **Minimum Road/Parking Elevation:** **26.5** (FT-NGVD) *(above the 10-year, 1-day design storm peak stage elevation)* **Minimum Perimeter Berm Elevation:** **27.35** (FT-NGVD) *(above the 25-year, 3-day design storm peak stage elevation)* **Minimum Finished Floor Elevation:** **28.0** (FT-NGVD) *(above the 100-year, 3-day design storm peak stage elevation and previously experienced high-water levels external to project)*
✓	2	**Flash Drive / CD** containing plans in both AutoCAD (.dwg file drawn in "State Plane Coordinates") and PDF formats.
✓	2	**Permit Application** (PARTS I & II)
✓	2	**Disclosure of Owner Form (signed and notarized)** (PART III)
✓	2	**Professional Consultant Form & Submittals** (PARTS V & VI)
✓	2	**Copy of Approved FDEP Notice of Intent (NOI)** for all sites one (1) acre and greater in size.
	2	**Copy of approved SWFMD permit**, for all sites required to get a permit, **or FDEP "10/2 Self-Certification"** approval.
✓	1	**Check for appropriate Permit Fees** made out to LA-MSID.

ATWELL

LETTER OF TRANSMITTAL

TO: Lehigh Acres Municipal Services
Improvement District
601 East County Lane
Lehigh Acres, FL 33936

DATE: 10/15/2025
PHONE: (239) 368-0044
FAX:

REFERENCE: Ibis Landing-Phase 3 & Beth Stacey Boulevard **JOB NO:** 3142L

COPIES	DESCRIPTION
2	Application for Permit Modification w/signatory authorization
2	Engineering Plans & Beth Stacey Boulevard Plans
2	FDEP NOI Approval
2	Stormwater Management Report
1	Check for $500.00 for review fees
1	CD of AutoCAD and PDF

REASON:
___ Your Comments
✓ Your Approval
___ Your Information
___ Your Review
___ Per Your Request

SENT VIA:
___ Regular Mail
___ FedEx / UPS
✓ Courier
___ Pick-up
___ E-mail

COMMENTS:
Please contact our office if you should have any questions or comments regarding the attached.

Thank you

Jennifer M. Sheppard
Project Coordinator
Email: jsheppard@atwell.com

cc:

C925448632544863

PAGE: 1 of 1

Lennar Corporation
Lennar Homes, LLC
Lee/Collier Homebuilding
10481 Ben C. Pratt/6 Mile Cypress Pkwy
Ft. Myers, FL 33966

DATE: October 2, 2025
CHECK NUMBER: 2544863
AMOUNT PAID: $500.00

00021 12804 CKS SD 25275 - 000254486З NNNNNNNNNNN 2755100005203 XIP3C9 C

LEHIGH ACRES MUNICIPAL SERV IMPROVE
601 E COUNTY LN
LEHIGH ACRES FL 33936

Vendor No: 13301953

Date	CO. #	Invoice Number	Payment Advice	Gross Amount	Discount	Net Amount
09/29/25	023300	202509 50000 FEE	MV-Ibis Ph3 Mod App Fee BethSt	$500.00	$0.00	$500.00
			MV-Ibis Ph3 Mod App Fee BethSt			
		TOTALS		**$500.00**	**$0.00**	**$500.00**

RECEIVED
OCT 20 2025
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

COPY

Permit # 23-07 (MI)

PLEASE DETACH BEFORE DEPOSITING CHECK

Lennar Corporation
Lennar Homes, LLC
Lee/Collier Homebuilding
10481 Ben C. Pratt/6 Mile Cypress Pkwy
Ft. Myers, FL 33966

CHECK NUMBER 2544863

70-2322
719

October 2, 2025

***** VOID AFTER 180 DAYS *****

PAY TO THE ORDER OF: LEHIGH ACRES MUNICIPAL SERV IMPROVE
601 E COUNTY LN
LEHIGH ACRES, FL 33936

CHECK AMOUNT
$500.00

EXACTLY *********500 DOLLARS AND 00 CENTS

Security features included. Details on back.

JPMorgan Chase Bank, N.A.
Chicago, IL

Authorized Signature

⑈2544863⑈ ⑆071923226⑆ 886293331⑈

ATWELL

February 23, 2026

Mr. Michael S. Cook, CDM
Lehigh Acres - Municipal Services Improvement District
601A East County Lane
Lehigh Acres, Florida 33936

Reference: **IBIS LANDING - PHASE 3 MODIFICATION**
APPLICATION NO. 23-07

Dear Mr. Cook:

This letter is written in response to your review comments dated January 6, 2026. With this letter we are submitting the following additional information you requested:

1. Revised Engineering Plans

Presented below are responses to each review comment.

Comment 1: Please describe the purpose of the yard drain structures located outside of the berm, on Sheet 11. It appears they are accepting runoff from outside the berm and conveying it to the on-site stormwater system.

Response: Please see revised section Q-Q on Sheet 15 of the revised civil engineering plans. The perimeter berm is located along the property line, which will include the yard drains as part of the stormwater management system.

Comment 2: Ensure that the perimeter berm ties seamlessly into the proposed roadway improvements. The minimum proposed berm elevation must be maintained continuously around the project boundary. The perimeter boundary may consist of a combination of earthen and hardened berm sections but must form a fully enclosed system.

Response: Please see sections A-A, Q-Q, N-N and R-R on sheet 15 of the revised civil engineering plans that show the continuous berm.

Comment 3: Prior to the commencement of construction, provide documentation of the SFWMD ERP No. 36-108291-P Modification approval to LAMSID.

Response: Acknowledged.

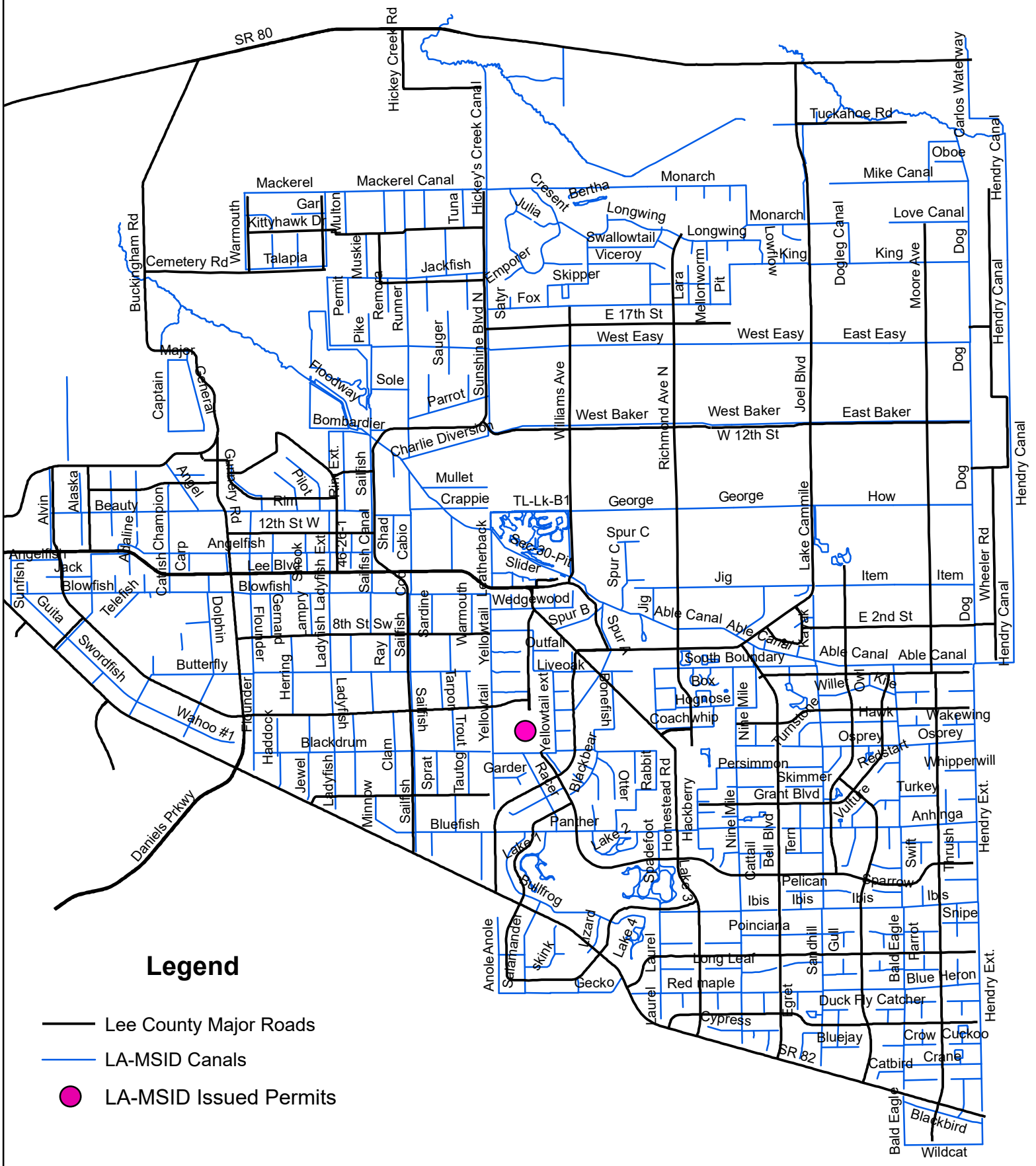
If you should have any questions or require additional information, please contact me at our Fort Myers office.

Sincerely,
ATWELL, LLC

Jennifer M. Sheppard
Senior Project Coordinator

10511 Six Mile Cypress Pkwy, Suite 101, Fort Myers, FL 33966
www.atwell-group.com

Permit # 2023-07 (M1)


SR 80
Hickey Creek Rd
Hickey's Creek Canal
Carlos Waterway
Tuckahoe Rd
Oboe
Mike Canal
Hendry Canal
Mackerel
Mackerel Canal
Gar
Kittyhawk Dr
Talapia
Cemetery Rd
Buckingham Rd
Warmouth
Multon
Muskie
Tuna
Jackfish
Permit
Remora
Runner
Pike
Sauger
Sole
Parrot
Sunshine Blvd N
Crescent
Bertha
Julia
Longwing
Monarch
Swallowtail
Viceroy
Emperor
Skipper
Satyr
Fox
Lara
Mellonworm
Pit
Lowman
King
Dogleg Canal
Love Canal
Dog
Moore Ave
E 17th St
West Easy
East Easy
Major General
Captain
Floodway
Bombardier
Charlie Diversion
Williams Ave
Richmond Ave N
Joel Blvd
West Baker
East Baker
W 12th St
Mullet
Crappie
TL-Lk-B1
George
How
Spur C
Jig
Item
Lake Cammile
Wheeler Rd
Alvin
Alaska
Beauty
Angel
Cathy Rd
Pilot
Rim
Rim Ext.
Sailfish
Champion
Catfish
Carp
Aldine
12th St W
Angelfish
Sunfish
Jack
Blowfish
Telefish
Guita
Lee Blvd
Blowfish Ext.
46-26-1
Shad
Cabio
Cod
Sailfish Canal
Ladyfish
Snook
Lampry
Gennard
Flounder
Dolphin
Butterfly
Swordfish
Wahoo #1
Daniels Prkwy
8th St Sw
Ray
Sailfish
Sardine
Warmouth
Yellowtail
Leatherback
Slider
Wedgewood
Outfall
Liveoak
Spur B
Able Canal
E 2nd St
Karak
South Boundary
Box
Hognose
Coachwhip
Willet
Owl
Kite
Hawk
Osprey
Wakewing
Redstart
Whipperwill
Turnstone
Nine Mile
Persimmon
Skimmer
Grant Blvd
Turkey
Vulture
Anhinga
Thrush
Swift
Hendry Ext.
Bell Blvd
Terr
Cattail
Pelican
Sparrow
Ibis
Snipe
Poinciana
Gull
Sandhill
Bald Eagle
Farrot
Blue Heron
Long Leaf
Red maple
Duck Fly Catcher
Egret
Cypress
Bluejay
Crow
Cuckoo
Crane
Catbird
SR 82
Blackbird
Wildcat
Haddock
Blackdrum
Jewel
Ladyfish
Clam
Minnow
Sprat
Tautog
Trout
Garder
Rabbit
Otter
Blackbear
Raccoon
Panther
Bluefish
Lake 1
Lake 2
Lake 3
Lake 4
Bullfrog
Lizard
Skink
Salamander
Anole
Gecko
Laurel
Homestead Rd
Hackberry
Spadefoot
Raccoon
Yellowtail ext
Tarpon
Sailfish
Ladyfish
Herring
Figunder
Ladyfish
Ladyfish
Sailfish
Sailfish
Sail

IBIS LANDING
(F.K.A. COPPERHEAD DEVELOPMENT)

S.F.W.M.D. PLANS

SECTION 06, TOWNSHIP 45 SOUTH, RANGE 27 EAST
LEE COUNTY, FLORIDA



GENERAL LOCATION MAP
NOT TO SCALE

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199 FAX: (239) 278-3174



SPECIFIC LOCATION MAP
NOT TO SCALE

UTILITY OWNER CONTACTS

WATER & WASTEWATER:	FLORIDA GOVERNMENTAL UTILITY AUTHORITY 280 WEKIVA SPRINGS RD. SUITE 203 LONGWOOD, FLORIDA 32779 PHONE: (407) 629-6900 FAX: (407) 629-6963
ELECTRIC POWER SERVICE:	LEE COUNTY ELECTRIC CO-OP P.O. BOX 3455 NORTH FORT MYERS, FLORIDA 33918 PHONE: (239) 995-2121 FAX: (239) 995-7904
TELEPHONE SERVICE:	SPRINT 2820 CARGO STREET BUILDING A FORT MYERS, FLORIDA 33916 PHONE: (239) 336-2003 FAX: (239) 336-2026

PROJECT CONTACTS

ENGINEER:	DAVID R. UNDERHILL, P.E. PROJECT MANAGER BANKS ENGINEERING 10511 SIX MILE CYPRESS PARKWAY FORT MYERS, FLORIDA 33966 PHONE: (239) 939-5490
SURVEYOR:	BANKS ENGINEERING 10511 SIX MILE CYPRESS PARKWAY FORT MYERS, FLORIDA 33966 PHONE: (239) 939-5490
DEVELOPER:	AQUABELLA DEVELOPMENT GROUP, LLC 10481 SIX MILE CYPRESS PARKWAY FORT MYERS, FLORIDA 33966 PHONE: (239) 278-1199

INDEX OF SHEETS

SHEET	TITLE
1	COVER SHEET
2	AERIAL MAP & EXISTING CONDITIONS PLAN
3	SITE DEVELOPMENT PLAN
4	STORM WATER MANAGEMENT PLAN
5	MASTER PAVING, GRADING, & DRAINAGE PLAN
6	PAVING, GRADING & DRAINAGE 1 (60 SCALE)
7	PAVING, GRADING & DRAINAGE 2 (60 SCALE)
8	PAVING, GRADING & DRAINAGE 3 (60 SCALE)
9	PAVING, GRADING & DRAINAGE 4 (60 SCALE)
10	PAVING, GRADING & DRAINAGE 5 (60 SCALE)
11	PAVING, GRADING & DRAINAGE 6 (60 SCALE)
12	PAVING, GRADING & DRAINAGE DETAILS (1)
13	PAVING, GRADING & DRAINAGE DETAILS (2)
14	DRAINAGE STRUCTURE SCHEDULE
14A	CONTROL STRUCTURE DETAILS 1
14B	CONTROL STRUCTURE DETAILS 2
14C	CONTROL STRUCTURE DETAILS 3
15	TYPICAL CROSS-SECTIONS
16	BEST MANAGEMENT PRACTICES PLAN
17	BEST MANAGEMENT PRACTICES DETAILS

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

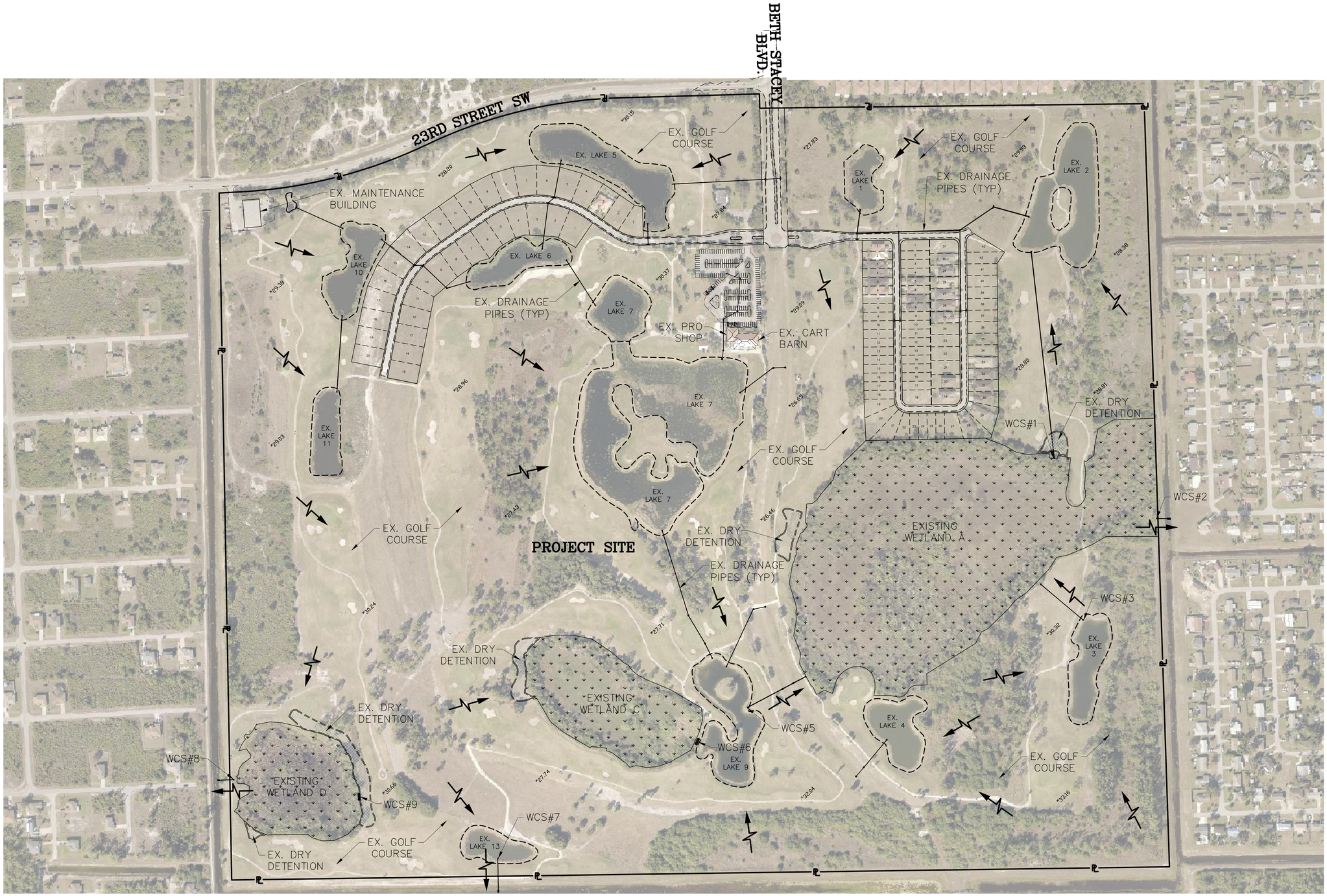
WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

NO.	DATE	REVISION DESCRIPTION	BY

IBIS LANDING

S:\USERS\DAVE.UNDERHILL\DOCUMENTS\REVIT FILES\IBIS LANDING_22-0146_DAVE.UNDERHILL@ATWELL-GROUP.COM.RVT 5/22/2025 12:55 PM JIMMY MCKINLEY



N

0 200' 400' 600'

GRAPHIC SCALE 1"= 200'

IBIS LANDING

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC

10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

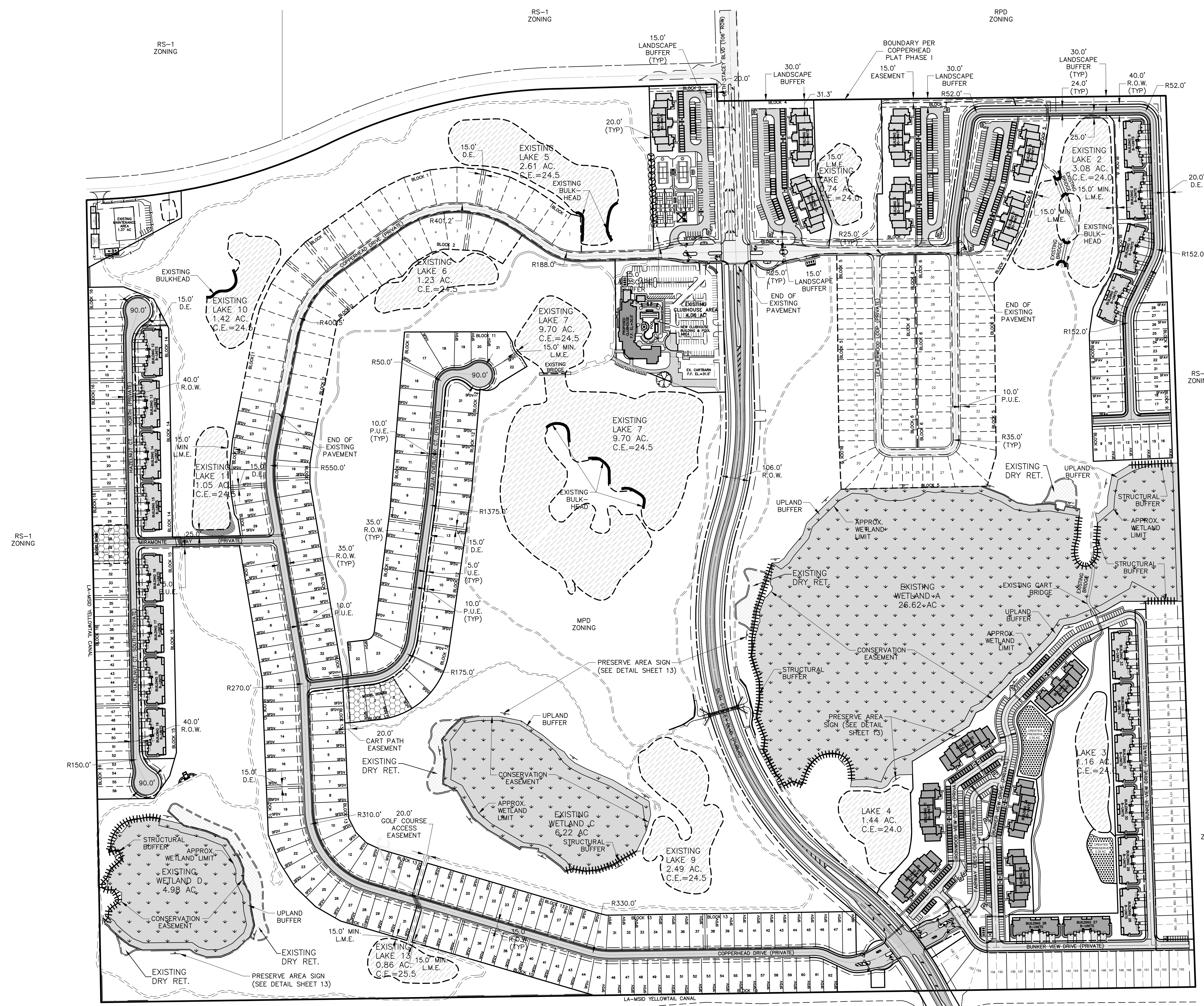
AERIAL MAP & EXISTING CONDITIONS PLAN

IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	AER	JLW	JLW	DRU	1"=200'	02

S:\JOBS\3142L\PHASE\QUADRENT\QEXPLANS\SFWMD_PLANS\WQSDP\3142L_02_AERIAL.DWG 9/22/2025 12:45 PM JIMMY WEAVY



SITE PLAN DATA:

PROJECT DEVELOPMENT AREA:	90.57 ACRES
PARCEL ZONING :	MPD
PARCEL LAND USE:	SINGLE FAMILY DETACHED VILLAS, SINGLE FAMILY ATTACHED VILLAS, TOWNHOMES, & MULTI-FAMILY RESIDENTIAL
FEMA FLOOD ZONE:	"X"
MAXIMUM LOT COVERAGE (BUILDINGS): MAXIMUM BUILDING HEIGHT SINGLE FAMILY RESIDENTIAL: MULTI-FAMILY RESIDENTIAL:	SEE PROPOSED DEVELOPMENT STANDARDS (SHEET 3A)

LAND USE:

TOTAL PROPERTY AREA:	90.57 Ac (100%)
PROJECT DEVELOPMENT AREA:	90.57 Ac (100%)
RESIDENTIAL AREA:	77.02 Ac (85.0%)
RECREATIONAL AREA:	1.35 Ac (1.5%)
RIGHT-OF-WAY AREA:	12.20 Ac (13.5%)

PARKING CALCULATIONS:

BLOCK 3: MULTI-FAMILY
TWO BEDROOM UNITS = 1.75 SPACES PER UNIT
84 UNITS x 1.75 = 147 SPACES
GUEST PARKING = 147 x 10% = 15

REQUIRED HANDICAP SPACES = 5
PROVIDED HANDICAP SPACES = 5

TOTAL PARKING SPACES REQUIRED = 162
TOTAL PARKING SPACES PROVIDED = 162

BLOCK 4: MULTI-FAMILY
TWO BEDROOM UNITS = 1.75 SPACES PER UNIT
60 UNITS x 1.75 = 105 SPACES
GUEST PARKING = 105 x 10% = 11

REQUIRED HANDICAP SPACES = 5
PROVIDED HANDICAP SPACES = 5

TOTAL PARKING SPACES REQUIRED = 116
TOTAL PARKING SPACES PROVIDED = 130

BLOCK 7: MULTI-FAMILY
TWO BEDROOM UNITS = 1.75 SPACES PER UNIT
108 UNITS x 1.75 = 189 SPACES
GUEST PARKING = 189 x 10% = 19

REQUIRED HANDICAP SPACES = 6
PROVIDED HANDICAP SPACES = 8

TOTAL PARKING SPACES REQUIRED = 208
TOTAL PARKING SPACES PROVIDED = 208

BLOCK 1 (LOTS 21-29), 2 (LOTS 16-33), 5 (LOT 44), 10, 11, 12, & 13: SINGLE FAMILY DETACHED VILLAS
2.0 SPACES REQUIRED PER UNIT
186 UNITS x 2.0 = 372 SPACES
ALL PARKING FOR SINGLE FAMILY DETACHED VILLA LOTS ARE PROVIDED FOR ON UNITS LOT WHETHER IN 2 CAR GARAGE OR ON DRIVEWAY

TOTAL PARKING SPACES REQUIRED = 372
TOTAL PARKING SPACES PROVIDED = 372

BLOCK 14, 15, & 16: TOWNHOMES
2.0 SPACES REQUIRED PER UNIT
142 UNITS x 2.0 = 284 SPACES
ALL PARKING FOR TOWNHOME LOTS ARE PROVIDED FOR ON UNITS LOT WHETHER IN 2 CAR GARAGE OR ON DRIVEWAY

TOTAL PARKING SPACES REQUIRED = 284
TOTAL PARKING SPACES PROVIDED = 284

BLOCK 17, 18, 19, 20, & 21: TOWNHOMES
2.0 SPACES REQUIRED PER UNIT
243 UNITS x 2.0 = 486 SPACES
ALL PARKING FOR TOWNHOME LOTS ARE PROVIDED FOR ON UNITS LOT WHETHER IN 2 CAR GARAGE OR ON DRIVEWAY

TOTAL PARKING SPACES REQUIRED = 486
TOTAL PARKING SPACES PROVIDED = 486

BLOCK 5 (LOTS 45-56), 8, & 9: SINGLE FAMILY ATTACHED VILLAS
2.0 SPACES REQUIRED PER UNIT
58 UNITS x 2.0 = 116 SPACES
ALL PARKING FOR SINGLE FAMILY ATTACHED VILLA LOTS ARE PROVIDED FOR ON UNITS LOT WHETHER IN 2 CAR GARAGE OR ON DRIVEWAY

TOTAL PARKING SPACES REQUIRED = 116
TOTAL PARKING SPACES PROVIDED = 116

LEGEND:

- EXISTING ASPHALT
- EXISTING E.O.P.
- PROPOSED ASPHALT PAVEMENT
- PROPOSED TOP OF BANK
- LANDSCAPE BUFFER
- RIGHT-OF-WAY LINE
- EXISTING STORM WATER MANAGEME
- WETLAND AREA & BUFFER
- LANDSCAPE BUFFER
- ZONING LINE
- PROPERTY LINE
- PROPOSED BERM
- EXISTING BULKHEAD
- PROPOSED DRAINAGE EASEMENT
- PROPOSED LAKE MAINTENANCE EA
- PROPOSED STRUCTURAL BUFFER
- EXISTING DEVELOPMENT ORDER #DOS 991203400D & COPPERHEAD PLAT
- MODEL HOME LOCATION
- LAKE FILL AREA

0 200' 400' 600'
GRAPHIC SCALE 1"= 200'

SOLID WASTE DISPOSAL FACILITIES:

REQUIRED WASTE DISPOSAL FACILITY AREA = 168 SF FOR THE FIRST 25 UNITS + 4.8 SF FOR EACH REMAINING UNIT

BLOCK 3: MULTI-FAMILY
84 UNITS = 168 SF + (4.8 SF UNIT x 59 UNITS) = 451 SF
DISPOSAL FACILITY AREA REQUIRED = 451 SF
4-12' X 12' DUMPSTER PADS = 576 SF
TOTAL DISPOSAL FACILITY AREA REQUIRED = 451 SF
TOTAL DISPOSAL FACILITY AREA PROVIDED = 576 SF

BLOCK 4: MULTI-FAMILY
60 UNITS = 168 SF + (4.8 SF UNIT x 35 UNITS) = 336 SF
DISPOSAL FACILITY AREA REQUIRED = 336 SF
2-12' X 14' DUMPSTER PADS = 336 SF
TOTAL DISPOSAL FACILITY AREA REQUIRED = 336 SF
TOTAL DISPOSAL FACILITY AREA PROVIDED = 336 SF

BLOCK 7: MULTI-FAMILY
108 UNITS = 168 SF + (4.8 SF UNIT x 83 UNITS) = 567 SF
DISPOSAL FACILITY AREA REQUIRED = 567 SF
4-12' X 14' DUMPSTER PADS = 672 SF
TOTAL DISPOSAL FACILITY AREA REQUIRED = 567 SF
TOTAL DISPOSAL FACILITY AREA PROVIDED = 672 SF

EXISTING UNIT BREAKDOWN PER PLAT "COPPERHEAD PHASE 1":

SINGLE FAMILY 60' x 100' MIN. BLOCKS:1 (LOTS 1-20), & 2 (LOTS 1-15)	= 35
SINGLE FAMILY DETACHED VILLAS (50' x 100') BLOCKS:5 (LOTS 27-43), & 6 (LOTS 19-31)	= 30
DUPLEX (37' x 100') BLOCKS:5 (LOTS 1-26), & 6 (LOTS 1-18)	= 44
TOTAL EXISTING UNITS	**= 109**

PROPOSED UNIT BREAKDOWN:

SINGLE FAMILY DETACHED VILLAS (50' x 100') BLOCKS:1 (LOTS 21-29), 2 (LOTS 16-33), 5 (LOT 44), 10, 11, 12, & 13	= 186
TOWNHOMES 22' x 130' BLOCKS:14, 15, & 16	= 142
TOWNHOMES 18' x 130' BLOCKS:20, & 21	= 146
TOWNHOMES 18' x 130' BLOCKS:17, 18, & 19	= 97
MULTI-FAMILY AREA BLOCK 3	= 84
MULTI-FAMILY AREA BLOCK 4	= 60
MULTI-FAMILY AREA BLOCK 7	= 108
DUPLEX (37' x 100') BLOCKS:5 (LOTS 45-56), 8, & 9	= 58
TOTAL PROPOSED UNITS	**= 881**

NOTE:

1) SEE PHASING PLAN FOR DEVELOPMENT STANDARDS SHEET 3A.

2) NO PARKING LOT LIGHTING IS PROPOSED, STREET LIGHTING WILL BE PROVIDED BY LCEC.

3) THE PROPOSED STRUCTURAL BUFFER'S MATERIAL SHALL MEET ALL S.F.W.M.D. REQUIREMENTS.

4) ALL IMPROVEMENTS WITHIN BETH STACEY BLVD R.O.W. INCLUDING, BUT NOT LIMITED TO, SIDEWALKS, PROPOSED ACCESS CONNECTIONS, STRIPING, & DRAINAGE STRUCTURES SHALL BE ADDRESSED WITHIN THE FUTURE AMENDMENT OF THE EXISTING DEVELOPMENT ORDER #DOS 991203400D

5) ALL TOWNHOME & 4-STORY MULTI-FAMILY BUILDINGS ARE PROPOSED TO BE PROTECTED BY FIRE SPRINKLER SYSTEMS. ALL SINGLE FAMILY ATTACHED & DETACHED VILLAS ARE PROPOSED NOT TO BE PROTECTED BY FIRE SPRINKLER SYSTEMS.

6) SEE PAVING, GRADING, & DRAINAGE PLANS SHEETS 6 THRU 11 FOR STREET & REGULATORY SIGN LOCATIONS.

7) ALL EXOTIC VEGETATION WILL BE REMOVED ON A PER PHASE BASIS.

SITE GENERAL NOTES:

1) DISTURBED & FILLED AREAS OF THE LA-MSID CANAL R.O.W. OR COUNTY R.O.W. SHALL BE RESTORED, STABILIZED, SODDED, & MAINTAINED UNTIL WELL ESTABLISHED. ALL SLOPED AREAS WITHIN THE PROJECT SHALL LIKEWISE BE SODDED.
2) CONTRACTOR SHALL SEED AND MULCH ALL OTHER DISTURBED AREAS WITHIN THE PROJECT AREA AND/OR OTHERWISE RESTORE THEM TO EXISTING CONDITION OR BETTER.
3) CONTRACTOR SHALL COORDINATE ALL WORK IN THE COUNTY RIGHT-OF-WAY WITH THE LEE COUNTY D.O.T.&E.
4) ALL CONSTRUCTION TO BE IN ACCORDANCE WITH FLORIDA DEPT. OF TRANSPORTATION (F.D.O.T.) STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, CURRENT EDITION.
5) THE CONTRACTOR SHALL SUPPLY SATISFACTORY DENSITY TESTS OF SUBGRADE & LIMEROCK BASE MATERIALS TO THE ENGINEER PRIOR TO PAYMENT &/OR FINAL ACCEPTANCE. ONE TEST PER COURSE PER 600 SQUARE YARDS.
6) CONTRACTOR SHALL PROVIDE AND MAINTAIN TRAFFIC CONTROL IN ACCORDANCE WITH F.D.O.T. ROADWAY AND TRAFFIC DESIGN STANDARDS CURRENT EDITION & THE FEDERAL HIGHWAY ADMINISTRATION'S MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD), PART IV.
7) ALL PAVEMENT STRIPING TO BE TWO COATS OF HEAVY DUTY ACRYLIC BASED TRAFFIC PAINT.
8) ALL TRAFFIC SIGNS TO BE DRIVEN STEEL POST TYPE GROUND SIGNS IN ACCORDANCE WITH F.D.O.T. INDEX DRAWINGS 11860 & 11863 (80 M.P.H. WIND LOAD) AND F.D.O.T. SPECIFICATIONS SECTION 700-HIGHWAY SIGNING.
9) HANDICAP RAMPS TO MEET ALL CURRENT ADA REQUIREMENTS.
10) A RIGHT-OF-WAY USE PERMIT WILL BE REQUIRED FOR ALL WORK WITHIN COUNTY RIGHT-OF-WAY.
11) THIS PROJECT PRESENTS NO ADVERSE IMPACTS ON LOCAL GROUNDS OR SURFACE WATERS, & DOES NOT ADVERSELY IMPACT THE FLOOD ZONE.
12) THIS SITE CAN BE USED FOR THE DEVELOPMENT AS SHOWN WITHOUT UNIQUE DANGER FROM FLOOD, ADVERSE SOIL, &/OR FOUNDATION CONDITIONS, SUBJECT TO PROPER GEOTECHNICAL INVESTIGATION, SITE DESIGN, & STRUCTURE DESIGN.

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

ABBREVIATIONS

W/	=	WITH
LF	=	LINEAR FOOT
P.U.E.	=	PUBLIC UTILITY EASEMENT
D.E.	=	DRAINAGE EASEMENT
L.M.E.	=	LAKE MAINTENANCE EASEMENT
R.O.W.	=	RIGHT-OF-WAY
T.O.B.	=	TOP OF BANK
U.E.	=	UTILITY EASEMENT
AC	=	ACRES
SF	=	SQUARE FEET
E.O.P.	=	EDGE OF PAVEMENT
H.C.	=	HANDICAPPED
SWMA	=	STORMWATER MANAGEMENT AREA
LSB	=	LANDSCAPE BUFFER
S.F.A.V.	=	SINGLE FAMILY ATTACHED VILLA
S.F.D.V.	=	SINGLE FAMILY DETACHED VILLA

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

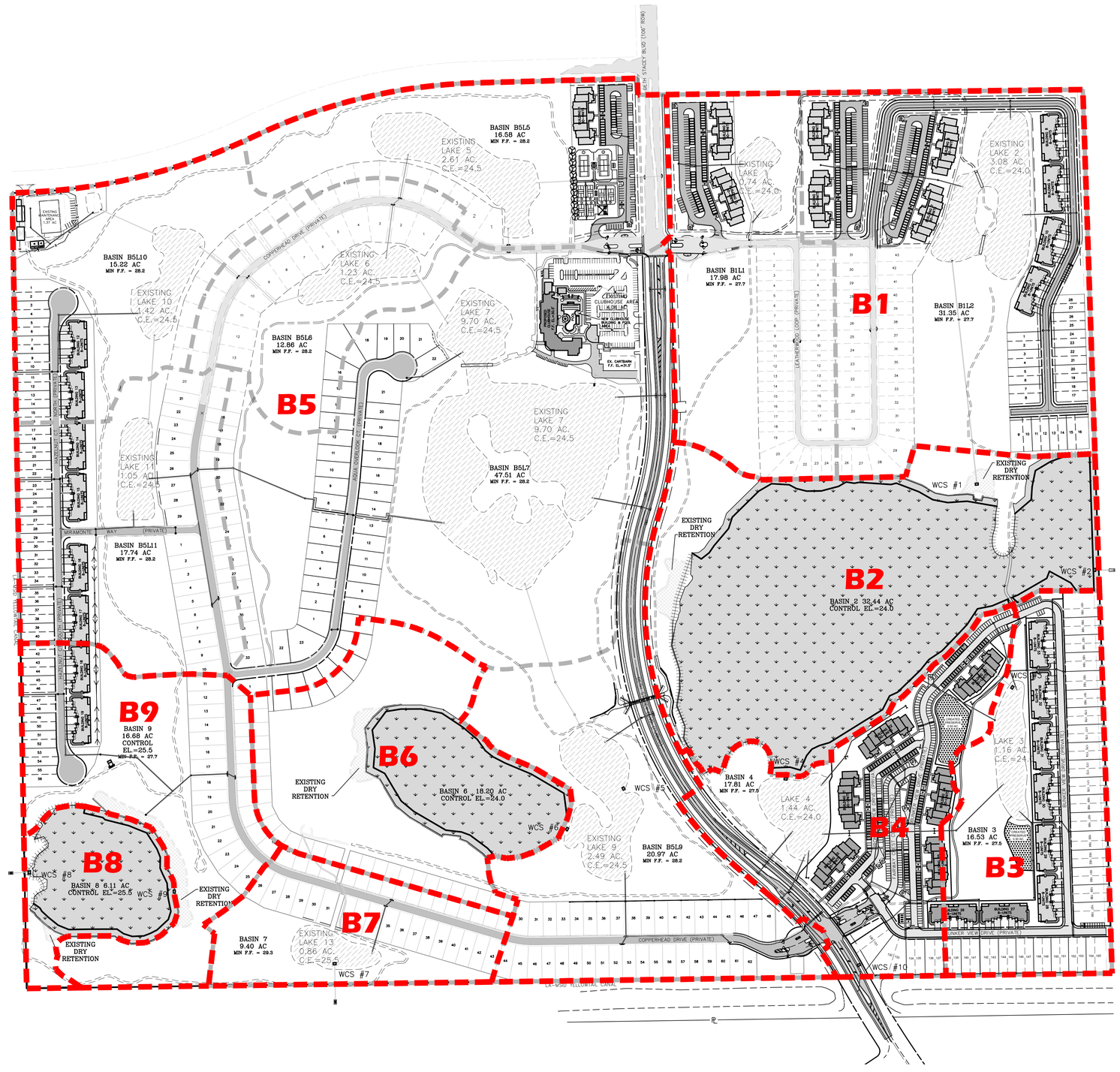
DAVID R. UNDERHILL JR.
P.E. #47029

SITE DEVELOPMENT PLAN
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	SDP	JLW	JLW	DRU	1"=200'	03

IBIS LANDING

S:\JOBS\31XX\3142L\ENGINEERING\DEVPLANS\SFWMD PLANS\MODIFICATION\3142-SFWMD_MOD_03_SDP.DWG 9/22/2025 12:47 PM JIMMY WHEWAY





LEGEND:

- EXISTING ASPHALT
- EXISTING E.O.P.
- PROPOSED ASPHALT PAVEMENT
- PROPOSED TOP OF BANK
- EXISTING DETENTION–RETENTION AREA
- LANDSCAPE BUFFER
  RIGHT–OF–WAY LINE
- EXISTING STORM WATER MANAGEMEI
- PROPOSED CONCRETE
- WETLAND AREA & BUFFER
- PROPERTY LINE
- PROPOSED BERM
- SUB–BASIN LINE
- MAJOR BASIN LINE
- PROPOSED STRUCTURAL BUFFER

STORMWATER MANAGEMENT SYSTEM
MINIMUM ELEVATIONS (NGVD)

Basin	WCS#	Control Elev.	5 Yr. Flood Stage	10 Yr. Flood Stage	Min. Road	25 Yr. Flood Stage	Min. Berm	100 Yr. Flood Stage	Min. Floor Elev.
B1	WCS 1	24.00	24.54	25.80	26.50	27.04	27.40*	27.44	28.00
B2	WCS 2	24.00	24.67	24.85	26.50	25.97	26.45	27.44	28.00
B3	WCS 3	24.00	25.94	26.07	26.50	26.91	27.35	27.44	28.00
B4	WCS 4	24.00	25.96	26.09	26.50	26.75	27.20	27.44	28.00
B5	WCS 5	24.50	25.54	25.87	26.50	27.32	27.85	27.95	28.30
B6	WCS 6	24.00	25.39	25.63	26.50	26.75	N/A	27.57	28.00
B7	WCS 7	25.50	27.13	27.44	27.50	28.52	29.05	29.27	29.30
B8	WCS 8	25.50	26.17	26.36	27.50	27.52	28.05	28.39	28.40
B9	WCS 9	25.50	26.17	26.36	27.50	27.52	28.05	28.39	28.40

* EXISTING PERIMETER BERM WILL PROVIDE 4" OF FREEBOARD ALONG NORTH PROPERTY LINE. MINIMUM BERM HEIGHT IS 27.6' ALONG LA–MSID CANALS.

PLEASE NOTE: B2, B7, & B10 WILL HAVE BERM IMPROVEMENTS TO REACH REQUIRED 6" FREEBOARD.

5 YEAR 24 HR RAINFALL IS 5.20"
10 YEAR 24 HR TOTAL RAINFALL IS 6.00"
25 YEAR 72 HR TOTAL RAINFALL IS 10.20"
100 YEAR 72 HR TOTAL RAINFALL IS 14.20"

ALLOWABLE DISCHARGE PER EXISTING SFWMD PERMIT (55 CSM) = 25.68 CFS
TOTAL PROJECT DISCHARGE = 13.55 CFS (28.6 CSM)

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278–1199
FAX: (239) 278–3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939–5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625–1165

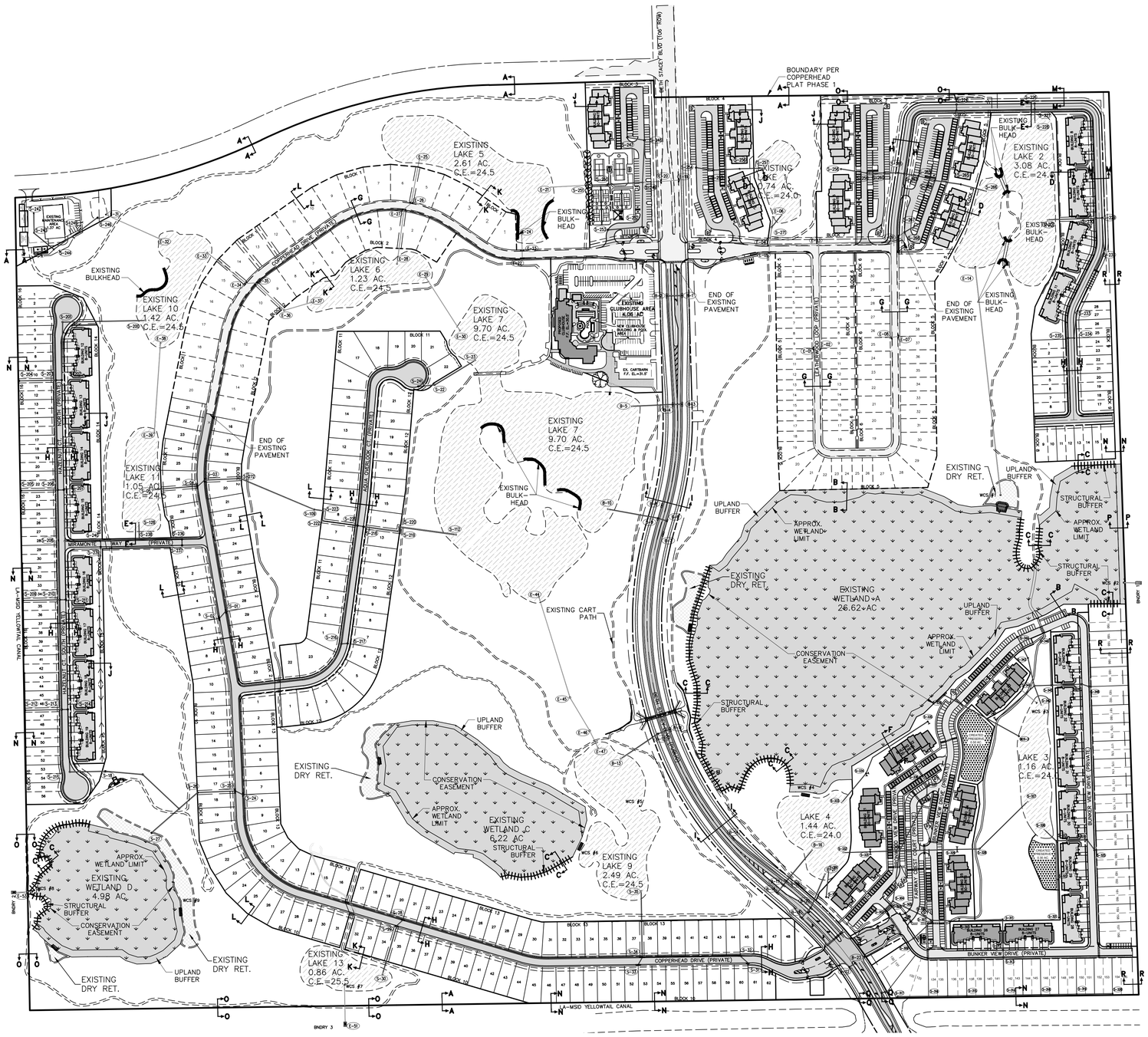
WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

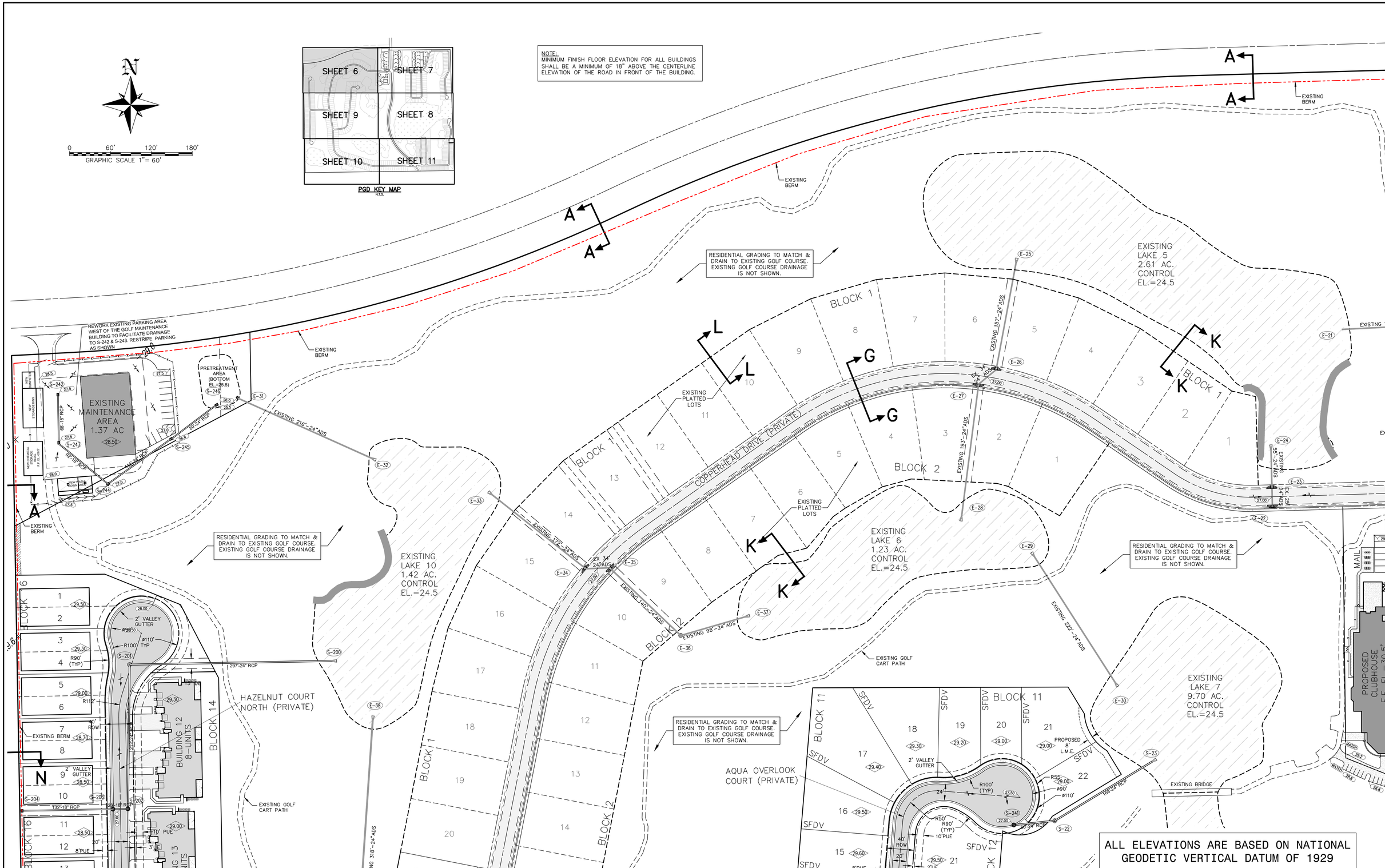
DAVID R. UNDERHILL JR.
P.E. #47029

STORM WATER MANAGEMENT PLAN
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	SWP	JLW	JLW	DRU	1"=200'	04

S:\JOBS\3142L\CIVIL\ENGINEERING\DEVPLANS\SFWMD PLANS\MISPOSITION\3142L-SFWMD_MOD_04_MPAVING 9/24/2025 1:59 PM JIMMY PIERNEY


EXISTING LAKE 5 2.61 AC. C.E.=24.5
EXISTING LAKE 1 0.74 AC. C.E.=24.0
EXISTING LAKE 2 3.08 AC. C.E.=24.0
EXISTING LAKE 6 1.23 AC. C.E.=24.5
EXISTING LAKE 10 1.42 AC. C.E.=24.5
EXISTING LAKE 7 9.70 AC. C.E.=24.5
EXISTING LAKE 11 1.05 AC. C.E.=24.5
EXISTING LAKE 7 9.70 AC. C.E.=24.5
EXISTING LAKE 3 1.16 AC. C.E.=24.
LAKE 4 1.44 AC. C.E.=24.0
EXISTING LAKE 9 2.49 AC. C.E.=24.5
EXISTING LAKE 13 0.86 AC. C.E.=25.5
EXISTING WETLAND A 26.62±AC
EXISTING WETLAND C 6.22 AC
EXISTING WETLAND D 4.98 AC
EXISTING CLUBHOUSE AREA 4.06 AC
EXISTING BULKHEAD
EXISTING BULK-HEAD
END OF EXISTING PAVEMENT
EXISTING CART PATH
EXISTING DRY RET.
UPLAND BUFFER
APPROX. WETLAND LIMIT
CONSERVATION EASEMENT
STRUCTURAL BUFFER
BOUNDARY PER COPPERHEAD PLAT PHASE 1
SOUTH STACEY BLVD (100' ROW)
CROSSFIRED DRIVE (PRIVATE)
MIRAMONTE WAY (PRIVATE)
COPPERHEAD DRIVE (PRIVATE)
BUNKER VIEW DRIVE (PRIVATE)
TEE TIME DRIVE (PRIVATE)
BIRDIE DRIVE (PRIVATE)
EXISTING CLUBHOUSE & POOL
NEW CLUBHOUSE & POOL UNDER CONSTRUCTION
EX. CARTBARN F.F. EL.=3.5'
EXISTING MAINTENANCE BLDG. F.F. EL.=3.7'
EXISTING SALES CENTER F.F. EL.=3.5'
EXISTING MODEL BLDG. F.F. EL.=3.5'
EXISTING MODEL BLDG. F.F. EL.=3.7'
EXISTING MODEL BLDG. F.F. EL.=3.8'
EXISTING MODEL BLDG. F.F. EL.=3.9'
EXISTING MODEL BLDG. F.F. EL.=4.0'
EXISTING MODEL BLDG. F.F. EL.=4.1'
EXISTING MODEL BLDG. F.F. EL.=4.2'
EXISTING MODEL BLDG. F.F. EL.=4.3'
EXISTING MODEL BLDG. F.F. EL.=4.4'
EXISTING MODEL BLDG. F.F.



PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE PLAN 1

IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-1	JLW	JLW	DRU	1"=60'	06

BLOCK 3
BLOCK 4
BLOCK 5
BLOCK 6
BLOCK 7
BLOCK 8
BLOCK 9
BLOCK 10
BLOCK 11
BUILDING 1 30-UNITS
BUILDING 2 30-UNITS
BUILDING 3 30-UNITS
BUILDING 4 30-UNITS
BUILDING 5 30-UNITS
BUILDING 6 30-UNITS
BUILDING 7 30-UNITS
BUILDING 8 8-UNITS
BUILDING 9 8-UNITS
BUILDING 10 8-UNITS
BUILDING 11 8-UNITS
TENNIS COURTS
POOL
EXISTING GOLF CART PATH
COPPERHEAD DRIVE (PRIVATE)
COPPERHEAD COURT NORTH (PRIVATE)
LEATHERWOOD LOOP (PRIVATE)
PROPOSED BERM
EXISTING BERM
RESIDENTIAL GRADING TO MATCH & DRAIN TO EXISTING GOLF COURSE. EXISTING GOLF COURSE DRAINAGE IS NOT SHOWN.
EXISTING LAKE 1 0.74 AC. CONTROL EL.=24.0
EXISTING LAKE 2 3.08 AC. CONTROL EL.=24.0
PROPOSED 15.0' L.M.E.
PROPOSED MIN. L.M.E.
PROPOSED 15' MIN. L.M.E.
15.0' GOLF CART ACCESS EASEMENT
EXISTING 20.0' DRAINAGE EASEMENT
EXISTING 15.0' DRAINAGE EASEMENT
EXISTING GOLF CART PATH TO BE REMOVED
EXISTING PIPE TO BE REMOVED & REROUTED
EXISTING BRIDGE
EXISTING CLUBHOUSE AREA 4.06 AC
NEW CLUBHOUSE BUILDING & POOL AREA
EX. CARTBARN F.F. EL.=31.5'
EXISTING IRRIGATION PUMP STATION
F.F. EL.=30.5'
EXISTING PLATTED LOTS
EXISTING GOLF CART PATH
RESIDENTIAL GRADING TO MATCH & DRAIN TO EXISTING GOLF COURSE. EXISTING GOLF COURSE DRAINAGE IS NOT SHOWN.
RESIDENTIAL GRADING TO MATCH & DRAIN TO EXISTING GOLF COURSE. EXISTING GOLF COURSE DRAINAGE IS NOT SHOWN.
EXISTING 178'-36" ADS
EXISTING 264'-36" ADS
EXISTING 380'-24" ADS
EXISTING 348'-24" ADS
EXISTING 834'-36" ADS
EX 22' 24" ADS
EX 62'-24" RCP
EX 64'-24" RCP
S 134'-24" RCP
75'-24" RCP
125'-24" RCP
152'-24" RCP
161'-18" RCP
125'-18" RCP
63'-18" RCP
95'-24" RCP
85'-24" RCP
90'-15" RCP
94'-24" RCP
62'-24" RCP
83'-24" RCP
94'-36" RCP
91'-36" RCP
174'-30" RCP
165'-18" RCP
45'-24" RCP
121'-24" RCP
R20.0' (TYP)
R30.0' (TYP)
R52.0'
R152.0'
22.0' (TYP)
24.0' (TYP)
18.0' (TYP)
15.0' DE
40.0' ROW
5' SIDEWALK TYP.
2' VALLEY GUTTER TYP.
MAILBOXES
STOP R1-1 W/ STREET SIGN
W14-2
GRAPHIC SCALE 1" = 60'
0 60' 120' 180'
N
IBIS LANDING
SHEET 6
SHEET 7
SHEET 8
SHEET 9
SHEET 10
SHEET 11
PGD KEY MAP
N.T.S.
NOTE:
MINIMUM FINISH FLOOR ELEVATION FOR ALL BUILDINGS SHALL BE A MINIMUM OF 18" ABOVE THE CENTERLINE ELEVATION OF THE ROAD IN FRONT OF THE BUILDING.
ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida
FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490
PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165
WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690
DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE PLAN 2
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-2	JLW	JLW	DRU	1"=60'	07

S:\JOBS\3XXX\3142L\ENGINEERING\DEVELOPMENT\PLANS\MASTER\DWG\3142L-SITMAS_MOD_06-11_2025.DWG 9/22/2025 12:49 PM JIMMY MCKAY

IBIS LANDING

N

0 60' 120' 180'
GRAPHIC SCALE 1" = 60'

B-3 B-4 B-5 B-6 B-7 B-8 B-9 B-10

180'-24"RCP

BETH STACEY BLVD (LOCAL)

EXISTING GOLF CART PATH

EXISTING GOLF CART PATH

RESIDENTIAL GRADING TO MATCH & DRAIN TO EXISTING GOLF COURSE. EXISTING GOLF COURSE DRAINAGE IS NOT SHOWN.

EXISTING PLATTED LOTS

BLOCK 5

BLOCK 6

LEATHERWOOD LOOP (PRIVATE)

EXISTING WETLAND A
26.62 AC

STRUCTURAL BUFFER

OVERHEAD GOLF CART BRIDGE

EXISTING GOLF CART PATH

RESIDENTIAL GRADING TO MATCH & DRAIN TO EXISTING GOLF COURSE. EXISTING GOLF COURSE DRAINAGE IS NOT SHOWN.

WCS #1

EXISTING OUTFALL TO WETLAND A

STRUCTURAL BUFFER

EXISTING BRIDGE

WCS #2

STRUCTURAL BUFFER

EXISTING 45"-24"RCP W/ MITERED/ CUT PIPE END

INSTALL RIP RAP AT EX. PIPE ENDS (SEE DETAIL ON SHEET 14)

BNDRY

COPPERHEAD COURT NORTH (PRIVATE)

PROPOSED BERM

STRUCTURAL BUFFER

PROPOSED BERM

BERM TO BE IMPROVED SEE SECTION

BUNKER VIEW DRIVE (PRIVATE)

BUILDING 33
8-UNITS

BUILDING 32
30-UNITS

BUILDING 32
8-UNITS

2' VALLEY GUTTER

5' SIDEWALK TYP.

EX. WCS #3 & OUTFALL TO BE REMOVED & RELOCATED (SEE THIS SHEET & 11)

RELOCATED WCS #3 & OUTFALL (SEE SHEET 11)

WCS #3

GOLF CART PATH

40.0' ROW

24.0'

NOTE:
MINIMUM FINISH FLOOR ELEVATION FOR ALL BUILDINGS SHALL BE A MINIMUM OF 18" ABOVE THE CENTERLINE ELEVATION OF THE ROAD IN FRONT OF THE BUILDING.

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PGD KEY MAP
N.T.S.

SHEET 6 SHEET 7
SHEET 9 SHEET 8
SHEET 10 SHEET 11

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

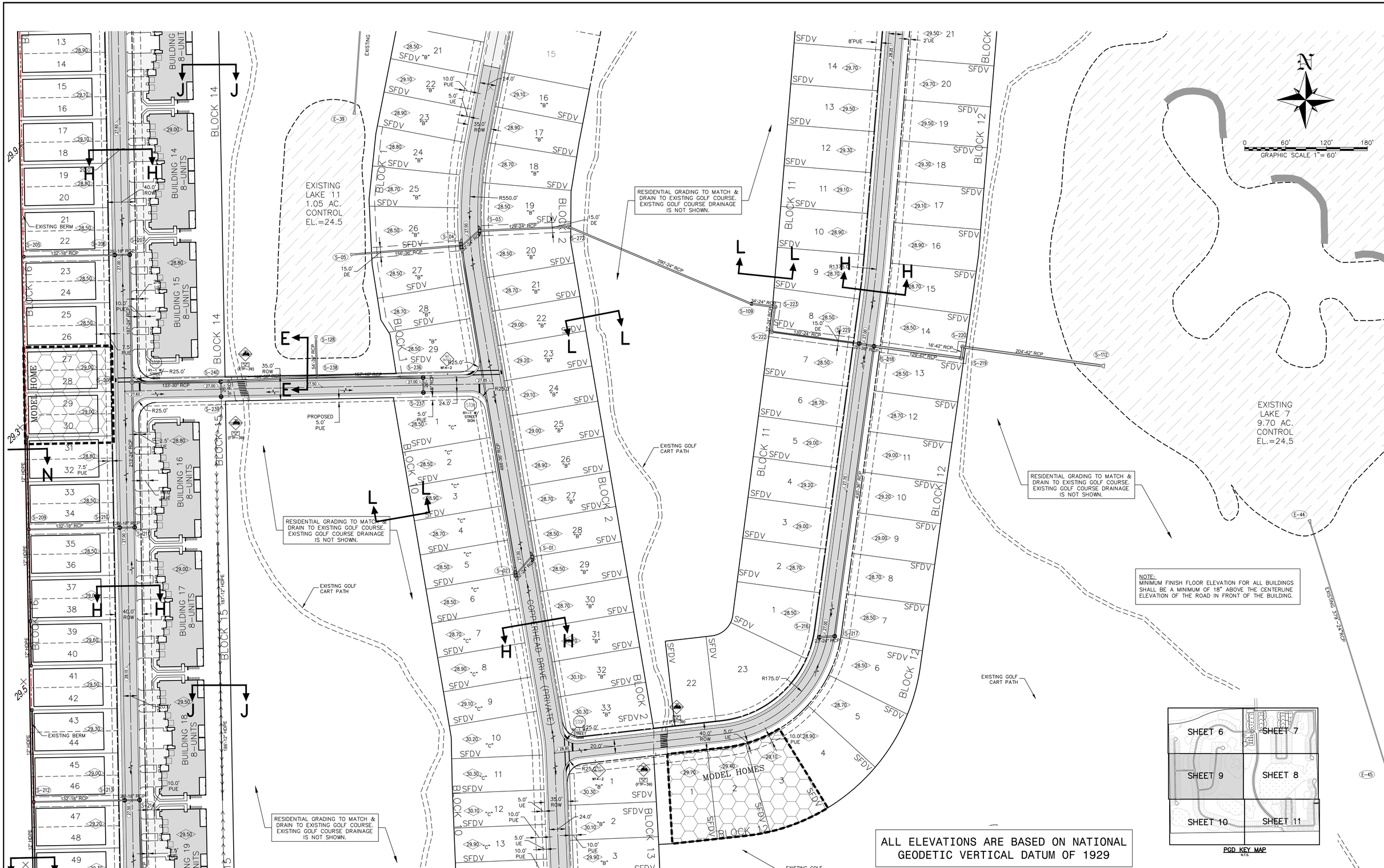
PAVING, GRADING & DRAINAGE PLAN 3

IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-3	JLW	JLW	DRU	1"=60'	08

S:\JOBS\3142L\CAD\LDP\ENGINEERING\LDP\PLANS\GP\IMAGES\PLANS\GP\GRADE PLANS\GP\GRADE PLANS_3.DWG 5/22/2025 12:59 PM JIMMY MCKAY



NOTE:
MINIMUM FINISH FLOOR ELEVATION FOR ALL BUILDINGS SHALL BE A MINIMUM OF 18" ABOVE THE CENTERLINE ELEVATION OF THE ROAD IN FRONT OF THE BUILDING.

PGD KEY MAP
N.T.S.

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

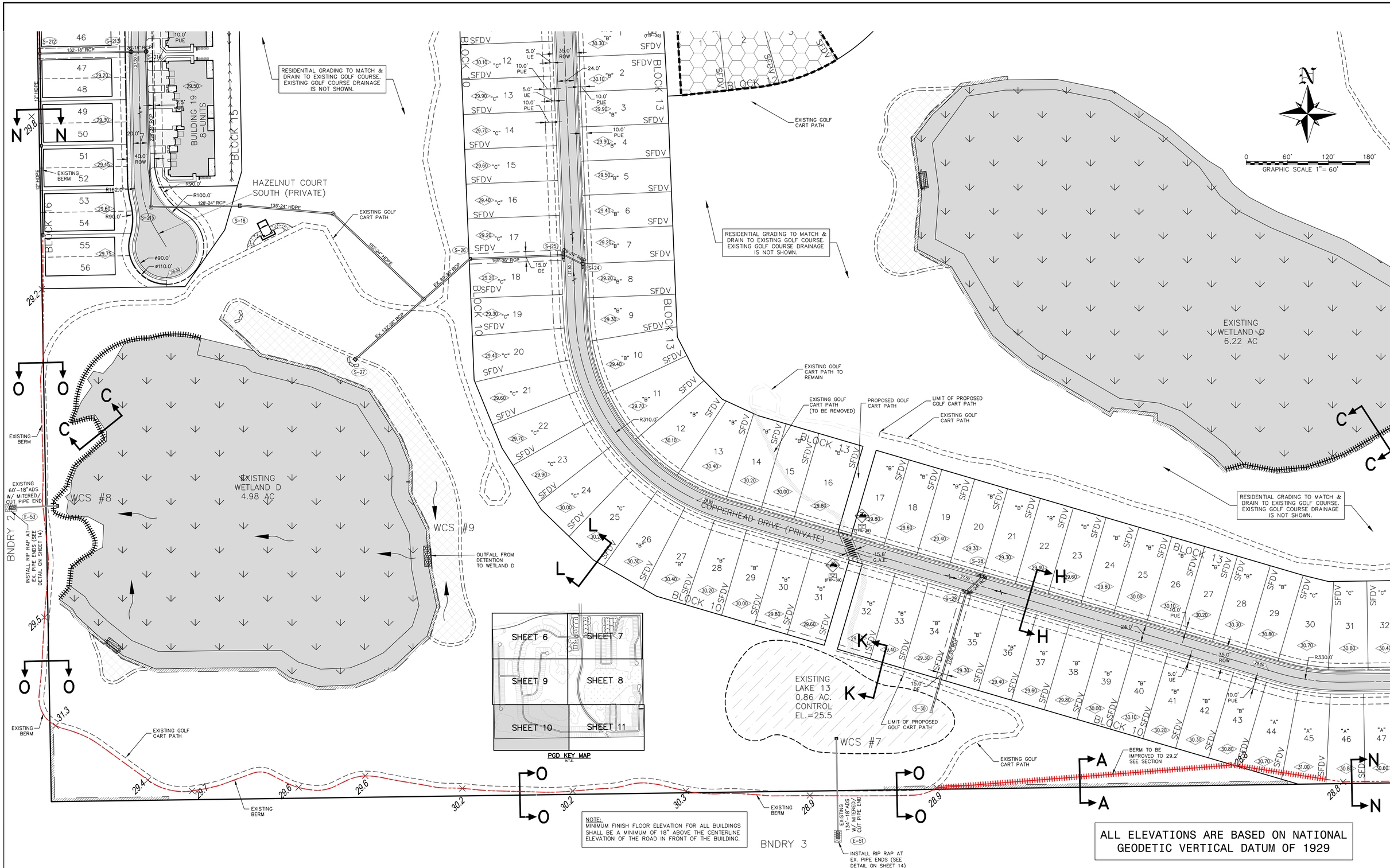
DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE PLAN 4

IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-4	JLW	JLW	DRU	1"=60'	09



NOTE:
MINIMUM FINISH FLOOR ELEVATION FOR ALL BUILDINGS SHALL BE A MINIMUM OF 18" ABOVE THE CENTERLINE ELEVATION OF THE ROAD IN FRONT OF THE BUILDING.

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

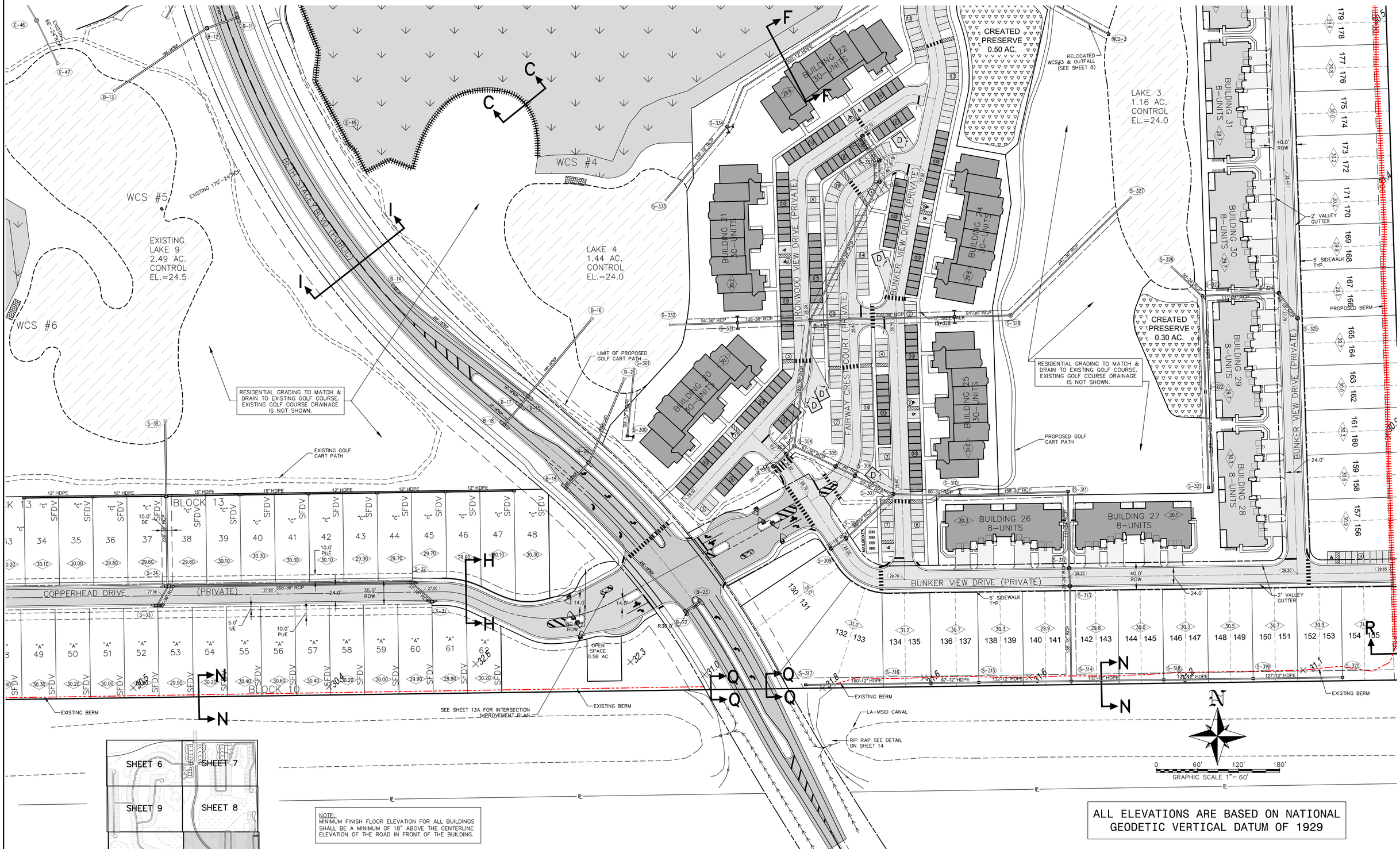
PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

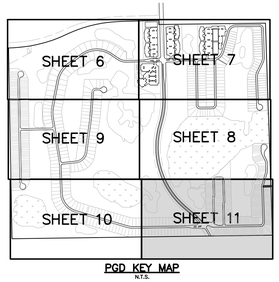
WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE PLAN 5
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-5	JLW	JLW	DRU	1"=60'	10





PGD KEY MAP
N.T.S.

NOTE:
MINIMUM FINISH FLOOR ELEVATION FOR ALL BUILDINGS SHALL BE A MINIMUM OF 18" ABOVE THE CENTERLINE ELEVATION OF THE ROAD IN FRONT OF THE BUILDING.

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

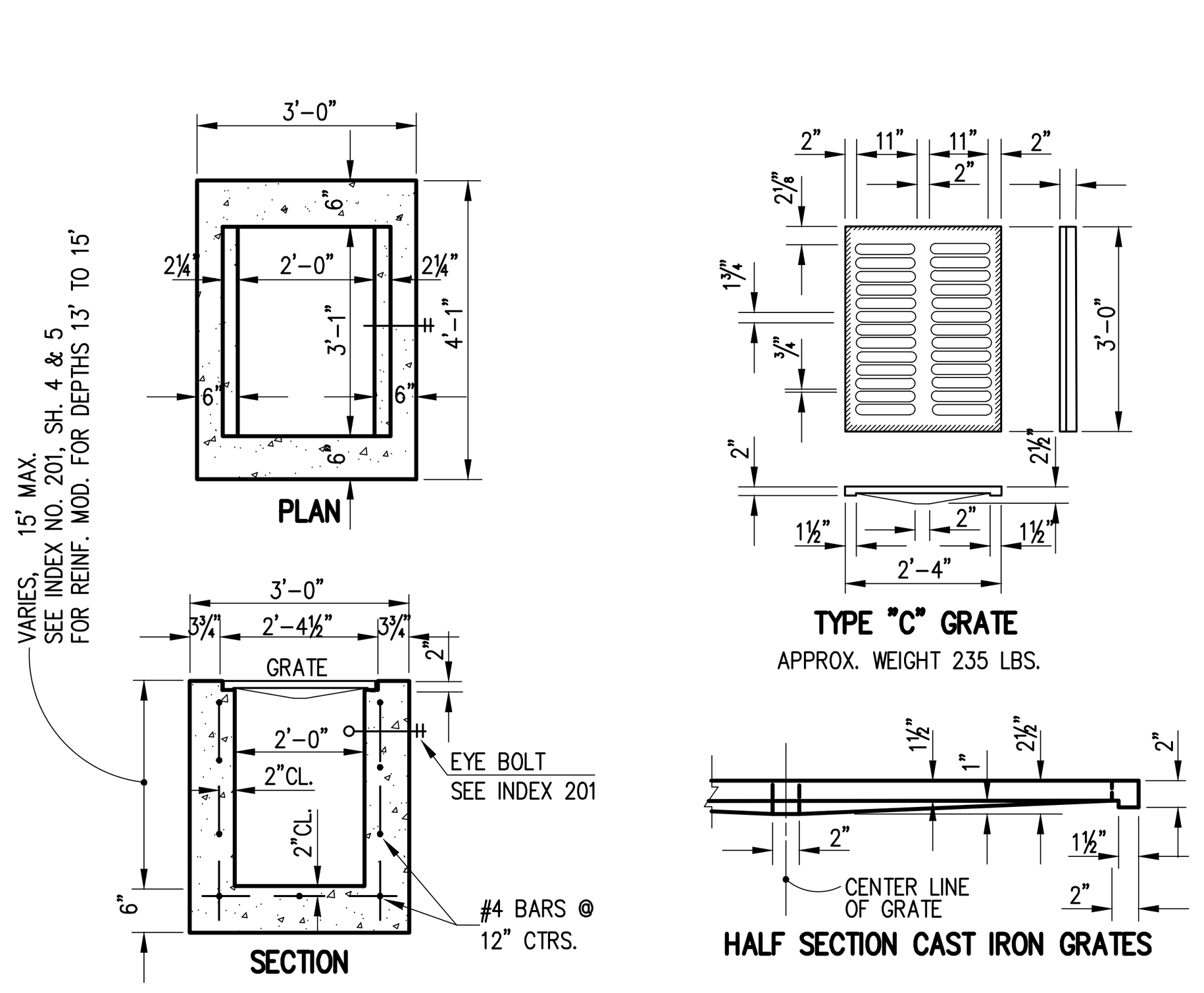
DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE PLAN 6

IBIS LANDING

LEE COUNTY, FLORIDA

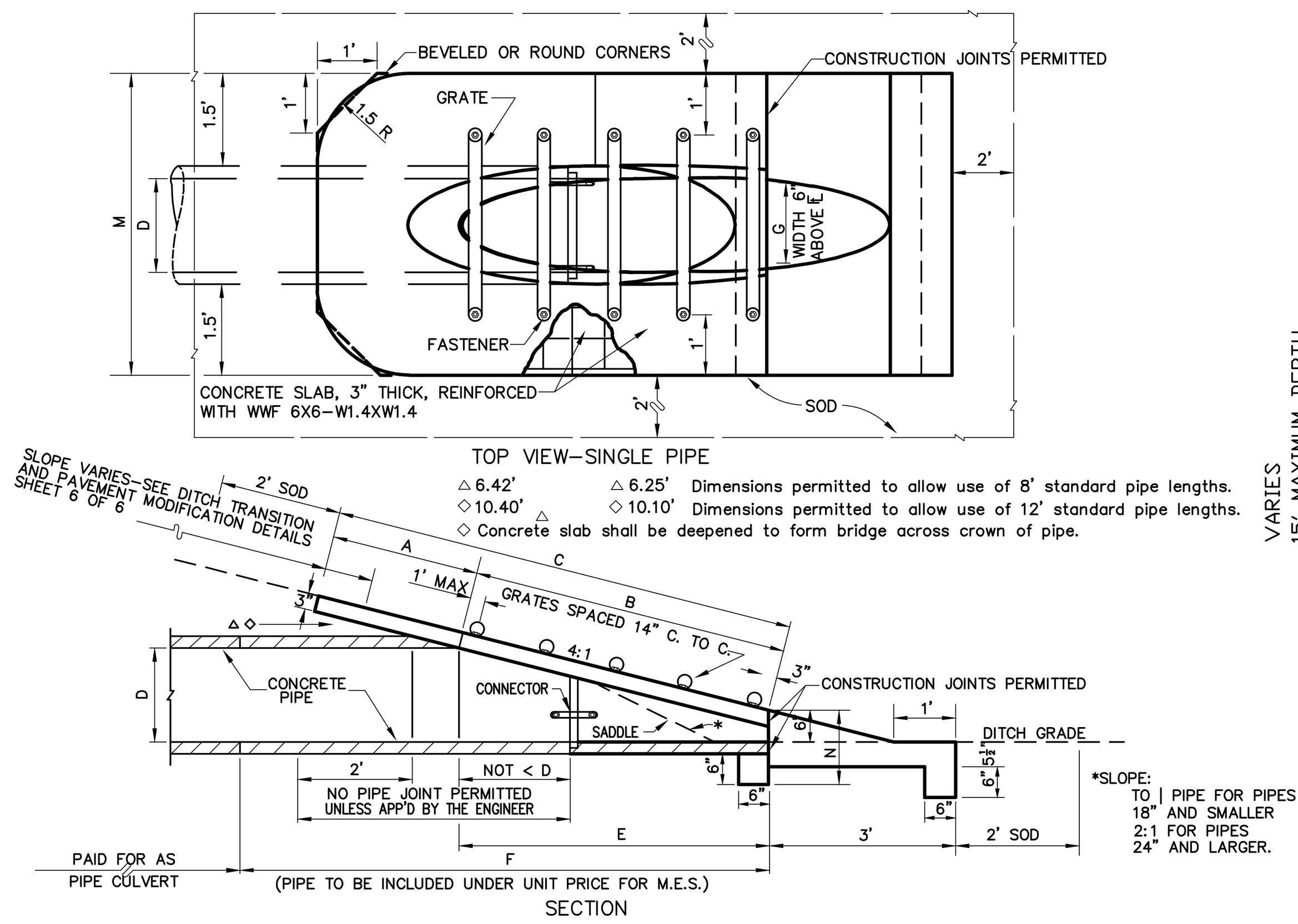
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-6	JLW	JLW	DRU	1"=60'	11



MAXIMUM PIPE SIZE RECOMMENDED FOR EACH OF THE WALL SIZES:
2'-0" WALL – 18" PIPE
3'-1" WALL – 24" PIPE

NOTES:
1. 6" WALL THICKNESS MUST BE USED FOR PRE-CAST INLETS IN ACCORDANCE WITH F.D.O.T. INDEX 201.
2. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH)
3. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 425.

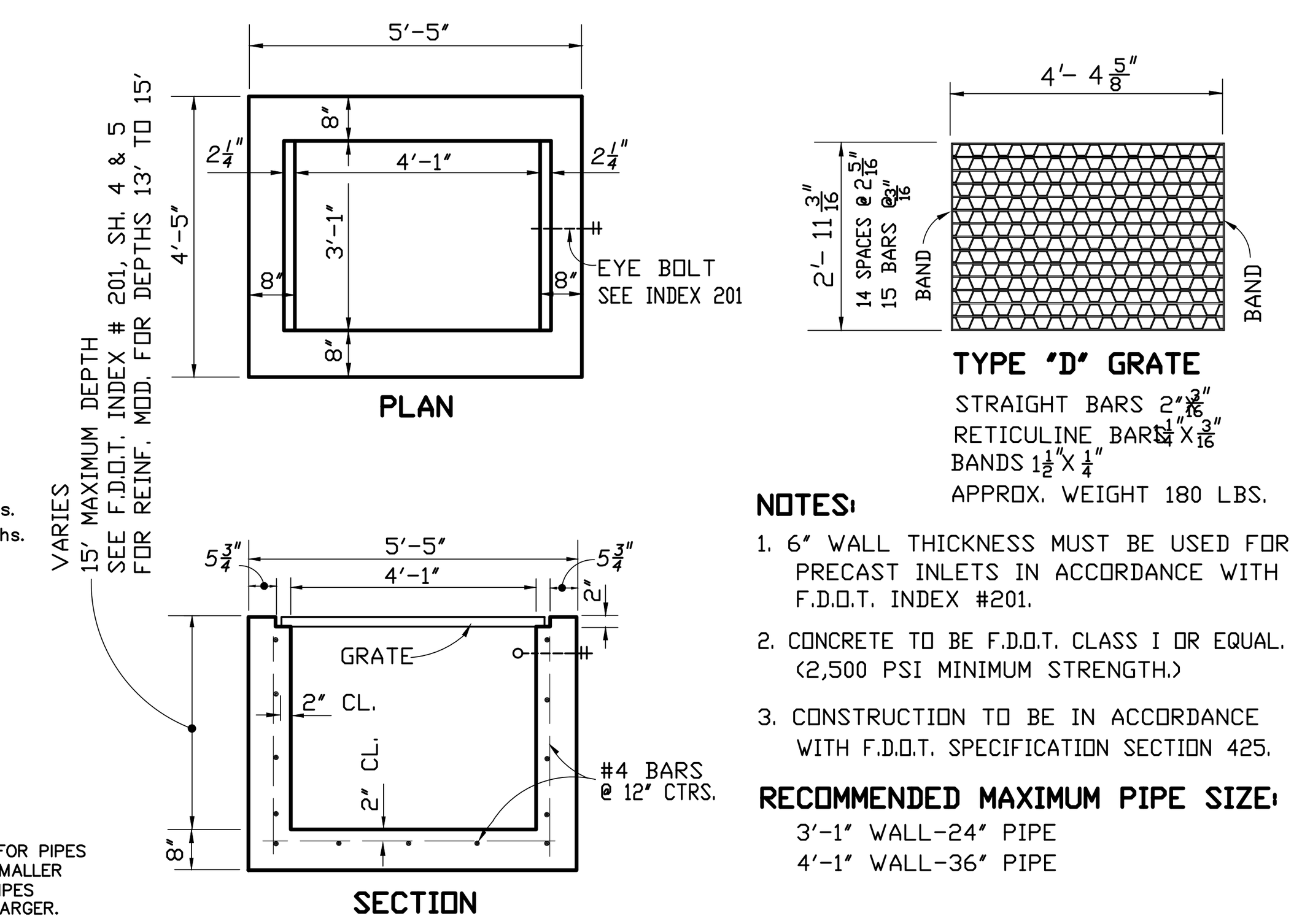
TYPE "C" INLET DETAIL
N.T.S.



DIMENSIONS & QUANTITIES

D	X	A	B	C	E	F	G	M Single Pipe	M Double Pipe	M Triple Pipe	M Quad Pipe	N	Grate Pipe Size
15"	2'-7"	2.27'	4.09'	6.36'	4.03'	8'	1.22'	4.63'	7.21'	9.79'	12.37'	1.19'	None
18"	2'-10"	2.36'	5.12'	7.48'	5.03'	9'	1.41'	4.92'	7.75'	10.58'	13.42'	1.21'	None
24"	3'-5"	2.53'	7.18' △	9.71'	7.03' △	11'	1.73'	5.50'	8.92'	12.33'	15.75'	1.25'	None
30"	4'-3"	2.70'	9.25'	11.95'	9.03'	13'	2.00'	6.08'	10.33'	14.58'	18.83'	1.29'	2½"
36"	5'-1"	2.87'	11.31' ◇	14.18'	11.03' ◇	15'	2.24'	6.67'	11.75'	16.83'	21.92'	1.33'	2½"
42"	6'-0"	3.05'	13.37'	16.42'	13.03'	17'	2.45'	7.25'	13.25'	19.25'	25.25'	1.38'	2½"
48"	6'-9"	3.22'	15.43'	18.65'	15.03'	19'	2.65'	7.83'	14.58'	21.33'	28.08'	1.42'	2½"
54"	7'-8"	3.39'	17.49'	20.88'	17.03'	21'	2.83'	8.42'	16.08'	23.75'	31.42'	1.46'	3"
60"	8'-6"	3.56'	19.55'	23.11'	19.03'	23'	3.00'	9.00'	17.50'	26.00'	34.50'	1.50'	3"

MITERED END SECTION
N.T.S.

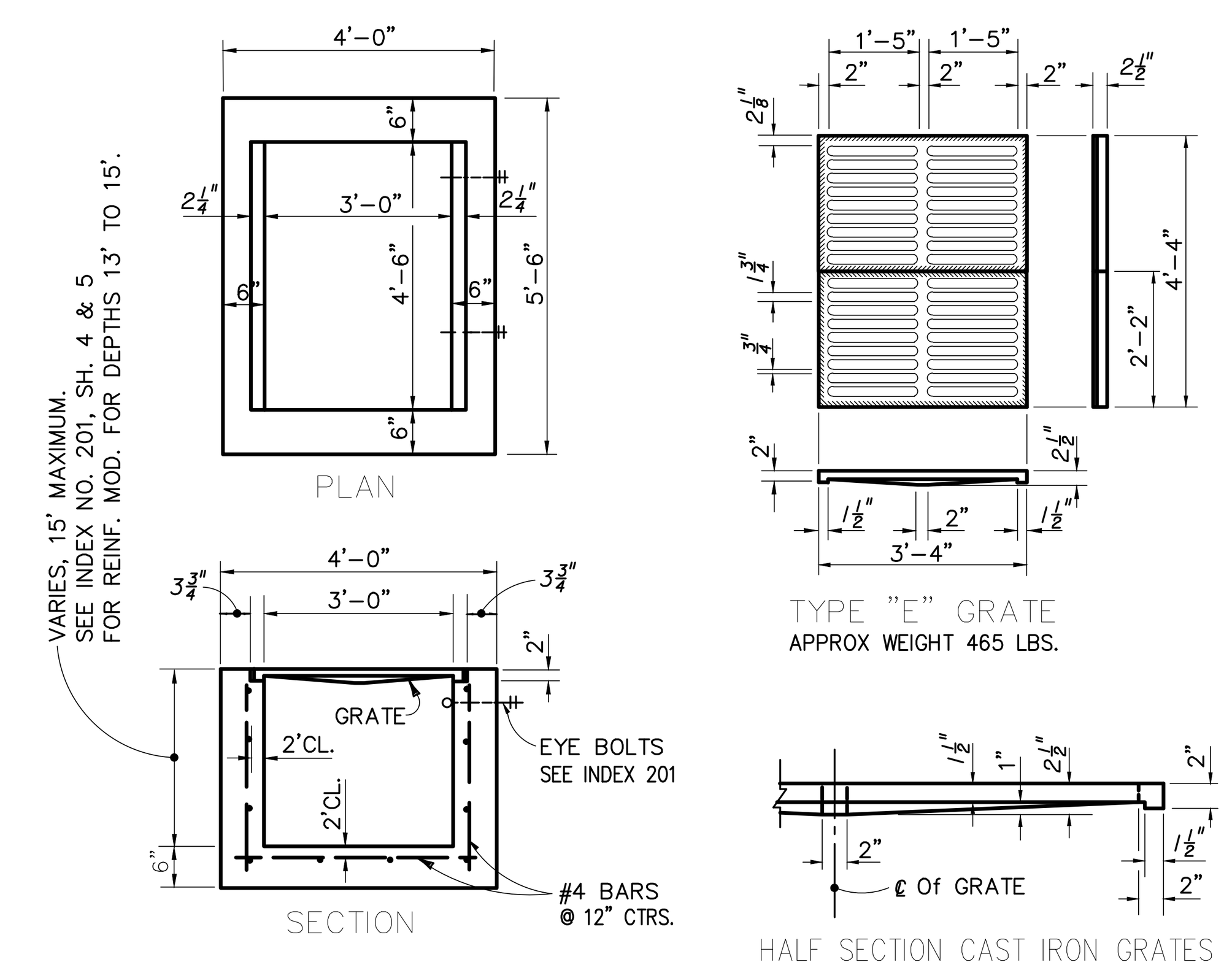


NOTES:
1. 6" WALL THICKNESS MUST BE USED FOR PRECAST INLETS IN ACCORDANCE WITH F.D.O.T. INDEX #201.
2. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH.)
3. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 425.

RECOMMENDED MAXIMUM PIPE SIZE:
3'-1" WALL-24" PIPE
4'-1" WALL-36" PIPE

TYPE 'D' INLET DETAIL
N.T.S.

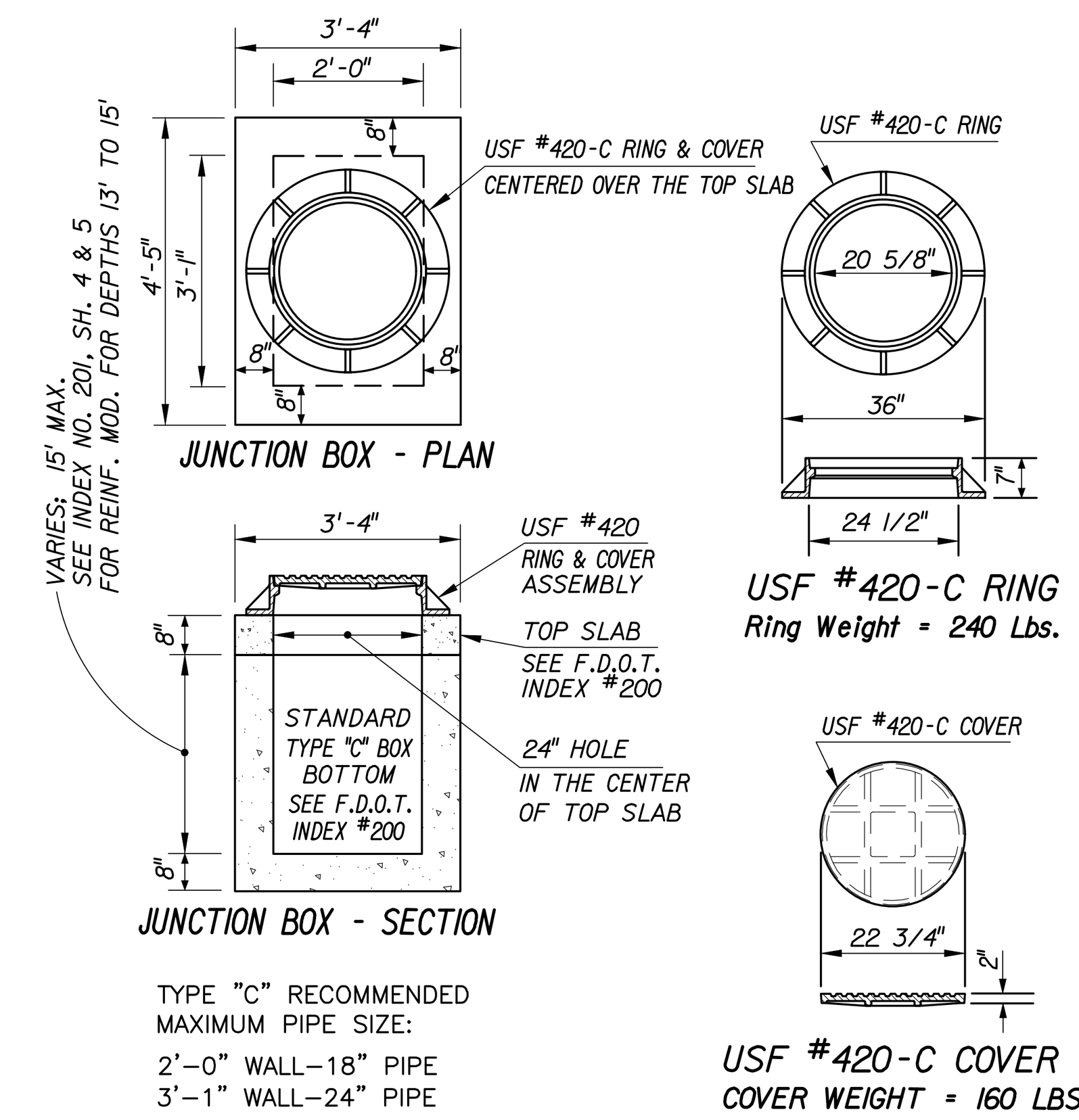
REINFORCING NOTES:
1. REINFORCING BARS TO BE PLACED A MINIMUM OF 2" FROM BOTTOM & 3" FROM SIDES OF CURB & GUTTER.



MAXIMUM PIPE SIZES RECOMMENDED FOR EACH OF THE WALL SIZES.
3'-0" WALL – 24" PIPE
4'-6" WALL – 42" PIPE

NOTE:
1. 6" WALL THICKNESS MUST BE USED FOR PRE-CAST INLETS IN ACCORDANCE WITH F.D.O.T. INDEX 201.
2. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH)
3. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATIONS SECTION 425.

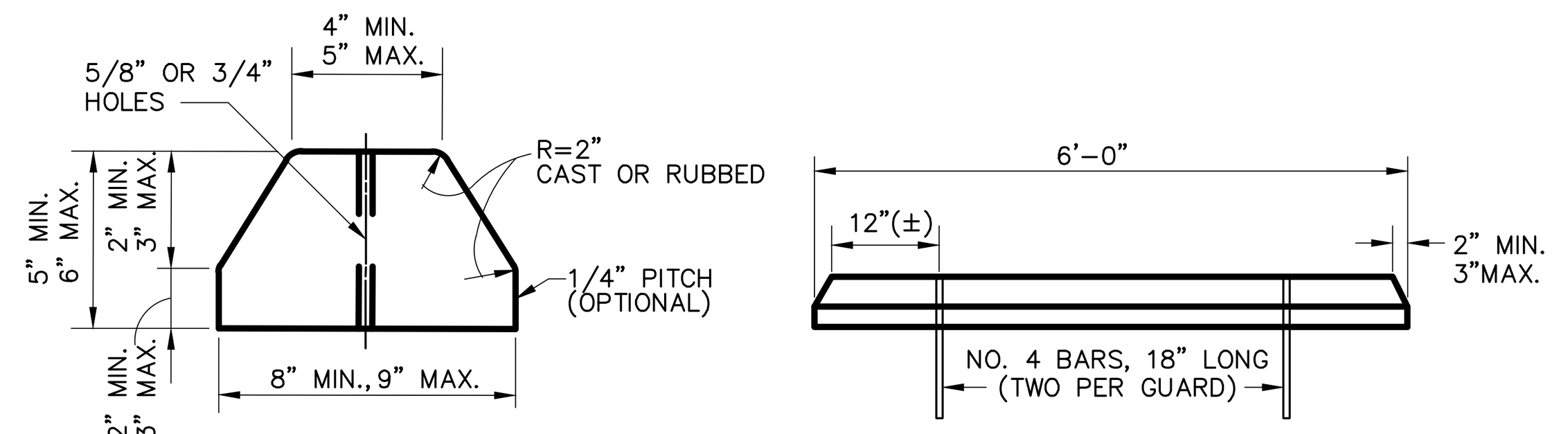
TYPE "E" (DITCH BOTTOM) INLET DETAIL
N.T.S.



TYPE "C" RECOMMENDED MAXIMUM PIPE SIZE:
2'-0" WALL–18" PIPE
3'-1" WALL–24" PIPE

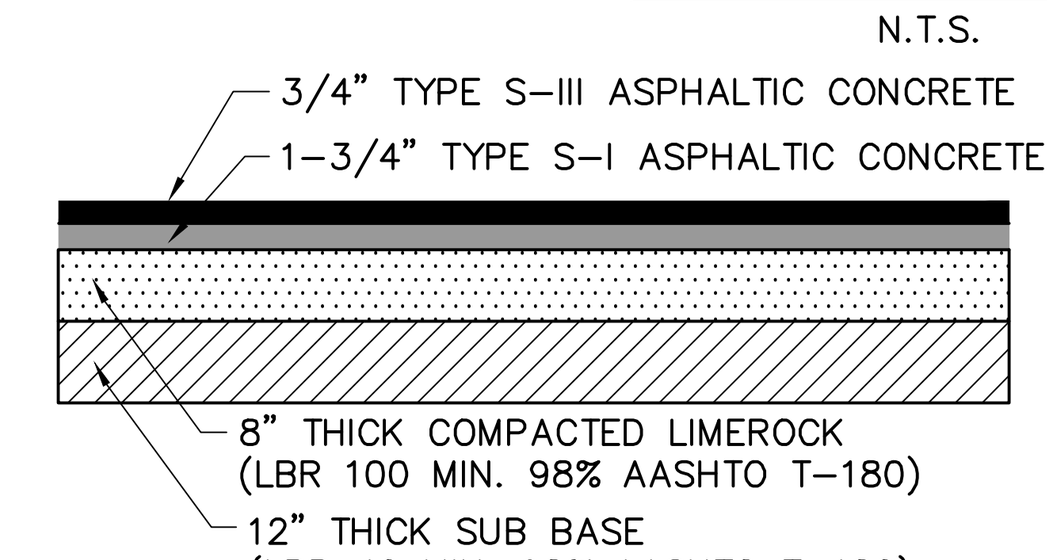
NOTE:
1. 6" WALL THICKNESS MUST BE USED FOR PRE-CAST INLETS IN ACCORDANCE ACCORDANCE WITH F.D.O.T. INDEX #201.
2. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH)
3. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATIONS SECTION 450.

TYPE "C" JUNCTION BOX DETAIL
N.T.S.

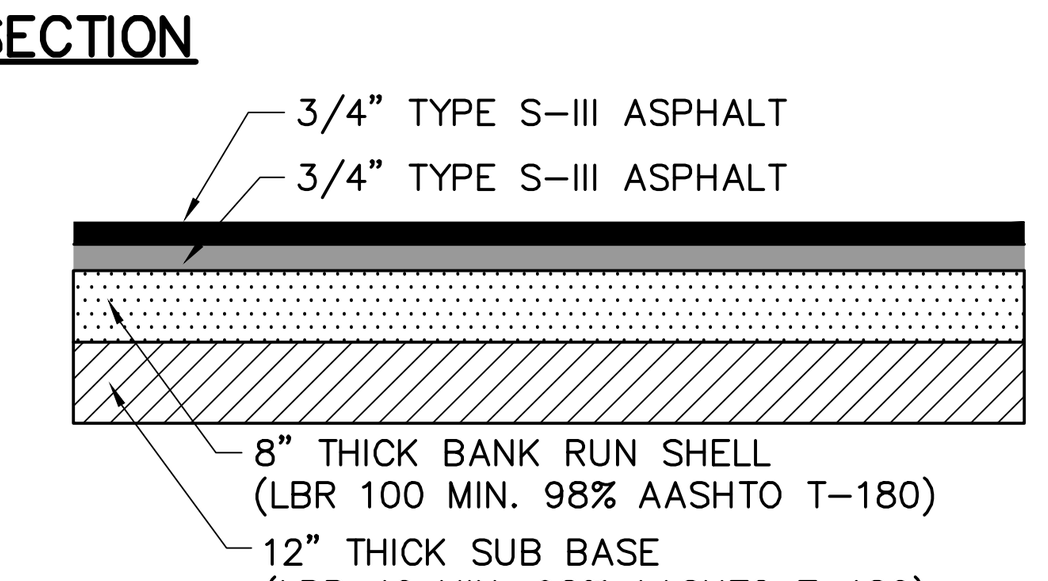


NOTE: CONCRETE TO BE 2,500 PSI STRENGTH MINIMUM.

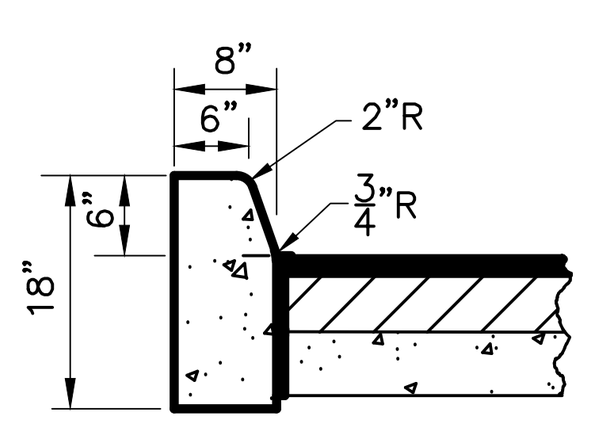
WHEEL STOP DETAIL
N.T.S.



BETH STACEY R.O.W. PAVEMENT SECTION, TYPICAL
N.T.S.

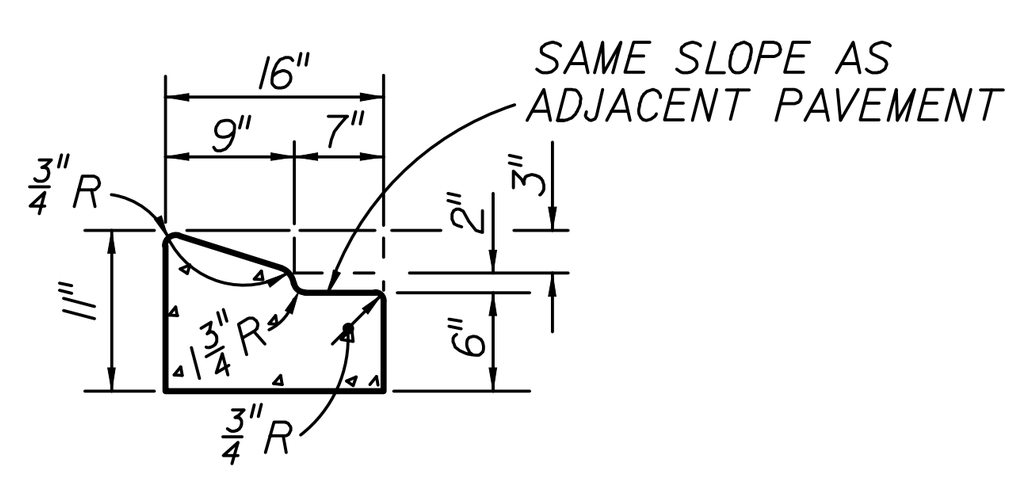


LOCAL STREET & PARKING LOT PAVEMENT SECTION, TYPICAL
N.T.S.

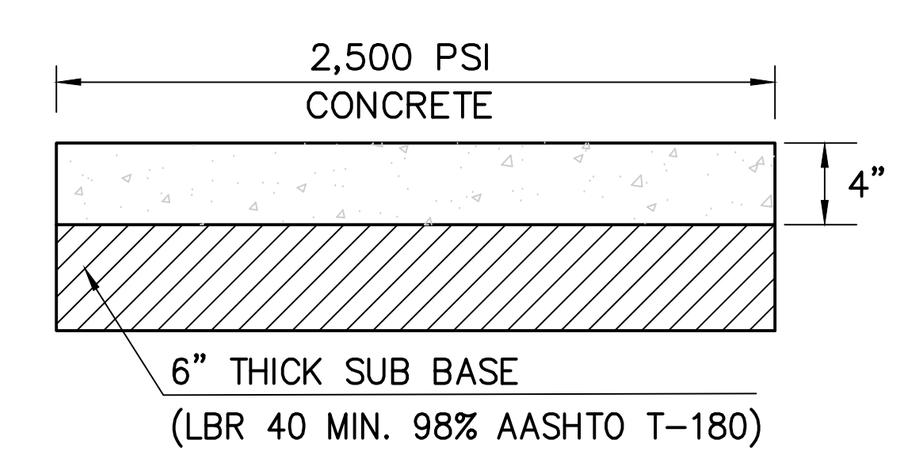


NOTES:
1. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH).
2. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 520.

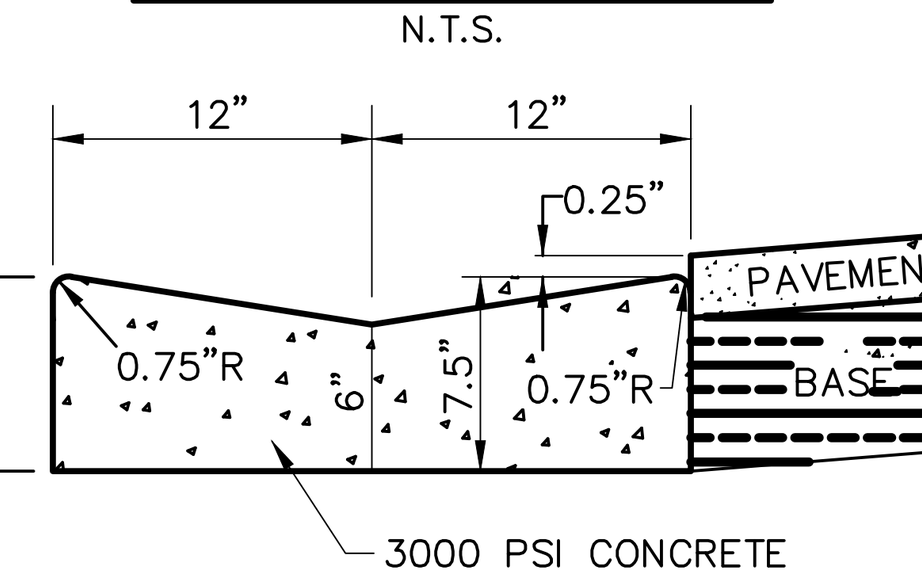
TYPE "D" CURB DETAIL
N.T.S.



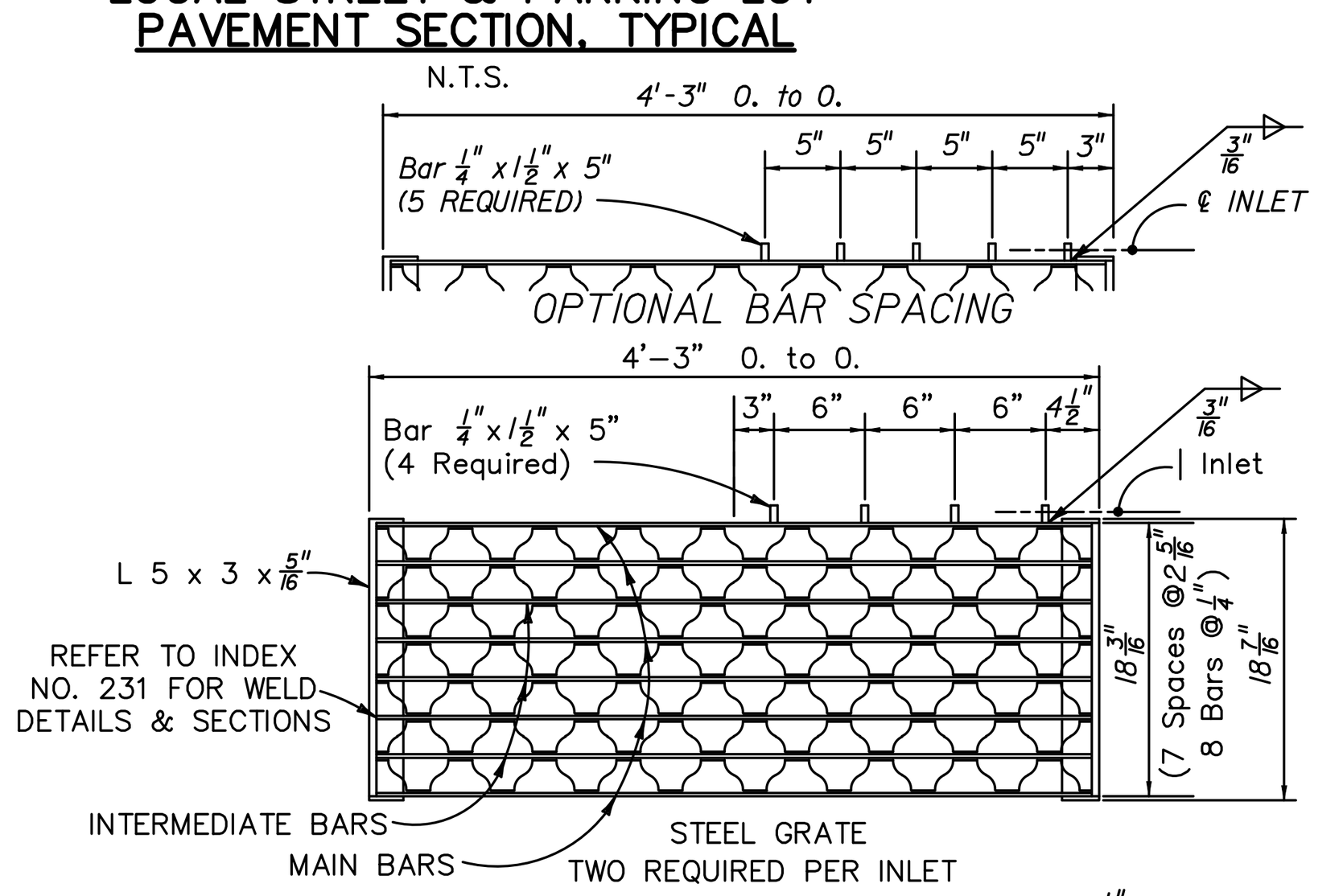
STANDARD FDOT TYPE A CURB DETAIL
N.T.S.



SIDEWALK DETAIL
N.T.S.

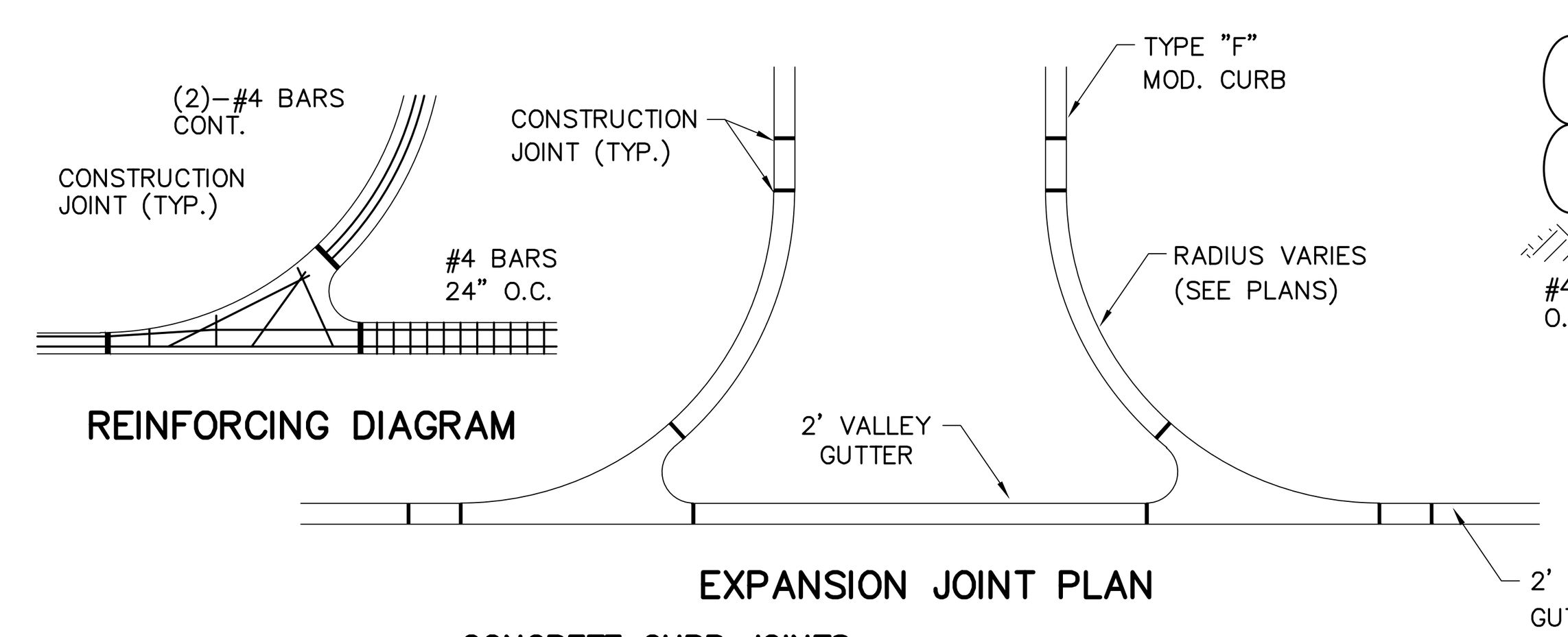


STANDARD 2' CONCRETE VALLEY GUTTER DETAIL
N.T.S.



STEEL GRATE : MANUFACTURED BY BORDEN, FLORIDA STEEL, U.S. FOUNDRY IRVING, RELIANCE, GREULICH (OR EQUAL).

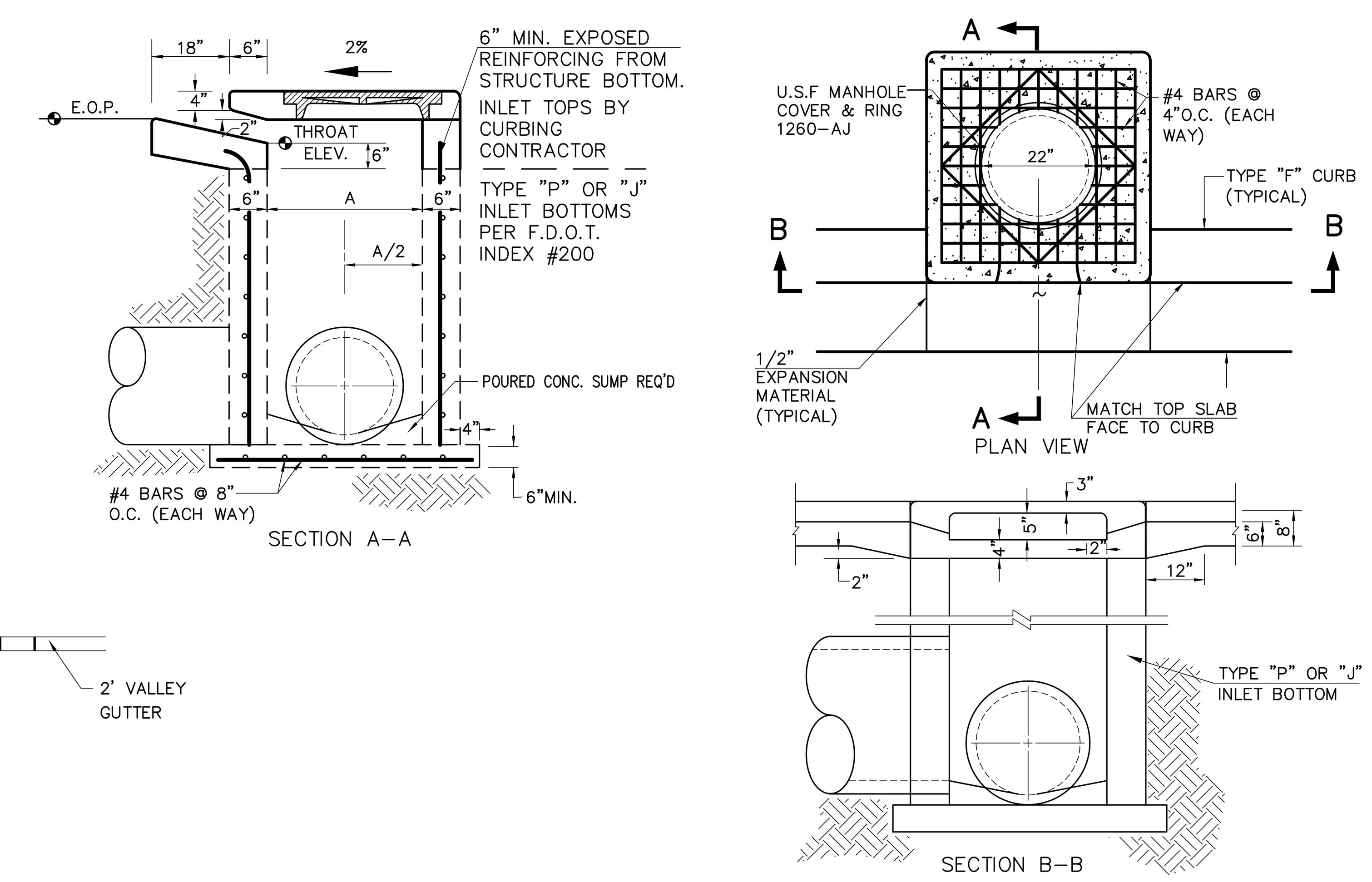
GRATE DETAIL
N.T.S.



EXPANSION JOINT PLAN

CONCRETE CURB JOINTS:
1. CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH OF 3,000 PSI IN 28 DAYS. UNLESS OTHERWISE NOTED.
2. AN EXPANSION JOINT WILL BE PLACED AT THE END OF ALL RETURNS AT INTERVALS NOT TO EXCEED 50' WITH CONTRACTION JOINTS AT 10' INTERVALS BETWEEN.
3. PVC SLIP JOINTS SHALL BE CONSTRUCTED TO PERMIT EXPANSION AND CONTRACTION OF STEEL ACROSS JOINT.
4. EXPANSION JOINTS SHALL BE CONSTRUCTED WITH 1/2" BITUMINOUS IMPREGNATED WITH EXPANSION JOINT MATERIAL.

VALLEY CROSSING DETAIL
N.T.S.

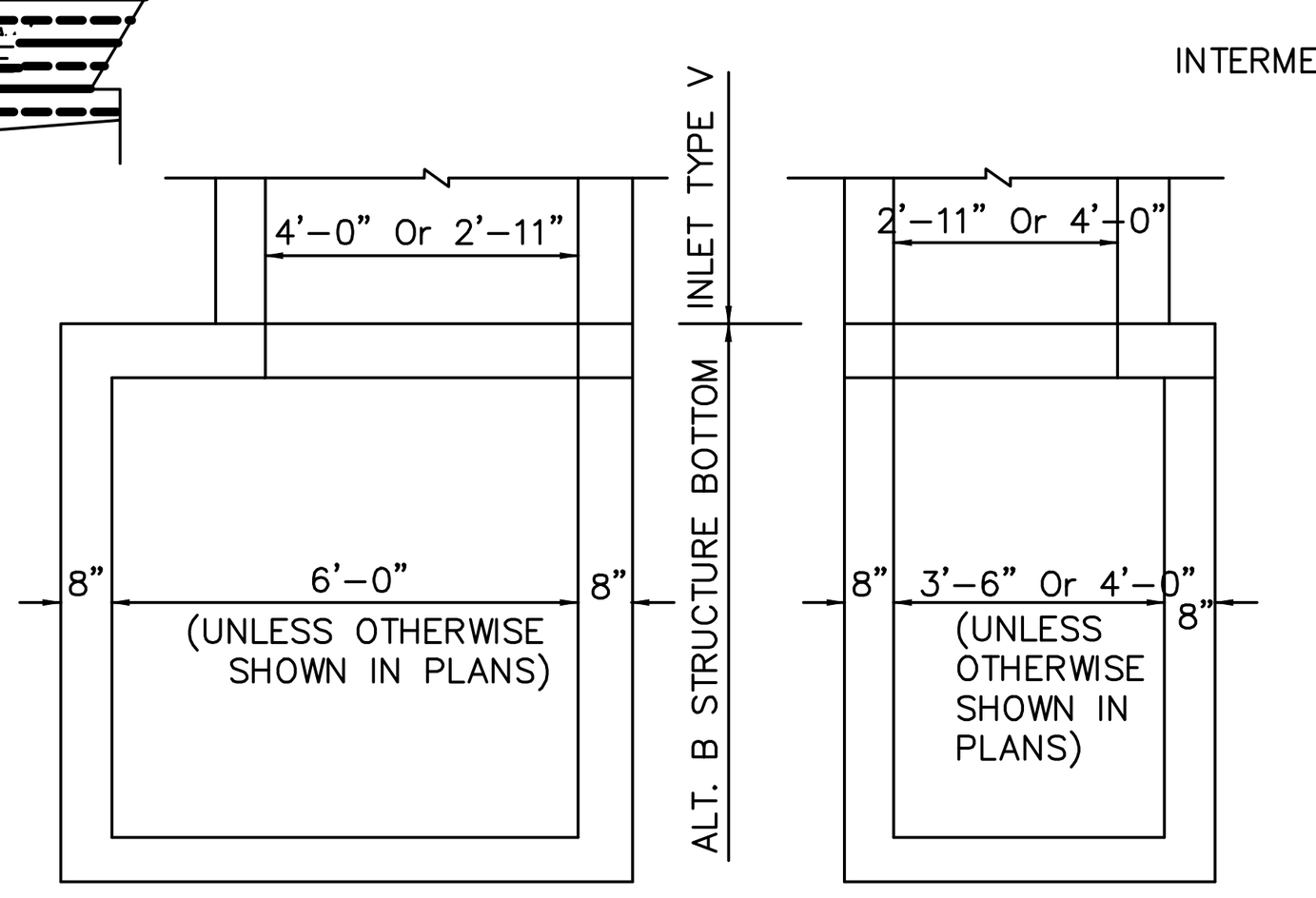


CURB INLET DETAIL
N.T.S.



NOTE: FTP-20-04 HANDICAPPED PARKING SIGN SHALL BE INSTALLED 84" ABOVE GRADE MEASURED TO THE BOTTOM OF THE SIGN.

FTP-20-04 HANDICAP PARKING SIGN DETAIL
N.T.S.



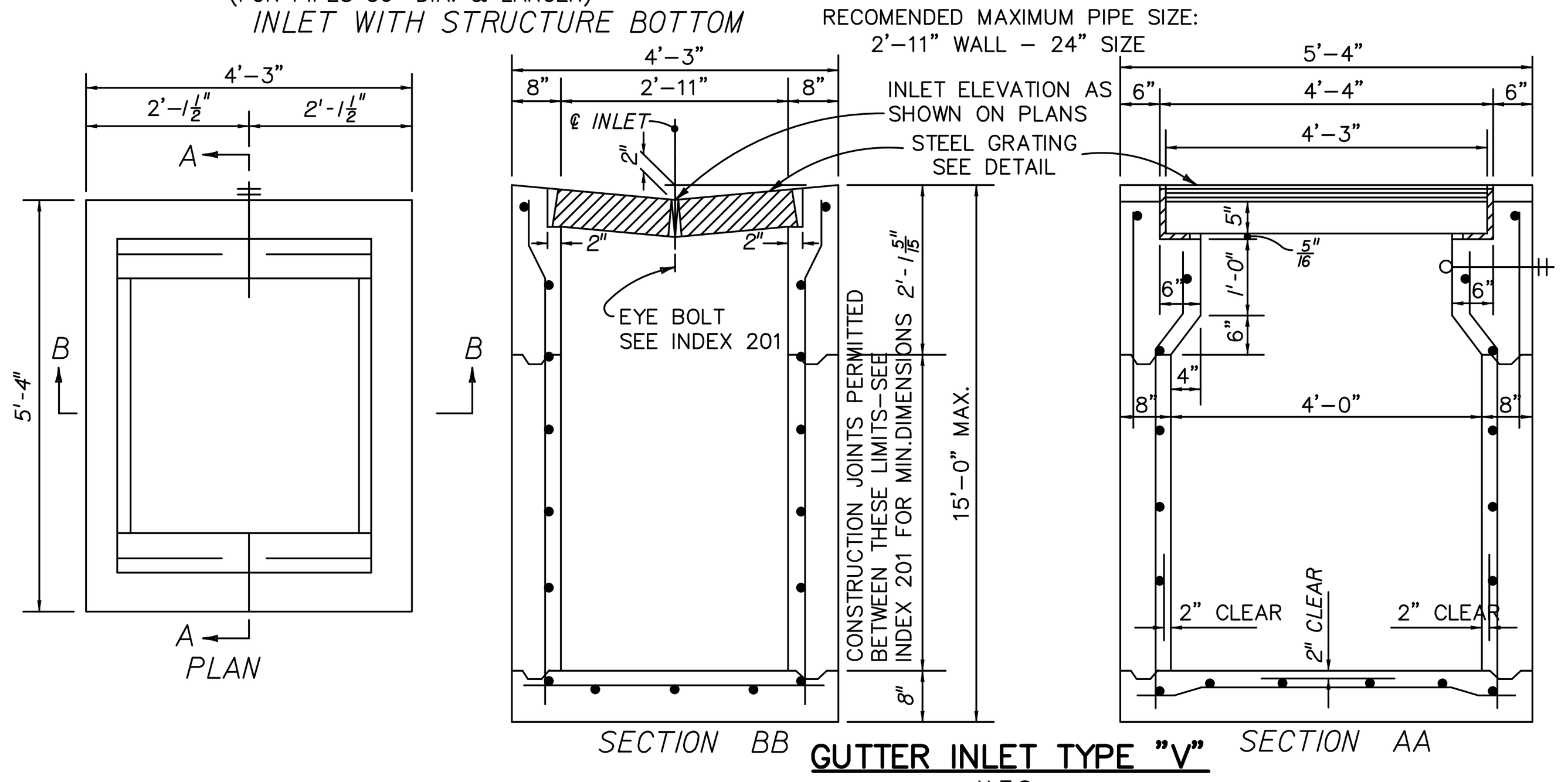
NOTE: ALT. B STRUCTURE BOTTOM ONLY. SEE INDEX NO. 200 (FOR PIPES 30" DIA. & LARGER)

INLET WITH STRUCTURE BOTTOM

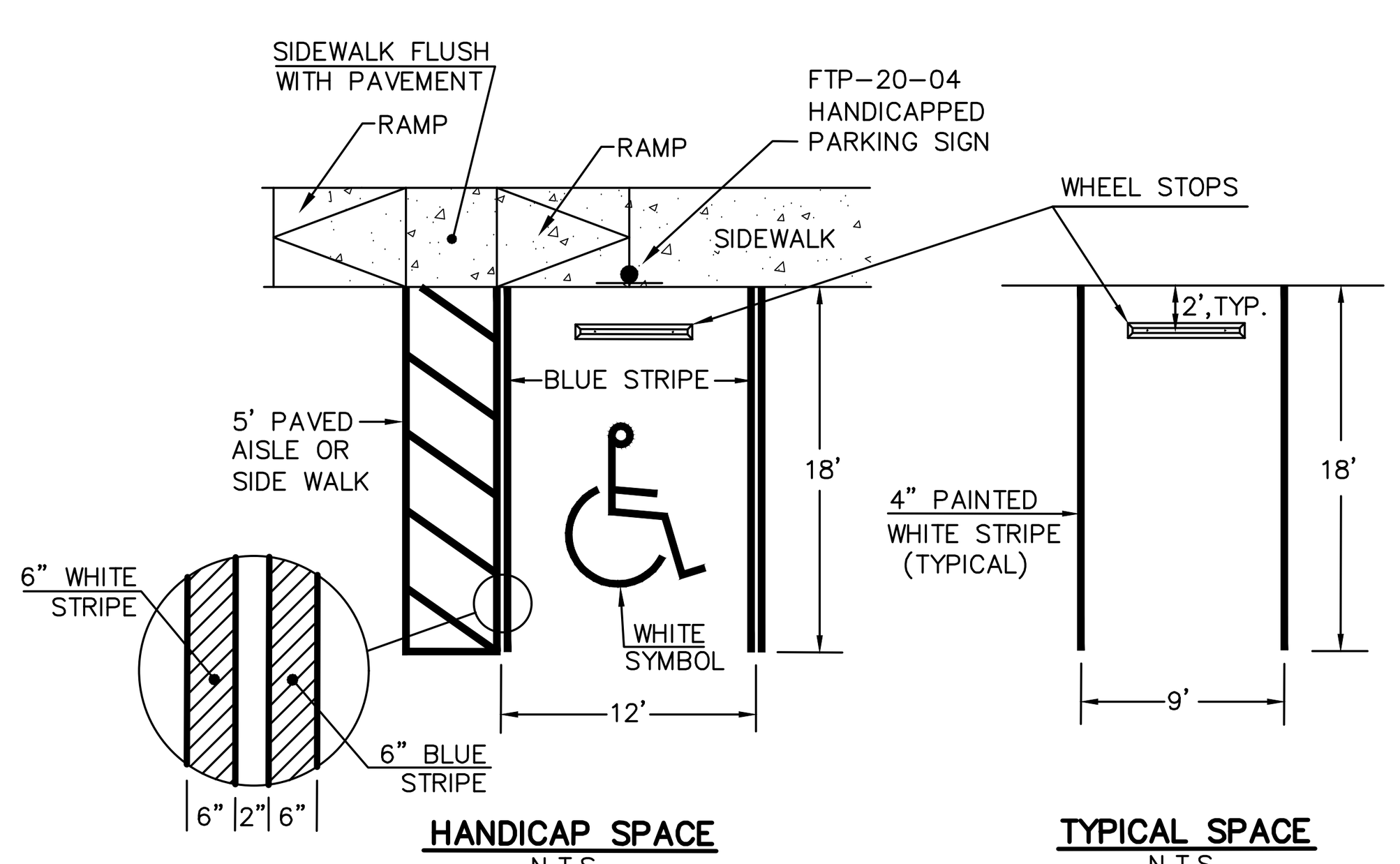
GENERAL NOTES

1. THIS INLET IS DESIGNED FOR VILLAGE SWALES, DITCHES, OR OTHER AREAS SUBJECT TO HEAVY WHEEL LOADS, MINIMUM DEBRIS & SUBJECT TO PEDESTRIAN &/OR BICYCLE TRAFFIC.
2. WHEN ALTERNATE "G" GRATE IS SPECIFIED IN PLANS, THE GRATE IS TO BE HOT DIPPED GALVANIZED AFTER FABRICATION.
3. REINFORCING - NO. 4 BARS AT 12" CTRS. BOTH WAYS. CUT OR BEND BARS OUT OF WAY OF PIPE TO CLEAR PIPE 1-1/2".
4. ALL EXPOSED EDGES AND CORNERS SHALL BE TOOLED TO 1/4" RADIUS.
5. RECOMENDED MAXIMUM PIPE SIZES SHOWN ARE FOR CONCRETE PIPE.
6. FOR SUPPLEMENTARY DETAILS SEE INDEX NO. 201.

RECOMENDED MAXIMUM PIPE SIZE:
2'-11" WALL – 24" SIZE

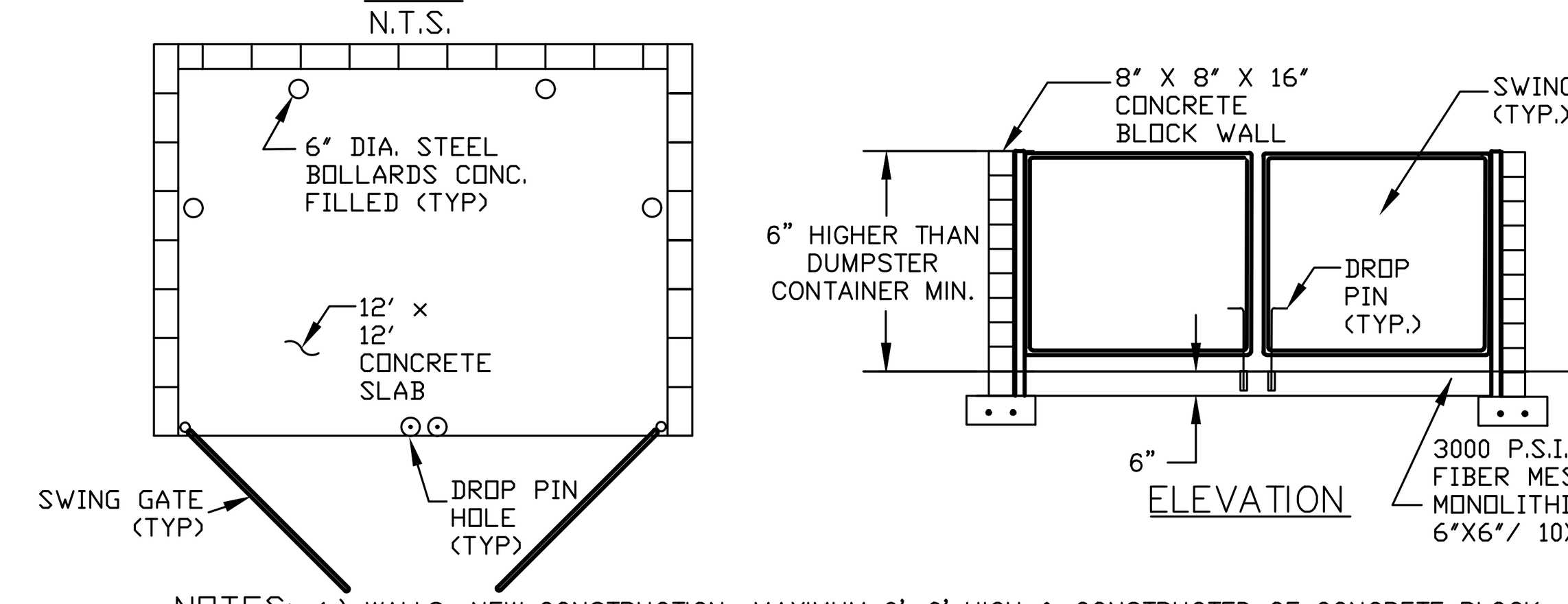


GUTTER INLET TYPE "V"
N.T.S.



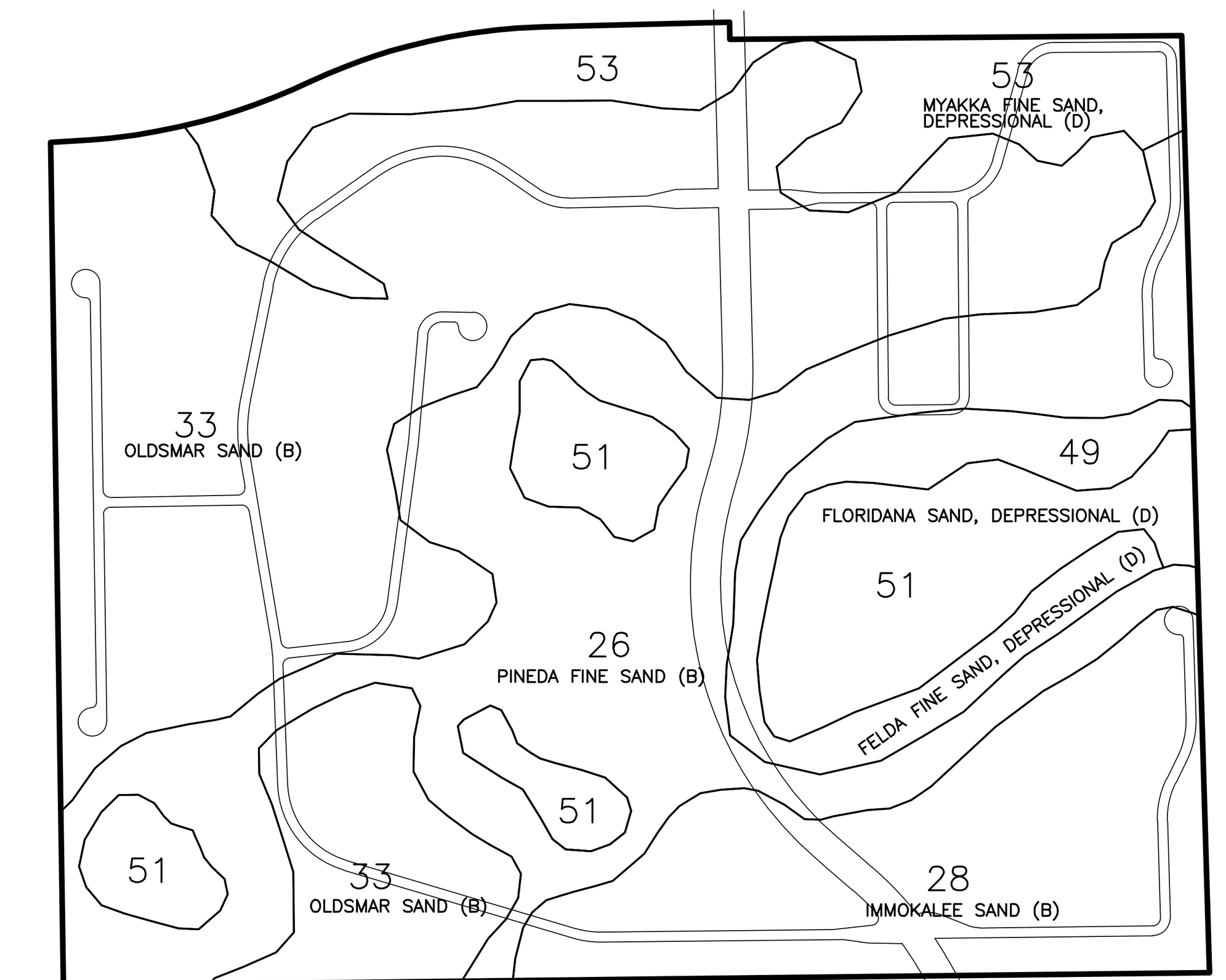
NOTE: ALL STRIPING TO BE HEAVY DUTY ACRYLIC BASED TRAFFIC PAINT.

PARKING SPACE DETAIL
N.T.S.



NOTES: 1.) WALLS: NEW CONSTRUCTION: MAXIMUM 6'-0' HIGH & CONSTRUCTED OF CONCRETE BLOCK (8"X8"X16"). MATERIALS USED SHOULD BE CONSISTENT WITH THOSE USED IN THE CONSTRUCTION OF & ARCHITECTURAL STYLE OF THE PRINCIPAL BUILDING.
2.) GATES: SHALL BE CONSTRUCTED OF WOOD OR OTHER COUNTY APPROVED OPAQUE MATERIAL.
3.) DROP PIN: 1/2" DIA. 18" LONG METAL ROD GATE LATCH. PROVIDE 3/4" DIA. HOLE 6" INTO CONCRETE FOR RODS. IN ASPHALT PROVIDE A 3/4" P.V.C. SLEEVE, 6" LONG, OR EQUAL.
4.) GRADING: SEE PLANS FOR GRADING OF CONCRETE SLAB.

DUMPSTER DETAIL
N.T.S.



SCS SOIL MAP DETAIL
N.T.S.

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE DETAILS 1
IBIS LANDING
LEE COUNTY, FLORIDA

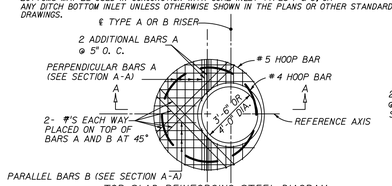
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-DT-1	JLW	JLW	DRU	NTS	12

IBIS LANDING

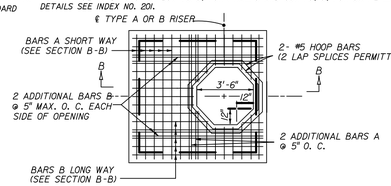
S:\JOBS\31XX\3142L\ENGINEERING\DEVPLANS\SFWMD PLANS\MODIFICATION\3142-SFWMD_MOD_12_DT-PGD-1.DWG 9/22/2025 12:50 PM JIMMY WHEWAY

STRUCTURE BOTTOM NOTES:

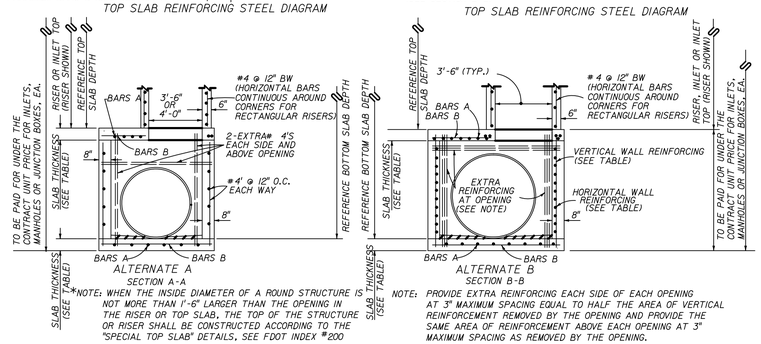
1. STANDARD STRUCTURE BOTTOMS 4'-0" DIAMETER & SMALLER (ALT. A) & 3'-6" SQUARE (ALT. B) ARE DESIGNATED TYPE P. LARGER STANDARD STRUCTURE BOTTOMS ARE DESIGNATED TYPE J. RISERS ARE PERMITTED FOR ALL STRUCTURES.
2. WALLS OF CIRCULAR STRUCTURES (ALTERNATE A) CONSTRUCTED IN PLACE MAY BE OF NON-REINFORCED CONCRETE OR BRICK OR REINFORCED CONCRETE. PRECAST AND RECTANGULAR STRUCTURES (ALTERNATE B) SHALL BE CONSTRUCTED OF REINFORCED CONCRETE ONLY.
3. WALL THICKNESS & REINFORCEMENT ARE FOR EITHER REINFORCED CAST-IN-PLACE OR PRECAST CONCRETE UNITS EXCEPT THAT PRECAST CIRCULAR UNITS MAY BE FURNISHED WITH WALLS IN ACCORDANCE WITH EITHER A.S.T.M. C-478 (UP TO 96" DIAMETER) OR A.S.T.M. C-76, CLASS III, B WALL, MODIFIED WHERE THE ELLIPTICAL STEEL CAGE AREA IS PLACED IN THE CENTER ONE-THIRD OF THE WALL.
4. TOP AND FLOOR SLAB THICKNESS AND REINFORCEMENT ARE FOR PRECAST & CAST IN PLACE CONSTRUCTION. TOP & FLOOR SLABS SHALL BE OF CLASS II CONCRETE. CONCRETE AS SPECIFIED IN A.S.T.M. C-478 (4000 PSI) MAY BE USED IN LIEU OF CLASS I AND CLASS II CONCRETE IN PRECAST ITEMS MANUFACTURED IN PLANTS WHICH ARE UNDER THE 'STANDARD OPERATING PROCEDURES FOR THE INSPECTION OF PRECAST DRAINAGE PRODUCTS.
5. ALL REINFORCEMENT SHALL BE A.S.T.M. A615, GRADE 60 OR 65 KSI WELDED WIRE FABRIC, EITHER SMOOTH OR DEFORMED.
6. STRUCTURE BOTTOMS MAY BE USED IN CONJUNCTION WITH CURB INLET TOPS TYPES I, 2, 3, 4, 5, 6, 9, AND 10, & ANY MANHOLE OR JUNCTION BOX UNLESS OTHERWISE SHOWN IN THE PLANS OR OTHER STANDARD DRAWINGS. ALT. B STRUCTURE BOTTOMS MAY BE USED IN CONJUNCTION WITH CURB INLET TYPES 7 & 8, OR ANY DITCH BOTTOM INLET UNLESS OTHERWISE SHOWN IN THE PLANS OR OTHER STANDARD DRAWINGS.
7. RECTANGULAR STRUCTURES MAY BE ROTATED AS DIRECTED BY THE ENGINEER IN ORDER TO FACILITATE CONNECTIONS BETWEEN THE STRUCTURE WALLS & STORM SEWER PIPES.
8. EXCEPT WHEN ACI HOOKS ARE SPECIFICALLY REQUIRED, EMBEDMENT HOOKS IN THE TOP AND BOTTOM SLABS MAY BE REPLACED WITH STRAIGHT EMBEDMENTS OR PERIPHERAL REINFORCEMENT IN ACCORDANCE WITH THE REINFORCEMENT DETAIL SHOWN UNDER REBAR STRAIGHT END EMBEDMENT OR PERIPHERAL REINFORCEMENT IN LIEU OF ACI STANDARD HOOKS FOR TOP AND BOTTOM SLABS, INDEX NO. 201, SHEET 3 OF 6.
9. ALL STEEL BARS SHALL HAVE 1½" MINIMUM COVER UNLESS OTHERWISE SHOWN. HORIZONTAL STEEL IN RECTANGULAR STRUCTURES SHALL BE LAPPED A MINIMUM OF 24 BAR DIAMETERS AT CORNERS.
10. THE CORNER FILLETS SHOWN ARE NECESSARY FOR RECTANGULAR STRUCTURES USED WITH CIRCULAR RISERS & INLET THROATS & USED ON SKEW WITH RECTANGULAR RISERS, INLET & INLET THROATS. FILLETS WILL BE REQUIRED IN LIEU OF THE BOTTOM SLAB OF THE ALT. B RISER WHEN USED WITH THE ALT. A BOX. EACH FILLET SHALL BE REINFORCED WITH 2- #5 BARS.
11. INLET THROATS, RISER OR MANHOLE TOPS SHALL BE SECURED TO STRUCTURES AS SHOWN ON INDEX NO. 201.
12. STRUCTURES WITH DEPTHS OVER 14' ARE TO BE CHECKED FOR FLOATATION BY DESIGNER OF PROJECT DRAINAGE.
13. UNITS LARGER THAN SPECIFIED STANDARD MAY BE SUBSTITUTED AT THE CONTRACTOR'S OPTION WHEN THESE UNITS WILL NOT CAUSE OR INCREASE THE SEVERITY OF UTILITY CONFLICTS. SUCH LARGER UNITS SHALL BE FURNISHED AT NO ADDITIONAL COST TO THE DEPARTMENT. LARGER ALTERNATE A UNITS CANNOT REPLACE ALTERNATE B UNITS WITHOUT APPROVAL OF THE ENGINEER. THIS NOTE APPLIES TO THIS INDEX ONLY.
14. FOR MANHOLE AND JUNCTION BOX TOPS, FOR FRAMES AND COVERS, &, FOR SUPPLEMENTARY DETAILS SEE INDEX NO. 201.



TOP SLAB REINFORCING STEEL DIAGRAM



TOP SLAB REINFORCING STEEL DIAGRAM

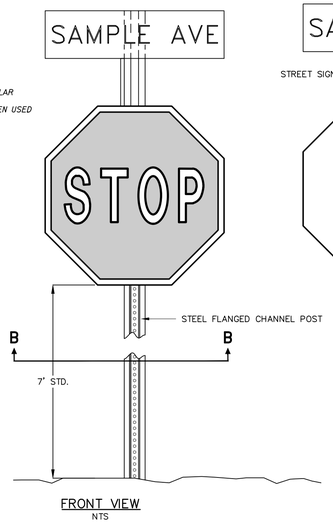


*NOTE: WHEN THE INSIDE DIAMETER OF A ROUND STRUCTURE IS NOT MORE THAN 1'-6" LARGER THAN THE OPENING IN THE RISER OR TOP SLAB, THE TOP OF THE STRUCTURE OR RISER SHALL BE CONSTRUCTED ACCORDING TO THE "SPECIAL TOP SLAB" DETAILS, SEE FDOT INDEX #200

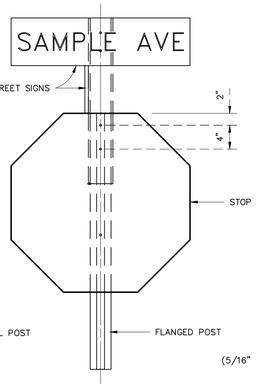
NOTE: PROVIDE EXTRA REINFORCING EACH SIDE OF EACH OPENING AT 3" MAXIMUM SPACING EQUAL TO HALF THE AREA OF VERTICAL REINFORCEMENT REMOVED BY THE OPENING AND PROVIDE THE SAME AREA OF REINFORCEMENT ABOVE EACH OPENING AT 3" MAXIMUM SPACING AS REMOVED BY THE OPENING.

TYPE "P" & "J" STRUCTURE BOTTOMS DETAIL

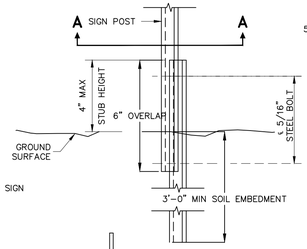
SEE FDOT INDEX #200 FOR ADDITIONAL DETAILS
N.T.S.

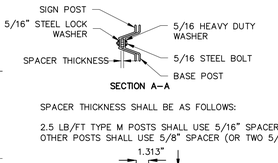


FRONT VIEW
NTS

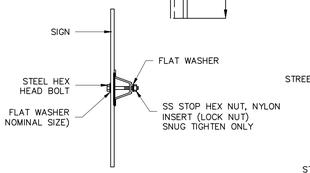


BACK VIEW
NTS

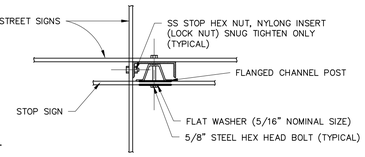




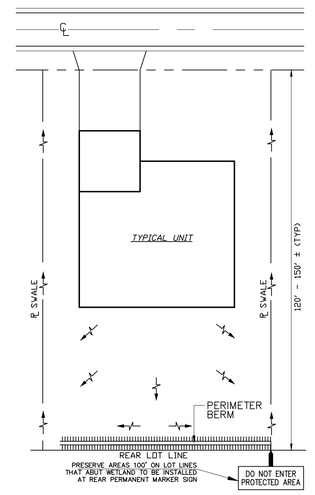
BACK VIEW
NTS



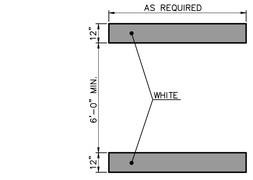
STREET SIGN ATTACHMENT DETAIL



TOP VIEW
NTS

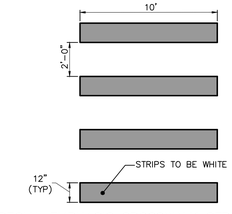


TYPICAL TYPE A LOT GRADING PLAN
LOT BACKING TO WETLANDS OR PROPERTY LINE



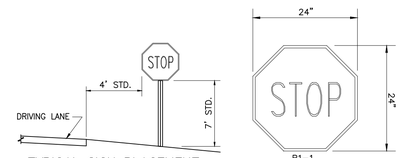
CROSSWALK STRIPING DETAIL
N.T.S.

1. TRAFFIC STRIPE LOCATION TO BE IN ACCORDANCE WITH F.D.O.T. DETAIL 17346
2. TRAFFIC STRIPE MATERIAL TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 710.



SPECIAL EMPHASIS CROSSWALK STRIPING DETAIL
N.T.S.

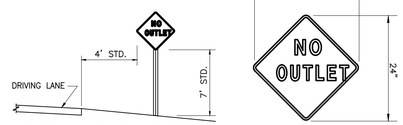
1. TRAFFIC STRIPE LOCATION TO BE IN ACCORDANCE WITH F.D.O.T. DETAIL 17346
2. TRAFFIC STRIPE MATERIAL TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 710.



TYPICAL SIGN PLACEMENT
N.T.S.

SIGN DETAIL
N.T.S.
R1-1
24" x 24"

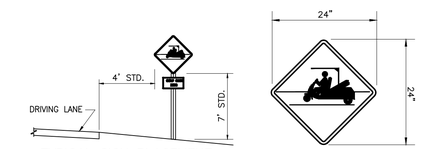
24" R1-1 STOP SIGN DETAIL
N.T.S.



TYPICAL SIGN PLACEMENT
N.T.S.

SIGN DETAIL
N.T.S.
W14-2
24" x 24"

24" 14-2 NO OUTLET SIGN DETAIL
N.T.S.

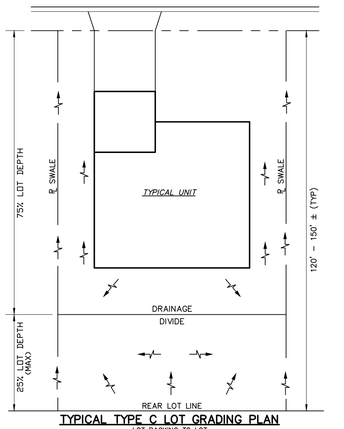


TYPICAL SIGN PLACEMENT
N.T.S.

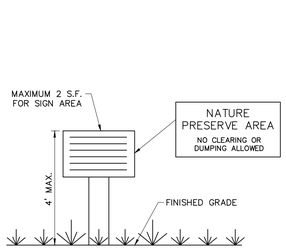
SIGN DETAIL
N.T.S.
FTP-39
24" x 24"

24" FTP-39 GOLF CART XING SIGN DETAIL
N.T.S.

TYPICAL TYPE B LOT GRADING PLAN
SPLIT DRAINAGE



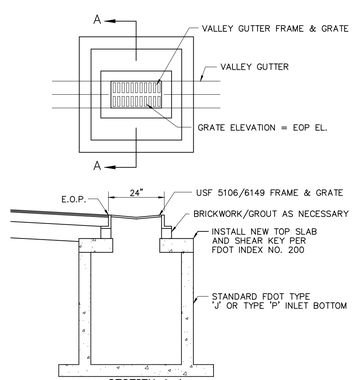
TYPICAL TYPE C LOT GRADING PLAN
LOT BACKING TO LOT



NOTES:

1. THE SIGN SHALL BE INSTALLED AT THE EDGE OF THE ONSITE PRESERVE BOUNDARY
2. SIGNS SHALL BE NO CLOSER THAN TEN FEET FROM THE DEVELOPMENT LIMITS AND BE LIMITED TO A MAXIMUM HEIGHT OF FOUR FEET AND A MAXIMUM SIZE OF TWO SQUARE FEET.
3. MAXIMUM SIGN SPACING SHALL BE 300 FEET.

PRESERVE AREA DETAIL
NTS



SECTION A-A

NOTE: A 2' SUMP IS REQUIRED ON ALL STRUCTURES SO THAT THE BOTTOM ELEVATION OF THE STRUCTURE IS A MINIMUM OF 2' BELOW THE LOWEST PIPE INVERT.

VALLEY GUTTER INLET DETAIL
N.T.S.

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE DETAILS 2

IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-DT-2	JLW	JLW	DRU	NTS	13

IBIS LANDING

S:\JOBS\3142L\ENGINEERING\GP\PLANS\GP1.DWG PLANSYS(GRADING)\GP1.DWG, 7/22/2025 12:55:24 PM, JIMMY MERKAY

STORM STRUCTURES:

S-300
YARD DRAIN
GRATE=28.70'
N. INV.=23.20'
E. INV.=25.20'

S-301
12" FLARED END
INV.=19.20'

S-302
V.G. INLET
GRATE=27.96'
NW. INV.=20.40'

S-303
V.G. INLET
GRATE=27.96'
NE. INV.=20.40'
SW. INV.=20.40'

S-304
JUNCTION BOX
GRATE=28.60'
N. INV.=19.20'
SE. INV.=19.20'
SW. INV.=20.00'

S-305
V.G. INLET
GRATE=27.90'
NW. INV.=19.20'
SE. INV.=21.70'

S-306
V.G. INLET
GRATE=27.90'
NW. INV.=21.70'
SE. INV.=20.20'

S-307
V.G. INLET
GRATE=28.60'
NW. INV.=20.20'
SW. INV.=20.20'
E. INV.=20.20'

S-308
V.G. INLET
GRATE=28.96'
NE. INV.=21.40'
SW. INV.=21.40'

S-309
V.G. INLET
GRATE=28.96'
NE. INV.=21.70'

S-310
TYPE 'D' INLET
GRATE=28.70'
W. INV.=20.20'
E. INV.=20.20'
N. INV.=25.70'

S-311
TYPE 'D' INLET
GRATE=28.70'
W. INV.=20.20'
S. INV.=20.20'

S-312
V.G. INLET
GRATE=27.98'
N. INV.=20.20'
S. INV.=21.70'

S-313
V.G. INLET
GRATE=27.96'
N. INV.=22.20'
S. INV.=22.20'

S-314
TYPE 'C' INLET
GRATE=28.40'
N. INV.=23.70'
E. INV.=24.20'
W. INV.=24.20'

S-315
YARD DRAIN
GRATE=28.80'
E. INV.=25.20'
W. INV.=25.20'

S-316
YARD DRAIN
GRATE=29.20'
E. INV.=25.20'
W. INV.=25.20'

S-317
YARD DRAIN
GRATE=29.29'
E. INV.=25.20'

S-318
YARD DRAIN
GRATE=28.50'
E. INV.=25.20'
W. INV.=25.20'

S-319
YARD DRAIN
GRATE=29.00'
E. INV.=25.70'
W. INV.=25.70'

S-320
YARD DRAIN
GRATE=29.40'
W. INV.=26.20'

S-321
YARD DRAIN
GRATE=28.20'
N. INV.=24.20'

S-322
YARD DRAIN
GRATE=28.20'
N. INV.=24.20'
S. INV.=24.20'

S-323
TYPE 'D' INLET
GRATE=28.20'
NW. INV.=19.70'
E. INV.=19.70'
S. INV.=24.20'

S-324
V.G. INLET
GRATE=27.46'
SE. INV.=22.20'
W. INV.=19.70'

S-325
V.G. INLET
GRATE=27.46'
NW. INV.=22.20'

S-326
24" FLARED END
INV.=19.20'

S-327
36" FLARED END
INV.=19.20'

S-328
NOT IN USE

S-329
TYPE 'D' INLET
GRATE=28.80'
W. INV.=22.20'
E. INV.=21.20'
N.&S. INV.=25.80'

S-330
V.G. INLET
GRATE=28.20'
W. INV.=19.20'
E. INV.=22.20'
N. INV.=16.20'
S. INV.=19.20'

S-331
TYPE 'D' INLET
GRATE=28.70'
W. INV.=19.20'
E. INV.=19.20'
N.&S. INV.=25.70'

S-332
36" FLARED END
INV.=18.20'

S-333
18" FLARED END
INV.=19.20'

S-334
TYPE 'C' INLET
GRATE=28.20'
N. INV.=24.20'
S. INV.=23.20'

S-335
YARD DRAIN
GRATE=28.20'
W. INV.=24.20'
S. INV.=25.20'

S-336
V.G. INLET
GRATE=27.70'
S. INV.=22.70'

S-337
V.G. INLET
GRATE=27.46'
N. INV.=22.20'
S. INV.=17.20'

S-338
V.G. INLET
GRATE=27.46'
S. INV.=16.20'
N. INV.=17.20'

S-339
TYPE 'D' INLET
GRATE=24.00'
E. INV.=18.20'

S-340
YARD DRAIN
GRATE=27.70'
E. INV.=22.20'
N. INV.=24.70'

S-341
JUNCTION BOX
RIM=27.70'
N. INV.=18.70'
E. INV.=18.70'
S. INV.=18.70'
W. INV.=22.20'

S-342
30" FLARED END
INV.=19.20'

S-343
TYPE 'C' INLET
GRATE=27.70'
N. INV.=19.20'
S. INV.=19.20'

S-344
V.G. INLET
GRATE=27.20'
S. INV.=19.70'

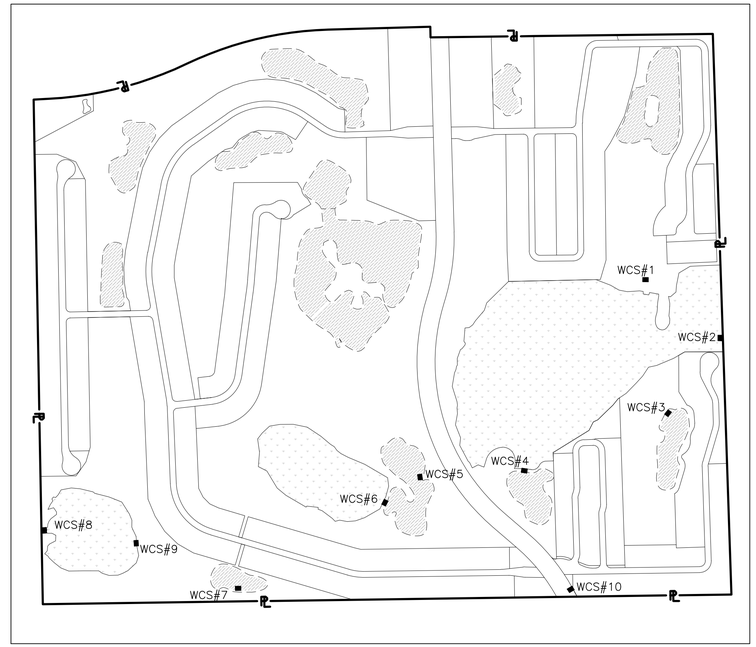
S-345
TYPE 'C' INLET
GRATE=27.70'
W. INV.=18.70'
E. INV.=18.70'
N. INV.=23.70'

S-346
YARD DRAIN
GRATE=27.70'
S. INV.=23.70'

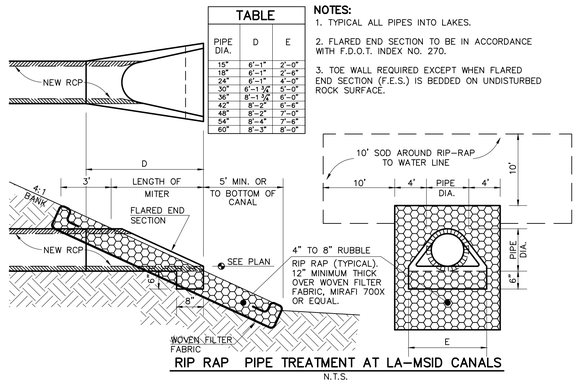
S-347
V.G. INLET
GRATE=26.96'
W. INV.=18.70'
E. INV.=18.70'

S-348
V.G. INLET
GRATE=26.96'
W. INV.=18.70'

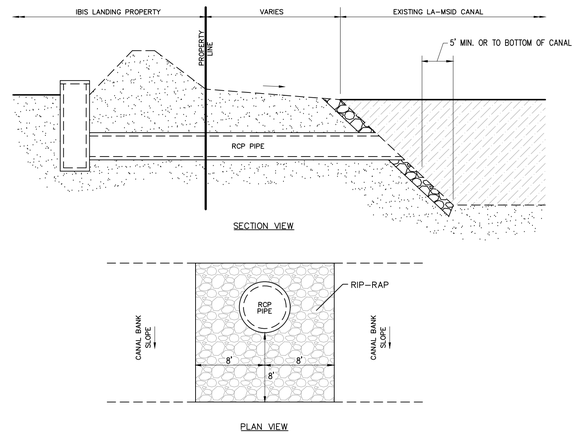
WCS-3
SEE DETAIL FOR INFO



CONTROL STRUCTURE LOCATION MAP



RIP RAP PIPE TREATMENT AT LA-MSID CANALS
N.T.S.



EXISTING OUTFALL & MITERED END
NTS

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

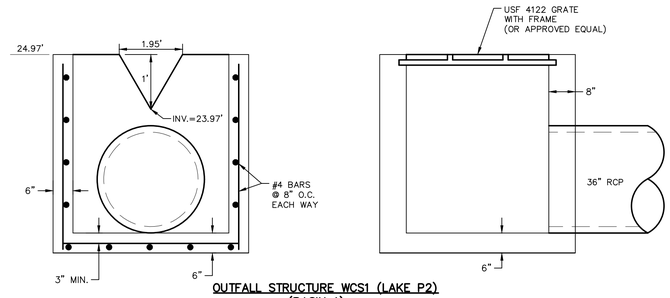
WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

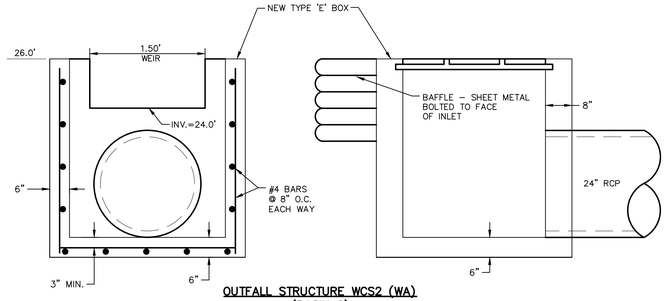
DRAINAGE STRUCTURE SCHEDULE
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	DSS	JLW	JLW	DRU	NTS	14

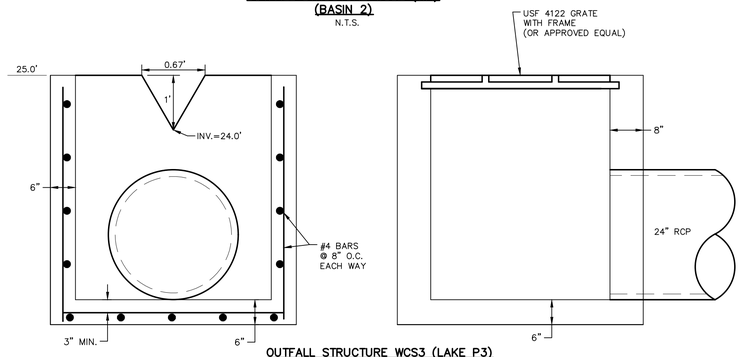
S:\JOBS\3142L\3142L\ENGINEERING\DEVELOPER\DRAINAGE PLANS\MASTER\DWG\3142L-STRMD_MSTR_14_250JUN5.DWG 6/27/2025 12:50 PM JIMMY MIENEY



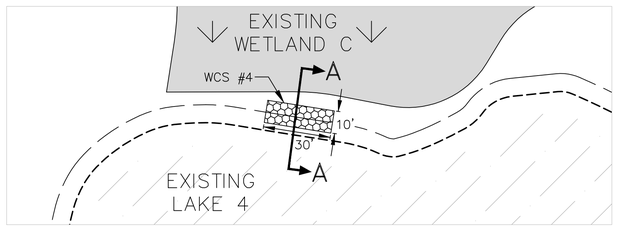
OUTFALL STRUCTURE WCS1 (LAKE P2)
(BASIN 1)
N.T.S.

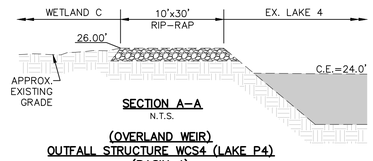


OUTFALL STRUCTURE WCS2 (WA)
(BASIN 2)
N.T.S.



OUTFALL STRUCTURE WCS3 (LAKE P3)
(BASIN 3)
N.T.S.

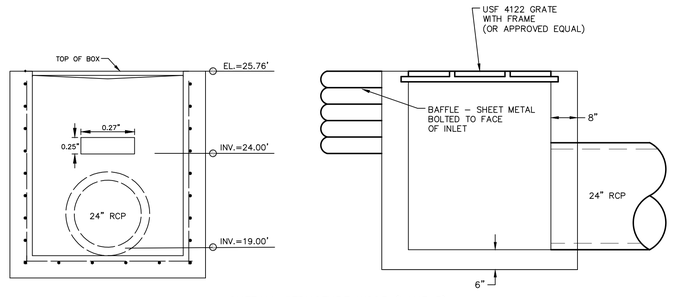




SECTION A-A
N.T.S.

(OVERLAND WEIR)
OUTFALL STRUCTURE WCS4 (LAKE P4)
(BASIN 4)
N.T.S.

PERMITTED (36-108291-P)



OUTFALL STRUCTURE WCS3 (LAKE 3)
(BASIN 3)
N.T.S.

PROPOSED

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

OUTFALL STRUCTURE DETAILS 1

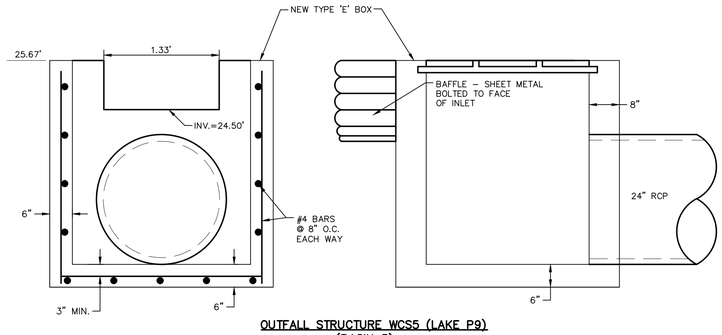
IBIS LANDING

LEE COUNTY, FLORIDA

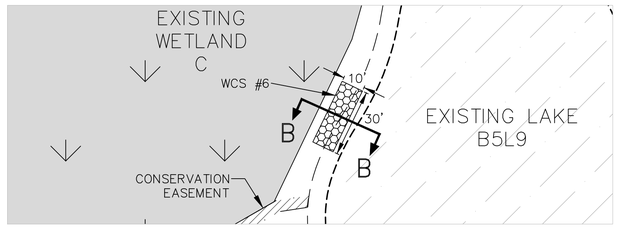
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	CS-DT-1	JLW	JLW	DRU	NTS	14A

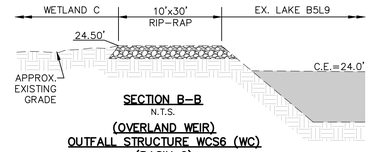
IBIS LANDING

S:\JOBS\3142L\CAD\ENGINEERING\DEVPLANS\SFWMD PLANS\MODIFICATION\3142-SFWMD_WCS_4A-4C_OUTFALL STRUCTURES EXHIBIT.DWG 9/24/2025 1:58 PM JIMMY WIECHAT



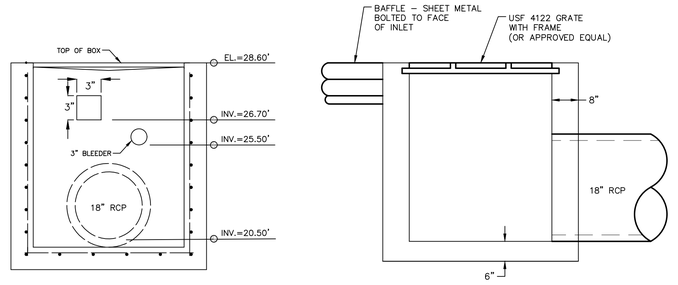
OUTFALL STRUCTURE WCS5 (LAKE P9)
(BASIN 5)
N.T.S.



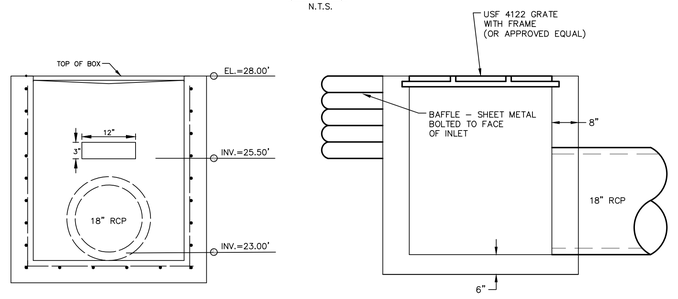


SECTION B–B
N.T.S.

(OVERLAND WEIR)
OUTFALL STRUCTURE WCS6 (WC)
(BASIN 6)
N.T.S.



OUTFALL STRUCTURE WCS7 (LAKE P13)
(BASIN 7)
N.T.S.



OUTFALL STRUCTURE WCS8 (WD)
(BASIN 8)
N.T.S.

PERMITTED (36-108291-P)

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

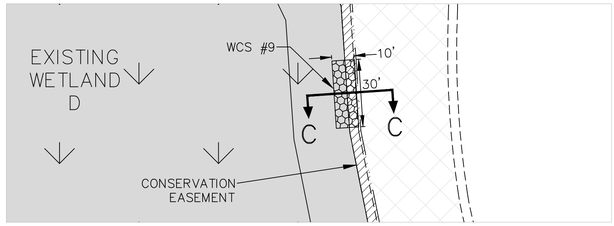
WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

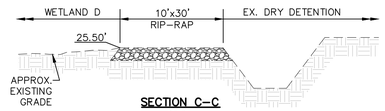
DAVID R. UNDERHILL JR.
P.E. #47029

OUTFALL STRUCTURE DETAILS 2
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	CS-DT-2	JLW	JLW	DRU	NTS	14B

IBIS LANDING





SECTION C-C
N.T.S.
(OVERLAND WEIR)
OUTFALL STRUCTURE WCS9 (LAKE P12)
(BASIN 9)
N.T.S.

PERMITTED (36-108291-P)

IBIS LANDING

S:\JOBS\3142L\ENGINEERING\DEVPLANS\SPMMD PLANS\MODIFICATION\3142-SPMMD_MOD_14A-14C_OUTFALL STRUCTURES EXHIBIT.DWG 9/22/2025 12:50 PM JIMMY WHEWAY

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

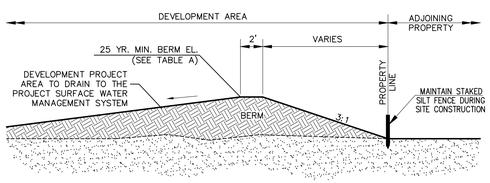
PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

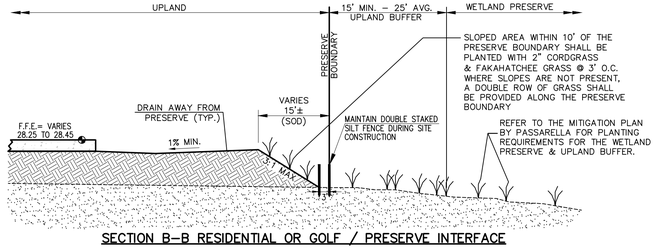
DAVID R. UNDERHILL JR.
P.E. #47029

OUTFALL STRUCTURE DETAILS 3
IBIS LANDING
LEE COUNTY, FLORIDA

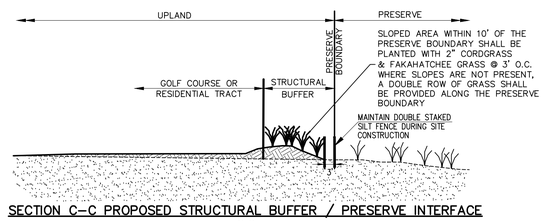
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	CS-DT-3	JLW	JLW	DRU	NTS	14C



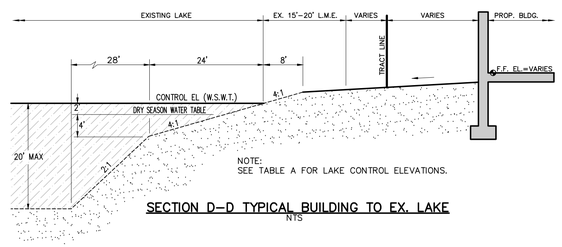
SECTION A–A TYPICAL BERM
NTS



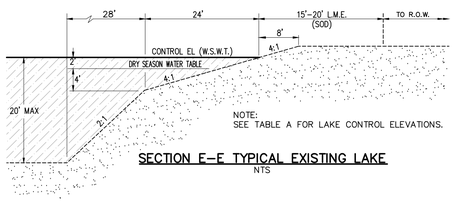
SECTION B–B RESIDENTIAL OR GOLF / PRESERVE INTERFACE
NTS



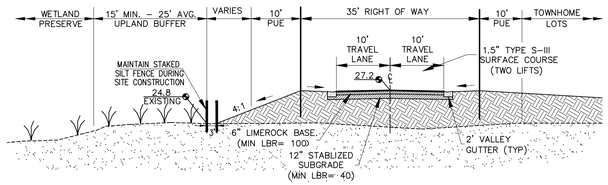
SECTION C–C PROPOSED STRUCTURAL BUFFER / PRESERVE INTERFACE
NTS



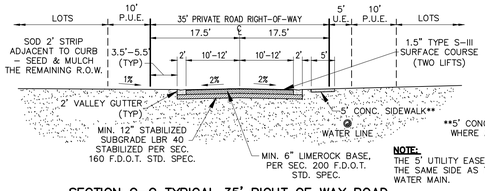
SECTION D–D TYPICAL BUILDING TO EX. LAKE
NTS



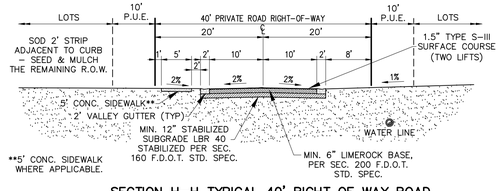
SECTION E–E TYPICAL EXISTING LAKE
NTS



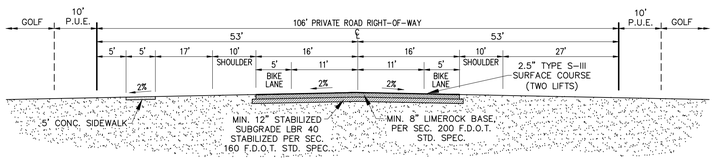
SECTION F–F TYPICAL ROAD TO WETLAND
NTS



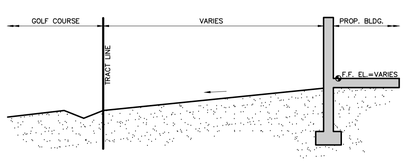
SECTION G–G TYPICAL 35' RIGHT OF WAY ROAD
NTS



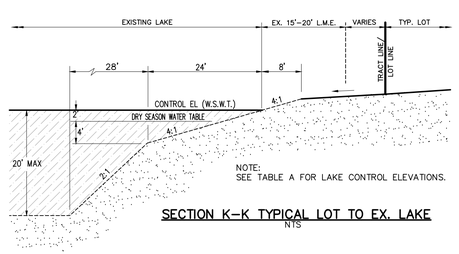
SECTION H–H TYPICAL 40' RIGHT OF WAY ROAD
NTS



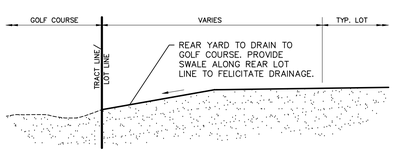
SECTION I–I TYPICAL 106' RIGHT OF WAY ROAD (BETH STACY BOULEVARD)
NTS



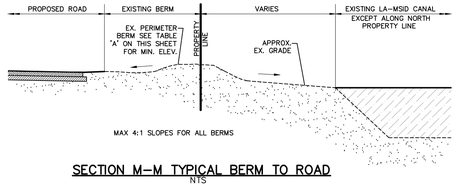
SECTION J–J TYPICAL BUILDING TO GOLF COURSE
NTS



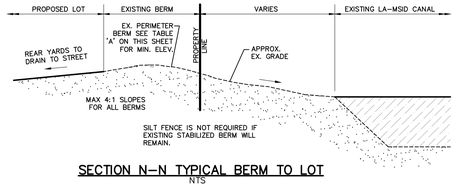
SECTION K–K TYPICAL LOT TO EX. LAKE
NTS



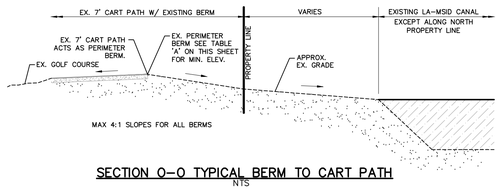
SECTION L–L TYPICAL LOT TO GOLF COURSE
NTS



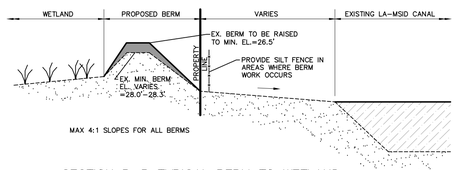
SECTION M–M TYPICAL BERM TO ROAD
NTS



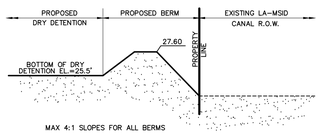
SECTION N–N TYPICAL BERM TO LOT
NTS



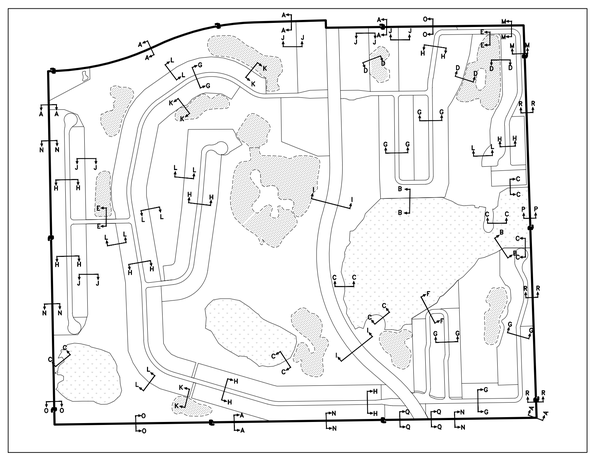
SECTION O–O TYPICAL BERM TO CART PATH
NTS



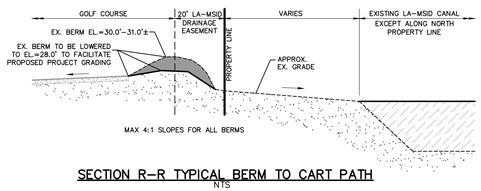
SECTION P–P TYPICAL BERM TO WETLAND
NTS



SECTION Q–Q PROPOSED BERM (BASIN 2)
NTS



CROSS SECTION KEY MAP



SECTION R–R TYPICAL BERM TO CART PATH
NTS

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

TABLE 'A'

STORMWATER MANAGEMENT SYSTEM
MINIMUM ELEVATIONS (NGVD)

Basin	WCS#	Control Elev.	5 Yr. Flood Stage	10 Yr. Flood Stage	Min. Road	25 Yr. Flood Stage	Min. Berm	100 Yr. Flood Stage	Min. Floor Elev.
B1	WCS 1	24.00	24.54	25.80	26.50	27.04	27.40*	27.44	28.00
B2	WCS 2	24.00	24.67	24.85	26.50	25.97	26.45	27.44	28.00
B3	WCS 3	24.00	25.94	26.07	26.50	26.91	27.35	27.44	28.00
B4	WCS 4	24.00	25.96	26.09	26.50	26.75	27.20	27.44	28.00
B5	WCS 5	24.50	25.54	25.87	26.50	27.32	27.85	27.95	28.30
B6	WCS 6	24.00	25.39	25.63	26.50	26.75	N/A	27.57	28.00
B7	WCS 7	25.50	27.13	27.44	27.50	28.52	29.05	29.27	29.30
B8	WCS 8	25.50	26.17	26.36	27.50	27.52	28.05	28.39	28.40
B9	WCS 9	25.50	26.17	26.36	27.50	27.52	28.05	28.39	28.40

* EXISTING PERIMETER BERM WILL PROVIDE 4" OF FREEBOARD ALONG NORTH PROPERTY LINE. MINIMUM BERM HEIGHT IS 27.6' ALONG LA–MSID CANALS.

PLEASE NOTE: B2, B7, & B10 WILL HAVE BERM IMPROVEMENTS TO REACH REQUIRED 6" FREEBOARD.

5 YEAR 24 HR RAINFALL IS 5.20"
10 YEAR 24 HR TOTAL RAINFALL IS 6.00"
25 YEAR 72 HR TOTAL RAINFALL IS 10.20"
100 YEAR 72 HR TOTAL RAINFALL IS 14.20"

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278–1199
FAX: (239) 278–3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939–5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625–1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

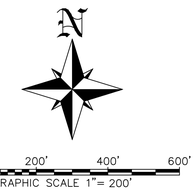
DAVID R. UNDERHILL JR.
P.E. #47029

TYPICAL CROSS-SECTIONS
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	XSEC	JLW	JLW	DRU	NTS	15

S:\JOBS\3142L\ENGINEERING\DEVPLANS\DPMAS\PLANS\MASTERPLAN\3142-DPMAS_MSTR_15_07-15-LINK 9/24/2025 2:00 PM JIMMY WESKEY





LEGEND:

- EXISTING E.O.P.
- PROPOSED ASPHALT PAVEMENT
- PROPOSED TOP OF BANK
- EXISTING DETENTION–RETENTION AREA
- LANDSCAPE BUFFER
- RIGHT–OF–WAY LINE
- EXISTING STORM WATER MANAGEMEI
- PROPOSED CONCRETE
- WETLAND AREA & BUFFER
- PROPERTY LINE
- PROPOSED BERM
- DRAINAGE HAYBALES
- DEWATERING SUMP
- DRAINAGE RIP–RAP
- FLOATING TURBIDITY BARRIER

SCHEDULE OF IMPLEMENTATION:

1) INSTALL ALL TEMPORARY BMP MEASURES PRIOR TO START OF CONSTRUCTION.

2) INSTALL ALL PERMANENT BMP MEASURES IMMEDIATELY AFTER FINAL GRADING.

3) REMOVE TEMPORARY BMP FEATURES AS APPLICABLE.

IBIS LANDING

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278–1199
FAX: (239) 278–3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939–5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625–1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

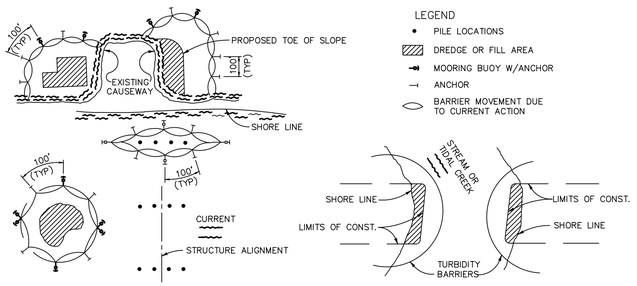
BEST MANAGEMENT PRACTICES PLAN

IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	BMP	JLW	JLW	DRU	1"=200'	16

S:\JOBS\3XXX\3142L\ENGINEERING\DEVELOP\DWG FILES\WQ\PLOTS\3142L_WQ_16_BMP.DWG 7/22/2025 12:51 PM JIMMY MCKINLY



NOTES:
1. TURBIDITY BARRIERS ARE TO BE USED IN ALL PERMANENT BODIES OF WATER REGARDLESS OF WATER DEPTH.
2. NUMBER AND SPACING OF ANCHORS DEPENDENT ON CURRENT VELOCITIES.
3. DEPLOYMENT OF BARRIER AROUND PILE LOCATIONS MAY VARY TO ACCOMMODATE CONSTRUCTION OPERATIONS.
4. NAVIGATION MAY REQUIRE SEGMENTING BARRIER DURING CONSTRUCTION OPERATIONS.
5. FOR ADDITIONAL INFORMATION SEE SECTION 104 OF THE STANDARD SPECIFICATIONS.

TURBIDITY BARRIER APPLICATION DETAIL
N.T.S.

NOTE:
TURBIDITY BARRIERS FOR FLOWING STREAMS AND TIDAL CREEKS MAY BE EITHER FLOATING, OR STAKED TYPES OR ANY COMBINATIONS OF TYPES THAT WILL SUIT SITE CONDITIONS AND MEET EROSION CONTROL AND WATER QUALITY REQUIREMENTS. THE BARRIER TYPE(S) WILL BE AT THE CONTRACTORS OPTION UNLESS OTHERWISE SPECIFIED IN THE PLANS, HOWEVER PAYMENT WILL BE UNDER THE PAY ITEM(S) ESTABLISHED IN THE STAKED TURBIDITY BARRIER. POSTS IN STAKED PLANS FOR FLOATING TURBIDITY BARRIER AND/OR TURBIDITY BARRIERS TO BE INSTALLED IN VERTICAL POSITION UNLESS OTHERWISE DIRECTED BY THE ENGINEER.

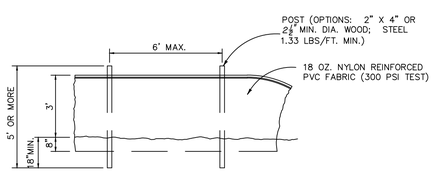
SEEDING RATES (LBS/AC) FOR NEW SHOULDERS AND SLOPES*								
TYPE OF SEED	ZONE 1				ZONE 2			
	COASTAL		INLAND		COASTAL		INLAND	
	MAR. TO NOV.	NOV. TO MAR.	MAR. TO OCT.	OCT. TO MAR.	FEB. TO DEC.	DEC. TO FEB.	FEB. TO DEC.	DEC. TO FEB.
PERMANENT GRASS								
UNHULLED BERMUDA	20	20	20	20	20	20	20	20
BAHIA ARGENTINA OR PENSACOLA			80	80			80	80
QUICK GROWING								
BROWN TOP MILLET	20		20		20		20	
ANNUAL RYE GRASS		20		20		20		20
TOTAL POUNDS PER ACRE	40	40	120	120	40	40	120	120

NOTE: THE SEEDING RATES SHOWN IN THIS TABLE APPLY ONLY WHEN SEED IS SPREAD BY AN APPROVED MECHANICAL SPREADER MEETING THE REQUIREMENTS OF SECTION 570 AND 577 OF THE STANDARD SPECIFICATIONS.
*SEE INDEX NO. 105 FOR ZONE BOUNDARIES AND SEEDING RATES FOR SHOULDER REWORKING.

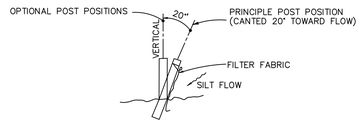
D_1 =5' STD. (SINGLE PANEL FOR DEPTHS 5' OR LESS).
D_2 =5' STD. (ADDITIONAL PANEL FOR DEPTHS > 5').
CURTAIN TO REACH BOTTOM UP TO DEPTHS OF 10 FEET. TWO(2) PANELS TO BE USED FOR DEPTHS GREATER THAN 10 FEET UNLESS SPECIAL DEPTH CURTAINS SPECIFICALLY CALLED FOR IN THE PLANS OR AS DETERMINED BY THE ENGINEER.

NOTICE: COMPONENTS OF TYPES ~ AND ~~ MAY BE SIMILAR OR IDENTICAL TO PROPRIETARY DESIGNS. ANY INFRINGEMENT ON THE PROPRIETARY RIGHTS OF THE DESIGNER SHALL BE THE SOLE RESPONSIBILITY OF THE USER. SUBSTITUTIONS FOR TYPES ~ AND ~~ SHALL BE AS APPROVED BY THE ENGINEER.

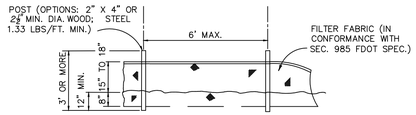
FLOATING TURBIDITY BARRIER DETAIL
N.T.S.



STAKED TURBIDITY BARRIER DETAIL
N.T.S.



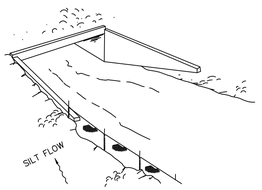
SECTION



ELEVATION

NOTE: SILT FENCE TO BE PAID FOR UNDER THE CONTRACT UNIT PRICE FOR STAKED SILT FENCE (LF).

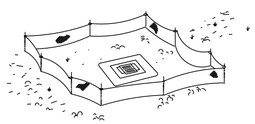
TYPE 3 SILT FENCE DETAIL
N.T.S.



TYPE 3 SILT FENCE



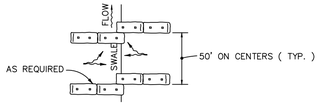
TYPE 3 SILT FENCE



TYPE 3 SILT FENCE PROTECTION AROUND DITCH BOTTOM INLETS.

DO NOT DEPLOY IN A MANNER THAT SILT FENCES WILL ACT AS A DAM ACROSS PERMANENT FLOWING WATERCOURSES. SILT FENCES ARE TO BE USED AT UPLAND LOCATIONS AND TURBIDITY BARRIERS USED AT PERMANENT BODIES OF WATER.

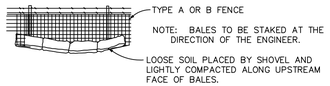
SILT FENCE APPLICATIONS DETAIL
N.T.S.



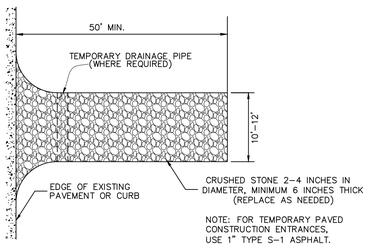
ANCHOR BALES WITH 2 – 2" X 2" X 4' STAKES PER BALE.
PLAN



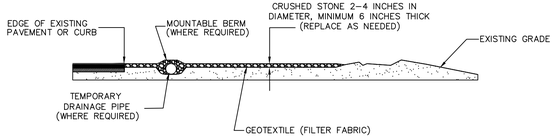
TO BE USED AT SELECTED SITES WHERE THE NATURAL GROUND SLOPES TOWARD THE TOE OF SLOPE
ELEVATION



BALES BACKED BY FENCE DETAIL
N.T.S.

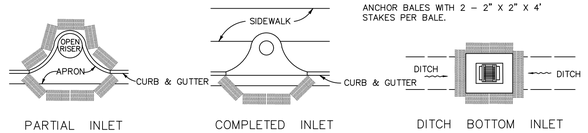


STABALIZED CONSTRUCTION ENTRANCE/EXIT PLAN VIEW
N.T.S.

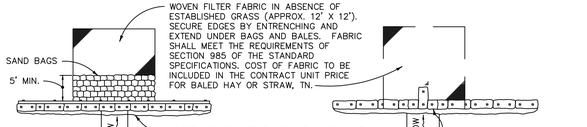


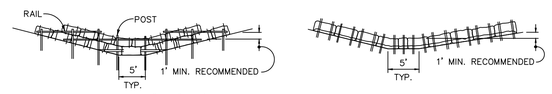
STABALIZED CONSTRUCTION ENTRANCE/EXIT SECTION VIEW
N.T.S.

GENERAL NOTES
1. EROSION CONTROL DETAILS ARE APPLICABLE TO NEW CONSTRUCTION, RECONSTRUCTION AND RRR PROJECTS.
2. FOR SODDING ADJACENT TO DITCHES AND AT HEADWALLS, SEE INDEX NO. 281.
3. ALL FRONT SLOPES STEEPER THAN 3:1 ARE TO BE SODDED.



PROTECTION AROUND INLETS OR SIMILAR STRUCTURES DETAIL
N.T.S.





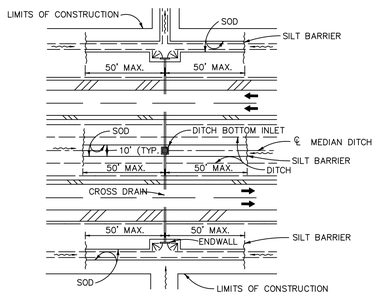
ANCHOR LOWER BALES WITH 2 – 2" X 2" X 4' STAKES PER BALE.
ANCHOR TOP BALES TO LOWER BALES WITH 2 – 2" X 2" X 4' STAKES PER BALE. ANCHOR BALES WITH 2 – 2" X 2" X 4' STAKES PER BALE
ELEVATION ELEVATION

APPLICATION AND SPACING: THE USE OF TYPES I & II BALE BARRIERS SHOULD BE LIMITED TO THE CONDITIONS OUTLINED IN CHART 1, SHEET 1 OF 3, INDEX NO. 102

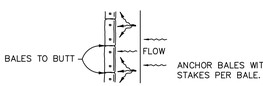
TYPE II TYPE I

BARRIER FOR UNPAVED DITCHES DETAIL
N.T.S.

GENERAL NOTES
1. EROSION CONTROL DETAILS ARE APPLICABLE TO NEW CONSTRUCTION, RECONSTRUCTION AND RRR PROJECTS.
2. FOR SODDING ADJACENT TO DITCHES AND AT HEADWALLS, SEE INDEX NO. 281.
3. ALL FRONT SLOPES STEEPER THAN 3:1 ARE TO BE SODDED.



DITCH INSTALLATIONS AT DRAINAGE STRUCTURES DETAIL
N.T.S.



TO BE USED AT SELECTED SITES WHERE THE NATURAL GROUND SLOPES AWAY FROM THE TOE OF SLOPE
ELEVATION

BARRIERS FOR FILL SLOPES DETAIL
N.T.S.

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

BEST MANAGEMENT PRACTICES DETAILS
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	BMP-DT	JLW	JLW	DRU	NTS	17

IBIS LANDING

S:\JOBS\3142L\ENGINEERING\QUICKPLANS\DETAILS PLANS\MASTER\3142-DETAILS_MASTER_17.DWG, 9/22/2025 10:59 PM, JIMMY WICKEY

IBIS LANDING
(F.K.A. COPPERHEAD DEVELOPMENT)

S.F.W.M.D. PLANS

SECTION 06, TOWNSHIP 45 SOUTH, RANGE 27 EAST
LEE COUNTY, FLORIDA



GENERAL LOCATION MAP
NOT TO SCALE

UTILITY OWNER CONTACTS

WATER & WASTEWATER:
FLORIDA GOVERNMENTAL UTILITY AUTHORITY
280 WEKIVA SPRINGS RD. SUITE 203
LONGWOOD, FLORIDA 32779
PHONE: (407) 629-6900
FAX: (407) 629-6963

ELECTRIC POWER SERVICE:
LEE COUNTY ELECTRIC CO-OP
P.O. BOX 3455
NORTH FORT MYERS, FLORIDA 33918
PHONE: (239) 995-2121
FAX: (239) 995-7904

TELEPHONE SERVICE:
SPRINT
2820 CARGO STREET BUILDING A
FORT MYERS, FLORIDA 33916
PHONE: (239) 336-2003
FAX: (239) 336-2026

PROJECT CONTACTS

ENGINEER:
DAVID R. UNDERHILL, P.E. PROJECT MANAGER
BANKS ENGINEERING
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

SURVEYOR:
BANKS ENGINEERING
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

DEVELOPER:
AQUABELLA DEVELOPMENT GROUP, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 278-1199

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199 FAX: (239) 278-3174



SPECIFIC LOCATION MAP
NOT TO SCALE

INDEX OF SHEETS

SHEET	TITLE
1	COVER SHEET
2	AERIAL MAP & EXISTING CONDITIONS PLAN
3	SITE DEVELOPMENT PLAN
4	STORM WATER MANAGEMENT PLAN
5	MASTER PAVING, GRADING, & DRAINAGE PLAN
6	PAVING, GRADING & DRAINAGE 1 (60 SCALE)
7	PAVING, GRADING & DRAINAGE 2 (60 SCALE)
8	PAVING, GRADING & DRAINAGE 3 (60 SCALE)
9	PAVING, GRADING & DRAINAGE 4 (60 SCALE)
10	PAVING, GRADING & DRAINAGE 5 (60 SCALE)
11	PAVING, GRADING & DRAINAGE 6 (60 SCALE)
12	PAVING, GRADING & DRAINAGE DETAILS (1)
13	PAVING, GRADING & DRAINAGE DETAILS (2)
14	DRAINAGE STRUCTURE SCHEDULE
14A	CONTROL STRUCTURE DETAILS 1
14B	CONTROL STRUCTURE DETAILS 2
14C	CONTROL STRUCTURE DETAILS 3
15	TYPICAL CROSS-SECTIONS
16	BEST MANAGEMENT PRACTICES PLAN
17	BEST MANAGEMENT PRACTICES DETAILS

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

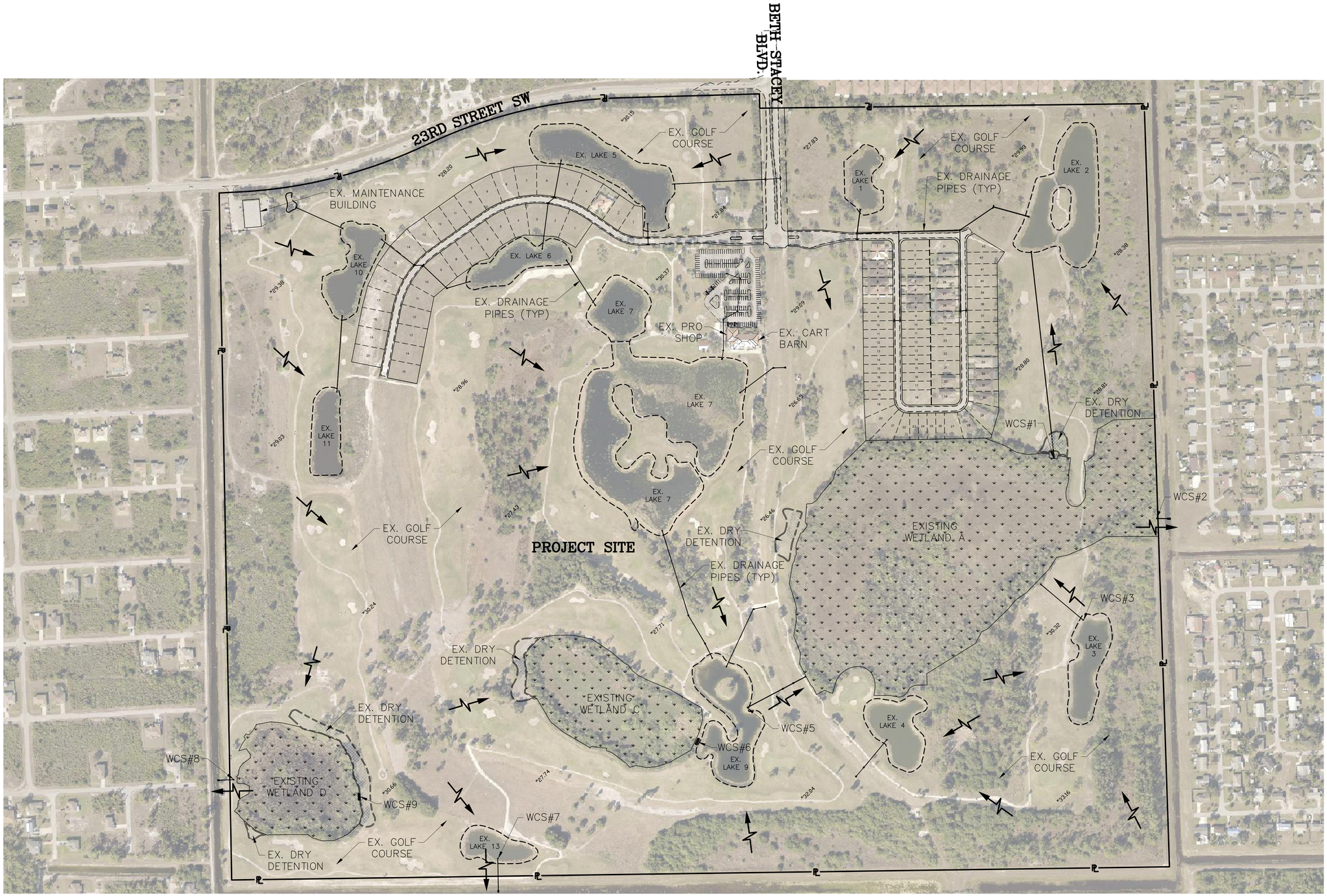
WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

NO.	DATE	REVISION DESCRIPTION	BY
1	12-11-25	REVISED PER SFWMD COMMENTS	JLW

IBIS LANDING

S:\USERS\DAVE.UNDERHILL\DOCUMENTS\WORK\IBIS LANDING\SFWMD PLANS\MASTER\COVER.DWG 2/5/2026 1:59 PM JIMMY WECKEY



ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

AERIAL MAP & EXISTING CONDITIONS PLAN

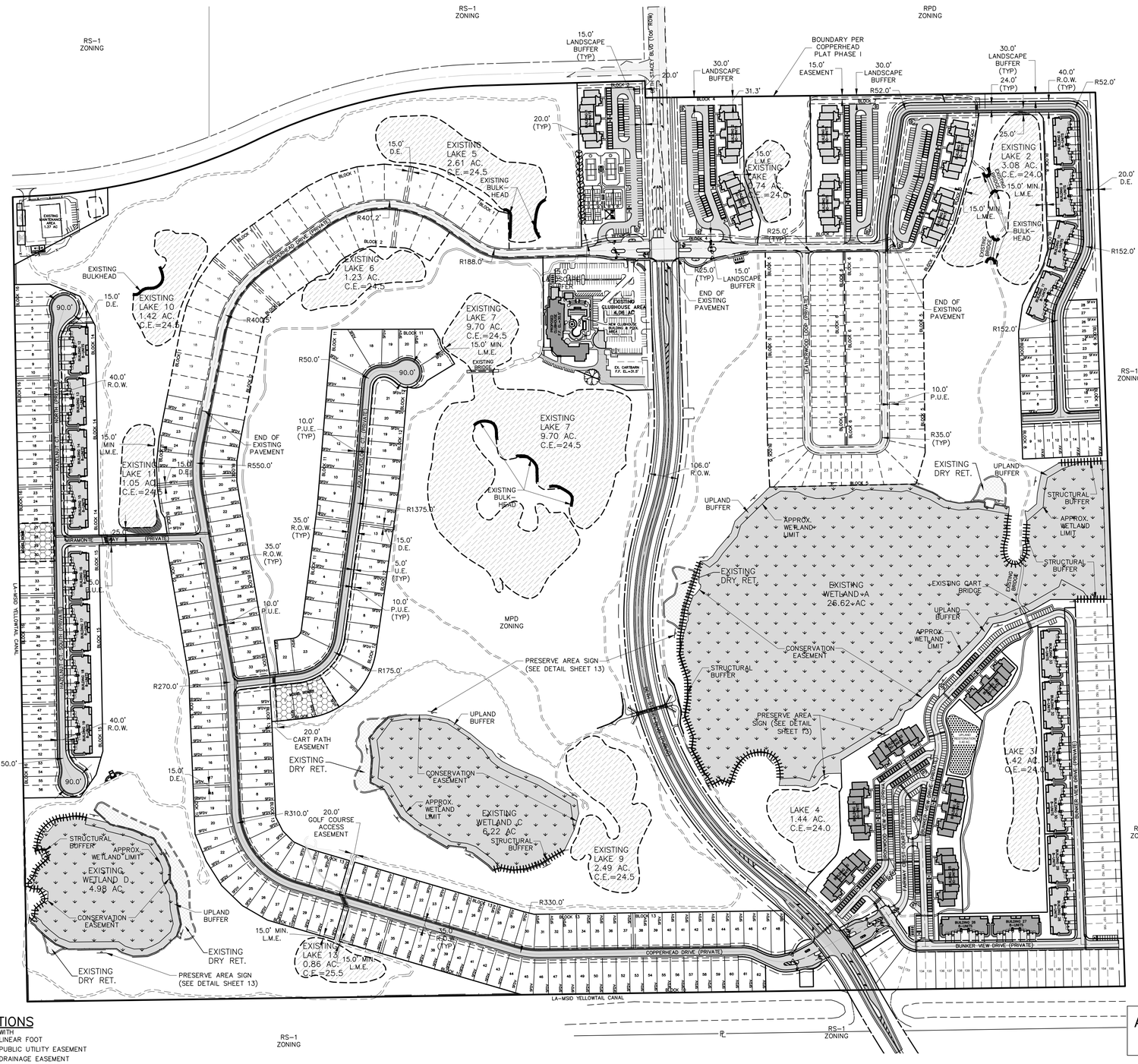
IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	AER	JLW	JLW	DRU	1"=200'	02

IBIS LANDING

S:\JOBS\3142L\DWG\MASTER\PLANS\SITEMAP\3142L_SITEMAP.DWG, 2/3/2026 1:59 PM, JIMMY MCKAY



SITE PLAN DATA:

PROJECT DEVELOPMENT AREA:	90.57 ACRES
PARCEL ZONING :	MPD
PARCEL LAND USE:	SINGLE FAMILY DETACHED VILLAS, SINGLE FAMILY ATTACHED VILLAS, TOWNHOMES, & MULTI-FAMILY RESIDENTIAL
FEMA FLOOD ZONE:	"X"
MAXIMUM LOT COVERAGE (BUILDINGS):	SEE PROPOSED
MAXIMUM BUILDING HEIGHT	DEVELOPMENT
SINGLE FAMILY RESIDENTIAL:	STANDARDS
MULTI-FAMILY RESIDENTIAL:	(SHEET 3A)

LAND USE:

TOTAL PROPERTY AREA:	90.57 Ac (100%)
PROJECT DEVELOPMENT AREA:	90.57 Ac (100%)
RESIDENTIAL AREA:	77.02 Ac (85.0%)
RECREATIONAL AREA:	1.35 Ac (1.5%)
RIGHT-OF-WAY AREA:	12.20 Ac (13.5%)

PARKING CALCULATIONS:

BLOCK 3: MULTI-FAMILY — TWO BEDROOM UNITS = 1.75 SPACES PER UNIT
84 UNITS x 1.75 = 147 SPACES
GUEST PARKING = 147 x 10% = 15
REQUIRED HANDICAP SPACES = 5
PROVIDED HANDICAP SPACES = 5
TOTAL PARKING SPACES REQUIRED = 162
TOTAL PARKING SPACES PROVIDED = 162

BLOCK 4: MULTI-FAMILY — TWO BEDROOM UNITS = 1.75 SPACES PER UNIT
60 UNITS x 1.75 = 105 SPACES
GUEST PARKING = 105 x 10% = 11
REQUIRED HANDICAP SPACES = 5
PROVIDED HANDICAP SPACES = 5
TOTAL PARKING SPACES REQUIRED = 116
TOTAL PARKING SPACES PROVIDED = 130

BLOCK 7: MULTI-FAMILY — TWO BEDROOM UNITS = 1.75 SPACES PER UNIT
108 UNITS x 1.75 = 189 SPACES
GUEST PARKING = 189 x 10% = 19
REQUIRED HANDICAP SPACES = 6
PROVIDED HANDICAP SPACES = 8
TOTAL PARKING SPACES REQUIRED = 208
TOTAL PARKING SPACES PROVIDED = 208

BLOCK 1 (LOTS 21-29), 2 (LOTS 16-33), 5 (LOT 44), 10, 11, 12, & 13: SINGLE FAMILY DETACHED VILLAS — 2.0 SPACES REQUIRED PER UNIT
186 UNITS x 2.0 = 372 SPACES
ALL PARKING FOR SINGLE FAMILY DETACHED VILLA LOTS ARE PROVIDED FOR ON UNITS LOT WHETHER IN 2 CAR GARAGE OR ON DRIVEWAY
TOTAL PARKING SPACES REQUIRED = 372
TOTAL PARKING SPACES PROVIDED = 372

BLOCK 14, 15, & 16: TOWNHOMES — 2.0 SPACES REQUIRED PER UNIT
142 UNITS x 2.0 = 284 SPACES
ALL PARKING FOR TOWNHOME LOTS ARE PROVIDED FOR ON UNITS LOT WHETHER IN 2 CAR GARAGE OR ON DRIVEWAY
TOTAL PARKING SPACES REQUIRED = 284
TOTAL PARKING SPACES PROVIDED = 284

BLOCK 17, 18, 19, 20, & 21: TOWNHOMES — 2.0 SPACES REQUIRED PER UNIT
243 UNITS x 2.0 = 486 SPACES
ALL PARKING FOR TOWNHOME LOTS ARE PROVIDED FOR ON UNITS LOT WHETHER IN 2 CAR GARAGE OR ON DRIVEWAY
TOTAL PARKING SPACES REQUIRED = 486
TOTAL PARKING SPACES PROVIDED = 486

BLOCK 5 (LO'S 45-56), 8, & 9: SINGLE FAMILY ATTACHED VILLAS — 2.0 SPACES REQUIRED PER UNIT
58 UNITS x 2.0 = 116 SPACES
ALL PARKING FOR SINGLE FAMILY ATTACHED VILLA LOTS ARE PROVIDED FOR ON UNITS LOT WHETHER IN 2 CAR GARAGE OR ON DRIVEWAY
TOTAL PARKING SPACES REQUIRED = 116
TOTAL PARKING SPACES PROVIDED = 116

NOTE:

1) SEE PHASING PLAN FOR DEVELOPMENT STANDARDS SHEET 3A.

2) NO PARKING LOT LIGHTING IS PROPOSED, STREET LIGHTING WILL BE PROVIDED BY LCEC.

3) THE PROPOSED STRUCTURAL BUFFER'S MATERIAL SHALL MEET ALL S.F.W.M.D. REQUIREMENTS.

4) ALL IMPROVEMENTS WITHIN BETH STACEY BLVD R.O.W. INCLUDING, BUT NOT LIMITED TO, SIDEWALKS, PROPOSED ACCESS CONNECTIONS, STRIPING, & DRAINAGE STRUCTURES SHALL BE ADDRESSED WITHIN THE FUTURE AMENDMENT OF THE EXISTING DEVELOPMENT ORDER #DOS 9912034000

5) ALL TOWNHOME & 4-STORY MULTI-FAMILY BUILDINGS ARE PROPOSED TO BE PROTECTED BY FIRE SPRINKLER SYSTEMS. ALL SINGLE FAMILY ATTACHED & DETACHED VILLAS ARE PROPOSED NOT TO BE PROTECTED BY FIRE SPRINKLER SYSTEMS.

6) SEE PAVING, GRADING, & DRAINAGE PLANS SHEETS 6 THRU 11 FOR STREET & REGULATORY SIGN LOCATIONS.

7) ALL EXOTIC VEGETATION WILL BE REMOVED ON A PER PHASE BASIS.

LEGEND:

- EXISTING ASPHALT
- EXISTING E.O.P.
- PROPOSED ASPHALT PAVEMENT
- PROPOSED TOP OF BANK
- LANDSCAPE BUFFER
- RIGHT-OF-WAY LINE
- EXISTING STORM WATER MANAGEME
- WETLAND AREA & BUFFER
- LANDSCAPE BUFFER
- ZONING LINE
- PROPERTY LINE
- PROPOSED BERM
- EXISTING BULKHEAD
- PROPOSED DRAINAGE EASEMENT
- PROPOSED LAKE MAINTENANCE EA:
- PROPOSED STRUCTURAL BUFFER
- EXISTING DEVELOPMENT ORDER #DOS 9912034000 & COPPERHEAD PLAT
- MODEL HOME LOCATION
- LAKE FILL AREA

N

0 200' 400' 600'
GRAPHIC SCALE 1"= 200'

SOLID WASTE DISPOSAL FACILITIES:

REQUIRED WASTE DISPOSAL FACILITY AREA = 168 SF FOR THE FIRST 25 UNITS + 4.8 SF FOR EACH REMAINING UNIT

BLOCK 3: MULTI-FAMILY
84 UNITS = 168 SF + (4.8 SF UNIT x 59 UNITS) = 451 SF
DISPOSAL FACILITY AREA REQUIRED = 451 SF
4-12' X 12' DUMPSTER PADS = 576 SF
TOTAL DISPOSAL FACILITY AREA REQUIRED = 451 SF
TOTAL DISPOSAL FACILITY AREA PROVIDED = 576 SF

BLOCK 4: MULTI-FAMILY
60 UNITS = 168 SF + (4.8 SF UNIT x 35 UNITS) = 336 SF
DISPOSAL FACILITY AREA REQUIRED = 336 SF
2-12' X 14' DUMPSTER PADS = 336 SF
TOTAL DISPOSAL FACILITY AREA REQUIRED = 336 SF
TOTAL DISPOSAL FACILITY AREA PROVIDED = 336 SF

BLOCK 7: MULTI-FAMILY
108 UNITS = 168 SF + (4.8 SF UNIT x 83 UNITS) = 567 SF
DISPOSAL FACILITY AREA REQUIRED = 567 SF
4-12' X 14' DUMPSTER PADS = 672 SF
TOTAL DISPOSAL FACILITY AREA REQUIRED = 567 SF
TOTAL DISPOSAL FACILITY AREA PROVIDED = 672 SF

EXISTING UNIT BREAKDOWN PER PLAT "COPPERHEAD PHASE 1":

SINGLE FAMILY 60' x 100' MIN.	
BLOCKS:1 (LOTS 1-20), & 2 (LOTS 1-15)	= 35
SINGLE FAMILY DETACHED VILLAS (50' x 100') BLOCKS:5 (LOTS 27-43), & 6 (LOTS 19-31)	= 30
DUPLEX (37' x 100') BLOCKS: 5 (LOTS 1-26), & 6 (LOTS 1-18)	= 44
TOTAL EXISTING UNITS	**= 109**

PROPOSED UNIT BREAKDOWN:

SINGLE FAMILY DETACHED VILLAS (50' x 100') BLOCKS:1 (LOTS 21-29), 2 (LOTS 16-33), 5 (LOT 44), 10, 11, 12, & 13	= 186
TOWNHOMES 22' x 130' BLOCKS:14, 15, & 16	= 142
TOWNHOMES 18' x 130' BLOCKS:20, & 21	= 146
TOWNHOMES 18' x 130' BLOCKS: 17, 18, & 19	= 97
MULTI-FAMILY AREA BLOCK 3	= 84
MULTI-FAMILY AREA BLOCK 4	= 60
MULTI-FAMILY AREA BLOCK 7	= 108
DUPLEX (37' x 100') BLOCKS: 5 (LOTS 45-56), 8, & 9	= 58
TOTAL PROPOSED UNITS	**= 881**

SITE GENERAL NOTES:

1) DISTURBED & FILLED AREAS OF THE LA-MSID CANAL R.O.W. OR COUNTY R.O.W. SHALL BE RESTORED, STABILIZED, SODDED, & MAINTAINED UNTIL WELL ESTABLISHED. ALL SLOPED AREAS WITHIN THE PROJECT SHALL LIKEWISE BE SODDED.
2) CONTRACTOR SHALL SEED AND MULCH ALL OTHER DISTURBED AREAS WITHIN THE PROJECT AREA AND/OR OTHERWISE RESTORE THEM TO EXISTING CONDITION OR BETTER.
3) CONTRACTOR SHALL COORDINATE ALL WORK IN THE COUNTY RIGHT-OF-WAY WITH THE LEE COUNTY D.O.T.&E.
4) ALL CONSTRUCTION TO BE IN ACCORDANCE WITH FLORIDA DEPT. OF TRANSPORTATION (F.D.O.T.) STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, CURRENT EDITION.
5) THE CONTRACTOR SHALL SUPPLY SATISFACTORY DENSITY TESTS OF SUBGRADE & LIMEROCK BASE MATERIALS TO THE ENGINEER PRIOR TO PAYMENT &/OR FINAL ACCEPTANCE. ONE TEST PER COURSE PER 600 SQUARE YARDS.
6) CONTRACTOR SHALL PROVIDE AND MAINTAIN TRAFFIC CONTROL IN ACCORDANCE WITH F.D.O.T. ROADWAY AND TRAFFIC DESIGN STANDARDS CURRENT EDITION & THE FEDERAL HIGHWAY ADMINISTRATION'S MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD), PART IV.
7) ALL PAVEMENT STRIPING TO BE TWO COATS OF HEAVY DUTY ACRYLIC BASED TRAFFIC PAINT.
8) ALL TRAFFIC SIGNS TO BE DRIVEN STEEL POST TYPE GROUND SIGNS IN ACCORDANCE WITH F.D.O.T. INDEX DRAWINGS 11860 & 11863 (80 M.P.H. WIND LOAD) AND F.D.O.T. SPECIFICATIONS SECTION 700-HIGHWAY SIGNING.
9) HANDICAP RAMPS TO MEET ALL CURRENT ADA REQUIREMENTS.
10) A RIGHT-OF-WAY USE PERMIT WILL BE REQUIRED FOR ALL WORK WITHIN COUNTY RIGHT-OF-WAY.
11) THIS PROJECT PRESENTS NO ADVERSE IMPACTS ON LOCAL GROUNDS OR SURFACE WATERS, & DOES NOT ADVERSELY IMPACT THE FLOOD ZONE.
12) THIS SITE CAN BE USED FOR THE DEVELOPMENT AS SHOWN WITHOUT UNIQUE DANGER FROM FLOOD, ADVERSE SOIL, &/OR FOUNDATION CONDITIONS, SUBJECT TO PROPER GEOTECHNICAL INVESTIGATION, SITE DESIGN, & STRUCTURE DESIGN.

ABBREVIATIONS

W/	=	WITH
LF	=	LINEAR FOOT
P.U.E.	=	PUBLIC UTILITY EASEMENT
D.E.	=	DRAINAGE EASEMENT
L.M.E.	=	LAKE MAINTENANCE EASEMENT
R.O.W.	=	RIGHT-OF-WAY
T.O.B.	=	TOP OF BANK
U.E.	=	UTILITY EASEMENT
AC	=	ACRES
SF	=	SQUARE FEET
E.O.P.	=	EDGE OF PAVEMENT
H.C.	=	HANDICAPPED
SWMA	=	STORMWATER MANAGEMENT AREA
LSB	=	LANDSCAPE BUFFER
S.F.A.V.	=	SINGLE FAMILY ATTACHED VILLA
S.F.D.V.	=	SINGLE FAMILY DETACHED VILLA

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
1	12-11-25	REVISED PER SFWMD COMMENTS	JLW

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

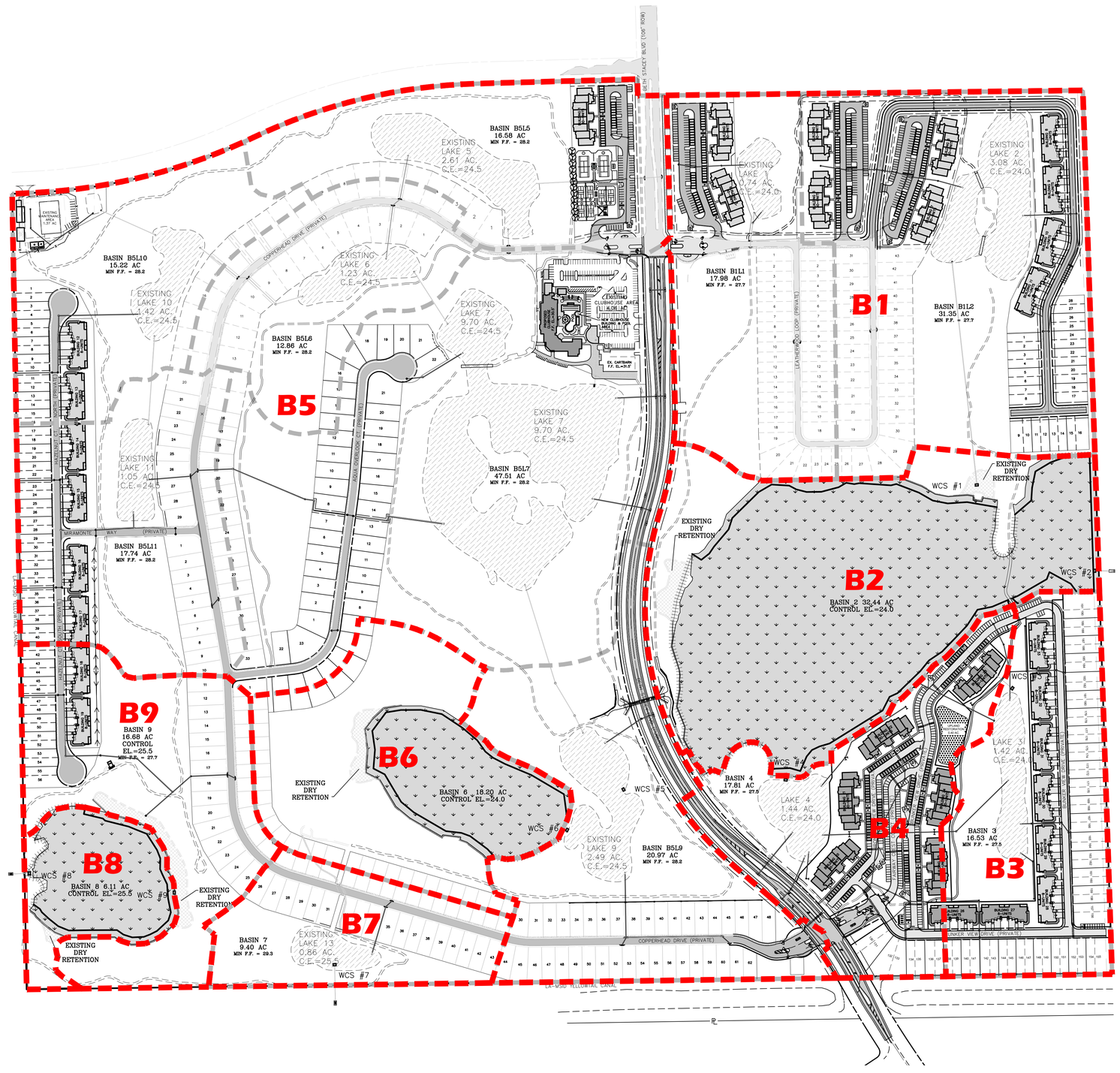
DAVID R. UNDERHILL JR.
P.E. #47029

SITE DEVELOPMENT PLAN
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	SDP	JLW	JLW	DRU	1"=200'	03

IBIS LANDING

S:\JOBS\3142L\ENGINEERING\DRAWINGS\SFWMD PLANS\MODIFICATION\3142-SFWMD_MOD_03_SDP.DWG 2/3/2026 2:01 PM JIMMY WESNEY





0 200' 400' 600'

GRAPHIC SCALE 1" = 200'

LEGEND:

- EXISTING ASPHALT
- EXISTING E.O.P.
- PROPOSED ASPHALT PAVEMENT
- PROPOSED TOP OF BANK
- EXISTING DETENTION–RETENTION AREA
- LANDSCAPE BUFFER RIGHT–OF–WAY LINE
- EXISTING STORM WATER MANAGEMEI
- PROPOSED CONCRETE
- WETLAND AREA & BUFFER
- PROPERTY LINE
- PROPOSED BERM
- SUB–BASIN LINE
- MAJOR BASIN LINE
- PROPOSED STRUCTURAL BUFFER

STORMWATER MANAGEMENT SYSTEM
MINIMUM ELEVATIONS (NGVD)

Basin	WCS#	Control Elev.	5 Yr. Flood Stage	10 Yr. Flood Stage	Min. Road	25 Yr. Flood Stage	Min. Berm	100 Yr. Flood Stage	Min. Floor Elev.
B1	WCS 1	24.00	24.54	25.80	26.50	27.04	27.40*	27.44	28.00
B2	WCS 2	24.00	24.66	24.84	26.50	25.97	26.45	27.43	28.00
B3	WCS 3	24.00	25.78	25.92	26.50	26.86	27.35	27.43	28.00
B4	WCS 4	24.00	25.79	25.97	26.50	26.73	27.20	27.43	28.00
B5	WCS 5	24.50	25.54	25.87	26.50	27.32	27.85	27.94	28.30
B6	WCS 6	24.00	25.39	25.63	26.50	26.75	N/A	27.57	28.00
B7	WCS 7	25.50	27.13	27.44	27.50	28.52	29.05	29.27	29.30
B8	WCS 8	25.50	26.17	26.36	27.50	27.52	28.05	28.39	28.40
B9	WCS 9	25.50	26.17	26.36	27.50	27.52	28.05	28.39	28.40

* EXISTING PERIMETER BERM WILL PROVIDE 4" OF FREEBOARD ALONG NORTH PROPERTY LINE. MINIMUM BERM HEIGHT IS 27.6' ALONG LA–MSID CANALS.

PLEASE NOTE: B2 & B7 WILL HAVE BERM IMPROVEMENTS TO REACH REQUIRED 6" FREEBOARD.

5 YEAR 24 HR RAINFALL IS 5.20"
10 YEAR 24 HR TOTAL RAINFALL IS 6.00"
25 YEAR 72 HR TOTAL RAINFALL IS 10.20"
100 YEAR 72 HR TOTAL RAINFALL IS 14.20"

ALLOWABLE DISCHARGE PER EXISTING SFWMD PERMIT (55 CSM) = 25.68 CFS
TOTAL PROJECT DISCHARGE = 13.55 CFS (28.6 CSM)

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278–1199
FAX: (239) 278–3174

NO.	DATE	REVISION DESCRIPTION	BY
1	12–11–25	REVISED PER SFWMD COMMENTS	JLW

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939–5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625–1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

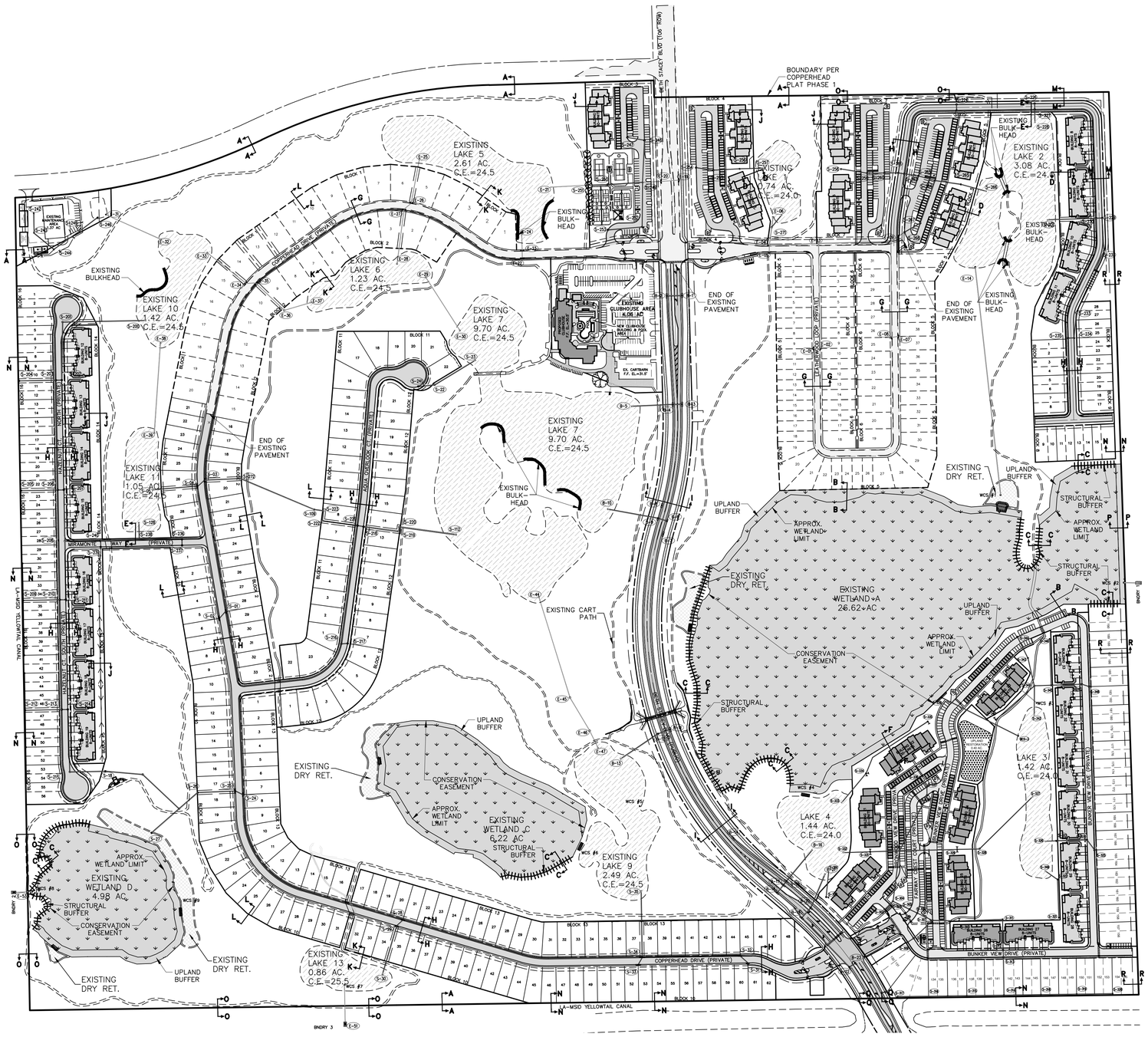
DAVID R. UNDERHILL JR.
P.E. #47029

STORM WATER MANAGEMENT PLAN
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	SWP	JLW	JLW	DRU	1"=200'	04

IBIS LANDING

S:\JOBS\3142L\CIVIL\ENGINEERING\DEVPLANS\SFWMD PLANS\MISC\3142L-SFWMD_MOD_04_LAYOUT.DWG 2/3/2026 2:02 PM JIMMY MCKENY



DRAINAGE GENERAL NOTES:

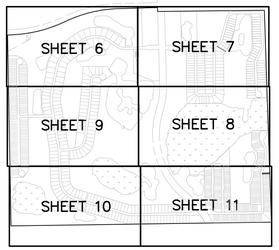
1) CONTRACTOR SHALL MAINTAIN A CURRENT AND UPDATED SET OF AS-BUILT DRAWINGS AT ALL TIMES AND PROVIDE ONE COPY TO THE ENGINEER UPON COMPLETION OF CONSTRUCTION.
2) ALL BACKFILL SHALL BE COMPACTED IN STRICT ACCORDANCE WITH THE BEDDING DETAILS.
3) CONTRACTOR SHALL COORDINATE ALL WORK IN THE COUNTY RIGHT-OF-WAY WITH THE LEE COUNTY D.O.T.&E.
4) ALL REINFORCED CONCRETE PIPE (RCP) CULVERTS SHALL MEET ASTM C 76, CLASS III, TYPE "B" WALL FOR ROUND PIPE AND MEET ASTM C 507, TABLE I FOR ELLIPTICAL PIPE AND OTHERWISE BE INSTALLED AND MANUFACTURED IN ACCORDANCE WITH F.D.O.T. SECTIONS 430 & 941.
5) ALL ROOF TOP DRAINS ARE TO BE DIRECTED TO DRAINAGE AREA.
6) CONTRACTOR SHALL SEED AND MULCH ALL DISTURBED AREAS AND OTHERWISE RESTORE ALL AREAS TO EXISTING CONDITION OR BETTER.
7) ALL WORK SHOWN BY INDEX NO. SHALL BE IN ACCORDANCE WITH F.D.O.T. ROADWAY AND TRAFFIC DESIGN STANDARDS, LATEST EDITION.
8) CONTRACTOR SHALL SOD ALL DISTURBED AREAS WITHIN THE COUNTY RIGHT-OF-WAY
9) THE GRADES REPRESENT THE FINISHED GRADE ON TOP OF THE SOD.
10) ALL SLOPES TO BE 3:1 MAXIMUM.
11) A RIGHT-OF-WAY USE PERMIT WILL BE REQUIRED FOR ALL WORK WITHIN COUNTY RIGHT-OF-WAY.
12) SEE PHASING PLAN FOR PHASE LAYOUT.
13) ALL LAKE AREAS ARE TAKEN FROM CONTROL ELEVATION (WSWT).
14) ALL DRY DETENTION AREAS TAKEN FROM TOP OF BANK.
15) THIS PROJECT PRESENTS NO ADVERSE IMPACTS ON LOCAL GROUNDS OR SURFACE WATERS, & DOES NOT ADVERSELY IMPACT THE FLOOD ZONE.
16) THIS SITE CAN BE USED FOR THE DEVELOPMENT AS SHOWN WITHOUT UNIQUE DANGER FROM FLOOD, ADVERSE SOIL, &/OR FOUNDATION CONDITIONS, SUBJECT TO PROPER GEOTECHNICAL INVESTIGATION, SITE DESIGN, & STRUCTURE DESIGN.

N

0 200' 400' 600'
GRAPHIC SCALE 1" = 200'

LEGEND:

- EXISTING ASPHALT
- EXISTING E.O.P.
- PROPOSED ASPHALT PAVEMENT
- PROPOSED TOP OF BANK
- EXISTING DETENTION-RETENTION AREA
- LANDSCAPE BUFFER
- EXISTING STORM PIPE
- PROPOSED STORM PIPE
- PROPOSED DRAINAGE STRUCTURE
- RIGHT-OF-WAY LINE
- EXISTING STORM WATER MANAGEMEN
- PROPOSED CONCRETE
- WETLAND AREA & BUFFER
- LANDSCAPE BUFFER
- ZONING LINE
- PROPERTY LINE
- PROPOSED STRUCTURAL BUFFER
- "A", "B", or "C" — TYPICAL LOT GRADING DESIGNATION (SEE SHEET 13 FOR DETAILS)
- EXISTING DEVELOPMENT ORDER #DOS 9912034000 & COPPERHEAD PLAT
- IRRIGATION CONDUIT SLEEVE



PGD KEY MAP
N.T.S.

PRIOR TO EXCAVATION CONTACT SUNSHINE STATE ONE - CALL OF FLORIDA 1 (800) 432 - 4770

CONTRACTOR SHALL SUBMIT FOR N.P.D.E.S. PERMIT A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION.

PERMIT APPLICATION SHALL BE SUBMITTED TO:

N.P.D.E.S. STORMWATER NOTICES CENTER, MS # 2510, FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION 2600 BLAIR STONE ROAD, TALLAHASEE, FLORIDA 32399-2400

ABBREVIATIONS

W/	=	WITH
RCP	=	REINFORCED CONCRETE PIPE
INV	=	INVERT
ERCP	=	ELLIPTICAL REINFORCED CONCRETE PIPE
LF	=	LINEAR FOOT
P.U.E.	=	PUBLIC UTILITY EASEMENT
D.E.	=	DRAINAGE EASEMENT
L.M.E.	=	LAKE MAINTENANCE EASEMENT
R.O.W.	=	RIGHT-OF-WAY
N.W.L.	=	NORMAL WATER LEVEL
D.H.W.L.	=	DESIGN HIGH WATER LEVEL
T.O.B.	=	TOP OF BANK
U.E.	=	UTILITY EASEMENT
AC	=	ACRES
SF	=	SQUARE FEET
E.O.P.	=	EDGE OF PAVEMENT
H.C.	=	HANDICAPPED
SWMA	=	STORMWATER MANAGEMENT AREA
D.S.W.T.	=	DRY SEASON WATER TABLE
W.S.W.T.	=	WET SEASON WATER TABLE
LSB	=	LANDSCAPE BUFFER
S.H.W.T.	=	SEASONAL HIGH WATER TABLE
A.D.S.	=	H.D.P.E. DRAINAGE PIPE
S.F.A.V.	=	SINGLE FAMILY ATTACHED VILLA
S.F.D.V.	=	SINGLE FAMILY DETACHED VILLA

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
1	12-11-25	REVISED PER SFWMD COMMENTS	JLW

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

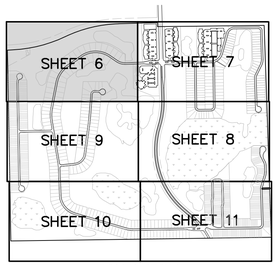
DAVID R. UNDERHILL JR. P.E. #47029

MASTER PAVING, GRADING & DRAINAGE PLAN
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	MDP	JLW	JLW	DRU	1"=200'	05

S:\JOBS\3142L\ENGINEERING\GENPLANS\SFWMD PLANS\MODIFICATION\3142-GPSMD_MOD_05_LAYOUT.DWG 12/3/2025 2:03 PM JIMMY MCKAY





PGD KEY MAP
N.T.S.

NOTE:
MINIMUM FINISH FLOOR ELEVATION FOR ALL BUILDINGS SHALL BE A MINIMUM OF 18" ABOVE THE CENTERLINE ELEVATION OF THE ROAD IN FRONT OF THE BUILDING.

IBIS LANDING

EXISTING BERM

RESIDENTIAL GRADING TO MATCH & DRAIN TO EXISTING GOLF COURSE. EXISTING GOLF COURSE DRAINAGE IS NOT SHOWN.

EXISTING LAKE 5
2.61 AC.
CONTROL
EL.=24.5

REWORK EXISTING PARKING AREA WEST OF THE GOLF MAINTENANCE BUILDING TO FACILITATE DRAINAGE TO S-242 & S-243. RESTRIPE PARKING AS SHOWN

EXISTING MAINTENANCE AREA
1.37 AC
<28.50>

PRETREATMENT AREA (BOTTOM EL.=25.5)

NEW FUEL TANK

NEW STORAGE BINS

NEW CHEMICAL STORAGE BLDG. F.F.=28.5'

NEW WASH RACK

68"-18" RCP

80"-24" RCP

EXISTING 216'-24" ADS

EXISTING 157'-24" ADS

EXISTING 193'-24" ADS

EXISTING 177'-24" ADS

EX. 34'-24" ADS

EXISTING 140'-24" ADS

EXISTING 98'-24" ADS

EXISTING 318'-24" ADS

EXISTING 134'

EXISTING 55'-24" ADS

EXISTING 225'-24" ADS

EXISTING 222'-24" ADS

BLOCK 1

BLOCK 2

BLOCK 12

BLOCK 13

BLOCK 14

BLOCK 16

COPPERHEAD DRIVE (PRIVATE)

EXISTING PLATTED LOTS

EXISTING LAKE 10
1.42 AC.
CONTROL
EL.=24.5

EXISTING LAKE 6
1.23 AC.
CONTROL
EL.=24.5

EXISTING LAKE 7
9.70 AC.
CONTROL
EL.=24.5

EXISTING GOLF CART PATH

HAZELNUT COURT NORTH (PRIVATE)

BUILDING 12
8-UNITS

BUILDING 13
8 UNITS

2' VALLEY GUTTER

R100' TYP

R90' (TYP)

ø110' TYP

297'-24" RCP

212'-24" RCP

132'-18" RCP

40' ROW

20'

10' PUE

8'PUE

AQUA OVERLOOK COURT (PRIVATE)

BLOCK 11

SFDV

PROPOSED 8" L.M.E.

R55'

ø90'

168'-24" RCP

EXISTING BRIDGE

PROPOSED CLUBHOUSE
F.F. EL.=30.5'

MAIL

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

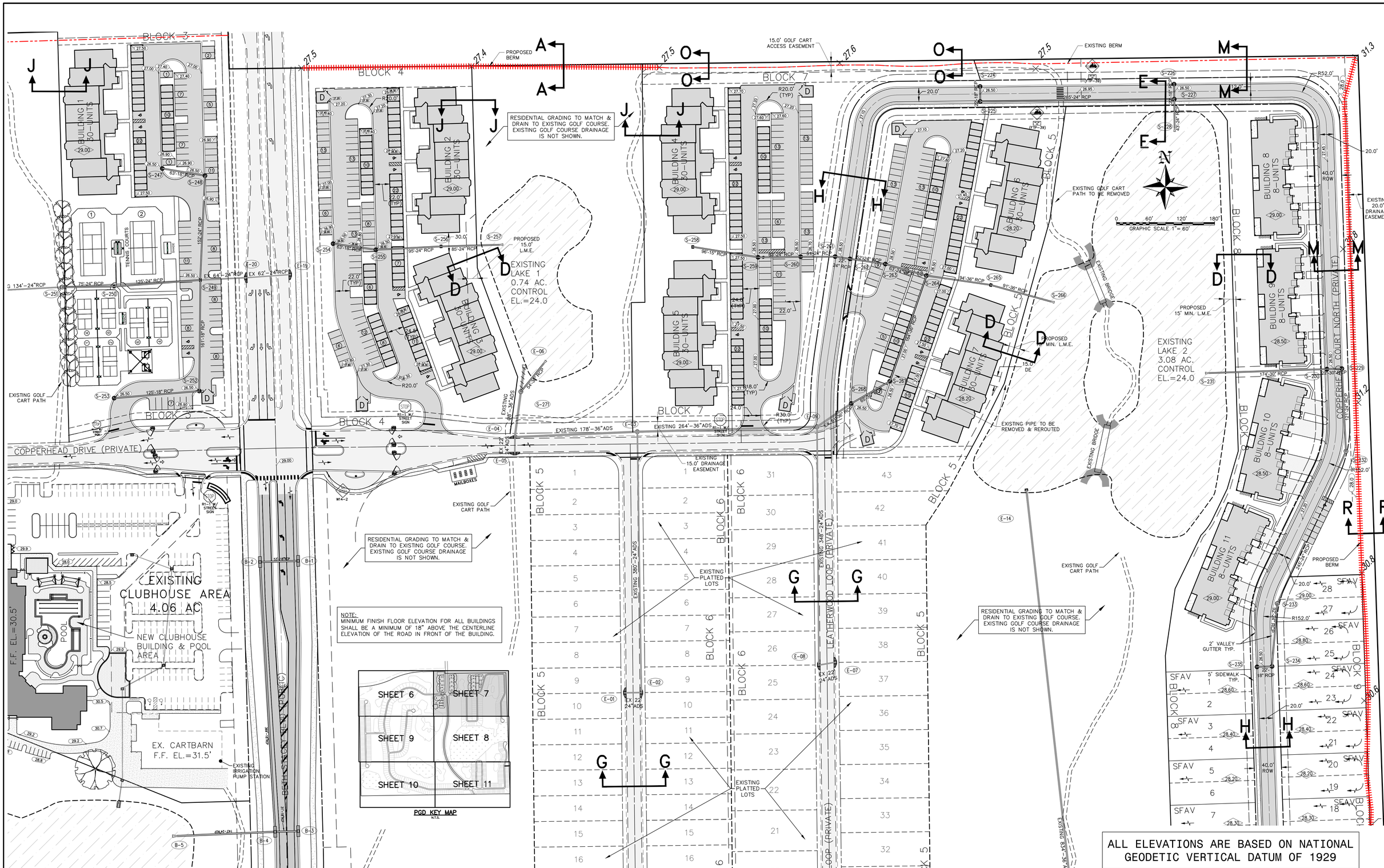
PAVING, GRADING & DRAINAGE PLAN 1

IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-1	JLW	JLW	DRU	1"=60'	06

S:\JOBS\3142L\CAD\LDW\ENGINEERING\DEVWORK\[3142L-SHEET_LAYOUT.DWG]PGD-1 2/3/2026 3:03 PM JIMMY WESKAY



ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

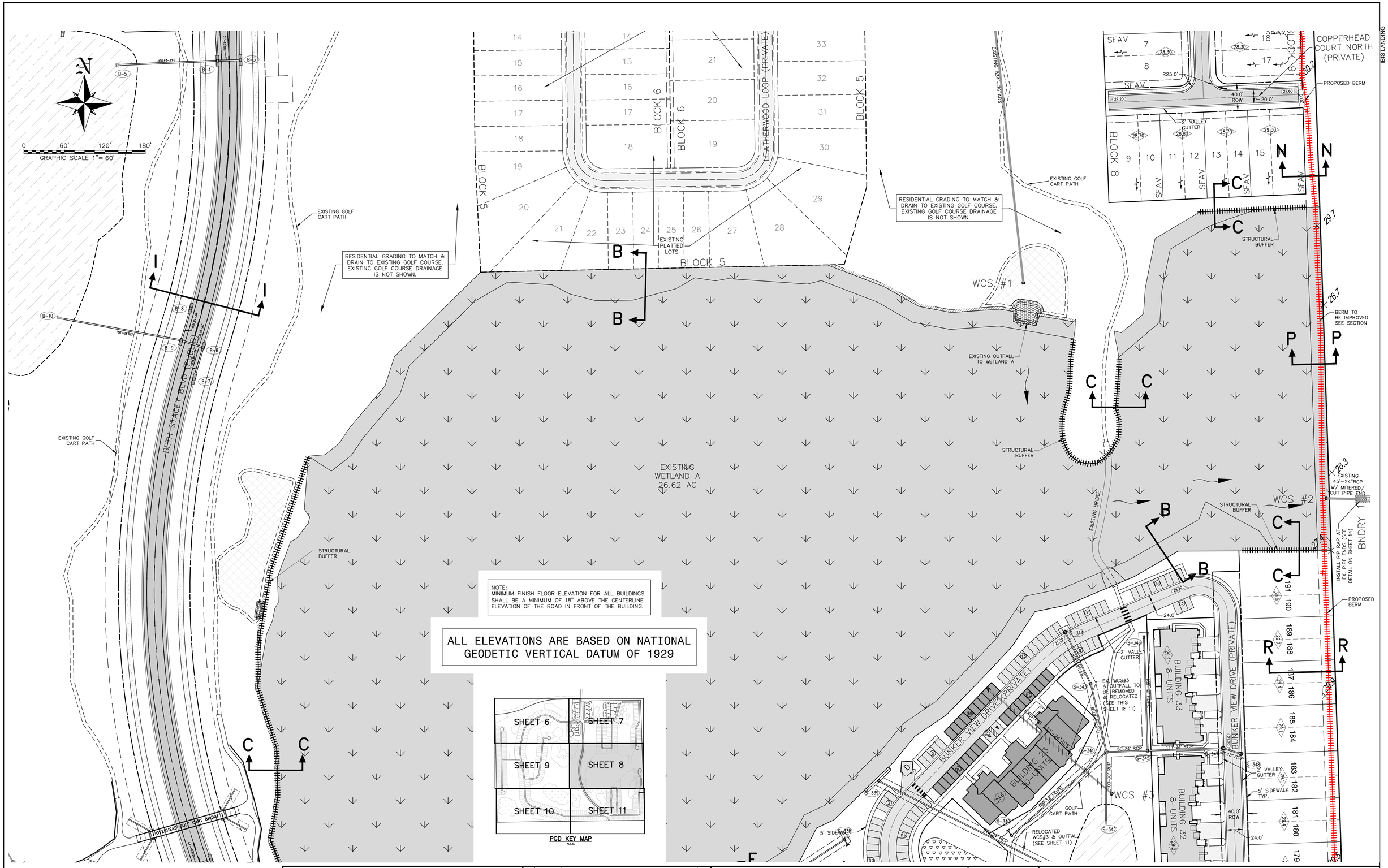
DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE PLAN 2

IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-2	JLW	JLW	DRU	1"=60'	07



NOTE:
MINIMUM FINISH FLOOR ELEVATION FOR ALL BUILDINGS SHALL BE A MINIMUM OF 18" ABOVE THE CENTERLINE ELEVATION OF THE ROAD IN FRONT OF THE BUILDING.

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
1	12-11-25	REVISED PER SFWMD COMMENTS	JLW

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

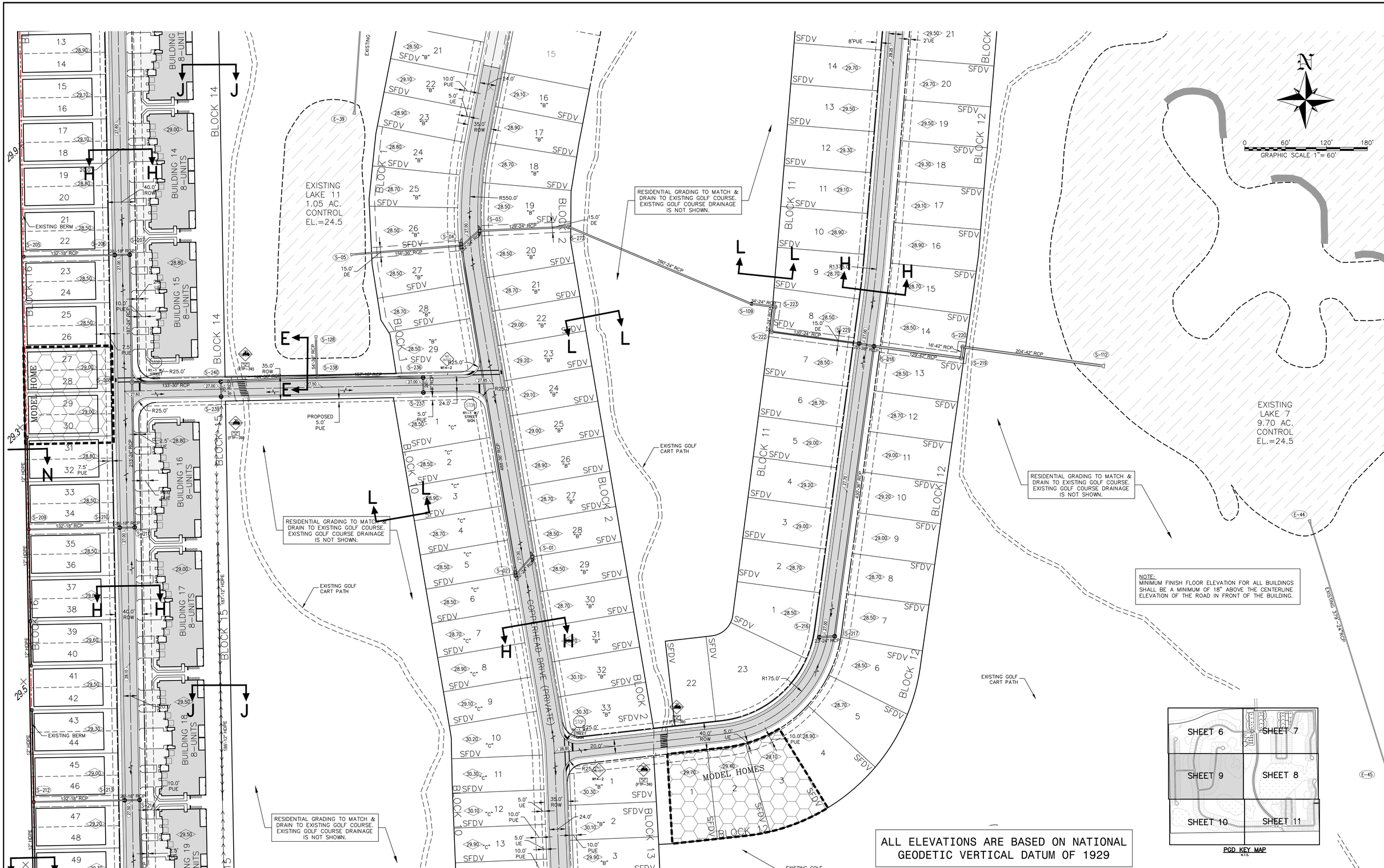
PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE PLAN 3
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-3	JLW	JLW	DRU	1"=60'	08



NOTE:
MINIMUM FINISH FLOOR ELEVATION FOR ALL BUILDINGS SHALL BE A MINIMUM OF 18" ABOVE THE CENTERLINE ELEVATION OF THE ROAD IN FRONT OF THE BUILDING.

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PGD KEY MAP
N.T.S.

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

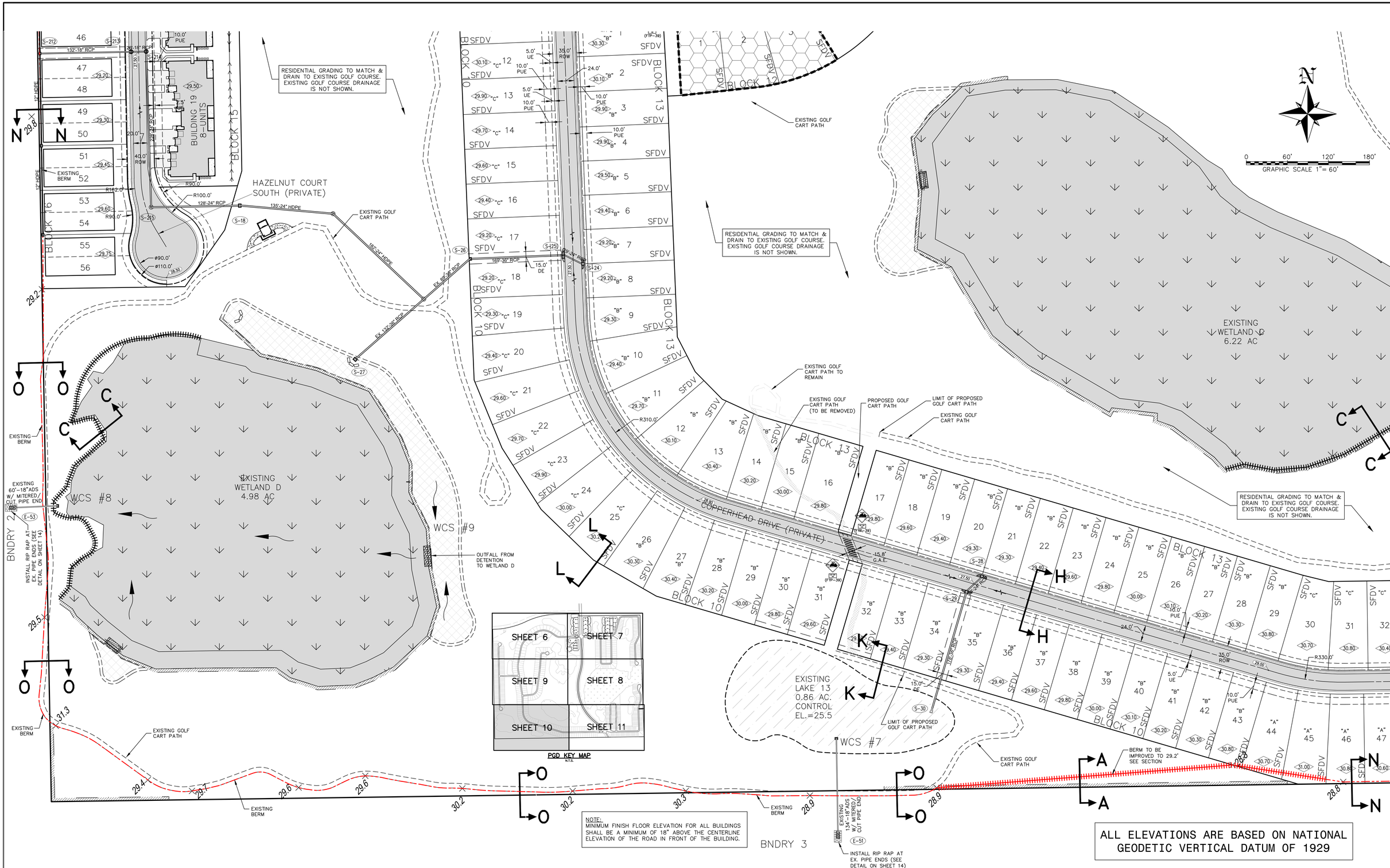
DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE PLAN 4

IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-4	JLW	JLW	DRU	1"=60'	09



NOTE:
MINIMUM FINISH FLOOR ELEVATION FOR ALL BUILDINGS SHALL BE A MINIMUM OF 18" ABOVE THE CENTERLINE ELEVATION OF THE ROAD IN FRONT OF THE BUILDING.

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE PLAN 5
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-5	JLW	JLW	DRU	1"=60'	10

WCS #5

WCS #4

LAKE 3
1.42 AC.
CONTROL
EL.=24.0

LAKE 4
1.44 AC.
CONTROL
EL.=24.0

EXISTING
LAKE 9
2.49 AC.
CONTROL
EL.=24.5

UPLAND
RESTORATION
0.40 AC.

RELOCATED
WCS#3 & OUTFALL
(SEE SHEET 8)

BELL TOWER BLVD (PUBLIC)

EXISTING 170'-24" RCP

BUNKER VIEW DRIVE (PRIVATE)

IRONWOOD VIEW DRIVE (PRIVATE)

FAIRWAY CREST COURT (PRIVATE)

COPPERHEAD DRIVE (PRIVATE)

BUILDING 21
30-UNITS

BUILDING 22
30-UNITS

BUILDING 24
30-UNITS

BUILDING 25
30-UNITS

BUILDING 26
8-UNITS

BUILDING 27
8-UNITS

BUILDING 28
8-UNITS

BUILDING 29
8-UNITS

BUILDING 30
8-UNITS

BUILDING 31
8-UNITS

BUILDING 20
30-UNITS

RESIDENTIAL GRADING TO MATCH & DRAIN TO EXISTING GOLF COURSE. EXISTING GOLF COURSE DRAINAGE IS NOT SHOWN.

RESIDENTIAL GRADING TO MATCH & DRAIN TO EXISTING GOLF COURSE. EXISTING GOLF COURSE DRAINAGE IS NOT SHOWN.

LIMIT OF PROPOSED GOLF CART PATH

EXISTING GOLF CART PATH

PROPOSED GOLF CART PATH

BLOCK 13

BLOCK 10

OPEN SPACE
0.58 AC

EXISTING BERM

PROPOSED BERM

LA-MSID CANAL

40.0' ROW

24.0'

2' VALLEY GUTTER

5' SIDEWALK TYP.

PROPOSED BERM

EXISTING BERM

5' SIDEWALK TYP.

10.0' PUE

5.0' UE

15.0' DE

SHEET 6 | SHEET 7
SHEET 9 | SHEET 8
SHEET 10 | SHEET 11

PGD KEY MAP
N.T.S.

NOTE:
MINIMUM FINISH FLOOR ELEVATION FOR ALL BUILDINGS SHALL BE A MINIMUM OF 18" ABOVE THE CENTERLINE ELEVATION OF THE ROAD IN FRONT OF THE BUILDING.

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

0 60' 120' 180'
GRAPHIC SCALE 1" = 60'

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
1	12-11-25	REVISED PER SFWMD COMMENTS	JLW

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

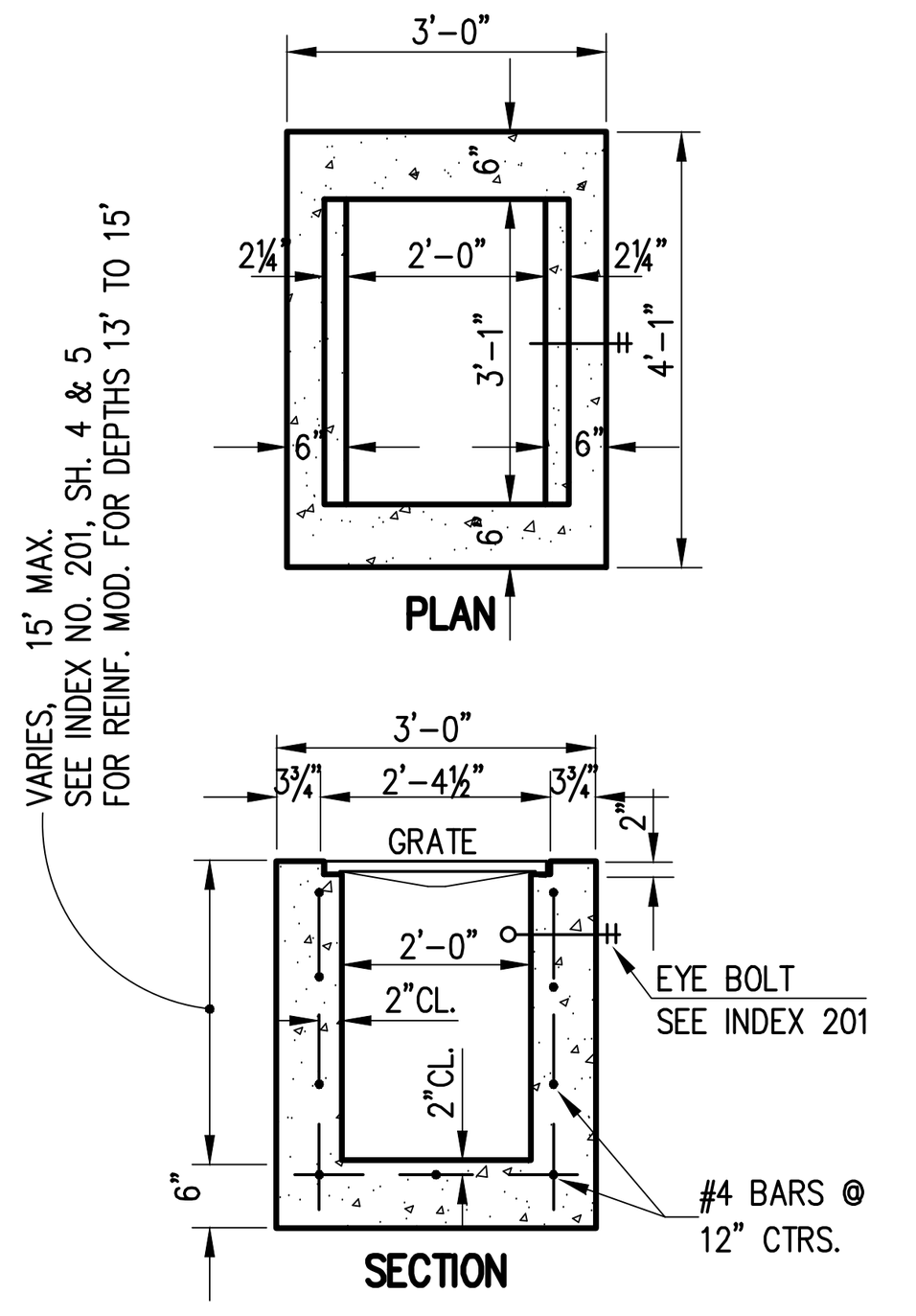
PAVING, GRADING & DRAINAGE PLAN 6

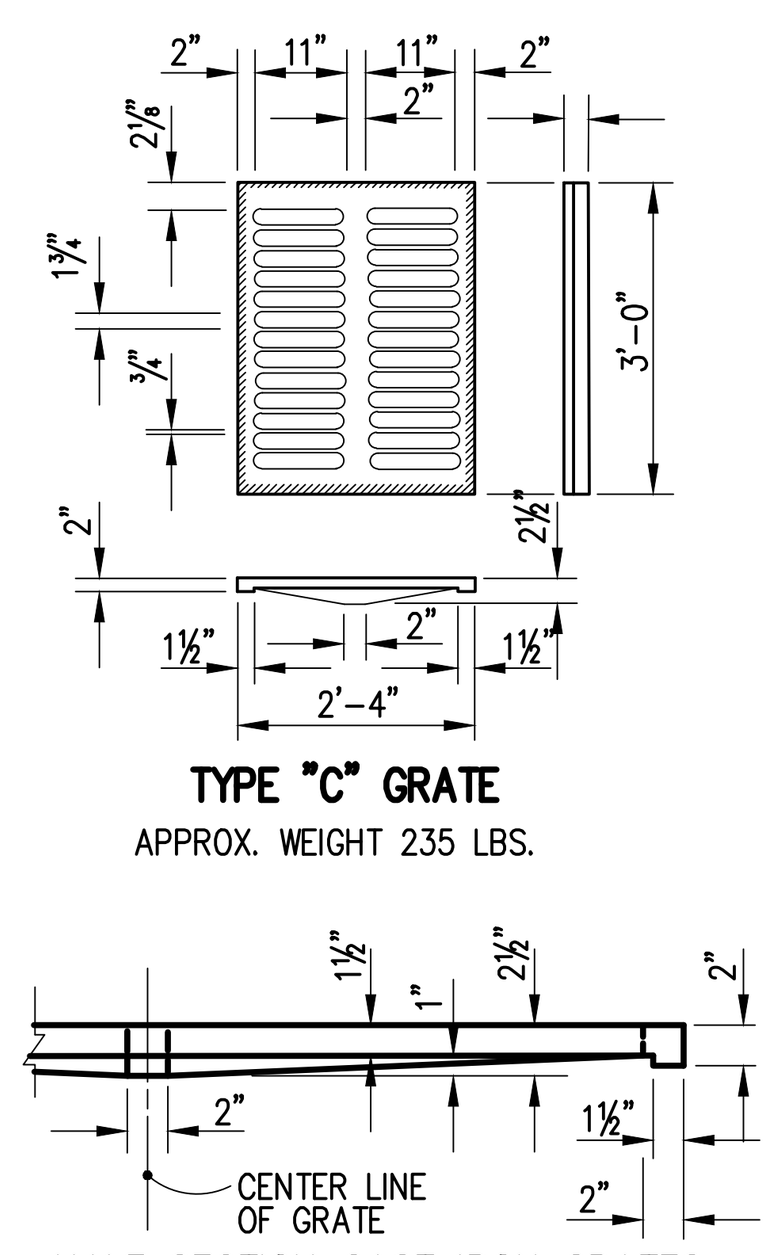
IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-6	JLW	JLW	DRU	1"=60'	11

S:\JOBS\3142L\31421L\ENGINEERING\GEAR_MAPS\SFWMD PLANS\MODIFICATION_1\141-SFWMD_MOD_06-11_PGD2.MXD 2/3/2026 2:14 PM JIMMY WEAKLY

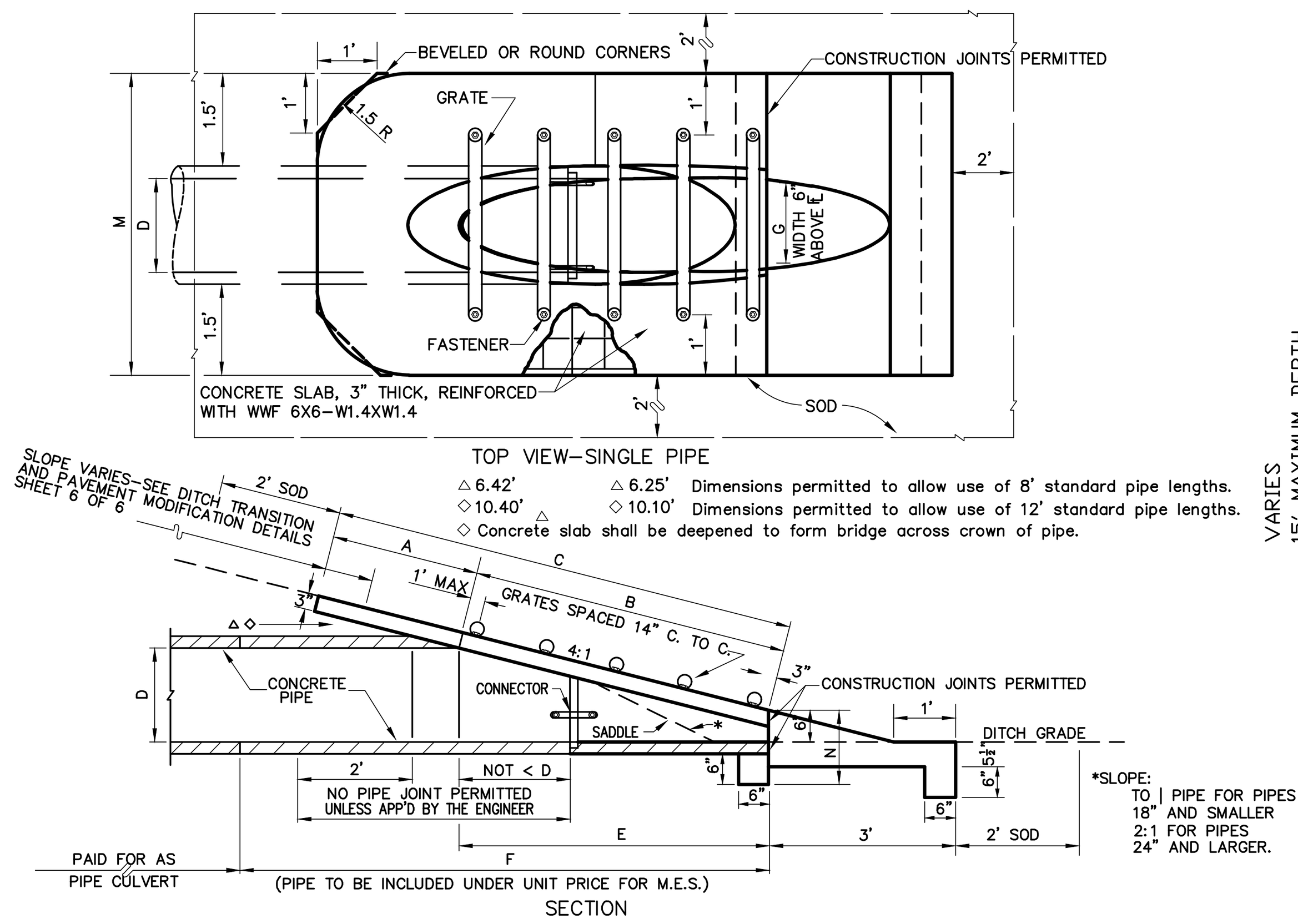




NOTES:
1. 6" WALL THICKNESS MUST BE USED FOR PRE-CAST INLETS IN ACCORDANCE WITH F.D.O.T. INDEX 201.
2. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH)
3. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 425.

MAXIMUM PIPE SIZE RECOMMENDED FOR EACH OF THE WALL SIZES:
2'-0" WALL – 18" PIPE
3'-1" WALL – 24" PIPE

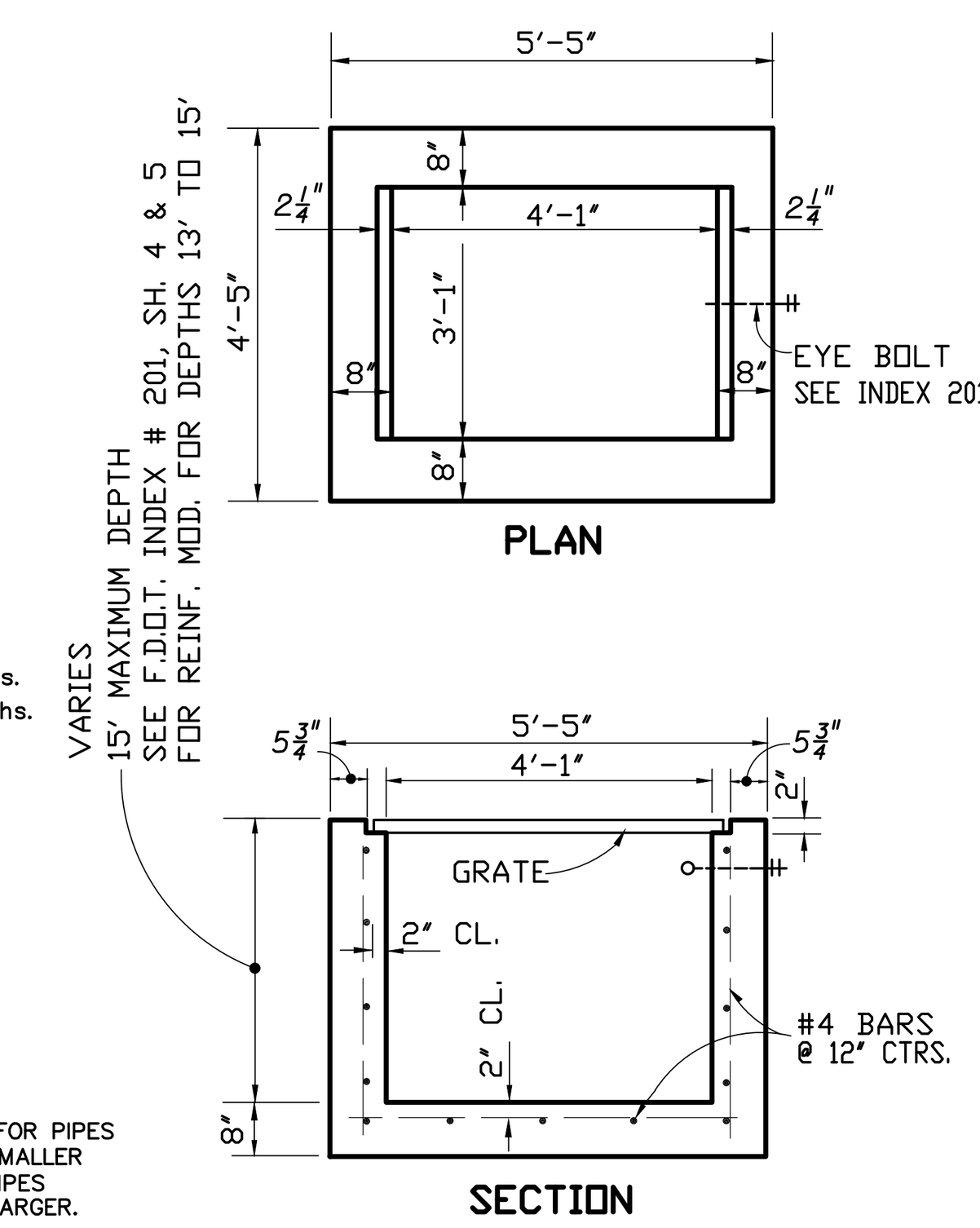
TYPE "C" INLET DETAIL
N.T.S.

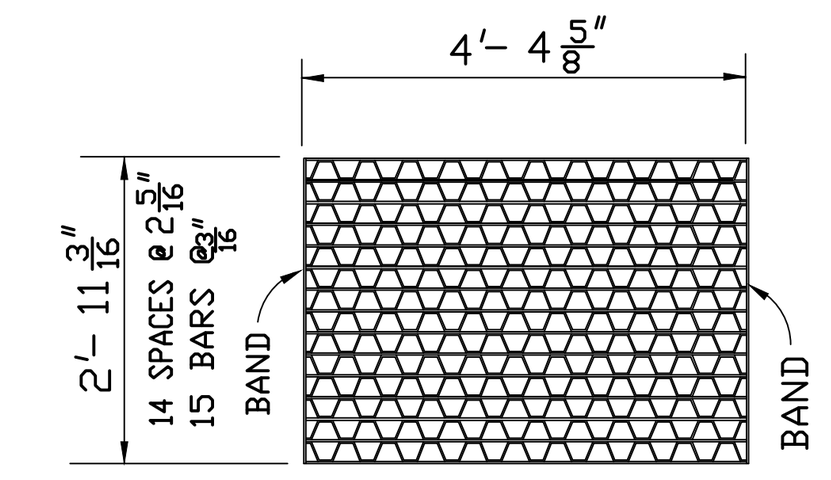


DIMENSIONS & QUANTITIES

D	X	A	B	C	E	F	G	M Single Pipe	M Double Pipe	M Triple Pipe	M Quad Pipe	N	Grate Pipe Size
15"	2'-7"	2.27'	4.09'	6.36'	4.03'	8'	1.22'	4.63'	7.21'	9.79'	12.37'	1.19'	None
18"	2'-10"	2.36'	5.12'	7.48'	5.03'	9'	1.41'	4.92'	7.75'	10.58'	13.42'	1.21'	None
24"	3'-5"	2.53'	7.18'△	9.71'	7.03'△	11'	1.73'	5.50'	8.92'	12.33'	15.75'	1.25'	None
30"	4'-3"	2.70'	9.25'	11.95'	9.03'	13'	2.00'	6.08'	10.33'	14.58'	18.83'	1.29'	2½"
36"	5'-1"	2.87'	11.31'◇	14.18'	11.03'◇	15'	2.24'	6.67'	11.75'	16.83'	21.92'	1.33'	2½"
42"	6'-0"	3.05'	13.37'	16.42'	13.03'	17'	2.45'	7.25'	13.25'	19.25'	25.25'	1.38'	2½"
48"	6'-9"	3.22'	15.43'	18.65'	15.03'	19'	2.65'	7.83'	14.58'	21.33'	28.08'	1.42'	2½"
54"	7'-8"	3.39'	17.49'	20.88'	17.03'	21'	2.83'	8.42'	16.08'	23.75'	31.42'	1.46'	3"
60"	8'-6"	3.56'	19.55'	23.11'	19.03'	23'	3.00'	9.00'	17.50'	26.00'	34.50'	1.50'	3"

MITERED END SECTION
N.T.S.





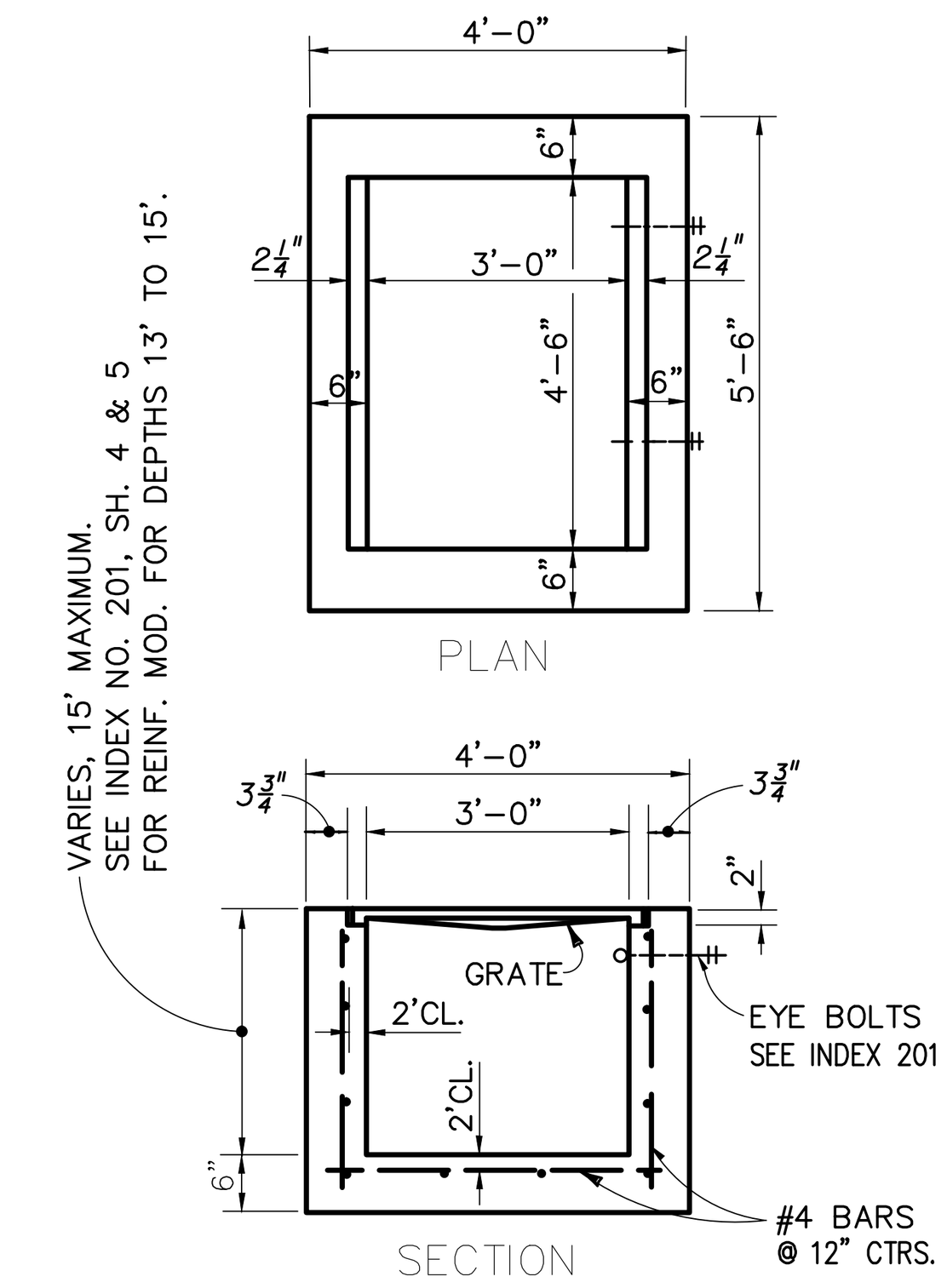
TYPE 'D' GRATE
STRAIGHT BARS 2"X 3/16"
RETICULINE BARS 1¼"X 3/16"
BANDS 1½"X ¼"
APPROX. WEIGHT 180 LBS.

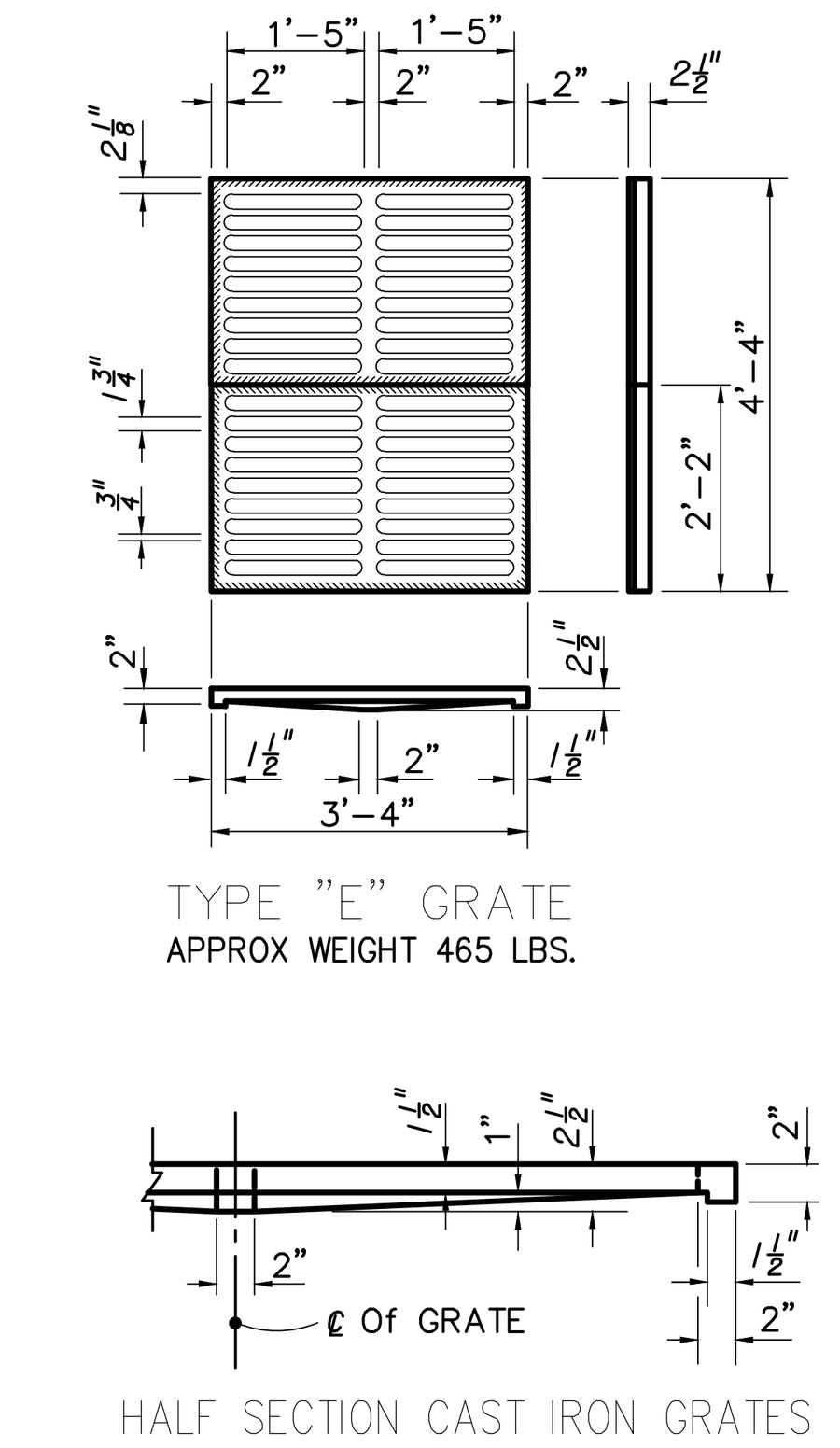
NOTES:
1. 6" WALL THICKNESS MUST BE USED FOR PRECAST INLETS IN ACCORDANCE WITH F.D.O.T. INDEX #201.
2. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH.)
3. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 425.

RECOMMENDED MAXIMUM PIPE SIZE:
3'-1" WALL-24" PIPE
4'-1" WALL-36" PIPE

TYPE 'D' INLET DETAIL
N.T.S.

REINFORCING NOTES:
1. REINFORCING BARS TO BE PLACED A MINIMUM OF 2" FROM BOTTOM & 3" FROM SIDES OF CURB & GUTTER.

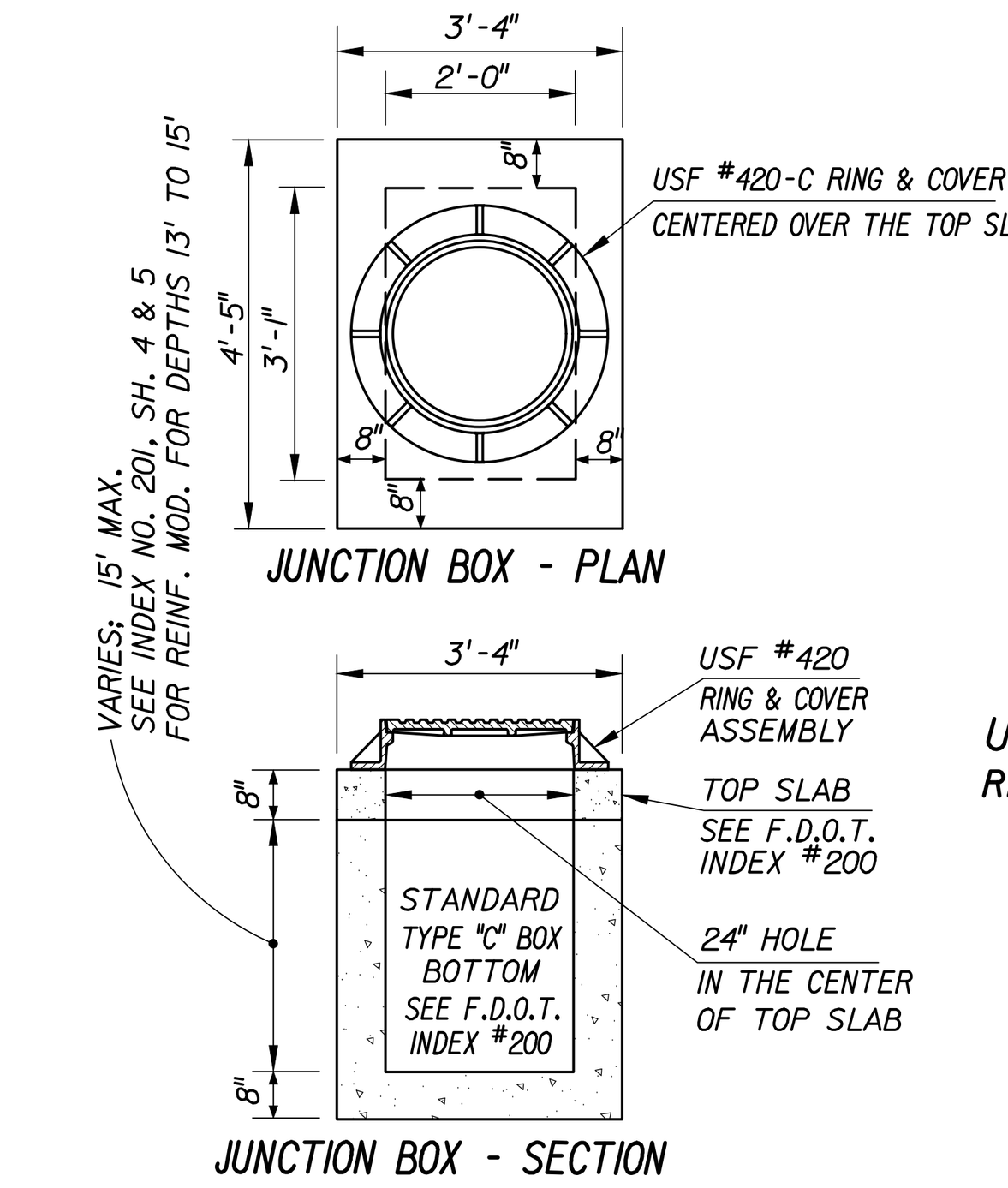


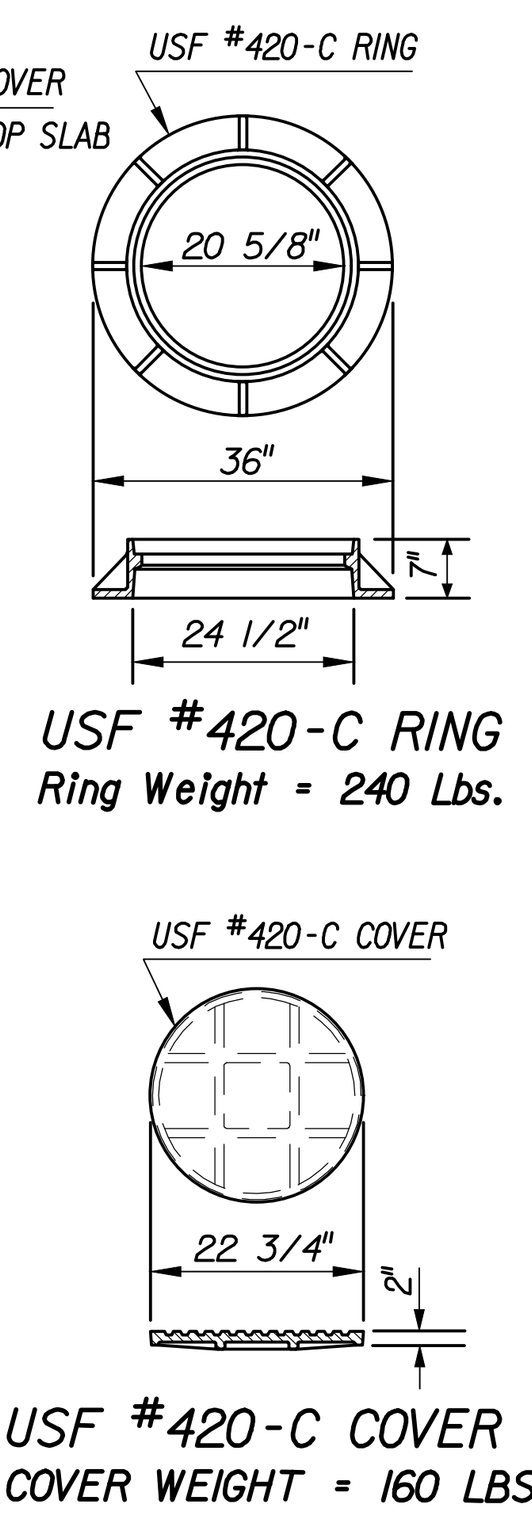


MAXIMUM PIPE SIZES RECOMMENDED FOR EACH OF THE WALL SIZES.
3'-0" WALL – 24" PIPE
4'-6" WALL – 42" PIPE

NOTE:
1. 6" WALL THICKNESS MUST BE USED FOR PRE-CAST INLETS IN ACCORDANCE WITH F.D.O.T. INDEX 201.
2. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH)
3. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATIONS SECTION 425.

TYPE "E" (DITCH BOTTOM) INLET DETAIL
N.T.S.

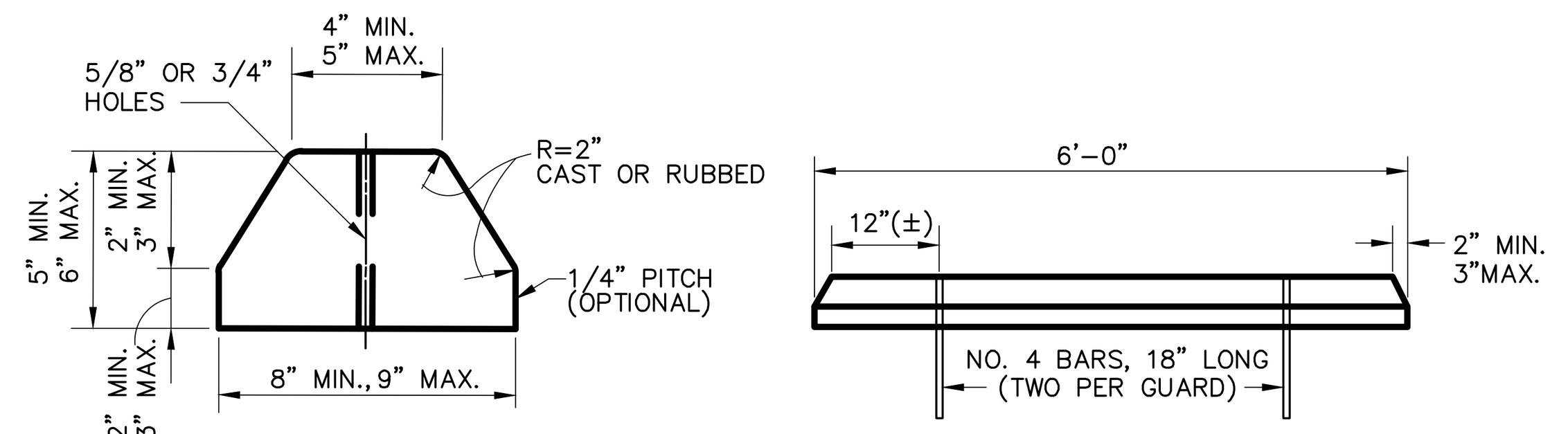




TYPE "C" RECOMMENDED MAXIMUM PIPE SIZE:
2'-0" WALL–18" PIPE
3'-1" WALL–24" PIPE

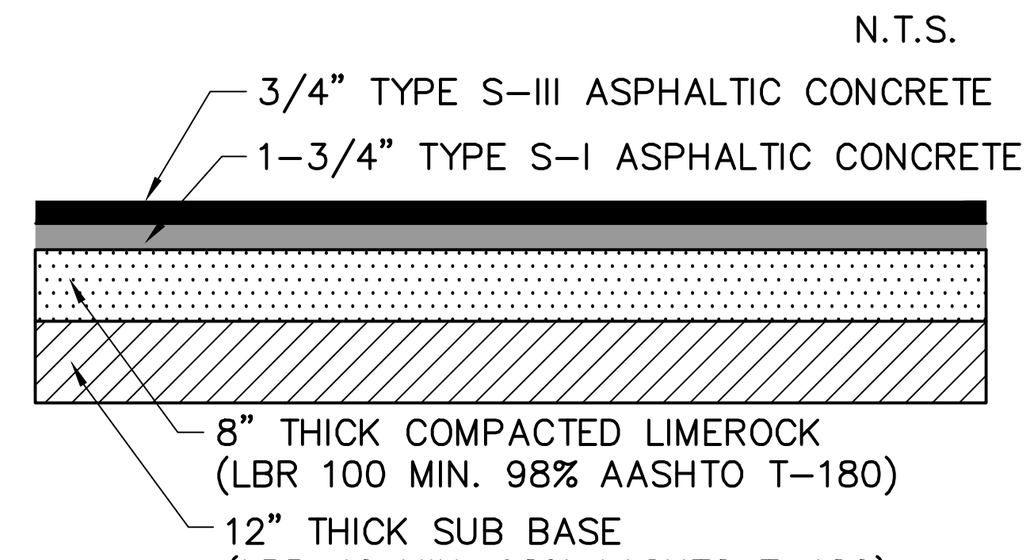
NOTE:
1. 6" WALL THICKNESS MUST BE USED FOR PRE-CAST INLETS IN ACCORDANCE ACCORDANCE WITH F.D.O.T. INDEX #201.
2. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH)
3. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATIONS SECTION 450.

TYPE "C" JUNCTION BOX DETAIL
N.T.S.

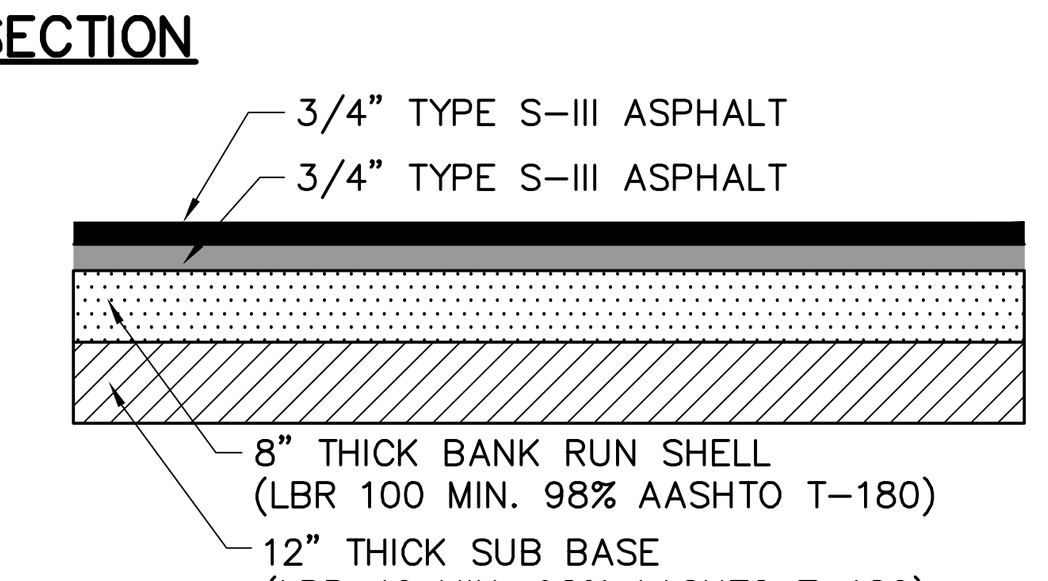


NOTE: CONCRETE TO BE 2,500 PSI STRENGTH MINIMUM.

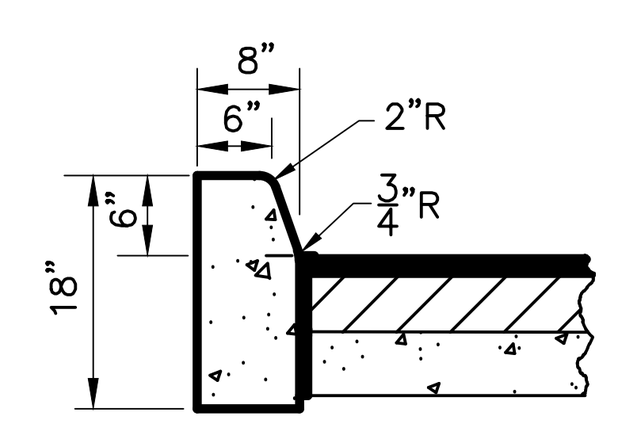
WHEEL STOP DETAIL
N.T.S.



BETH STACEY R.O.W. PAVEMENT SECTION, TYPICAL
N.T.S.

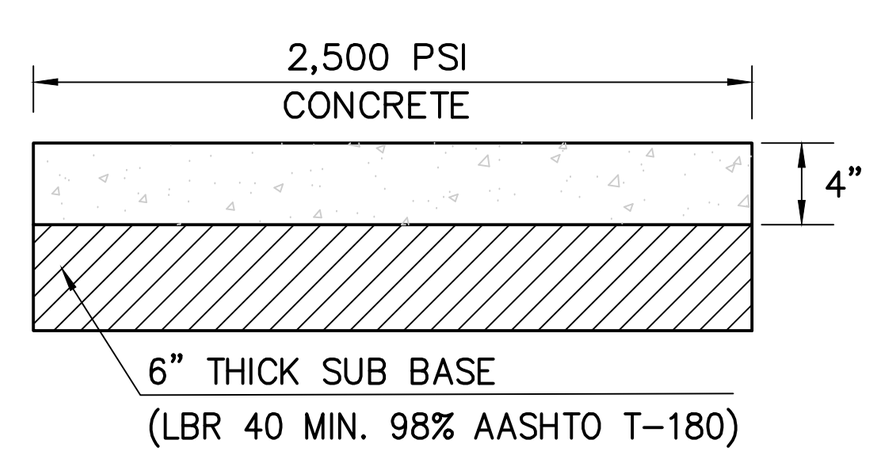


LOCAL STREET & PARKING LOT PAVEMENT SECTION, TYPICAL
N.T.S.

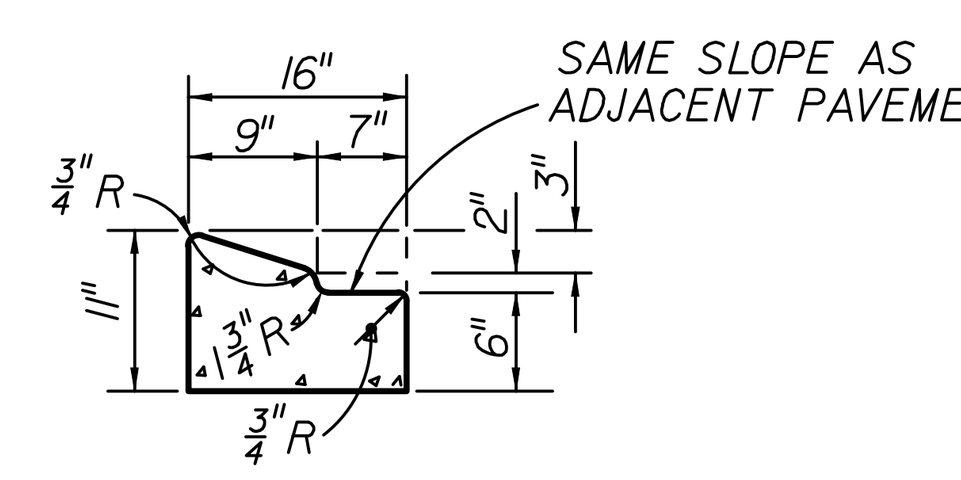


NOTES:
1. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH).
2. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 520.

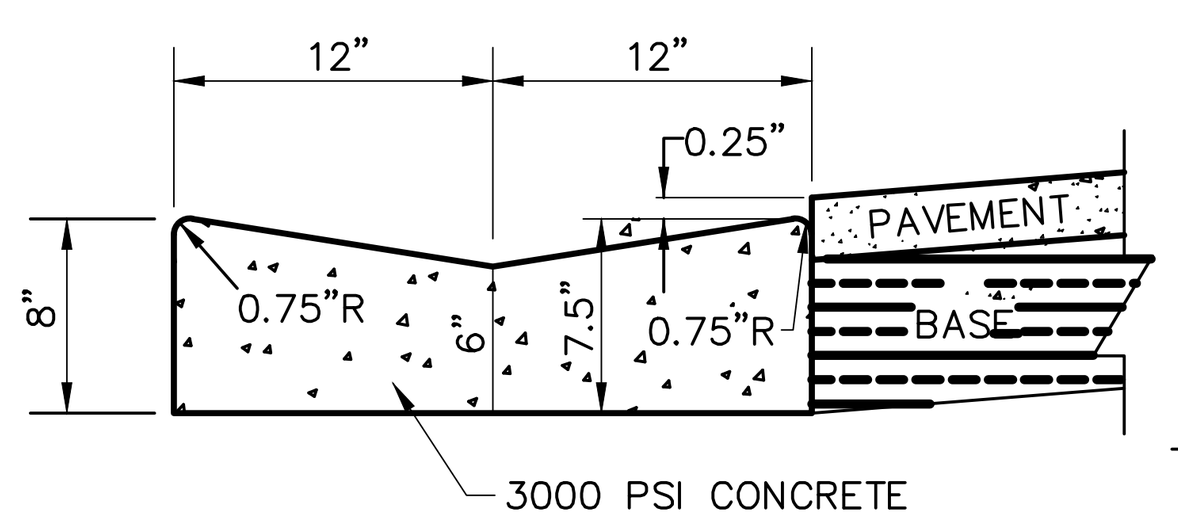
TYPE "D" CURB DETAIL
N.T.S.



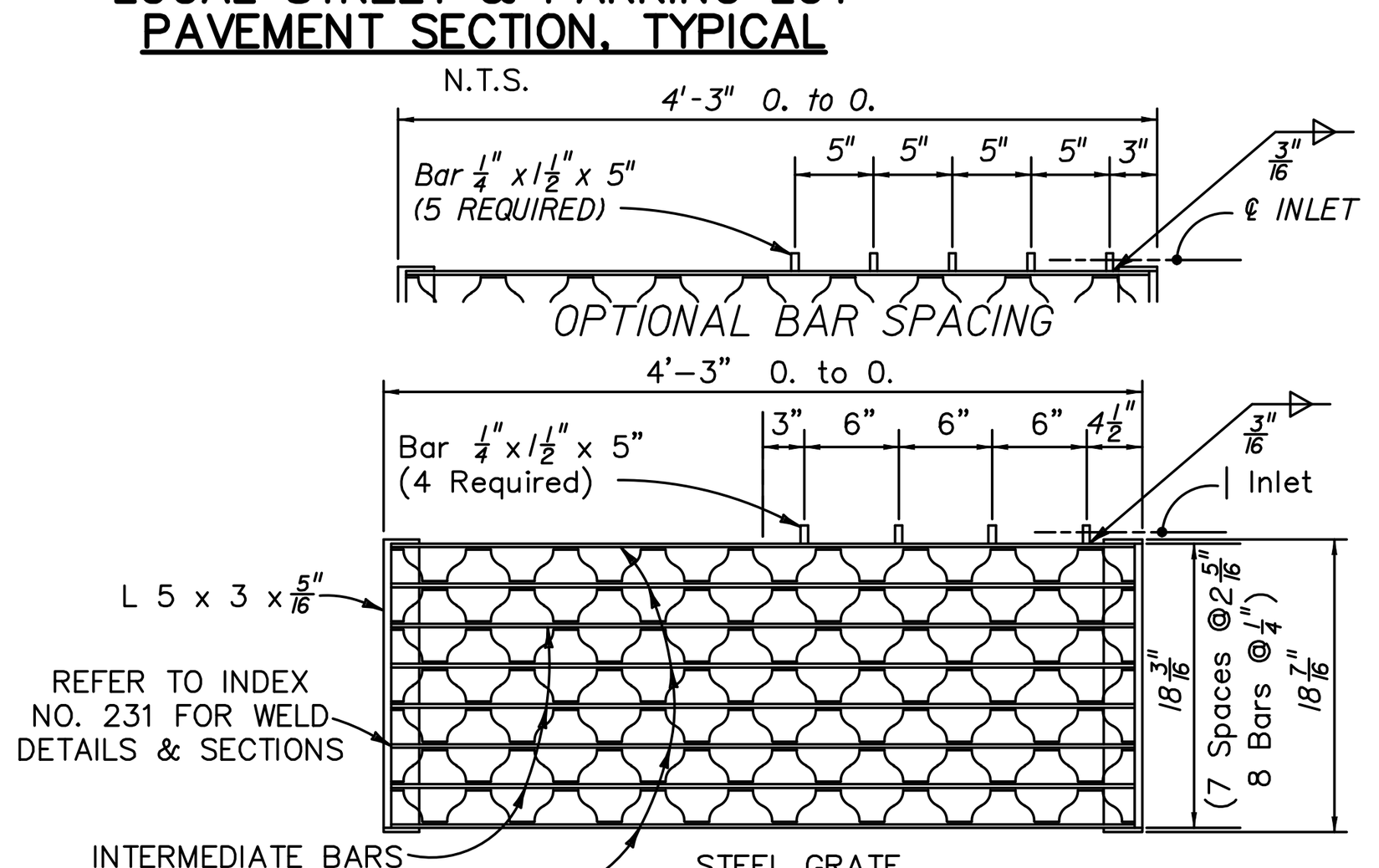
SIDEWALK DETAIL
N.T.S.



STANDARD FDOT TYPE A CURB DETAIL
N.T.S.



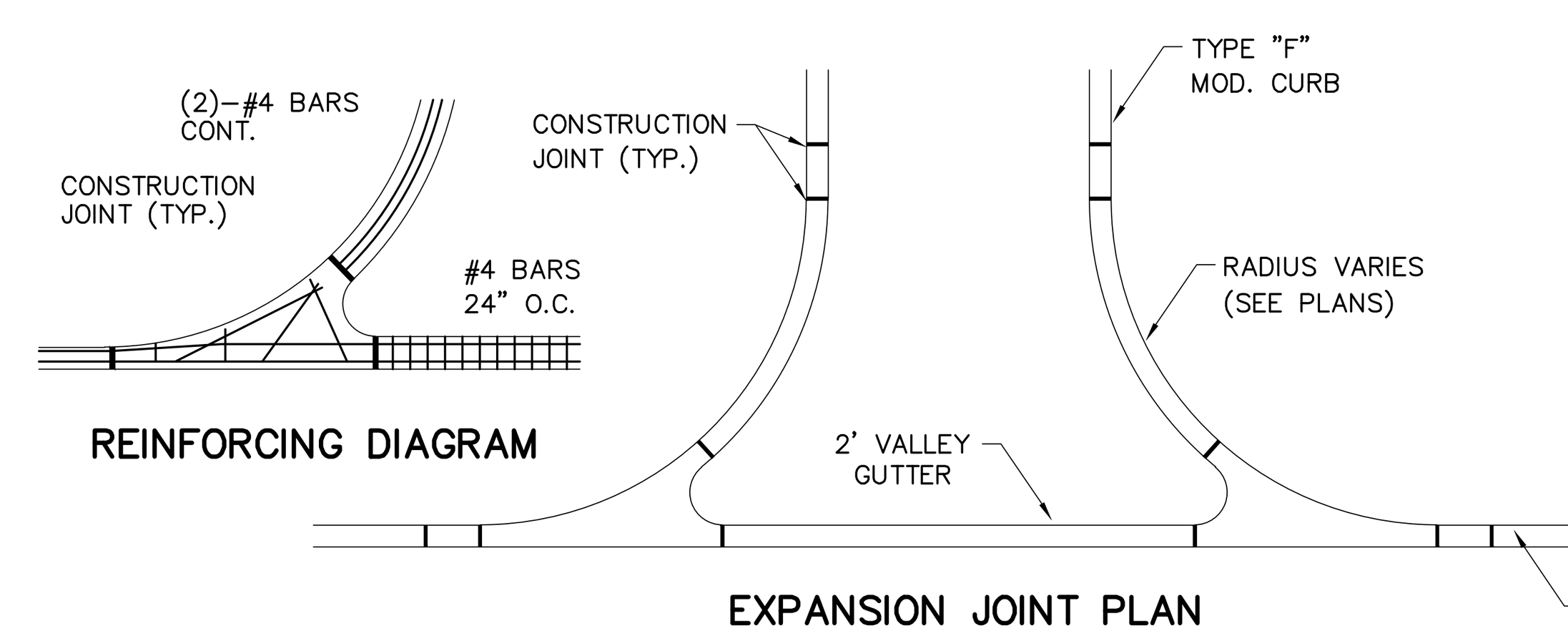
STANDARD 2' CONCRETE VALLEY GUTTER DETAIL
N.T.S.



5" Steel Grate Main Bars 5" x ¼"
Intermediate Bars 1½" x ¼" Reticuline Bars 1¼" x 3/16"

STEEL GRATE : MANUFACTURED BY BORDEN, FLORIDA STEEL, U.S. FOUNDRY IRVING, RELIANCE, GREULICH (OR EQUAL).

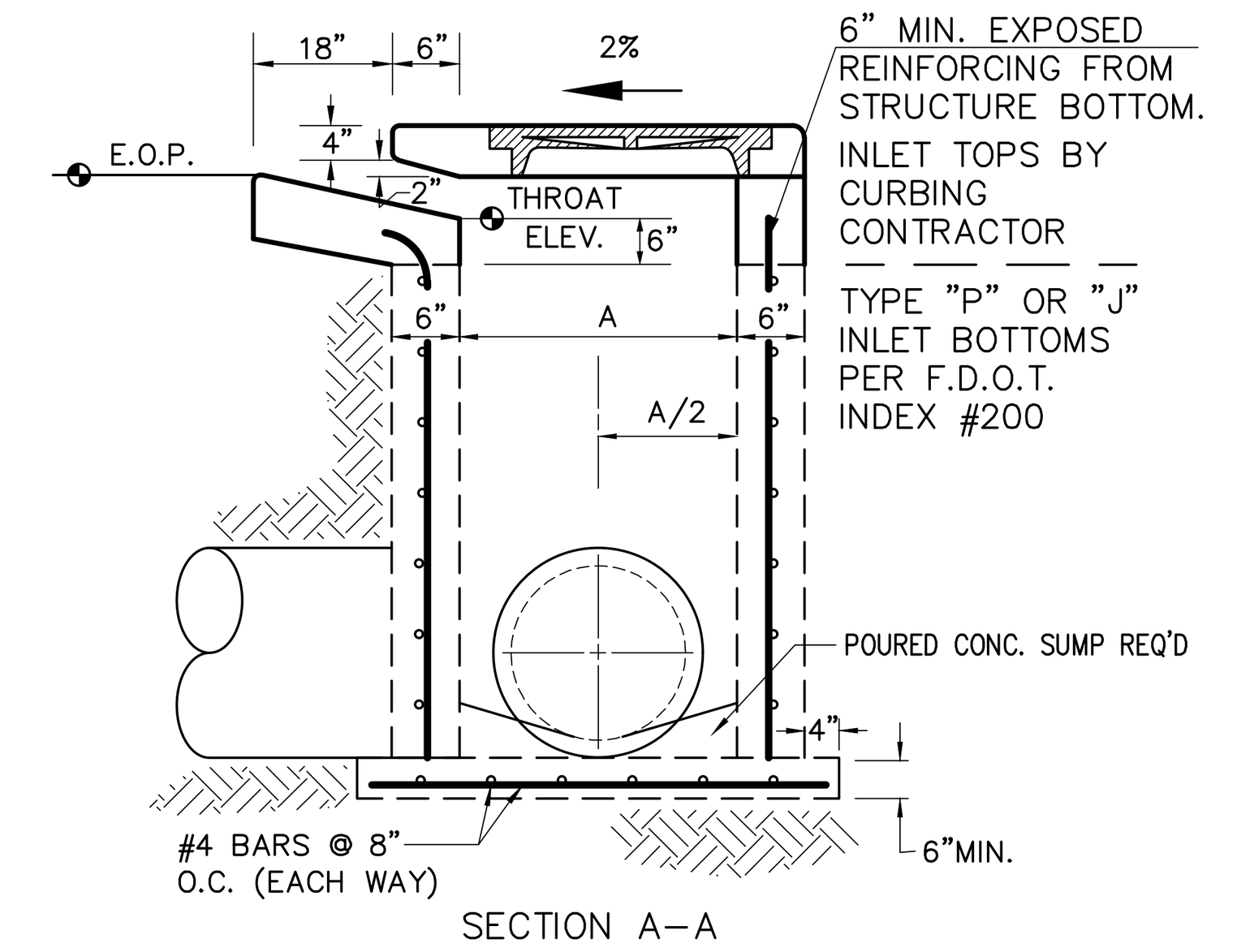
GRATE DETAIL
N.T.S.

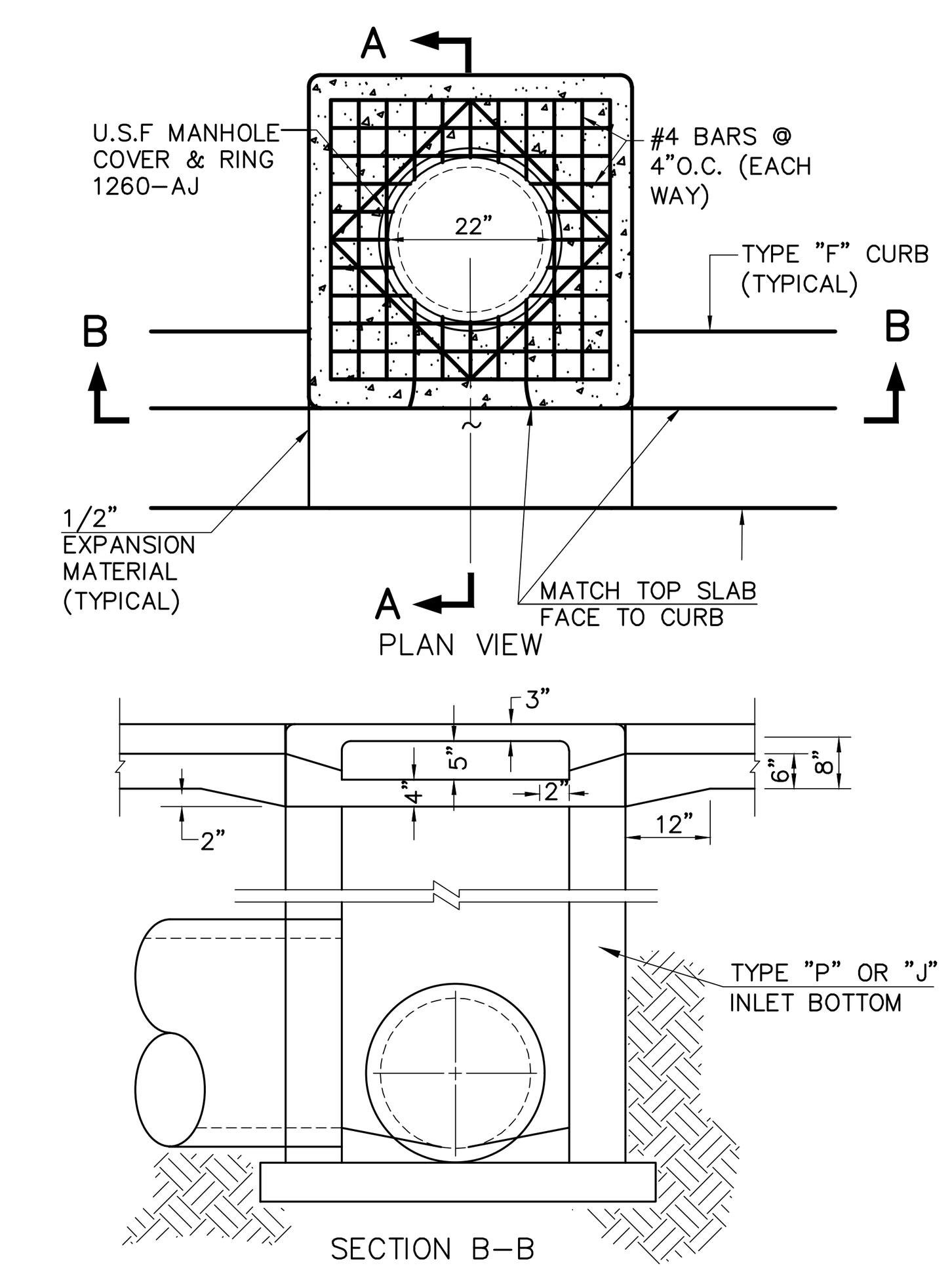


EXPANSION JOINT PLAN

CONCRETE CURB JOINTS:
1. CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH OF 3,000 PSI IN 28 DAYS. UNLESS OTHERWISE NOTED.
2. AN EXPANSION JOINT WILL BE PLACED AT THE END OF ALL RETURNS AT INTERVALS NOT TO EXCEED 50' WITH CONTRACTION JOINTS AT 10' INTERVALS BETWEEN.
3. PVC SLIP JOINTS SHALL BE CONSTRUCTED TO PERMIT EXPANSION AND CONTRACTION OF STEEL ACROSS JOINT.
4. EXPANSION JOINTS SHALL BE CONSTRUCTED WITH 1/2" BITUMINOUS IMPREGNATED WITH EXPANSION JOINT MATERIAL.

VALLEY CROSSING DETAIL
N.T.S.



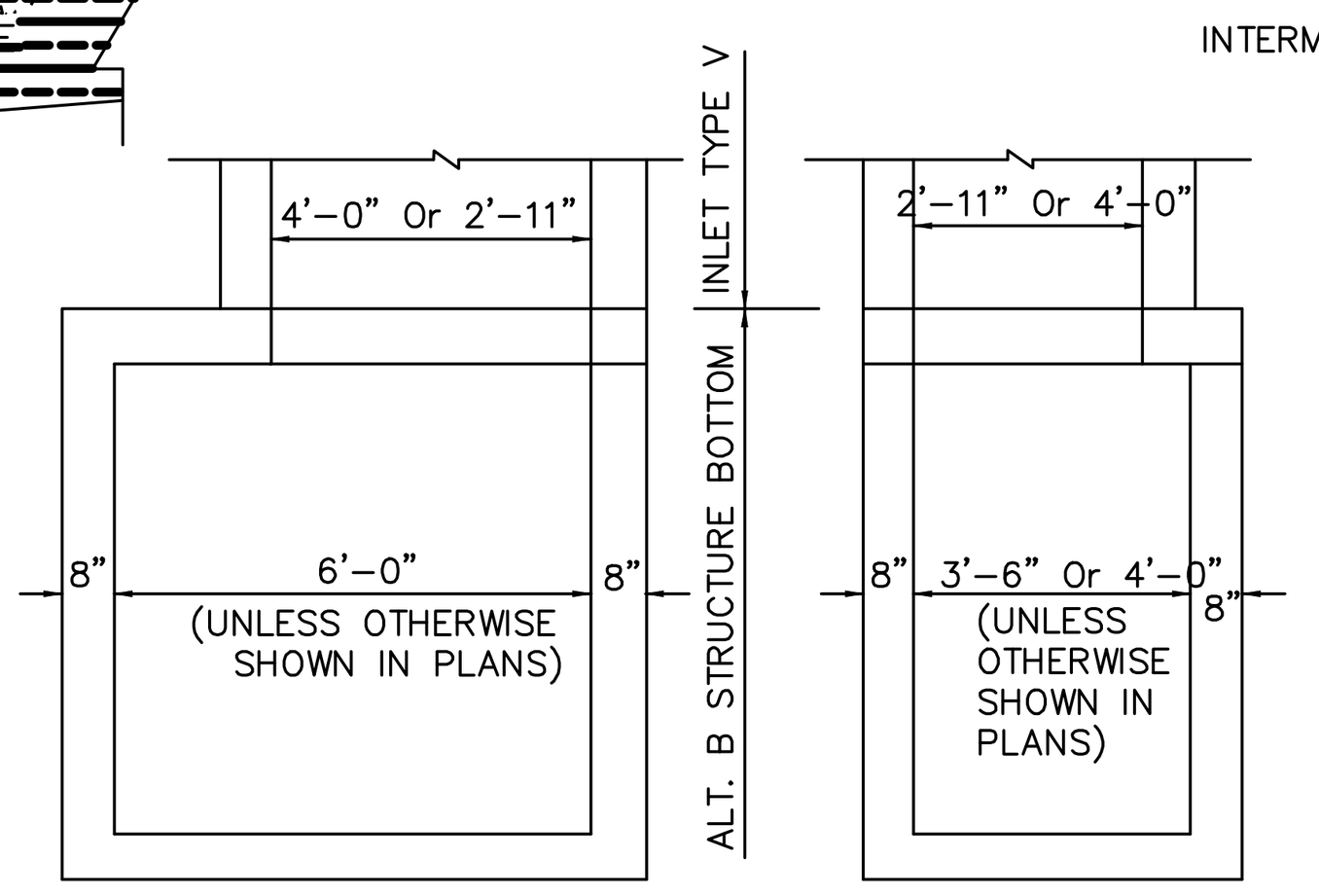


CURB INLET DETAIL
N.T.S.



NOTE: FTP-20-04 HANDICAPPED PARKING SIGN SHALL BE INSTALLED 84" ABOVE GRADE MEASURED TO THE BOTTOM OF THE SIGN.

FTP-20-04 HANDICAP PARKING SIGN DETAIL
N.T.S.

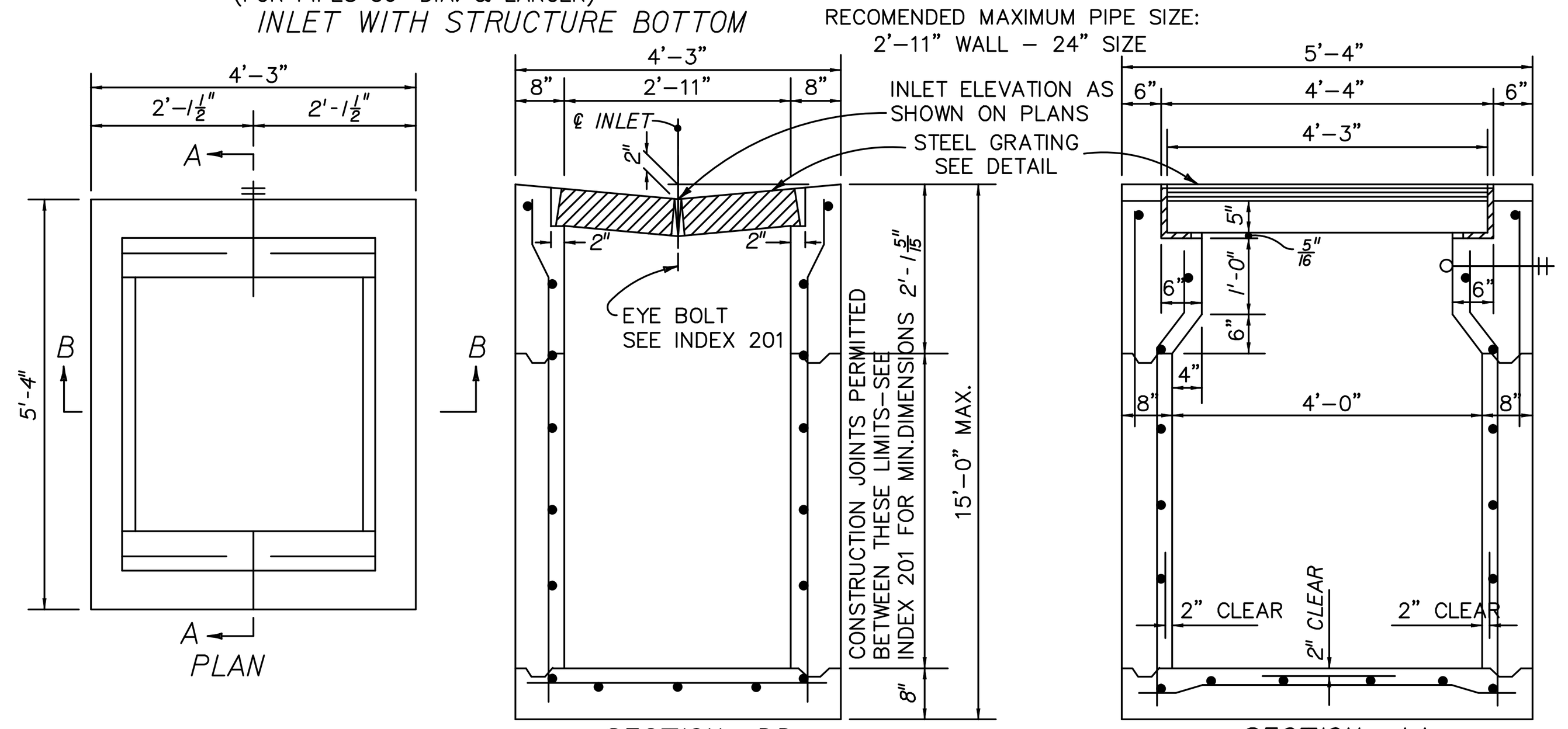


NOTE: ALT. B STRUCTURE BOTTOM ONLY. SEE INDEX NO. 200 (FOR PIPES 30" DIA. & LARGER)

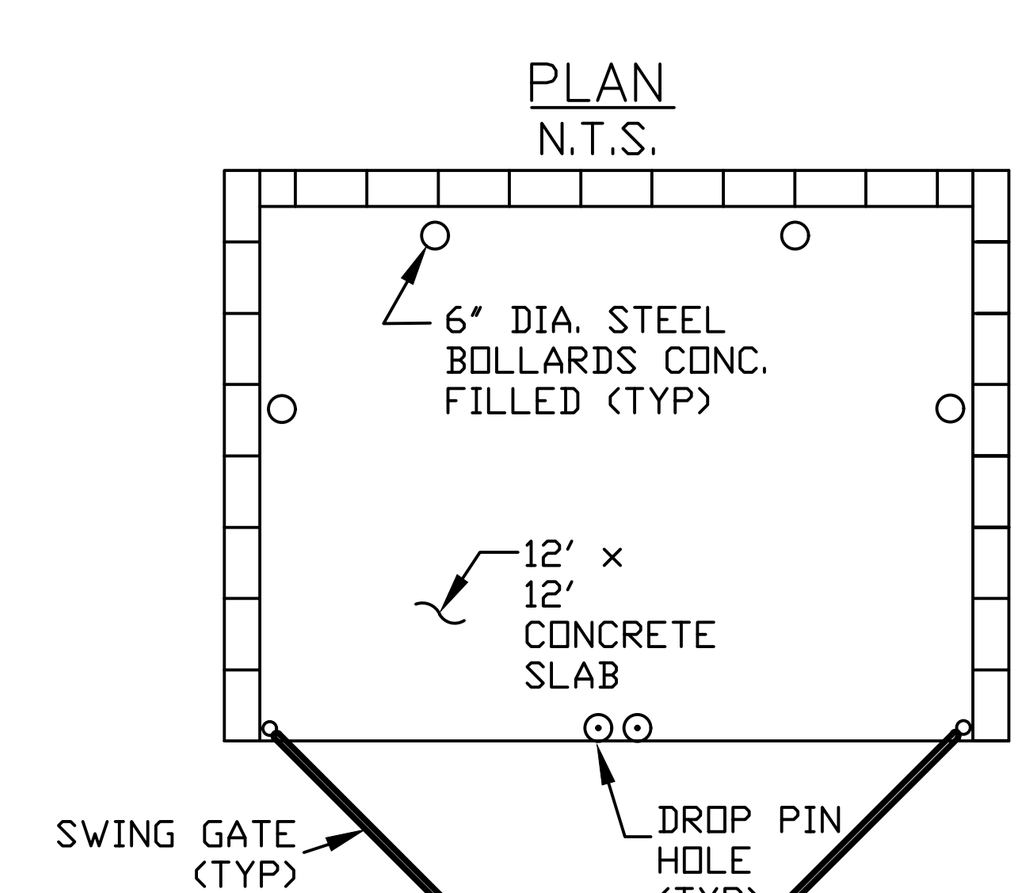
INLET WITH STRUCTURE BOTTOM

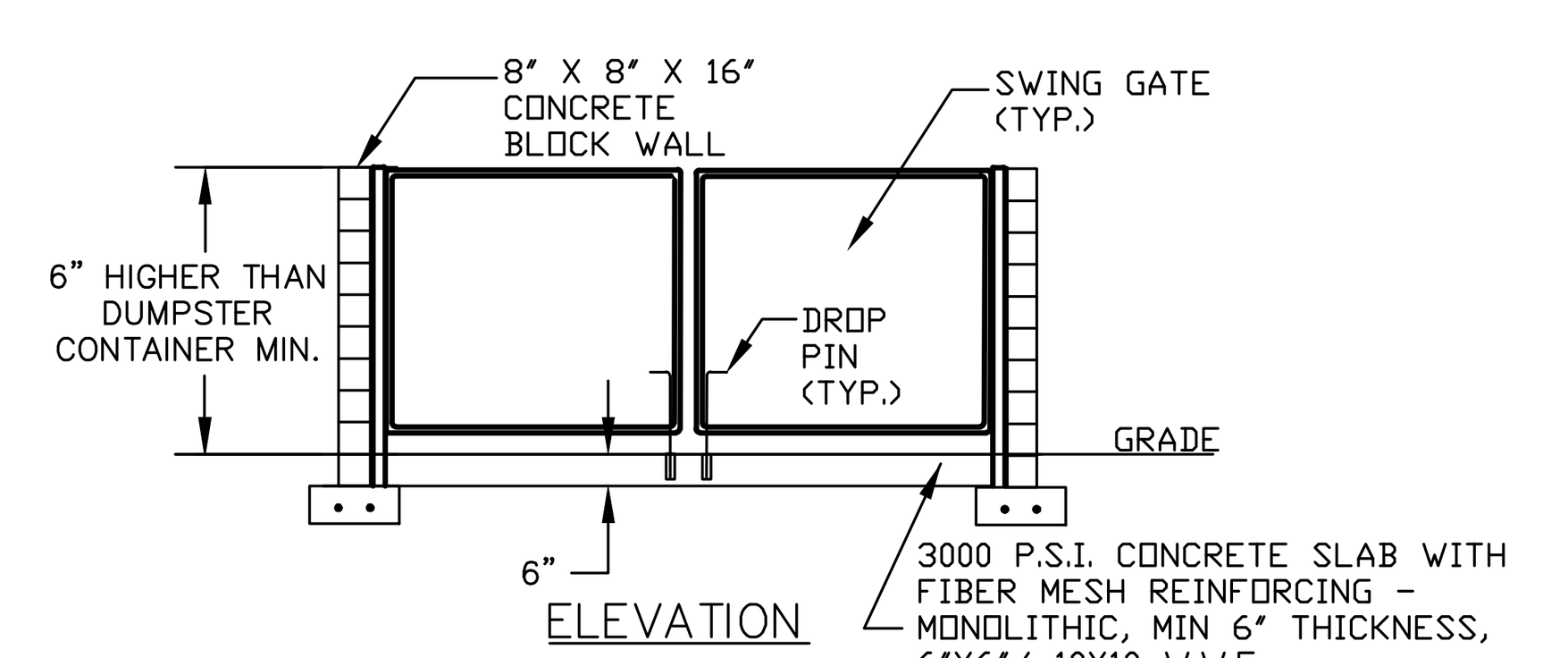
GENERAL NOTES
1. *THIS INLET IS DESIGNED FOR VILLAGE SWALES, DITCHES, OR OTHER AREAS SUBJECT TO HEAVY WHEEL LOADS, MINIMUM DEBRIS & SUBJECT TO PEDESTRIAN &/OR BICYCLE TRAFFIC.*
2. *WHEN ALTERNATE "G" GRATE IS SPECIFIED IN PLANS, THE GRATE IS TO BE HOT DIPPED GALVANIZED AFTER FABRICATION.*
3. *REINFORCING - NO. 4 BARS AT 12" CTRS. BOTH WAYS. CUT OR BEND BARS OUT OF WAY OF PIPE TO CLEAR PIPE 1-1/2".*
4. *ALL EXPOSED EDGES AND CORNERS SHALL BE TOOLED TO 1/4" RADIUS.*
5. *RECOMENDED MAXIMUM PIPE SIZES SHOWN ARE FOR CONCRETE PIPE.*
6. *FOR SUPPLEMENTARY DETAILS SEE INDEX NO. 201.*

RECOMENDED MAXIMUM PIPE SIZE:
2'-11" WALL – 24" SIZE



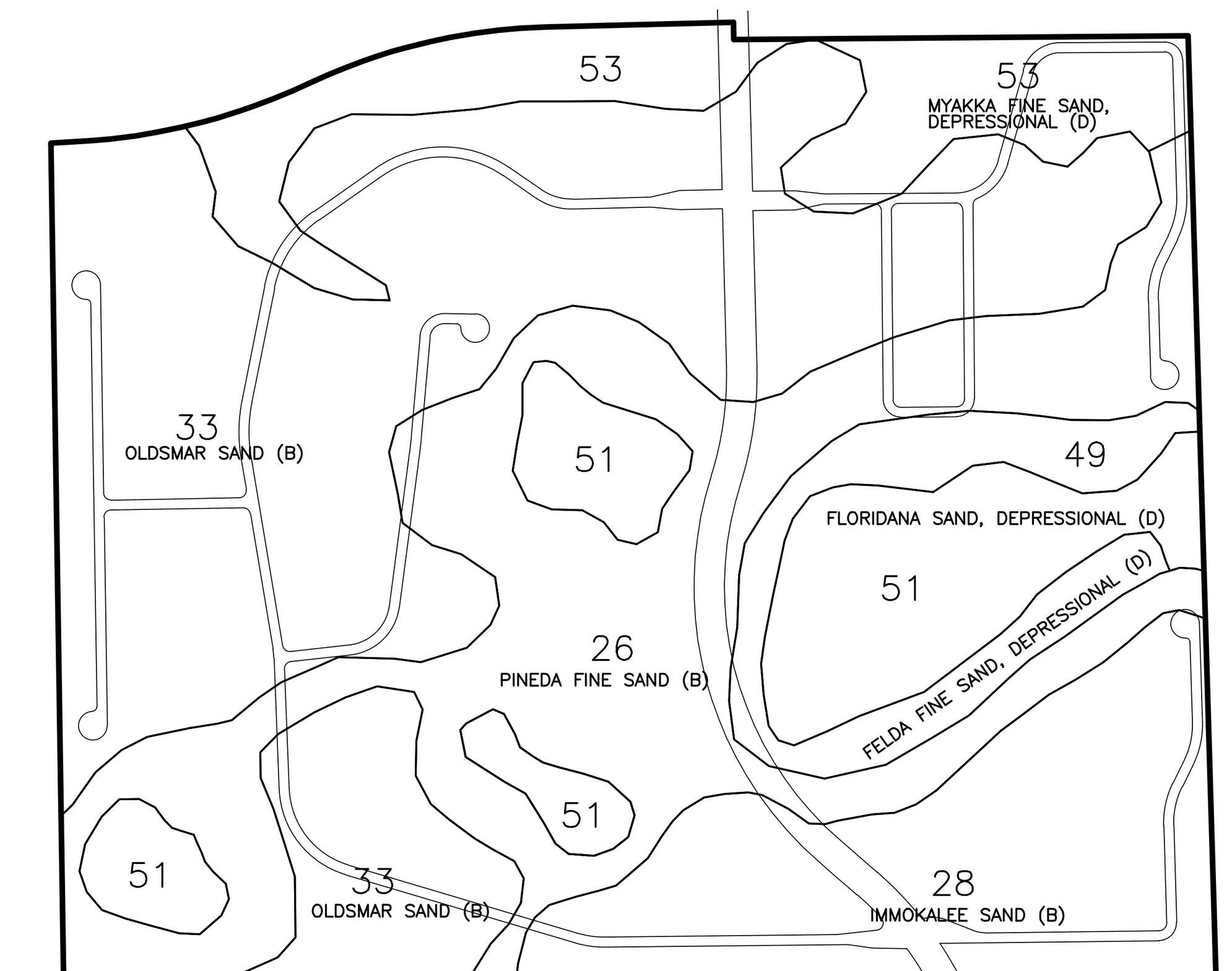
GUTTER INLET TYPE "V"
N.T.S.



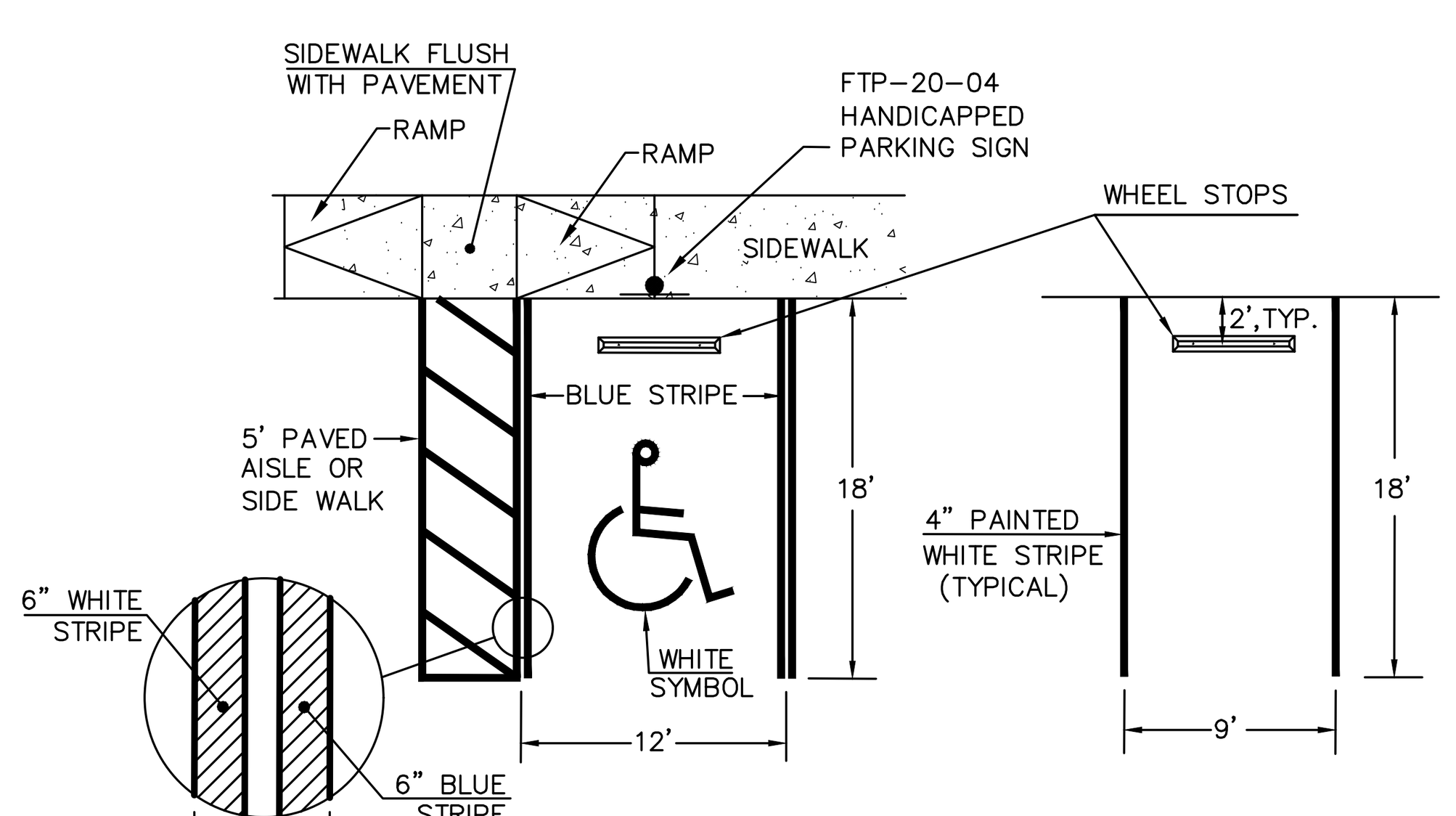


NOTES: 1.) WALLS: NEW CONSTRUCTION: MAXIMUM 6'-0' HIGH & CONSTRUCTED OF CONCRETE BLOCK (8"X8"X16"). MATERIALS USED SHOULD BE CONSISTENT WITH THOSE USED IN THE CONSTRUCTION OF & ARCHITECTURAL STYLE OF THE PRINCIPAL BUILDING.
2.) GATES: SHALL BE CONSTRUCTED OF WOOD OR OTHER COUNTY APPROVED OPAQUE MATERIAL.
3.) DROP PIN: 1/2" DIA. 18" LONG METAL ROD GATE LATCH. PROVIDE 3/4" DIA. HOLE 6" INTO CONCRETE FOR RODS. IN ASPHALT PROVIDE A 3/4" P.V.C. SLEEVE, 6" LONG, OR EQUAL.
4.) GRADING: SEE PLANS FOR GRADING OF CONCRETE SLAB.

DUMPSTER DETAIL
N.T.S.



SCS SOIL MAP DETAIL
N.T.S.



HANDICAP SPACE
N.T.S.

TYPICAL SPACE
N.T.S.

NOTE: ALL STRIPING TO BE HEAVY DUTY ACRYLIC BASED TRAFFIC PAINT.

PARKING SPACE DETAIL
N.T.S.

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE DETAILS 1

IBIS LANDING

LEE COUNTY, FLORIDA

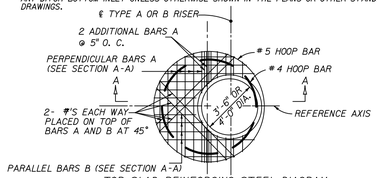
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-DT-1	JLW	JLW	DRU	NTS	12

IBIS LANDING

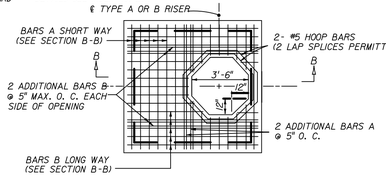
S:\JOBS\31XX\3142L\ENGINEERING\DEVPLANS\SFWMD PLANS\MODIFICATION\3142-SFWMD_MOD_12_DT-PGD-1.DWG 2/3/2026 2:04 PM JIMMY WHEWAY

STRUCTURE BOTTOM NOTES:

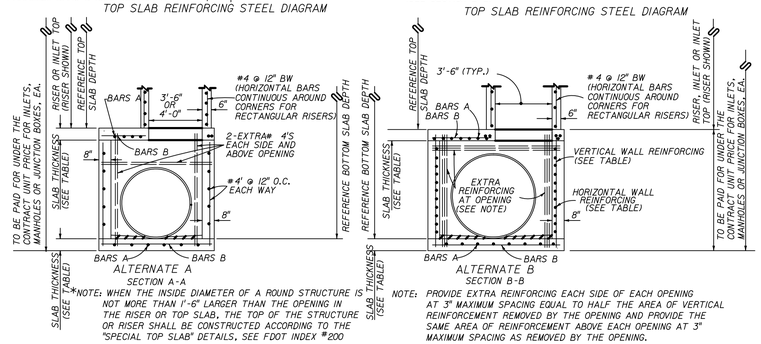
1. STANDARD STRUCTURE BOTTOMS 4'-0" DIAMETER & SMALLER (ALT. A) & 3'-6" SQUARE (ALT. B) ARE DESIGNATED TYPE P. LARGER STANDARD STRUCTURE BOTTOMS ARE DESIGNATED TYPE J. RISERS ARE PERMITTED FOR ALL STRUCTURES.
2. WALLS OF CIRCULAR STRUCTURES (ALTERNATE A) CONSTRUCTED IN PLACE MAY BE OF NON-REINFORCED CONCRETE OR BRICK OR REINFORCED CONCRETE. PRECAST AND RECTANGULAR STRUCTURES (ALTERNATE B) SHALL BE CONSTRUCTED OF REINFORCED CONCRETE ONLY.
3. WALL THICKNESS & REINFORCEMENT ARE FOR EITHER REINFORCED CAST-IN-PLACE OR PRECAST CONCRETE UNITS EXCEPT THAT PRECAST CIRCULAR UNITS MAY BE FURNISHED WITH WALLS IN ACCORDANCE WITH EITHER A.S.T.M. C-478 (UP TO 96" DIAMETER) OR A.S.T.M. C-76, CLASS III, B WALL, MODIFIED WHERE THE ELLIPTICAL STEEL CAGE AREA IS PLACED IN THE CENTER ONE-THIRD OF THE WALL.
4. TOP AND FLOOR SLAB THICKNESS AND REINFORCEMENT ARE FOR PRECAST & CAST IN PLACE CONSTRUCTION. TOP & FLOOR SLABS SHALL BE OF CLASS II CONCRETE. CONCRETE AS SPECIFIED IN A.S.T.M. C-478 (4000 PSI) MAY BE USED IN LIEU OF CLASS I AND CLASS II CONCRETE IN PRECAST ITEMS MANUFACTURED IN PLANTS WHICH ARE UNDER THE 'STANDARD OPERATING PROCEDURES FOR THE INSPECTION OF PRECAST DRAINAGE PRODUCTS.
5. ALL REINFORCEMENT SHALL BE A.S.T.M. A615, GRADE 60 OR 65 KSI WELDED WIRE FABRIC, EITHER SMOOTH OR DEFORMED.
6. STRUCTURE BOTTOMS MAY BE USED IN CONJUNCTION WITH CURB INLET TOPS TYPES I, 2, 3, 4, 5, 6, 9, AND 10, & ANY MANHOLE OR JUNCTION BOX UNLESS OTHERWISE SHOWN IN THE PLANS OR OTHER STANDARD DRAWINGS. ALT. B STRUCTURE BOTTOMS MAY BE USED IN CONJUNCTION WITH CURB INLET TYPES 7 & 8, OR ANY DITCH BOTTOM INLET UNLESS OTHERWISE SHOWN IN THE PLANS OR OTHER STANDARD DRAWINGS.
7. RECTANGULAR STRUCTURES MAY BE ROTATED AS DIRECTED BY THE ENGINEER IN ORDER TO FACILITATE CONNECTIONS BETWEEN THE STRUCTURE WALLS & STORM SEWER PIPES.
8. EXCEPT WHEN ACI HOOKS ARE SPECIFICALLY REQUIRED, EMBEDMENT HOOKS IN THE TOP AND BOTTOM SLABS MAY BE REPLACED WITH STRAIGHT EMBEDMENTS OR PERIPHERAL REINFORCEMENT IN ACCORDANCE WITH THE REINFORCEMENT DETAIL SHOWN UNDER REBAR STRAIGHT END EMBEDMENT OR PERIPHERAL REINFORCEMENT IN LIEU OF ACI STANDARD HOOKS FOR TOP AND BOTTOM SLABS, INDEX NO. 201, SHEET 3 OF 6.
9. ALL STEEL BARS SHALL HAVE 1½" MINIMUM COVER UNLESS OTHERWISE SHOWN. HORIZONTAL STEEL IN RECTANGULAR STRUCTURES SHALL BE LAPPED A MINIMUM OF 24 BAR DIAMETERS AT CORNERS.
10. THE CORNER FILLETS SHOWN ARE NECESSARY FOR RECTANGULAR STRUCTURES USED WITH CIRCULAR RISERS & INLET THROATS & USED ON SKEW WITH RECTANGULAR RISERS, INLET & INLET THROATS. FILLETS WILL BE REQUIRED IN LIEU OF THE BOTTOM SLAB OF THE ALT. B RISER WHEN USED WITH THE ALT. A BOX. EACH FILLET SHALL BE REINFORCED WITH 2- #5 BARS.
11. INLET THROATS, RISER OR MANHOLE TOPS SHALL BE SECURED TO STRUCTURES AS SHOWN ON INDEX NO. 201.
12. STRUCTURES WITH DEPTHS OVER 14' ARE TO BE CHECKED FOR FLOATATION BY DESIGNER OF PROJECT DRAINAGE.
13. UNITS LARGER THAN SPECIFIED STANDARD MAY BE SUBSTITUTED AT THE CONTRACTOR'S OPTION WHEN THESE UNITS WILL NOT CAUSE OR INCREASE THE SEVERITY OF UTILITY CONFLICTS. SUCH LARGER UNITS SHALL BE FURNISHED AT NO ADDITIONAL COST TO THE DEPARTMENT. LARGER ALTERNATE A UNITS CANNOT REPLACE ALTERNATE B UNITS WITHOUT APPROVAL OF THE ENGINEER. THIS NOTE APPLIES TO THIS INDEX ONLY.
14. FOR MANHOLE AND JUNCTION BOX TOPS, FOR FRAMES AND COVERS, &, FOR SUPPLEMENTARY DETAILS SEE INDEX NO. 201.



TOP SLAB REINFORCING STEEL DIAGRAM



TOP SLAB REINFORCING STEEL DIAGRAM

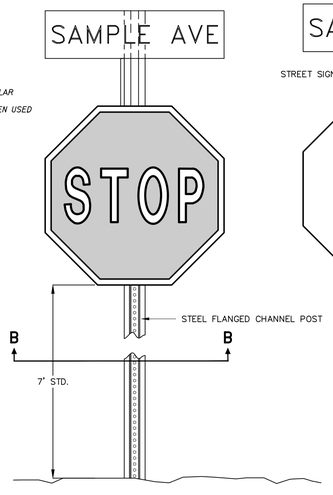


*NOTE: WHEN THE INSIDE DIAMETER OF A ROUND STRUCTURE IS NOT MORE THAN 1'-6" LARGER THAN THE OPENING IN THE RISER OR TOP SLAB, THE TOP OF THE STRUCTURE OR RISER SHALL BE CONSTRUCTED ACCORDING TO THE "SPECIAL TOP SLAB" DETAILS, SEE FDOT INDEX #200

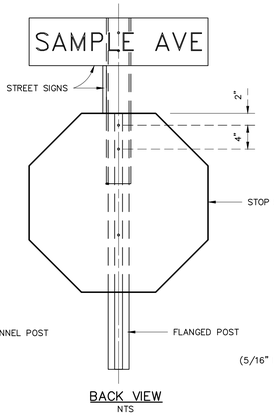
NOTE: PROVIDE EXTRA REINFORCING EACH SIDE OF EACH OPENING AT 3" MAXIMUM SPACING EQUAL TO HALF THE AREA OF VERTICAL REINFORCEMENT REMOVED BY THE OPENING AND PROVIDE THE SAME AREA OF REINFORCEMENT ABOVE EACH OPENING AT 3" MAXIMUM SPACING AS REMOVED BY THE OPENING.

TYPE "P" & "J" STRUCTURE BOTTOMS DETAIL

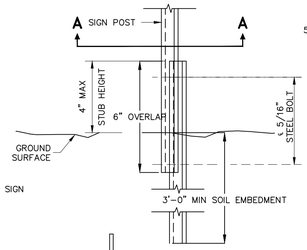
SEE FDOT INDEX #200 FOR ADDITIONAL DETAILS
N.T.S.

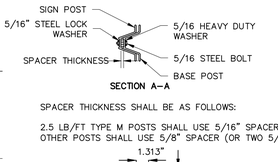


FRONT VIEW
NTS

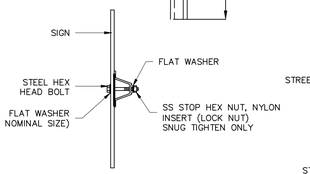


BACK VIEW
NTS

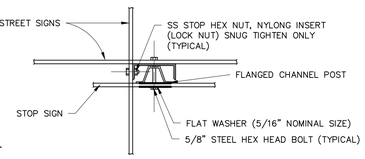




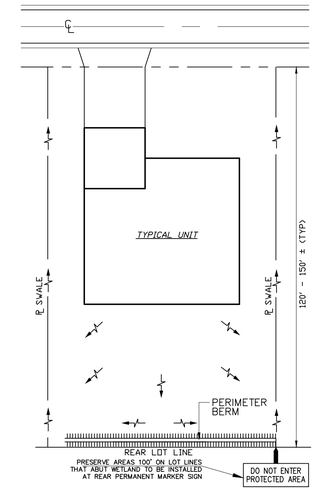
BACK VIEW
NTS



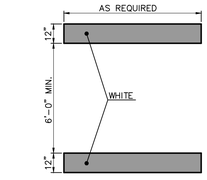
STREET SIGN ATTACHMENT DETAIL



TOP VIEW
NTS

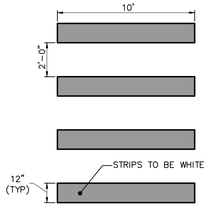


TYPICAL TYPE A LOT GRADING PLAN
LOT BACKING TO WETLANDS OR PROPERTY LINE



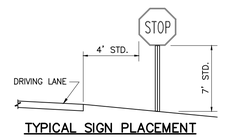
CROSSWALK STRIPING DETAIL
N.T.S.

1. TRAFFIC STRIPE LOCATION TO BE IN ACCORDANCE WITH F.D.O.T. DETAIL 17346
2. TRAFFIC STRIPE MATERIAL TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 710.

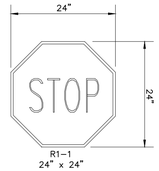


SPECIAL EMPHASIS CROSSWALK STRIPING DETAIL
N.T.S.

1. TRAFFIC STRIPE LOCATION TO BE IN ACCORDANCE WITH F.D.O.T. DETAIL 17346
2. TRAFFIC STRIPE MATERIAL TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 710.

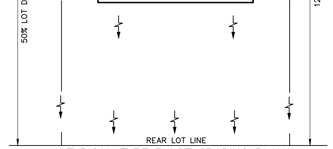


TYPICAL SIGN PLACEMENT
N.T.S.

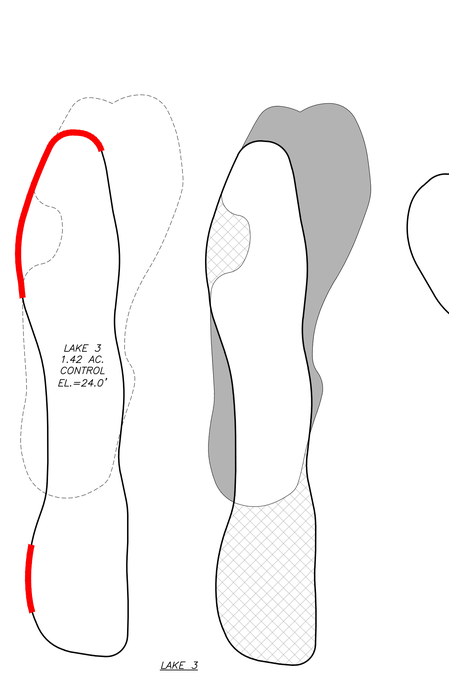


SIGN DETAIL
N.T.S.

24" R1-1 STOP SIGN DETAIL
N.T.S.



TYPICAL TYPE B LOT GRADING PLAN
SPLIT DRAINAGE

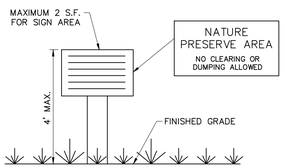


LAKE 3

LAKE FILL = 0.54 AC — TOTAL LENGTH OF LAKE SHORE LINE = 1,382 LF
LAKE EXCAVATE = 0.57 AC — 25% REQUIRED LITTORAL SHELF 1,382 LF x 25% = 346 LF
LITTORAL SHELF = 346 LF — 346 LF LITTORAL SHELF PROVIDED

LAKE 4

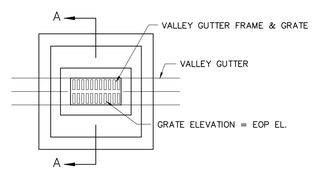
LAKE FILL = 0.04 AC — LENGTH OF MODIFIED SHORE LINE = 439 LF
LAKE EXCAVATE = 0.10 AC — 25% REQUIRED LITTORAL SHELF 439 LF x 25% = 110 LF
LITTORAL SHELF = 110 LF — 110 LF LITTORAL SHELF PROVIDED



NOTES:

1. THE SIGN SHALL BE INSTALLED AT THE EDGE OF THE ONSITE PRESERVE BOUNDARY
2. SIGNS SHALL BE NO CLOSER THAN TEN FEET FROM THE DEVELOPMENT LIMITS AND BE LIMITED TO A MAXIMUM HEIGHT OF FOUR FEET AND A MAXIMUM SIZE OF TWO SQUARE FEET.
3. MAXIMUM SIGN SPACING SHALL BE 300 FEET.

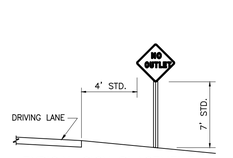
PRESERVE AREA DETAIL
NTS



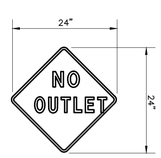
SECTION A-A

NOTE: A 2' SUMP IS REQUIRED ON ALL STRUCTURES SO THAT THE BOTTOM ELEVATION OF THE STRUCTURE IS A MINIMUM OF 2' BELOW THE LOWEST PIPE INVERT.

VALLEY GUTTER INLET DETAIL
N.T.S.

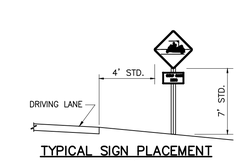


TYPICAL SIGN PLACEMENT
N.T.S.



SIGN DETAIL
N.T.S.

24" 14-2 NO OUTLET SIGN DETAIL
N.T.S.

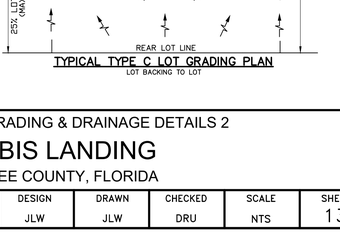


TYPICAL SIGN PLACEMENT
N.T.S.



SIGN DETAIL
N.T.S.

24" FTP-39 GOLF CART XING SIGN DETAIL
N.T.S.



TYPICAL TYPE C LOT GRADING PLAN
LOT BACKING TO LOT

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
1	12-11-25	REVISED PER SFWMD COMMENTS	JLW

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

PAVING, GRADING & DRAINAGE DETAILS 2

IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	PGD-DT-2	JLW	JLW	DRU	NTS	13

S:\JOBS\3XXX\3142L\ENGINEERING\DEVPLANS\SFWMD PLANS\WORKFILES\3142L-SFWMD_MOD_13.DT-PGD-2.DWG 2/3/2026 2:04 PM JIMMY MCKINLY

STORM STRUCTURES:

S-300
YARD DRAIN
GRATE=28.70'
N. INV.=23.20'
E. INV.=25.20'

S-301
12" FLARED END
INV.=19.20'

S-302
V.G. INLET
GRATE=27.96'
NW. INV.=20.40'

S-303
V.G. INLET
GRATE=27.96'
NE. INV.=20.40'
SW. INV.=20.40'

S-304
JUNCTION BOX
GRATE=28.60'
N. INV.=19.20'
SE. INV.=19.20'
SW. INV.=20.00'

S-305
V.G. INLET
GRATE=27.90'
NW. INV.=19.20'
SE. INV.=21.70'

S-306
V.G. INLET
GRATE=27.90'
NW. INV.=21.70'
SE. INV.=20.20'

S-307
V.G. INLET
GRATE=28.60'
NW. INV.=20.20'
SW. INV.=20.20'
E. INV.=20.20'

S-308
V.G. INLET
GRATE=28.96'
NE. INV.=21.40'
SW. INV.=21.40'

S-309
V.G. INLET
GRATE=28.96'
NE. INV.=21.70'

S-310
TYPE 'D' INLET
GRATE=28.70'
W. INV.=20.20'
E. INV.=20.20'
N. INV.=25.70'

S-311
TYPE 'D' INLET
GRATE=28.70'
W. INV.=20.20'
S. INV.=20.20'

S-312
V.G. INLET
GRATE=27.96'
N. INV.=20.20'
S. INV.=21.70'

S-313
V.G. INLET
GRATE=27.96'
N. INV.=22.20'
S. INV.=22.20'

S-314
TYPE 'C' INLET
GRATE=28.40'
N. INV.=23.70'
E. INV.=24.20'
W. INV.=24.20'

S-315
YARD DRAIN
GRATE=28.80'
E. INV.=25.20'
W. INV.=25.20'

S-316
YARD DRAIN
GRATE=29.20'
E. INV.=25.20'
W. INV.=25.20'

S-317
YARD DRAIN
GRATE=29.29'
E. INV.=25.20'

S-318
YARD DRAIN
GRATE=28.50'
E. INV.=25.20'
W. INV.=25.20'

S-319
YARD DRAIN
GRATE=29.00'
E. INV.=25.70'
W. INV.=25.70'

S-320
YARD DRAIN
GRATE=29.40'
W. INV.=26.20'

S-321
YARD DRAIN
GRATE=28.20'
N. INV.=24.20'

S-322
YARD DRAIN
GRATE=28.20'
N. INV.=24.20'
S. INV.=24.20'

S-323
TYPE 'D' INLET
GRATE=28.20'
NW. INV.=19.70'
E. INV.=19.70'
S. INV.=24.20'

S-324
V.G. INLET
GRATE=27.46'
SE. INV.=22.20'
W. INV.=19.70'

S-325
V.G. INLET
GRATE=27.46'
NW. INV.=22.20'

S-326
24" FLARED END
INV.=19.20'

S-327
36" FLARED END
INV.=19.20'

S-328
NOT IN USE

S-329
TYPE 'D' INLET
GRATE=28.80'
W. INV.=22.20'
E. INV.=21.20'
N.&S. INV.=25.80'

S-330
V.G. INLET
GRATE=28.20'
W. INV.=19.20'
E. INV.=22.20'
N. INV.=16.20'
S. INV.=19.20'

S-331
TYPE 'D' INLET
GRATE=28.70'
W. INV.=19.20'
E. INV.=19.20'
N.&S. INV.=25.70'

S-332
36" FLARED END
INV.=18.20'

S-333
18" FLARED END
INV.=19.20'

S-334
TYPE 'C' INLET
GRATE=28.20'
N. INV.=24.20'
S. INV.=23.20'

S-335
YARD DRAIN
GRATE=28.20'
W. INV.=24.20'
S. INV.=25.20'

S-336
V.G. INLET
GRATE=27.70'
S. INV.=22.70'

S-337
V.G. INLET
GRATE=27.46'
N. INV.=22.20'
S. INV.=17.20'

S-338
V.G. INLET
GRATE=27.46'
S. INV.=16.20'
N. INV.=17.20'

S-339
TYPE 'D' INLET
GRATE=24.00'
E. INV.=18.20'

S-340
YARD DRAIN
GRATE=27.70'
E. INV.=22.20'
N. INV.=24.70'

S-341
JUNCTION BOX
RIM=27.70'
N. INV.=18.70'
E. INV.=18.70'
S. INV.=18.70'
W. INV.=22.20'

S-342
30" FLARED END
INV.=19.20'

S-343
TYPE 'C' INLET
GRATE=27.70'
N. INV.=19.20'
S. INV.=19.20'

S-344
V.G. INLET
GRATE=27.20'
S. INV.=19.70'

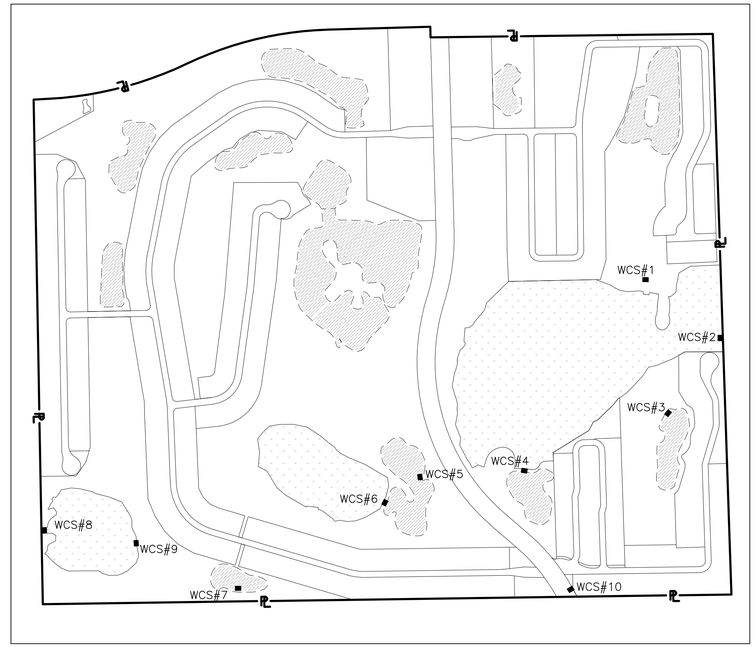
S-345
TYPE 'C' INLET
GRATE=27.70'
W. INV.=18.70'
E. INV.=18.70'
N. INV.=23.70'

S-346
YARD DRAIN
GRATE=27.70'
S. INV.=23.70'

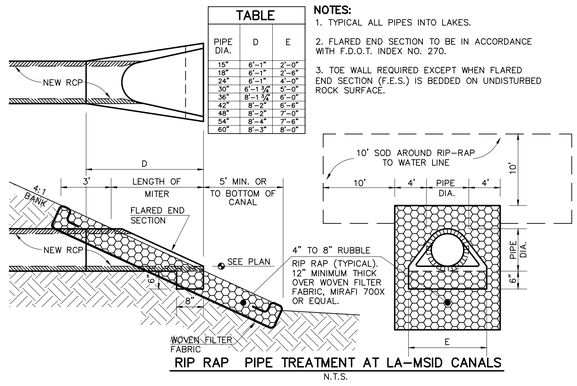
S-347
V.G. INLET
GRATE=26.96'
W. INV.=18.70'
E. INV.=18.70'

S-348
V.G. INLET
GRATE=26.96'
W. INV.=18.70'

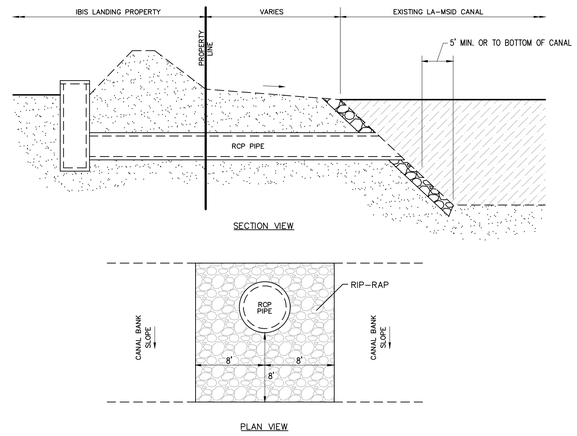
WCS-3
SEE DETAIL FOR INFO



CONTROL STRUCTURE LOCATION MAP



RIP RAP PIPE TREATMENT AT LA-MSID CANALS
N.T.S.



EXISTING OUTFALL & MITERED END
NTS

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

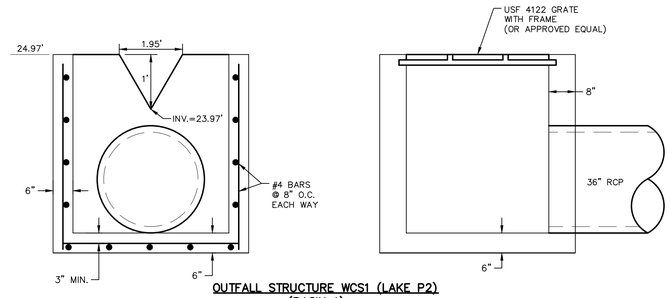
NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida
FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490
PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165
WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 8469 \ SURVEY LICENSE # LB 6690
DAVID R. UNDERHILL JR. P.E. #47029

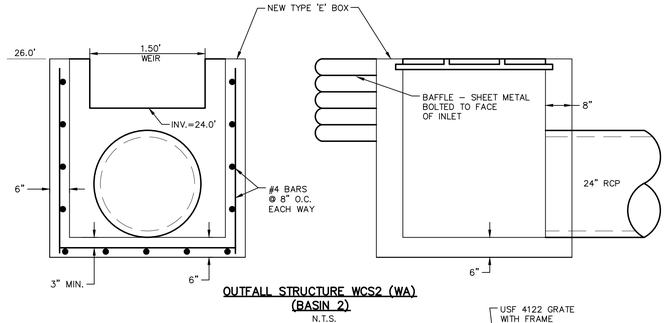
DRAINAGE STRUCTURE SCHEDULE
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	DSS	JLW	JLW	DRU	NTS	14

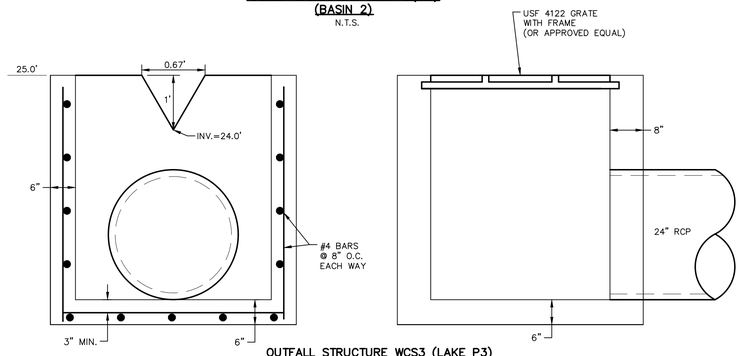
S:\JOBS\3142L\3142L\ENGINEERING\DRAINAGE\PLANS\MASTER\3142L-STRMD_MSTR_PLSHEETS.DWG 2/3/2026 2:04 PM JIMMY MCKAY



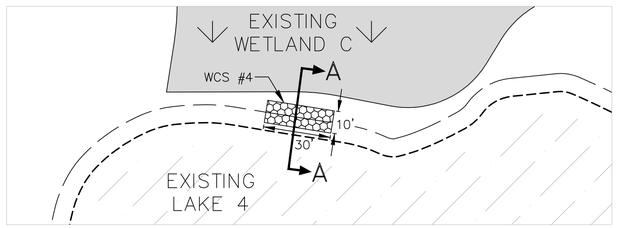
OUTFALL STRUCTURE WCS1 (LAKE P2)
(BASIN 1)
N.T.S.

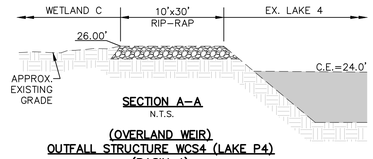


OUTFALL STRUCTURE WCS2 (WA)
(BASIN 2)
N.T.S.



OUTFALL STRUCTURE WCS3 (LAKE P3)
(BASIN 3)
N.T.S.

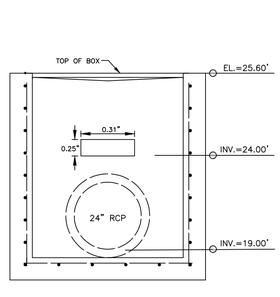


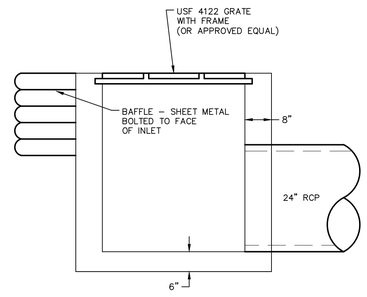


SECTION A-A
N.T.S.

(OVERLAND WEIR)
OUTFALL STRUCTURE WCS4 (LAKE P4)
(BASIN 4)
N.T.S.

PERMITTED (36-108291-P)





OUTFALL STRUCTURE WCS3 (LAKE 3)
(BASIN 3)
N.T.S.

PROPOSED

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
1	12-11-25	REVISED PER SFWMD COMMENTS	JLW

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

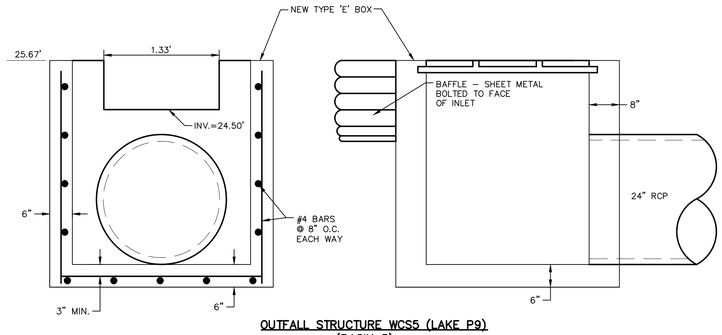
WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

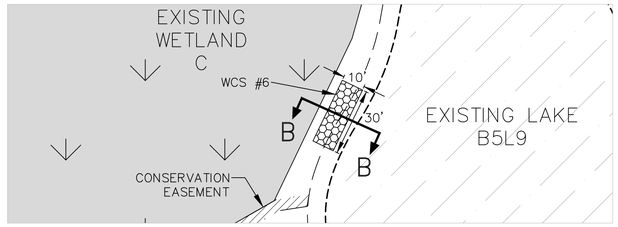
OUTFALL STRUCTURE DETAILS 1
IBIS LANDING
LEE COUNTY, FLORIDA

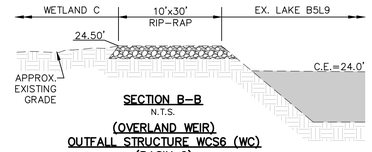
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	CS-DT-1	JLW	JLW	DRU	NTS	14A

S:\JOBS\3142L\CIVIL\ENGINEERING\DEVPLANS\SFWMD PLANS\MODIFICATION\3142L-SFWMD_WCS_1A-1AC_OUTFALL STRUCTURES EXHIBIT.DWG 2/3/2026 2:04 PM JIMMY WEWAY



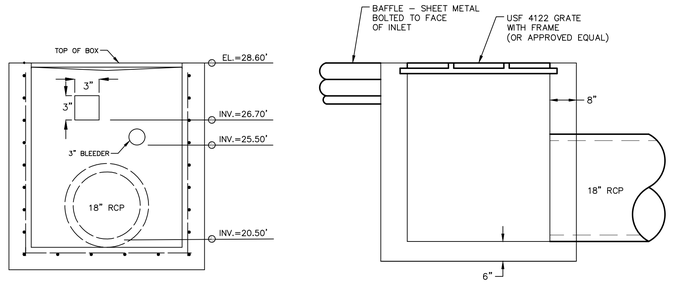
OUTFALL STRUCTURE WCS5 (LAKE P9)
(BASIN 5)
N.T.S.



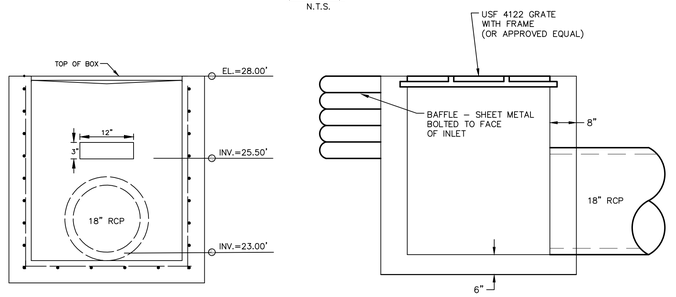


SECTION B–B
N.T.S.

(OVERLAND WEIR)
OUTFALL STRUCTURE WCS6 (WC)
(BASIN 6)
N.T.S.



OUTFALL STRUCTURE WCS7 (LAKE P13)
(BASIN 7)
N.T.S.



OUTFALL STRUCTURE WCS8 (WD)
(BASIN 8)
N.T.S.

PERMITTED (36-108291-P)

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

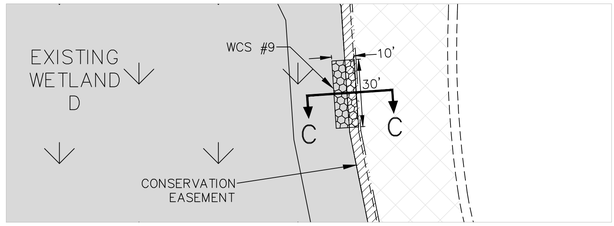
OUTFALL STRUCTURE DETAILS 2

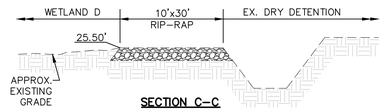
IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	CS-DT-2	JLW	JLW	DRU	NTS	14B

S:\JOBS\3142L\CIVIL\ENGINEERING\DEVPLANS\SFWMD PLANS\MODIFICATION\3142-SFWMD_MOD_14-14C_OUTFALL STRUCTURES EXHIBITS.dwg 2/3/2026 2:05 PM JIMMY MCKAY





SECTION C-C
N.T.S.
(OVERLAND WEIR)
OUTFALL STRUCTURE WCS9 (LAKE P12)
(BASIN 9)
N.T.S.

PERMITTED (36-108291-P)

IBIS LANDING

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

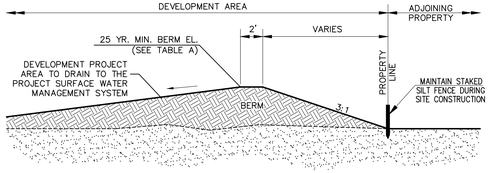
PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

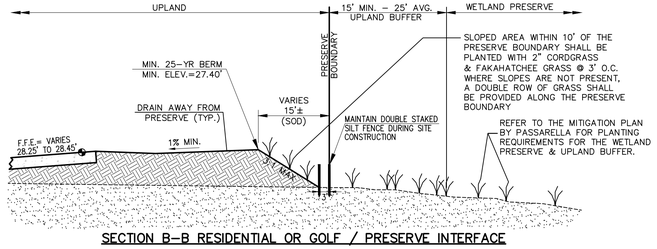
DAVID R. UNDERHILL JR.
P.E. #47029

OUTFALL STRUCTURE DETAILS 3
IBIS LANDING
LEE COUNTY, FLORIDA

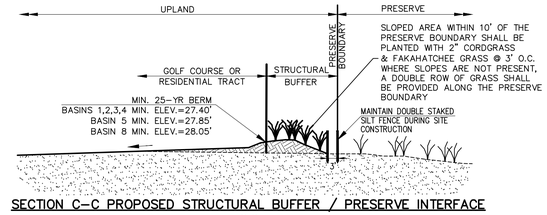
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	CS-DT-3	JLW	JLW	DRU	NTS	14C



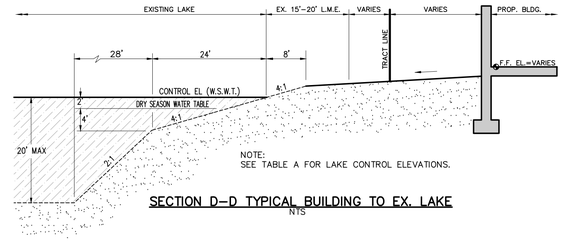
SECTION A–A TYPICAL BERM
NTS



SECTION B–B RESIDENTIAL OR GOLF / PRESERVE INTERFACE
NTS

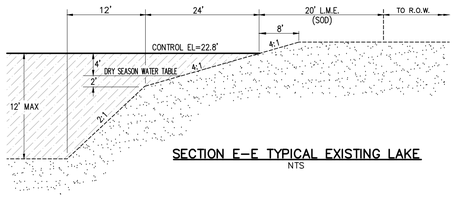


SECTION C–C PROPOSED STRUCTURAL BUFFER / PRESERVE INTERFACE
NTS

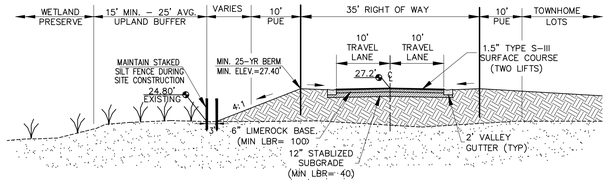


NOTE:
SEE TABLE A FOR LAKE CONTROL ELEVATIONS.

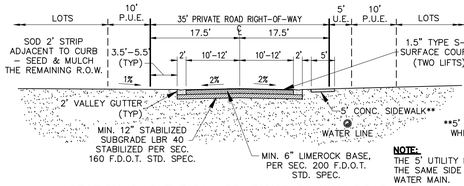
SECTION D–D TYPICAL BUILDING TO EX. LAKE
NTS



SECTION E–E TYPICAL EXISTING LAKE
NTS

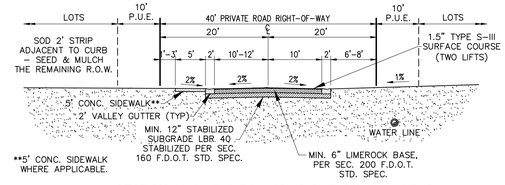


SECTION F–F TYPICAL ROAD TO WETLAND
NTS

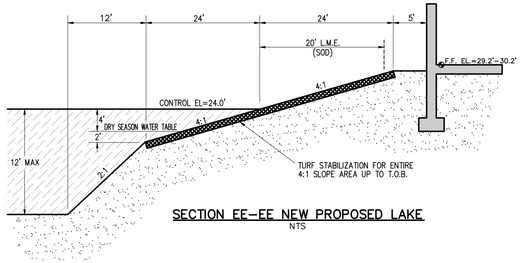


NOTE:
THE 5' UTILITY EASEMENT RUNS ON THE SAME SIDE AS THE PROPOSED WATER MAIN.

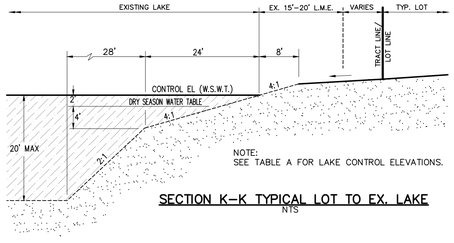
SECTION G–G TYPICAL 35' RIGHT OF WAY ROAD
NTS



SECTION H–H TYPICAL 40' RIGHT OF WAY ROAD
NTS

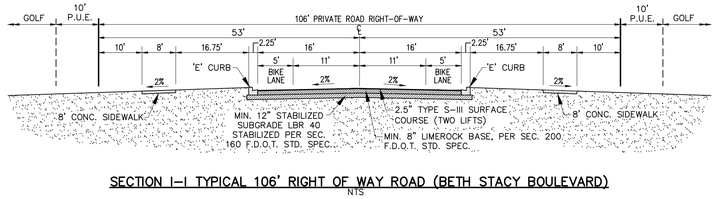


SECTION EE–EE NEW PROPOSED LAKE
NTS

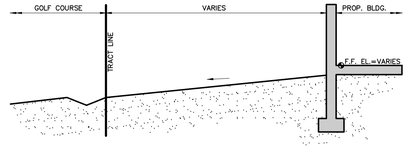


NOTE:
SEE TABLE A FOR LAKE CONTROL ELEVATIONS.

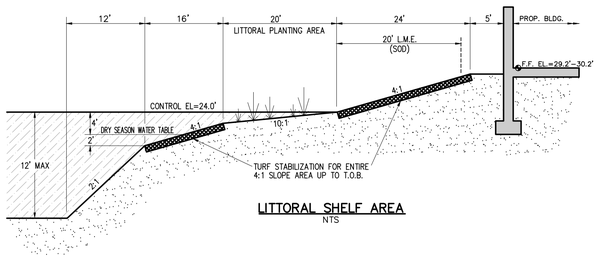
SECTION K–K TYPICAL LOT TO EX. LAKE
NTS



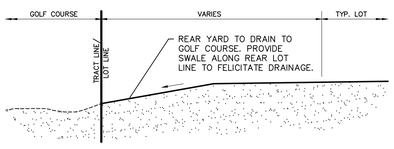
SECTION I–I TYPICAL 106' RIGHT OF WAY ROAD (BETH STACY BOULEVARD)
NTS



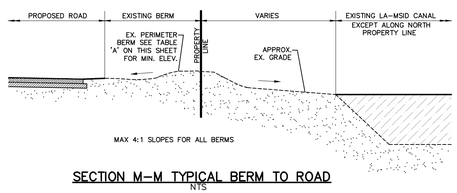
SECTION J–J TYPICAL BUILDING TO GOLF COURSE
NTS



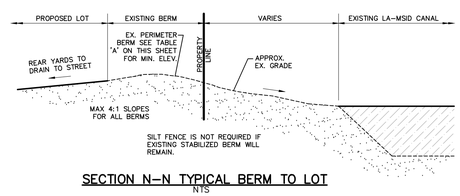
LITTORAL SHELF AREA
NTS



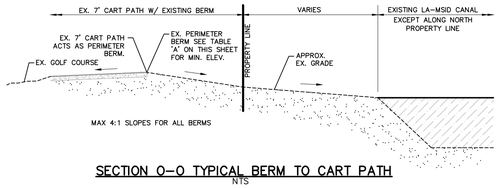
SECTION L–L TYPICAL LOT TO GOLF COURSE
NTS



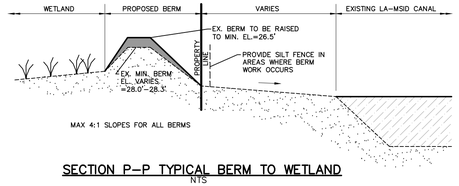
SECTION M–M TYPICAL BERM TO ROAD
NTS



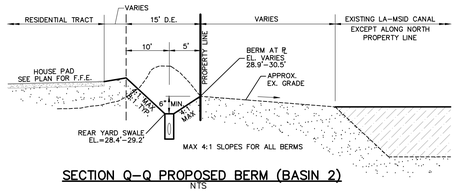
SECTION N–N TYPICAL BERM TO LOT
NTS



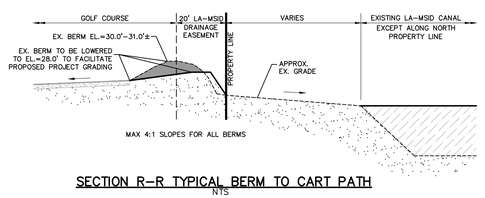
SECTION O–O TYPICAL BERM TO CART PATH
NTS



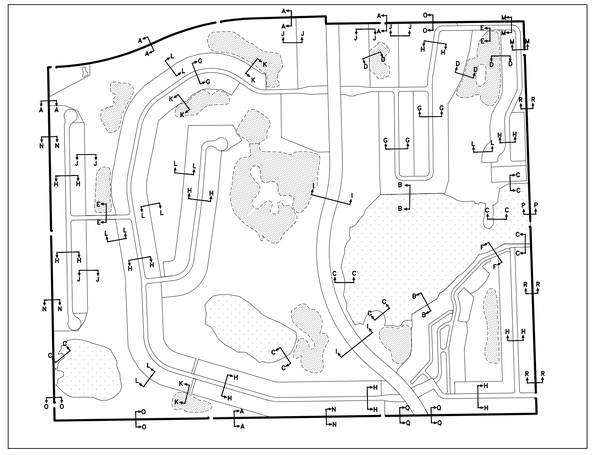
SECTION P–P TYPICAL BERM TO WETLAND
NTS



SECTION Q–Q PROPOSED BERM (BASIN 2)
NTS



SECTION R–R TYPICAL BERM TO CART PATH
NTS



CROSS SECTION KEY MAP

TABLE 'A'

STORMWATER MANAGEMENT SYSTEM MINIMUM ELEVATIONS (NGVD)

Basin	WCS#	Control Elev.	5 Yr. Flood Stage	10 Yr. Flood Stage	Min. Road	25 Yr. Flood Stage	Min. Berm	100 Yr. Flood Stage	Min. Floor Elev.
B1	WCS 1	24.00	24.54	25.80	26.50	27.04	27.40*	27.44	28.00
B2	WCS 2	24.00	24.66	24.84	26.50	25.97	26.45	27.43	28.00
B3	WCS 3	24.00	25.78	25.92	26.50	26.86	27.35	27.43	28.00
B4	WCS 4	24.00	25.79	25.97	26.50	26.73	27.20	27.43	28.00
B5	WCS 5	24.50	25.54	25.87	26.50	27.32	27.85	27.94	28.30
B6	WCS 6	24.00	25.39	25.63	26.50	26.75	N/A	27.57	28.00
B7	WCS 7	25.50	27.13	27.44	27.50	28.52	29.05	29.27	29.30
B8	WCS 8	25.50	26.17	26.36	27.50	27.52	28.05	28.39	28.40
B9	WCS 9	25.50	26.17	26.36	27.50	27.52	28.05	28.39	28.40

* EXISTING PERIMETER BERM WILL PROVIDE 4" OF FREEBOARD ALONG NORTH PROPERTY LINE. MINIMUM BERM HEIGHT IS 27.6' ALONG LA–MSID CANALS.

PLEASE NOTE: B2 & B7 WILL HAVE BERM IMPROVEMENTS TO REACH REQUIRED 6" FREEBOARD.

5 YEAR 24 HR RAINFALL IS 5.20"
10 YEAR 24 HR TOTAL RAINFALL IS 6.00"
25 YEAR 72 HR TOTAL RAINFALL IS 10.20"
100 YEAR 72 HR TOTAL RAINFALL IS 14.20"

ALL ELEVATIONS ARE BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278–1199
FAX: (239) 278–3174

NO.	DATE	REVISION DESCRIPTION	BY
1	12–11–25	REVISED PER SFWMD COMMENTS	JLW

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939–5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625–1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR. P.E. #47029

TYPICAL CROSS-SECTIONS
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	XSEC	JLW	JLW	DRU	NTS	15

S:\JOBS\3XXX\3142L\ENGINEERING\DEVELOPMENT\SFWMD PLANS\WORKFOLDER\3142L-SFWMD_MOD_15_07-16-1.DWG 2/9/2026 1:59 PM JIMMY MEKINY



LEGEND:

- EXISTING E.O.P.
- PROPOSED ASPHALT PAVEMENT
- PROPOSED TOP OF BANK
- EXISTING DETENTION-RETENTION AREA
- LANDSCAPE BUFFER
- RIGHT-OF-WAY LINE
- EXISTING STORM WATER MANAGEMEI
- PROPOSED CONCRETE
- WETLAND AREA & BUFFER
- PROPERTY LINE
- PROPOSED BERM
- DRAINAGE HAYBALES
- DEWATERING SUMP
- DRAINAGE RIP-RAP
- FLOATING TURBIDITY BARRIER

SCHEDULE OF IMPLEMENTATION:

1) INSTALL ALL TEMPORARY BMP MEASURES PRIOR TO START OF CONSTRUCTION.

2) INSTALL ALL PERMANENT BMP MEASURES IMMEDIATELY AFTER FINAL GRADING.

3) REMOVE TEMPORARY BMP FEATURES AS APPLICABLE.

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
1	12-11-25	REVISED PER SFWMD COMMENTS	JLW

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors
Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL - BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

BEST MANAGEMENT PRACTICES PLAN

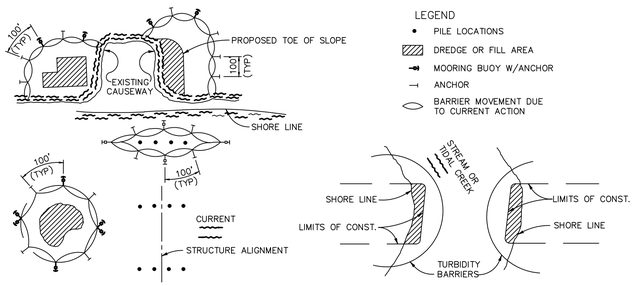
IBIS LANDING

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	BMP	JLW	JLW	DRU	1"=200'	16

IBIS LANDING

S:\JOBS\3XXX\3142L\ENGINEERING\DEVELOP\SFWMD PLANS\MODIFICATION\3142L_SFWMD_MOD_16_BMP.DWG 12/3/2025 2:06 PM JIMMY WEEKLY



NOTES:
1. TURBIDITY BARRIERS ARE TO BE USED IN ALL PERMANENT BODIES OF WATER REGARDLESS OF WATER DEPTH.
2. NUMBER AND SPACING OF ANCHORS DEPENDENT ON CURRENT VELOCITIES.
3. DEPLOYMENT OF BARRIER AROUND PILE LOCATIONS MAY VARY TO ACCOMMODATE CONSTRUCTION OPERATIONS.
4. NAVIGATION MAY REQUIRE SEGMENTING BARRIER DURING CONSTRUCTION OPERATIONS.
5. FOR ADDITIONAL INFORMATION SEE SECTION 104 OF THE STANDARD SPECIFICATIONS.

TURBIDITY BARRIER APPLICATION DETAIL
N.T.S.

NOTE:
TURBIDITY BARRIERS FOR FLOWING STREAMS AND TIDAL CREEKS MAY BE EITHER FLOATING, OR STAKED TYPES OR ANY COMBINATIONS OF TYPES THAT WILL SUIT SITE CONDITIONS AND MEET EROSION CONTROL AND WATER QUALITY REQUIREMENTS. THE BARRIER TYPE(S) WILL BE AT THE CONTRACTORS OPTION UNLESS OTHERWISE SPECIFIED IN THE PLANS, HOWEVER PAYMENT WILL BE UNDER THE PAY ITEM(S) ESTABLISHED IN THE STAKED TURBIDITY BARRIER. POSTS IN STAKED PLANS FOR FLOATING TURBIDITY BARRIER AND/OR TURBIDITY BARRIERS TO BE INSTALLED IN VERTICAL POSITION UNLESS OTHERWISE DIRECTED BY THE ENGINEER.

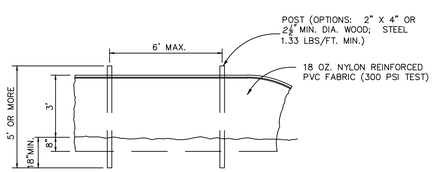
SEEDING RATES (LBS/AC) FOR NEW SHOULDERS AND SLOPES*								
TYPE OF SEED	ZONE 1				ZONE 2			
	COASTAL		INLAND		COASTAL		INLAND	
	MAR. TO NOV.	NOV. TO MAR.	MAR. TO OCT.	OCT. TO MAR.	FEB. TO DEC.	DEC. TO FEB.	FEB. TO DEC.	DEC. TO FEB.
PERMANENT GRASS								
UNHULLED BERMUDA	20	20	20	20	20	20	20	20
BAHIA ARGENTINA OR PENSACOLA			80	80			80	80
QUICK GROWING								
BROWN TOP MILLET	20		20		20		20	
ANNUAL RYE GRASS		20		20		20		20
TOTAL POUNDS PER ACRE	40	40	120	120	40	40	120	120

NOTE: THE SEEDING RATES SHOWN IN THIS TABLE APPLY ONLY WHEN SEED IS SPREAD BY AN APPROVED MECHANICAL SPREADER MEETING THE REQUIREMENTS OF SECTION 570 AND 577 OF THE STANDARD SPECIFICATIONS.
*SEE INDEX NO. 105 FOR ZONE BOUNDARIES AND SEEDING RATES FOR SHOULDER REWORKING.

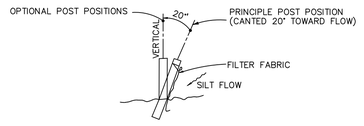
D_1 =5' STD. (SINGLE PANEL FOR DEPTHS 5' OR LESS).
D_2 =5' STD. (ADDITIONAL PANEL FOR DEPTHS > 5').
CURTAIN TO REACH BOTTOM UP TO DEPTHS OF 10 FEET. TWO(2) PANELS TO BE USED FOR DEPTHS GREATER THAN 10 FEET UNLESS SPECIAL DEPTH CURTAINS SPECIFICALLY CALLED FOR IN THE PLANS OR AS DETERMINED BY THE ENGINEER.

NOTICE: COMPONENTS OF TYPES ~ AND ~~ MAY BE SIMILAR OR IDENTICAL TO PROPRIETARY DESIGNS. ANY INFRINGEMENT ON THE PROPRIETARY RIGHTS OF THE DESIGNER SHALL BE THE SOLE RESPONSIBILITY OF THE USER. SUBSTITUTIONS FOR TYPES ~ AND ~~ SHALL BE AS APPROVED BY THE ENGINEER.

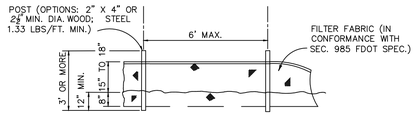
FLOATING TURBIDITY BARRIER DETAIL
N.T.S.



STAKED TURBIDITY BARRIER DETAIL
N.T.S.



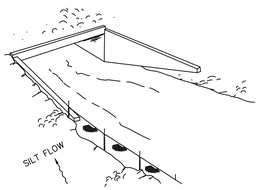
SECTION



ELEVATION

NOTE: SILT FENCE TO BE PAID FOR UNDER THE CONTRACT UNIT PRICE FOR STAKED SILT FENCE (LF).

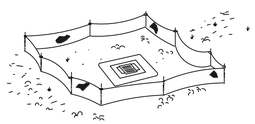
TYPE 3 SILT FENCE DETAIL
N.T.S.



TYPE 3 SILT FENCE



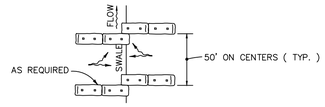
TYPE 3 SILT FENCE



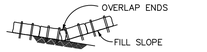
TYPE 3 SILT FENCE PROTECTION AROUND DITCH BOTTOM INLETS.

DO NOT DEPLOY IN A MANNER THAT SILT FENCES WILL ACT AS A DAM ACROSS PERMANENT FLOWING WATERCOURSES. SILT FENCES ARE TO BE USED AT UPLAND LOCATIONS AND TURBIDITY BARRIERS USED AT PERMANENT BODIES OF WATER.

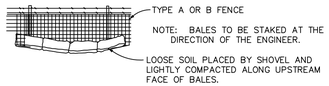
SILT FENCE APPLICATIONS DETAIL
N.T.S.



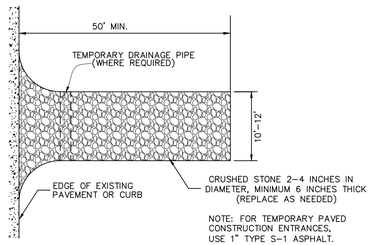
ANCHOR BALES WITH 2 – 2" X 2" X 4' STAKES PER BALE.
PLAN



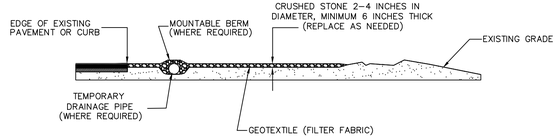
TO BE USED AT SELECTED SITES WHERE THE NATURAL GROUND SLOPES TOWARD THE TOE OF SLOPE
ELEVATION



BALES BACKED BY FENCE DETAIL
N.T.S.

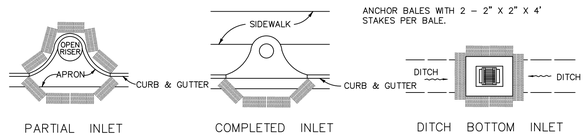


STABALIZED CONSTRUCTION ENTRANCE/EXIT PLAN VIEW
N.T.S.

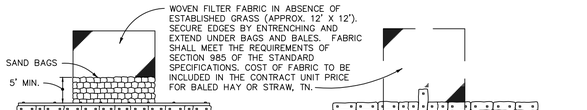


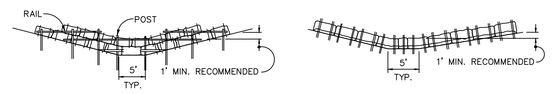
STABALIZED CONSTRUCTION ENTRANCE/EXIT SECTION VIEW
N.T.S.

GENERAL NOTES
1. EROSION CONTROL DETAILS ARE APPLICABLE TO NEW CONSTRUCTION, RECONSTRUCTION AND RRR PROJECTS.
2. FOR SODDING ADJACENT TO DITCHES AND AT HEADWALLS, SEE INDEX NO. 281.
3. ALL FRONT SLOPES STEEPER THAN 3:1 ARE TO BE SODDED.



PROTECTION AROUND INLETS OR SIMILAR STRUCTURES DETAIL
N.T.S.





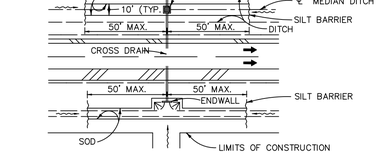
ANCHOR LOWER BALES WITH 2 – 2" X 2" X 4' STAKES PER BALE.
ANCHOR TOP BALES TO LOWER BALES WITH 2 – 2" X 2" X 4' STAKES PER BALE. ANCHOR BALES WITH 2 – 2" X 2" X 4' STAKES PER BALE
ELEVATION ELEVATION

APPLICATION AND SPACING: THE USE OF TYPES I & II BALE BARRIERS SHOULD BE LIMITED TO THE CONDITIONS OUTLINED IN CHART 1, SHEET 1 OF 3, INDEX NO. 102

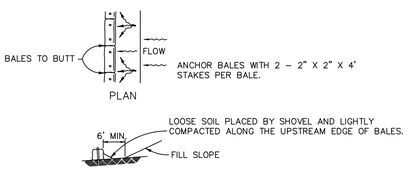
TYPE II TYPE I

BARRIER FOR UNPAVED DITCHES DETAIL
N.T.S.

GENERAL NOTES
1. EROSION CONTROL DETAILS ARE APPLICABLE TO NEW CONSTRUCTION, RECONSTRUCTION AND RRR PROJECTS.
2. FOR SODDING ADJACENT TO DITCHES AND AT HEADWALLS, SEE INDEX NO. 281.
3. ALL FRONT SLOPES STEEPER THAN 3:1 ARE TO BE SODDED.



DITCH INSTALLATIONS AT DRAINAGE STRUCTURES DETAIL
N.T.S.



TO BE USED AT SELECTED SITES WHERE THE NATURAL GROUND SLOPES AWAY FROM THE TOE OF SLOPE
ELEVATION

BARRIERS FOR FILL SLOPES DETAIL
N.T.S.

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
Professional Engineers, Planners, & Land Surveyors Serving Southwest Florida

FORT MYERS OFFICE:
10511 SIX MILE CYPRESS PKWY, STE. 101
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

PORT CHARLOTTE OFFICE:
4161 TAMIAMI TRAIL – BLDG 5 UNIT 501
PORT CHARLOTTE, FLORIDA 33952
PHONE: (941) 625-1165

WWW.BANKSENGFLA.COM
ENGINEERING LICENSE # EB 6469 \ SURVEY LICENSE # LB 6690

DAVID R. UNDERHILL JR.
P.E. #47029

BEST MANAGEMENT PRACTICES DETAILS
IBIS LANDING
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JULY 2025	3142L	BMP-DT	JLW	JLW	DRU	NTS	17

IBIS LANDING

S:\JOBS\3142L\DESIGN\ENGINEERING\PLANS\PLANS\STAND PLANS\BMP\3142-BMP_DET_1.DWG, 7/3/2025 2:08 PM, JIMMY MCKEWY

ATWELL

DAVID R. UNDERHILL JR. LICENSE No. 47029 STATE OF FLORIDA PROFESSIONAL ENGINEER

IBIS LANDING (FKA COPPERHEAD)
WATER MANAGEMENT REPORT

PROJECT INFORMATION

This application is a request for a permit modification to the Ibis Landing Environmental Resource Permit (Permit Number 36-108291-P) for site plan revisions in the southeast corner of the project. Ibis Landing is a partially constructed 298.86-acre residential golf course project formerly known as Copperhead. The site is located along Beth Stacey Blvd, south of SW 23rd Street in Lehigh Acres, Lee County, Florida. The project is being developed by Lennar under the name Ibis Landing.

PROJECT BACKGROUND

On April 13, 2000, Permit Number 36-03765-P (Application Number 990415-2) was approved for construction and operation approval of a surface water management of a residential golf course development located on a 297.8-acre site known as Copperhead. The entire surface water management system, golf course and the first phase of residential development (approximately 100 lots) were constructed in 2001 and placed in service.

A new ERP (Permit Number 36-108291-P) was issued to reauthorize construction of the undeveloped residential areas in September 2023. The residential infrastructure for the western and northeastern areas of the project have been constructed over the last two years. Beth Stacy Blvd (Milwaukee Blvd) will be extended from north to south as part of the Ibis Landing project. This roadway will ultimately be owned by Lee County and will drain to the project's surface water management system.

SURFACE WATER MANAGEMENT SYSTEM

The existing onsite surface water management system consists of 9 primary surface water management basins which use the existing lakes and dry detention areas for treatment and storage. A 10th basin was proposed for the southern (offsite) portion of Beth Stacey Blvd that has not been constructed. The site is surrounded by Lehigh Acres Municipal Services Improvement District (LAMSID) Yellowtail Canal and Yellowtail Canal Extensions on the West, South and East sides. The project discharges to these canals which are part of the Orange River Watershed. The discharge rate is 30.1 cfm which is the allowable rate per LAMSID. No modifications to the previously permitted wetland preserves are proposed.

Stormwater calculations are only being provided for the basins that are being modified (Basins 2-5). No changes to the permitted control elevations are proposed. Per the permit, the eastern portion of the site (modified area) has a control elevation of 24.0' NGVD. The Ibis Landing Surface Water Management System provides the required water quality treatment and attenuation for the 25-year 3-day storm event. Water quality treatment volume is one inch plus an additional 50%. Water quality calculations are provided at the end of the calculations for the basins that discharge offsite (Basins 2).

The eastern portion of the site (Basins 1, 3, 4, 5 and 6) discharge to Basin 2, a 26.62 wetland which discharges to the east to the Yellowtail Canal. This permitted drainage pattern will be maintained. No modification to any weirs that discharge offsite are planned.

PROPOSED PROJECT MODIFICATIONS

This application proposes site plan revisions within Basins 3 and 4 in the southeast corner of the project. The existing lake in Basins 3 is being reshaped which results in a reduction in the lake size of 0.4 acres. However,

the existing lake in Basin 4 is being reshaped which results in a increase in the lake size of 0.3 acres. Basins 3 and Basin 4 are also connected to help equalize the stages in these basins. The outfall location in Basin 4 is being maintained and will operate as previously permitted/constructed. The outfall location is Basin 3 is being modified to accommodate the lake reshaping. The bleeder in Basin 3 was sized to account for both Basin 3 and Basin 4 water quality bleed down. Minor revisions to Beth Stacey Blvd are also proposed which consist of providing closed drainage (curb and gutter) for the roadway along with the addition of a second 8' concrete sidewalk along the roadway (which is within Basin 5). The proposed Basin 10, which provided storage for runoff from the southern portion of Beth Stacey Blvd will be eliminated and the roadway will drain to Basins 4 and 5 which increase the sizes of these basins. Approximately 250' offsite roadway will drain to the proposed roadway swales. Compensating water quality storage will be provided for this area of the offsite roadway within Basin 5.

WATER QUALITY

All basins provide the required water quality treatment prior to connecting to the onsite wetland or before discharging offsite. Water quality treatment is provided in the interconnected lakes and dry detention ponds throughout the site. The required water quality volume of 150% of the greater of one inch or 2.5" over the impervious areas is provided. Compensating water quality treatment in the amount of 2.5 inches times the offsite impervious area is provided for the offsite portion of Beth Stacey Blve.

Basin #	Total Area (AC)	Control Elev. (NGVD)	Sub-basins Included	Treatment Provided By	Final Outfall WCS
1	49.33	24.00	B1L1 & B1L2	Wet Detention	WCS 2
2	32.44	24.00	N/A	Dry Detention	WCS 2
3	16.53	24.00	N/A	Wet Detention	WCS 2
4	18.34	24.00	N/A	Wet Detention	WCS 2
5	131.83	24.50	B5L5, B5L6, B5L7, B5L9, B5L10, B5L11	Wet Detention	WCS 2
6	18.20	24.00	N/A	Dry Detention	WCS 2
7	9.40	25.50	N/A	Wet Detention	WCS 7
8	6.11	25.50	N/A	Dry Detention	WCS 8
9	16.68	25.50	N/A	Dry Detention	WCS 8

Basin	CE	Proposed Basin Area	Permitted WQ V (ac-ft)	Proposed Construction WQ V (ac-ft)
B1 (P1-A, P1-B, P2-A, P2-B, P2-C & P2-D)	24.00	49.33	3.81	N/A
B2 (WA)	24.00	32.44	2.97	N/A
B3 (P3)	24.00	16.53	1.82	2.07
B4 (P4)	24.00	18.34	1.76	2.51
B5 (P5, P6, P7, P8-A, P8-B, P9, P10, & P11)	24.50	131.83	23.00	N/A
B6 (WC-A, WC-B, & WC-C)	24.00	18.20	1.75	N/A
B7 (P13)	25.50	9.40	1.18	N/A
B8 (WD)	25.50	6.11	0.93	N/A
B9 (P12)	25.50	16.68	0.38	N/A
	Totals	**298.86**	**37.60**	**4.58**

The proposed project provides 150% of the 1 inch over the project area. 298.86 x 1.5 inches = ***37.36 ac-ft***

The proposed project provides 2.5 inches times the percentage of imperviousness for the offsite improvements. 0.22 ac x 2.5 inches = ***0.05 ac-ft***

Total Required = ***37.41 ac-ft***

Total Provided = ***42.18 ac-ft***

WATER QUANTITY AND MODEL

The project is divided into 9 Basins with Basin 1 and Basin 5 having multiple interconnected lakes/sub-basins. The land use, CN, and stage storage was calculated by sub-basin. An Interconnected Pond Routing model (ICPR 4) was created to simulate the flood routing for the existing SWM system design. The weirs used in the model for Basins 1, 4, 5, 6, & 9 are modeled as they exist. The weir between Lake 3 and the downstream Wetland is proposed to be constructed. This weir will provide pre-treatment for areas that connect to Wetlands. The weirs, for Basins 2, 7, and 8, that discharge offsite will operate as previously permitted/constructed to meet the design parameters of water quality, allowable bleed down, and allowable discharge for SFWMD.

The design elevations are as follows and the flood routing results are included for comparison in the tables on the next page.

	CONDITION	
	Permitted (App#230103-37076)	Proposed (Current)
BASIN	WA	B2
5-YR	24.68	24.66
10-YR	24.86	24.84
25-YR	25.94	25.97
100-YR	27.42	27.44
DISCHARGE	10.98	11.12

	CONDITION	
	Permitted (App#230103-37076)	Proposed (Current)
BASIN	P3	B3
5-YR	25.43	25.94
10-YR	25.61	26.07
25-YR	26.81	26.91
100-YR	27.42	27.44
DISCHARGE	N/A (Internal)	N/A

BASIN	P4	B4
5-YR	25.64	25.96
10-YR	25.97	26.09
25-YR	26.68	26.75
100-YR	27.42	27.44
DISCHARGE	N/A (Internal)	N/A (Internal)

BASIN	P5-P11,P14-P15	B5
5-YR	25.54	25.54
10-YR	25.87	25.87
25-YR	27.32	27.32
100-YR	27.94	27.95
DISCHARGE	N/A (Internal)	N/A

The next page contains a land use comparison between the total permitted land use from Permit No. 36-108291-P application 230103-37076 and the proposed modification.

LAND USE COMPARISON

	Permitted Total	% of Proposed
Basin	B2	
Building	0.00	0.0%
Impervious	0.16	0.5%
Pervious	7.15	22.0%
Dry Detentiion	0.44	1.4%
Wetland	24.69	76.1%
Total	**32.44**	100.0%
Basin	B3	
Building	3.24	19.6%
Impervious	4.59	27.8%
Pervious	6.83	41.3%
Lake	1.87	11.3%
Total	**16.53**	100.0%
Basin	B4	
Building	2.37	13.3%
Impervious	3.74	21.0%
Pervious	10.33	58.0%
Lake	1.37	7.7%
Total	**17.81**	100.0%
Basin	B5	
Building	16.24	12.4%
Impervious	24.18	18.5%
Pervious	71.75	54.8%
Dry Detention	0.07	0.1%
Lake	18.64	14.2%
Total	**130.88**	100.0%
Basin	B10	
Building	0.04	2.7%
Impervious	0.12	8.1%
Pervious	1.11	75.0%
Dry Detention	0.21	14.2%
Total	**1.48**	100.0%

Proposed Modification Total	% of Permitted
B2	
0.00	0.0%
0.16	0.5%
7.15	22.0%
0.44	1.4%
24.69	76.1%
32.44	100.0%
B3	
3.05	18.5%
4.71	28.5%
7.61	46.0%
1.16	7.0%
16.53	100.0%
B4	
1.36	7.4%
7.38	40.2%
8.16	44.5%
1.44	7.9%
18.34	100.0%
P5, P6, P7, P8-A, P8-B, P9, P10, & P11	
16.25	12.3%
24.73	18.8%
72.14	54.7%
0.07	0.1%
18.64	14.1%
131.83	100.0%

Basin 10 has been removed and added to Basins 4 and 5 as shown above.

0.53 ac of Basin 10 was added into Basin 4.

0.95 ac of Basin 10 was added into Basin 5.

OFFSITE DISCHARGES

CS	Contributing Basins	Contributing Area (AC)	Allowable Discharge* (cfs)	Model Discharge (cfs)
WCS-2	B1, B2, B3, B4, B5, & B6	266.67	12.54	11.12

* Based on 30.1 SCM

The project lies within the Lehigh Acres Municipal Services Improvement District which has an allowable discharge limit of 30.1 CSM. The proposed surface water management system for the 266.67-acre basin has an allowable discharge rate of 12.54 cfs. The total discharge is 11.12 cfs which is less than allowed.

TOTAL SITE

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	25.78	8.6%	0.00	0.00	0.00	25.78
LAKE BANK @ CE + 1	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE TOB	2.28	0.8%	2.28	0.00	0.00	0.00
DRY DETENTION	1.65	0.6%	1.65	0.00	0.00	0.00
PRESERVE	37.83	12.7%	37.83	0.00	0.00	0.00
ROW	32.59	10.9%	13.04	19.55	0.00	0.00
SF	63.92	21.4%	25.57	19.18	19.18	0.00
MF	25.26	8.5%	5.05	10.10	10.10	0.00
GOLF COURSE	96.62	32.3%	93.73	2.90	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	0.5%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	2.3%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**298.86**	**100%**	**185.38**	**55.01**	**32.56**	**25.92**

TOTAL PERVIOUS AREA	185.38	62.0%	
TOTAL IMPERVIOUS AREA	113.49	38.0%	*(INCLUDES WATER SURFACE AREA)*

TOTAL SITE

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 62.0% x 6.57 in.
S = 4.07 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 71

III. SURFACE STORAGE

Basin Area =	298.86	ac.			
Bldg. Coverage Area =	32.56	ac.			
Impervious =	55.01				
Lake Area (at Control)	25.92	ac.	***EL. =***	***24.00***	***(NGVD)***
Preserve Area (at control)	37.83	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less Preserved)=	147.55	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**25.92**	**0.00**	**0.00**	**0.00**	**25.92**	**0.00**
25.00	**28.06**	**0.00**	**0.00**	**0.00**	**28.06**	**26.99**
26.50	**30.34**	**0.00**	**0.00**	**0.00**	**30.34**	**70.79**
27.00	**32.62**	**7.57**	**29.51**	**27.51**	**97.20**	**102.68**
27.50	**32.93**	**15.13**	**59.02**	**49.51**	**156.59**	**166.12**
28.50	**32.93**	**18.92**	**132.79**	**55.01**	**239.65**	**364.25**

IV. WATER QUALITY DETENTION VOLUME

TOTAL SITE

Total Area =	298.86	ac.
Bldg. Coverage =	32.56	ac.
Water Surface Area =	25.92	ac.
Pervious Area =	185.38	ac.

First one (1) Inch of runoff over basin:

= 1" x 298.86 ac. x1'/12" = **24.91** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	240.38	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	55.01	ac.
% Impervious = Impervious Area / Site Area	22.9%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**13.01**	**ac-ft**
Required Water Quality Detention Volume	**24.91**	**ac-ft**
Additional 50% Water Quality Detention Volume	**12.45**	**ac-ft**
Total Required Water Quality Detention Volume	**37.36**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**37.36**	**ac-ft**

Between El. 24.00 and 25.36

WCS Stage-Storage Table

Stage	Site Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	25.92	0.00
25.00	28.06	26.99
26.50	30.34	70.79
27.00	97.20	102.68
27.50	156.59	166.12
28.50	239.65	364.25

Water Quality Min. Weir Elevation = **25.36** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event:

= 55.0 CSM x WCS Area x 1 SM/640 ac.

= **25.68** **cfs**

METHOD OF DETERMINATION:		
Orange River Watershed (Approved ERP Appl # 060503-10)	55.0	CSM

Basin B2

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 1	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE TOB	0.00	0.0%	0.00	0.00	0.00	0.00
DRY DETENTION	0.44	1.4%	0.44	0.00	0.00	0.00
PRESERVE	26.62	82.1%	26.62	0.00	0.00	0.00
ROW	0.00	0.0%	0.00	0.00	0.00	0.00
SF	0.00	0.0%	0.00	0.00	0.00	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	5.38	16.6%	5.22	0.16	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**32.44**	**100%**	**32.28**	**0.16**	**0.00**	**0.00**

TOTAL PERVIOUS AREA	32.28	99.5%
TOTAL IMPERVIOUS AREA	0.16	0.5%

Basin B2

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** in.

Available Soil Storage:
S = (% of Pervicus Area) * (Soil Storage Depth)
S = 99.5% x 6.57 in.
S = 6.53 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 60

III. SURFACE STORAGE

Basin Area =	32.44	ac.			
Bldg. Coverage Area =	0.00	ac.			
Dry Detention TOS (at CE + 1)	0.44	ac.	*EL. =*	***25.00***	***(NGVD)***
Dry Detention TOB (at CE + 2)	0.54	ac.	*EL. =*	***26.00***	***(NGVD)***
Golf Course	5.22	ac.			
Preserve Area (at control)	26.62	ac.	*EL. =*	***24.00***	***(NGVD)***
Pervious Area =	32.28	ac.			

Stage-Storage Table

Stage	Dry Detention	Preserve Area	Golf Course	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac-ft
24.00	0.00	26.62	0.00	26.62	0.00
25.00	0.44	26.62	0.00	27.06	26.84
26.50	0.44	26.62	1.04	28.10	68.21
27.00	0.44	26.62	2.09	29.15	82.53
27.50	0.44	26.62	2.61	29.67	97.23

IV. WATER QUALITY DETENTION VOLUME
The permitted 2.97 ac-ft of water quality was provided in the existing condition and no modification to the basin is proposed.

V. ALLOWABLE DISCHARGE

BASIN	**2**		
Contributing Flows			
Basin	1	49.33	
Basin	2	32.44	
Basin	3	16.53	
Basin	4	18.34	
Basin	5	131.83	
Basin	6	18.20	
Total Area =		266.68	ac.

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event.

=	55.0	CSM x WCS Area x 1 SM/640 ac.
=	**22.92**	**cfs**

METHOD OF DETERMINATION:		
55 CSM allowed in Orange River Watershed & Approved ERP Appl # 934015-2	55.0	CSM

VI. WEIR DESIGN

Total Basin

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**266.68 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**11.11 Ac-FT**
½" Discharge Rate:	=		=	**5.72 CFS**
Water Quality Stage:	=		=	**24.56 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**22.22 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation)	=	**24.23 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**24.39 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)	=	**0.39 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 5.72 **CFS**

Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 7.39 Length of weir **A 1.50 ft wide rectangular weir was used to meet the maximum discharge rate**

Basin B3L3

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.16	7.0%	0.00	0.00	0.00	1.16
LAKE BANK @ CE + 1	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE TOB	0.14	0.8%	0.14	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	2.62	15.8%	1.05	1.57	0.00	0.00
SF	5.83	35.3%	2.33	1.75	1.75	0.00
MF	3.25	19.7%	0.65	1.30	1.30	0.00
GOLF COURSE	3.26	19.7%	3.16	0.10	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.53**	**100%**	**7.61**	**4.72**	**3.05**	**1.16**

TOTAL PERVIOUS AREA	7.61	46.0%	
TOTAL IMPERVIOUS AREA	8.93	54.0%	*(INCLUDES WATER SURFACE AREA)*

Basin B3L3

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:

= (Average Developed Grade)-(Wet Season Water Table)

= 27.50 - 24.00

= 3.50 ft.

Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:

S = (% of Pervious Area) * (Soil Storage Depth)

S = 46.0% x 6.57 in.

S = 3.02 in.

Curve Number:

Cn = 1000/(S + 10)

Cn = 77

III. SURFACE STORAGE

Basin Area =	16.53	ac.			
Bldg. Coverage Area =	3.05	ac.			
Lake Area (at Control)	1.16	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.30	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.44	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.57	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	3.16	ac.			
Pervious Area (less bank & GC) =	4.03	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.16**	**0.00**	**0.00**	**0.00**	**1.16**	**0.00**
25.00	**1.30**	**0.00**	**0.00**	**0.00**	**1.30**	**1.23**
26.50	**1.44**	**0.00**	**0.00**	**0.00**	**1.44**	**3.28**
27.00	**1.57**	**0.63**	**0.81**	**2.36**	**5.37**	**4.98**
27.50	**1.57**	**1.26**	**1.61**	**4.25**	**8.70**	**8.50**
28.50	**1.57**	**1.58**	**3.63**	**4.72**	**11.50**	**18.60**

IV. WATER QUALITY DETENTION VOLUME

BASIN	**3**		
Total Area =		16.53	ac.
Bldg. Coverage =		3.05	ac.
Water Surface Area =		1.16	ac.
Pervious Area =		7.61	ac.

First one (1) Inch of runoff over basin:

= 1" x	16.53	ac. x1'/12" =	**1.38**	**ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	12.32	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	4.72	ac.
% Impervious = Impervious Area / Site Area	38.3%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.23**	**ac-ft**
Required Water Quality Detention Volume	**1.38**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.69**	**ac-ft**
Total Required Water Quality Detention Volume	**2.07**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.07**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.16	0.00
25.00	1.30	1.23
26.50	1.44	3.28
27.00	1.57	4.03
27.50	1.57	4.82
28.50	1.57	6.39

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.16 ac + 1.44 ac (Lake 3 and Lake 4) = 2.60 ac, the WQ is provided at 25.76 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin (The required bleed down for B4L4 is accounted for in this basin)

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**34.88 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**1.45 Ac-FT**
½" Discharge Rate:	=		=	**0.75 CFS**
Water Quality Stage:	=		=	**25.76 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)=		**3.12 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation) =		**24.07 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2 =		**24.92 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)=		**0.92 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 0.75 **CFS**

Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 0.27 Length of weir **A 0.27 ft wide rectangular weir was used to meet the maximum bleeder rate**

Basin B4L4

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.44	7.9%	0.00	0.00	0.00	1.44
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	9.88	53.9%	3.95	5.93	0.00	0.00
SF	1.42	7.7%	0.57	0.43	0.43	0.00
MF	2.34	12.8%	0.47	0.94	0.94	0.00
GOLF COURSE	2.87	15.6%	2.78	0.09	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**18.34**	**100%**	**8.16**	**7.38**	**1.36**	**1.44**

TOTAL PERVIOUS AREA	8.16	44.5%	
TOTAL IMPERVIOUS AREA	10.18	55.5%	*(INCLUDES WATER SURFACE AREA)*

Basin B4L4

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** in.

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 44.5% x 6.57 in.
S = 2.92 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 77

III. SURFACE STORAGE

Basin Area =	18.34	ac.			
Bldg. Coverage Area =	1.36	ac.			
Lake Area (at Control)	1.44	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.57	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.70	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.83	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	0.00	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less bank & GC) =	4.99	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.44**	**0.00**	**0.00**	**0.00**	**1.44**	**0.00**
25.00	**1.57**	**0.00**	**0.00**	**0.00**	**1.57**	**1.51**
26.50	**1.70**	**0.00**	**0.00**	**0.00**	**1.70**	**3.96**
27.00	**1.83**	**0.56**	**1.00**	**3.69**	**7.07**	**6.15**
27.50	**1.83**	**1.11**	**2.00**	**6.64**	**11.58**	**10.82**
28.50	**1.83**	**1.39**	**4.49**	**7.38**	**15.09**	**24.15**

IV. WATER QUALITY DETENTION VOLUME

BASIN **4**

Total Area =	18.34	ac.
Bldg. Coverage =	1.36	ac.
Water Surface Area =	1.44	ac.
Pervious Area =	8.16	ac.

First one (1) Inch of runoff over basin:

= 1" x 18.34 ac. x1'/12" = **1.53** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	15.54	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	7.38	ac.
% Impervious = Impervious Area / Site Area	47.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.67**	**ac-ft**
Required Water Quality Detention Volume	**1.67**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.84**	**ac-ft**
Total Required Water Quality Detention Volume	**2.51**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.51**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.44	0.00
25.00	1.57	1.51
26.50	1.70	3.96
27.00	1.83	4.84
27.50	1.83	5.76
28.50	1.83	7.59

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.16 ac + 1.44 ac (Lake 3 and Lake 4) = 2.60 ac, the WQ is provided at 25.76 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

The Existing weir, as approved in ERP 36-03765-P (App 990415-2) was utilized.

Basin B5L5

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.61	15.7%	0.00	0.00	0.00	2.61
LAKE BANK @ CE + 1	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE TOB	0.23	1.4%	0.23	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	0.66	4.0%	0.26	0.40	0.00	0.00
SF	2	12.1%	0.80	0.60	0.60	0.00
MF	3.99	24.1%	0.80	1.60	1.60	0.00
GOLF COURSE	6.62	39.9%	6.42	0.20	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.58**	**100%**	**8.98**	**2.79**	**2.20**	**2.61**

TOTAL PERVIOUS AREA	8.98	54.2%	
TOTAL IMPERVIOUS AREA	7.60	45.8%	*(INCLUDES WATER SURFACE AREA)*

Basin B5L5

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 54.2% x 6.57 in.
S = 3.56 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	16.58	ac.			
Bldg. Coverage Area =	2.20	ac.			
Lake Area (at Control)	2.61	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.84	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.08	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	3.31	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	6.62	ac.			
Pervious Area (less bank & GC) =	1.86	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.61**	**0.00**	**0.00**	**0.00**	**2.61**	**0.00**
25.50	**2.84**	**0.00**	**0.00**	**0.00**	**2.84**	**2.73**
27.00	**3.08**	**0.00**	**0.00**	**0.00**	**3.08**	**7.17**
27.50	**3.31**	**1.32**	**0.37**	**1.40**	**6.40**	**9.54**
28.00	**3.31**	**2.65**	**0.74**	**2.51**	**9.22**	**13.44**
29.00	**3.31**	**3.31**	**1.68**	**2.79**	**11.09**	**23.60**

Lake B5L6

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.23	9.6%	0.00	0.00	0.00	1.23
LAKE BANK @ CE + 1	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE TOB	0.16	1.2%	0.16	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	1.40	10.9%	0.56	0.84	0.00	0.00
SF	5.62	43.7%	2.25	1.69	1.69	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	4.14	32.2%	4.01	0.12	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**12.86**	**100%**	**7.29**	**2.65**	**1.69**	**1.23**

TOTAL PERVIOUS AREA	7.29	56.7%	
TOTAL IMPERVIOUS AREA	5.57	43.3%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L6

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 56.7% x 6.57 in.
S = 3.72 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 73

III. SURFACE STORAGE

Basin Area =	12.86	ac.			
Bldg. Coverage Area =	1.69	ac.			
Lake Area (at Control)	1.23	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	0.16	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	0.16	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	1.39	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.01	ac.			
Pervious Area (less bank & GC) =	2.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	1.23	0.00	0.00	0.00	1.23	0.00
25.50	1.31	0.00	0.00	0.00	1.31	1.27
27.00	1.39	0.00	0.00	0.00	1.39	3.29
27.50	1.39	0.80	0.56	1.33	4.08	4.66
28.00	1.39	1.61	1.12	2.39	6.50	7.30
29.00	1.39	2.01	2.53	2.65	8.57	14.84

Lake B5L7

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	9.70	20.4%	0.00	0.00	0.00	9.70
LAKE BANK @ CE + 1	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE TOB	0.71	1.5%	0.71	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	3.50	7.4%	1.40	2.10	0.00	0.00
SF	7.01	14.8%	2.80	2.10	2.10	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	18.16	38.2%	17.61	0.54	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	14.7%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**47.51**	**100%**	**25.36**	**7.55**	**4.90**	**9.70**

TOTAL PERVIOUS AREA	25.36	53.4%	
TOTAL IMPERVIOUS AREA	22.15	46.6%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L7

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.4% x 6.57 in.
S = 3.50 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	47.51	ac.			
Bldg. Coverage Area =	4.90	ac.			
Lake Area (at Control)	9.70	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	10.41	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	11.13	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	11.84	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	18.16	ac.			
Pervious Area (less bank & GC) =	5.60	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**9.70**	**0.00**	**0.00**	**0.00**	**9.70**	**0.00**
25.50	**10.41**	**0.00**	**0.00**	**0.00**	**10.41**	**10.06**
27.00	**11.13**	**0.00**	**0.00**	**0.00**	**11.13**	**26.21**
27.50	**11.84**	**3.63**	**1.12**	**3.77**	**20.37**	**34.09**
28.00	**11.84**	**7.26**	**2.24**	**6.79**	**28.14**	**46.21**
29.00	**11.84**	**9.08**	**5.04**	**7.55**	**33.51**	**77.04**

Lake B5L9

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.49	11.4%	0.00	0.00	0.00	2.49
LAKE BANK @ CE + 1	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE TOB	0.28	1.3%	0.28	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.60	11.9%	1.04	1.56	0.00	0.00
SF	6.92	31.6%	2.77	2.08	2.08	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	9.06	41.3%	8.79	0.27	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**21.92**	**100%**	**13.45**	**3.91**	**2.08**	**2.49**

TOTAL PERVIOUS AREA	13.45	61.3%	
TOTAL IMPERVIOUS AREA	8.47	38.7%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L9

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 61.3% x 6.57 in.
S = 4.03 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 71

III. SURFACE STORAGE

Basin Area =	21.92	ac.			
Bldg. Coverage Area =	2.08	ac.			
Lake Area (at Control)	2.49	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.77	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.05	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	2.77	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	9.06	ac.			
Pervious Area (less bank & GC) =	3.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.49**	**0.00**	**0.00**	**0.00**	**2.49**	**0.00**
25.50	**2.77**	**0.00**	**0.00**	**0.00**	**2.77**	**2.63**
27.00	**3.05**	**0.00**	**0.00**	**0.00**	**3.05**	**7.00**
27.50	**3.05**	**1.81**	**0.76**	**1.95**	**7.58**	**9.66**
28.00	**3.05**	**3.63**	**1.52**	**3.52**	**11.72**	**14.49**
29.00	**3.05**	**4.53**	**3.43**	**3.91**	**14.92**	**27.81**

Lake B5L10

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.42	9.3%	0.00	0.00	0.00	1.42
LAKE BANK @ CE + 1	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE TOB	0.17	1.1%	0.17	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	0.61	4.0%	0.24	0.37	0.00	0.00
SF	1.37	9.0%	0.55	0.41	0.41	0.00
MF	4.10	26.9%	0.82	1.64	1.64	0.00
GOLF COURSE	5.85	38.5%	5.68	0.18	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	9.0%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**15.22**	**100%**	**8.06**	**3.07**	**2.53**	**1.56**

TOTAL PERVIOUS AREA	8.06	53.0%	
TOTAL IMPERVIOUS AREA	7.16	47.0%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L10

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.0% x 6.57 in.
S = 3.48 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	15.22	ac.			
Bldg. Coverage Area =	2.53	ac.			
Lake Area (at Control)	1.56	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.72	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.89	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.92	ac.	***EL. =***	***26.50***	***(NGVD)***
Golf Course	5.68	ac.			
Pervious Area (less bank & GC) =	1.89	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.56**	**0.00**	**0.00**	**0.00**	**1.56**	**0.00**
25.50	**1.72**	**0.00**	**0.00**	**0.00**	**1.72**	**1.64**
27.00	**1.89**	**0.00**	**0.00**	**0.00**	**1.89**	**4.35**
27.50	**1.92**	**1.14**	**0.38**	**1.54**	**4.96**	**6.06**
28.00	**1.92**	**2.27**	**0.75**	**2.76**	**7.71**	**9.23**
29.00	**1.92**	**2.84**	**1.70**	**3.07**	**9.52**	**17.84**

Lake B5L11

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.05	5.9%	0.00	0.00	0.00	1.05
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.94	16.6%	1.18	1.76	0.00	0.00
SF	6.61	37.3%	2.64	1.98	1.98	0.00
MF	2.20	12.4%	0.44	0.88	0.88	0.00
GOLF COURSE	4.55	25.6%	4.41	0.14	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**17.74**	**100%**	**9.06**	**4.76**	**2.86**	**1.05**

TOTAL PERVIOUS AREA	9.06	51.1%	
TOTAL IMPERVIOUS AREA	8.68	48.9%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L11

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 51.1% x 6.57 in.
S = 3.35 In.

Curve Number:
Cn = 1000/(S + 10)
Cn = 75

III. SURFACE STORAGE

Basin Area =	17.74	ac.			
Bldg. Coverage Area =	2.86	ac.			
Lake Area (at Control)	1.05	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.18	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.31	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.44	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.41	ac.			
Pervious Area (less bank & GC) =	4.26	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.05**	**0.00**	**0.00**	**0.00**	**1.05**	**0.00**
25.50	**1.18**	**0.00**	**0.00**	**0.00**	**1.18**	**1.12**
27.00	**1.31**	**0.00**	**0.00**	**0.00**	**1.31**	**2.98**
27.50	**1.44**	**0.88**	**0.85**	**2.38**	**5.56**	**4.70**
28.00	**1.44**	**1.76**	**1.70**	**4.29**	**9.20**	**8.39**
29.00	**1.44**	**2.21**	**3.83**	**4.76**	**12.25**	**19.11**

IV. WATER QUALITY DETENTION VOLUME

BASIN **5**

Total Area =	131.83	ac.
Bldg. Coverage =	16.25	ac.
Water Surface Area =	18.64	ac.
Pervious Area =	72.21	ac.

First one (1) Inch of runoff over basin:

= 1" x 131.83 ac. x1'/12" = **10.99** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	96.94	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	24.73	ac.
% Impervious = Impervious Area / Site Area	25.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**6.02**	**ac-ft**
Required Water Quality Detention Volume	**10.99**	**ac-ft**
Additional 50% Water Quality Detention Volume	**5.49**	**ac-ft**

Beth Stacy Offsite Imperviousness

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	0.22	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	0.22	ac.
% Impervious = Impervious Area / Site Area	100.0%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**0.05**	**ac-ft**
Total Required Water Quality Detention Volume	**16.52**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**16.52**	**ac-ft**

Between El. 24.00 and 25.35

Stage-Storage Table for Water Quality Calculations

Stage (NGVD)	Lake Storage Area ac	Lake Storage Volume ac-ft
24.50	18.64	0.00
25.50	20.24	19.44
27.00	21.85	51.01
27.50	22.95	62.20
28.00	22.95	73.68
29.00	22.95	96.63

23 ac-ft of total wq volume is provided

Water Quality Min. Weir Elevation = **25.35** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**131.83 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**5.49 Ac-FT**
½" Discharge Rate:	=		=	**2.83 CFS**
Water Quality Stage:	=		=	**25.67 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**17.51 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	eatment Vol. After 24 Hr. / Total Basin Stora (+ Control Elevation)	=	**24.84 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**25.26 Ft-NAVD**
Average Height During Bleed Down:	=	Ave. Elev. During Bleed Down - Control Elev.	=	**0.76 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 2.83 **CFS**

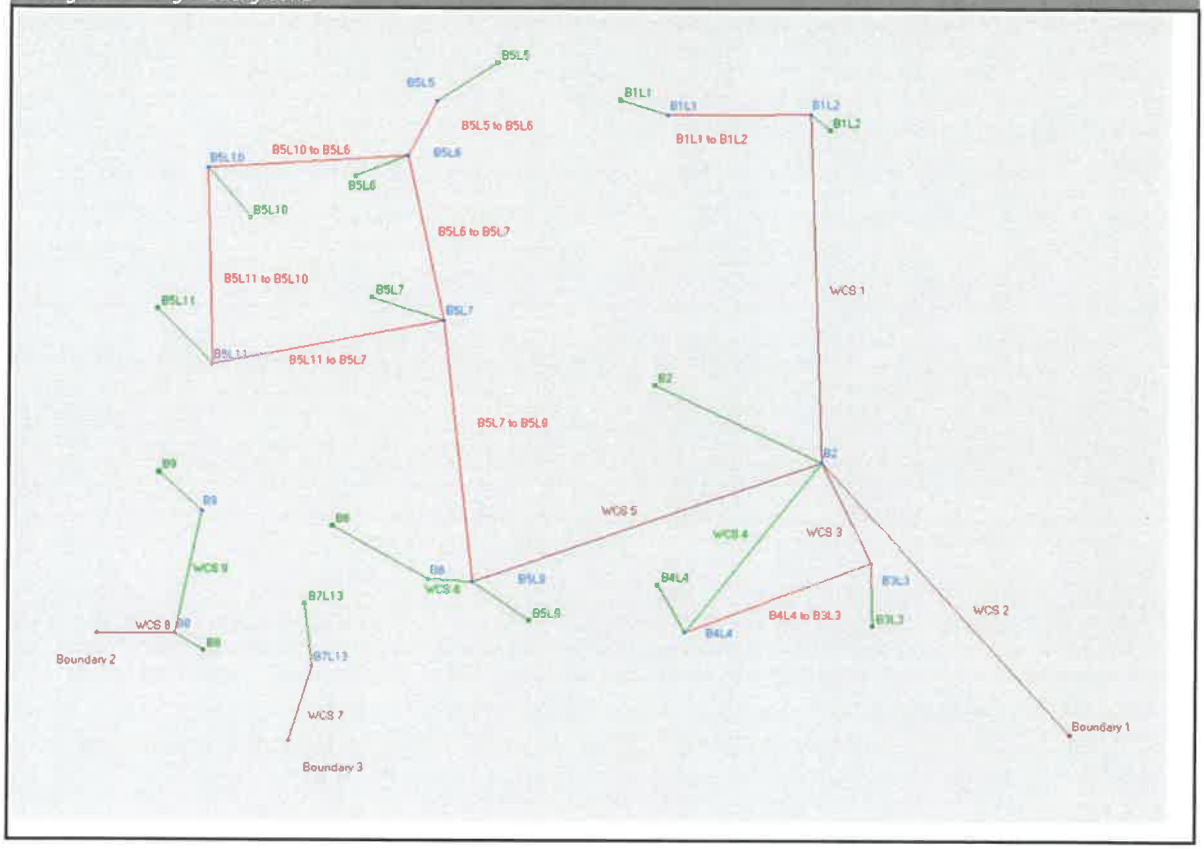
Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 1.37 Length of weir **A 1.33 ft wide rectangular weir was used to meet the maximum bleeder rate**

Background Image: Background



Impervious: Scenario 1 [Set]

Land Cover Zone	% Impervious	% DCIA	% Direct	Ia/S [dec]	Ia Impervious [in]	Ia Pervious [in]
Impervious & Pervious	0.00	0.00	0.00	0.00	0.000	0.000

Curve Number: Scenario 1 [Set]

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 60	60.0
Impervious & Pervious	Calculated CN 63	63.0
Impervious & Pervious	Calculated CN 64	64.0
Impervious & Pervious	Calculated CN 70	70.0
Impervious & Pervious	Calculated CN 71	71.0
Impervious & Pervious	Calculated CN 72	72.0

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 73	73.0
Impervious & Pervious	Calculated CN 74	74.0
Impervious & Pervious	Calculated CN 75	75.0
Impervious & Pervious	Calculated CN 76	76.0
Impervious & Pervious	Calculated CN 77	77.0

Manual Basin: B2

Scenario: Scenario1
Node: B2
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 60.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 32.4400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
32.4400	Impervious & Pervious	Calculated CN 60	

Comment:

Manual Basin: B3L3

Scenario: Scenario1
Node: B3L3
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5300 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5300	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B4L4

Scenario: Scenario1
Node: B4L4

Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 18.3400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
18.3400	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B5L10

Scenario: Scenario1
Node: B5L10
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 15.2200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
15.2200	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L11

Scenario: Scenario1
Node: B5L11
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 17.7400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
17.7400	Impervious & Pervious	Calculated CN 75	

Comment:

Manual Basin: B5L5

Scenario: Scenario1
Node: B5L5
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5800 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5800	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L6

Scenario: Scenario1
Node: B5L6
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 12.8600 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
12.8600	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L7

Scenario: Scenario1
Node: B5L7
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 47.5100 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
47.5100	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L9

Scenario: Scenario1
Node: B5L9
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 21.9200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
21.9200	Impervious & Pervious	Calculated CN 71	

Comment:

Node: B2

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	26.6200	1159567
25.00	27.0600	1178734
26.50	28.1000	1224036
27.00	29.1500	1269774
27.50	29.6700	1292425

Comment:

Node: B3L3

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft

Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.1600	50530
25.00	1.3000	56628
26.50	1.4400	62726
27.00	5.3700	233917
27.50	8.7000	378972
28.50	11.5000	500940

Comment:

Node: B4L4

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.4400	62726
25.00	1.5700	68389
26.50	1.7000	74052
27.00	7.0700	307969
27.50	11.5800	504425
28.50	15.0900	657320

Comment:

Node: B5L10

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.5600	67954
25.50	1.7200	74923
27.00	1.8900	82328
27.50	4.9600	216058
28.00	7.7100	335848
29.00	9.5200	414691

Comment:

Node: B5L11

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.0500	45738
25.50	1.1800	51401
27.00	1.3100	57064
27.50	5.5600	242194
28.00	9.2000	400752
29.00	12.2500	533610

Comment:

Node: B5L5

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.6100	113692
25.50	2.8400	123710
27.00	3.0800	134165
27.50	6.4000	278784
28.00	9.2200	401623
29.00	11.0900	483080

Comment:

Node: B5L6

Scenario: Scenario1
Type: Stage/Area

Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.2300	53579
25.50	1.3100	57064
27.00	1.3900	60548
27.50	4.0800	177725
28.00	6.5000	283140
29.00	8.5700	373309

Comment:

Node: B5L7

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	9.7000	422532
25.50	10.4100	453460
27.00	11.1300	484823
27.50	20.3700	887317
28.00	28.1400	1225778
29.00	33.5100	1459696

Comment:

Node: B5L9

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.4900	108464
25.50	2.7700	120661
27.00	3.0500	132858

Stage [ft]	Area [ac]	Area [ft2]
27.50	7.5800	330185
28.00	11.7200	510523
29.00	14.9200	649915

Comment:

Node: Boundary 1

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 23.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	23.00
0	0	0	60.0000	24.00
0	0	0	80.0000	25.00
0	0	0	100.0000	24.50
0	0	0	115.0000	23.30
0	0	0	336.0000	23.00

Comment:

Pipe Link: B4L4 to B3L3

Scenario: Scenario1
From Node: B3L3
To Node: B4L4
Link Count: 1
Flow Direction: Both
Damping: 0.0000 ft
Length: 731.00 ft
FHWA Code: 0
Entr Loss Coef: 1
Exit Loss Coef: 1
Bend Loss Coef: 0
Bend Location: 0.00 dec
Energy Switch: Energy

Upstream		Downstream	
Invert:	21.00 ft	Invert:	21.00 ft
Manning's N:	0.0130	Manning's N:	0.0130
Geometry:	Circular	Geometry:	Circular
Max Depth:	3.00 ft	Max Depth:	3.00 ft
Bottom Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000
Top Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L10 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.70 ft	Invert:	20.60 ft
From Node:	B5L10	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry:	Circular	Geometry:	Circular
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	450.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L10		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.15 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L10	Geometry:	Circular	Geometry:	Circular
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	320.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	21.00 ft	Invert:	20.50 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry:	Circular	Geometry:	Circular
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	430.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	1	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			

Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L5 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft
From Node:	B5L5	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	385.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L6 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.40 ft
From Node:	B5L6	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	220.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L7 to B5L9		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft

From Node:	B5L7	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L9	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.50 ft	Max Depth:	2.50 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	585.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Drop Structure Link: WCS 2		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B2	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	Boundary 1	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	65.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Sharp Crested Vertical	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	24.00 ft	Ref Node:	
Control Elevation:	24.00 ft	Discharge Coefficients	
Max Depth:	999.00 ft	Weir Default:	3.200
Max Width:	1.50 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600

Orifice Table:

Weir Comment:

Drop Structure Comment:

Drop Structure Link: WCS 3		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B3L3	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B2	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	237.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Sharp Crested Vertical	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	24.00 ft	Ref Node:	
Control Elevation:	24.00 ft	Discharge Coefficients	
Max Depth:	0.25 ft	Weir Default:	3.200
Max Width:	0.27 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component		Bottom Clip	
Weir:	2	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Horizontal		

Geometry Type:	Rectangular
Invert:	25.76 ft
Control Elevation:	25.76 ft
Max Depth:	3.00 ft
Max Width:	5.00 ft
Fillet:	0.00 ft

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	3.200
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Drop Structure Comment:

Weir Link: WCS 4

Scenario:	Scenario1
From Node:	B4L4
To Node:	B2
Link Count:	1
Flow Direction:	Both
Damping:	0.0000 ft
Weir Type:	Broad Crested Vertical
Geometry Type:	Rectangular
Invert:	26.00 ft
Control Elevation:	26.00 ft
Max Depth:	999.00 ft
Max Width:	30.00 ft
Fillet:	0.00 ft

Bottom Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Top Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	2.800
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Comment:

Simulation: 100yr3d

Scenario:	Scenario1
Run Date/Time:	9/18/2025 10:11:53 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	216.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr

Ia/S: 0.20 dec

Smp/Man Basin Rain Opt: Global

Rainfall Name: ~SFWMD-72
Rainfall Amount: 14.20 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 10yr 1d

Scenario: Scenario1
Run Date/Time: 9/23/2025 3:10:07 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec

IA Recovery Time: 12.0000 hr

Ia/S: 0.20 dec

dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FLMOD
		Rainfall Amount:	6.00 in
		Storm Duration:	24.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 25yr3d

Scenario: Scenario1
Run Date/Time: 9/23/2025 3:10:16 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:	
Unit Hydrograph Folder:	

Lookup Tables

Boundary Stage Set:	
Extern Hydrograph Set:	
Curve Number Set:	Scenario 1
Green-Ampt Set:	
Vertical Layers Set:	
Impervious Set:	Scenario 1

Tolerances & Options

Time Marching:	SAOR
Max Iterations:	6
Over-Relax Weight Fact:	0.5 dec
dZ Tolerance:	0.0010 ft
Max dZ:	1.0000 ft
Link Optimizer Tol:	0.0001 ft
IA Recovery Time:	24.0000 hr
Ia/S:	0.20 dec
Smp/Man Basin Rain Opt:	Global
Rainfall Name:	~SFWMD-72
Rainfall Amount:	10.20 in
Storm Duration:	72.0000 hr
Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (1D):	100 ft2
Energy Switch (1D):	Energy

Comment:

Simulation: 5yr1d

Scenario:	Scenario1
Run Date/Time:	9/23/2025 3:10:43 PM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 12.0000 hr

Ia/S: 0.20 dec

Smp/Man Basin Rain Opt: Global

Rainfall Name: ~FLMOD
Rainfall Amount: 5.20 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	10yr 1d	B2	24.84	42.0862
Scenario1	10yr 1d	B3L3	26.07	13.1869
Scenario1	10yr 1d	B4L4	26.09	13.2166
Scenario1	10yr 1d	B5L10	25.86	13.0674
Scenario1	10yr 1d	B5L11	25.87	12.8033
Scenario1	10yr 1d	B5L5	25.73	15.8862
Scenario1	10yr 1d	B5L6	25.72	24.0696
Scenario1	10yr 1d	B5L7	25.71	24.1362
Scenario1	10yr 1d	B5L9	25.64	24.5112
Scenario1	25yr3d	B2	25.98	74.1436
Scenario1	25yr3d	B3L3	26.91	60.4359
Scenario1	25yr3d	B4L4	26.75	60.2153
Scenario1	25yr3d	B5L10	27.32	61.3671
Scenario1	25yr3d	B5L11	27.32	60.8888
Scenario1	25yr3d	B5L5	27.19	64.9794
Scenario1	25yr3d	B5L6	27.16	64.4186
Scenario1	25yr3d	B5L7	27.08	72.0602
Scenario1	25yr3d	B5L9	26.77	72.7019
Scenario1	5yr1d	B2	24.66	50.1783
Scenario1	5yr1d	B3L3	25.94	15.5401
Scenario1	5yr1d	B4L4	25.96	15.5473
Scenario1	5yr1d	B5L10	25.54	13.0172
Scenario1	5yr1d	B5L11	25.54	12.8222
Scenario1	5yr1d	B5L5	25.45	15.5878
Scenario1	5yr1d	B5L6	25.44	24.0950
Scenario1	5yr1d	B5L7	25.44	24.1700
Scenario1	5yr1d	B5L9	25.40	24.4366

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	100yr3d	B2	27.44	216.0017
Scenario1	100yr3d	B3L3	27.44	216.0017
Scenario1	100yr3d	B4L4	27.44	216.0017
Scenario1	100yr3d	B5L10	27.94	64.2212
Scenario1	100yr3d	B5L11	27.93	64.2212
Scenario1	100yr3d	B5L5	27.95	72.0934
Scenario1	100yr3d	B5L6	27.92	72.1017
Scenario1	100yr3d	B5L7	27.86	72.2184
Scenario1	100yr3d	B5L9	27.59	80.0184

Sim	Link Name	Maximum Flow Rate [cfs]	Time to Maximum Flow Rate [hrs]
25yr3d	WCS 2	11.12	109.5011

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	0.0000	24.00	0.00
25yr3d	B2	0.2525	24.00	0.00
25yr3d	B2	0.5010	24.00	0.00
25yr3d	B2	0.7552	24.00	0.00
25yr3d	B2	1.0052	24.00	0.00
25yr3d	B2	1.2552	24.00	0.00
25yr3d	B2	1.5052	24.00	0.00
25yr3d	B2	1.7552	24.00	0.00
25yr3d	B2	2.0052	24.00	0.00
25yr3d	B2	2.2552	24.00	0.00
25yr3d	B2	2.5052	24.00	0.00
25yr3d	B2	2.7552	24.00	0.00
25yr3d	B2	3.0052	24.00	0.00
25yr3d	B2	3.2552	24.00	0.00
25yr3d	B2	3.5052	24.00	0.00
25yr3d	B2	3.7552	24.00	0.00
25yr3d	B2	4.0052	24.00	0.00
25yr3d	B2	4.2552	24.00	0.00
25yr3d	B2	4.5052	24.00	0.00
25yr3d	B2	4.7552	24.00	0.00
25yr3d	B2	5.0052	24.00	0.00
25yr3d	B2	5.2552	24.00	0.00
25yr3d	B2	5.5052	24.00	0.00
25yr3d	B2	5.7552	24.00	0.00
25yr3d	B2	6.0052	24.00	0.00
25yr3d	B2	6.2552	24.00	0.00
25yr3d	B2	6.5052	24.00	0.00
25yr3d	B2	6.7552	24.00	0.00
25yr3d	B2	7.0052	24.00	0.00
25yr3d	B2	7.2552	24.00	0.00
25yr3d	B2	7.5052	24.00	0.00
25yr3d	B2	7.7552	24.00	0.00
25yr3d	B2	8.0052	24.00	0.00
25yr3d	B2	8.2552	24.00	0.00
25yr3d	B2	8.5052	24.00	0.00
25yr3d	B2	8.7552	24.00	0.00
25yr3d	B2	9.0052	24.00	0.00
25yr3d	B2	9.2552	24.00	0.00
25yr3d	B2	9.5052	24.00	0.00
25yr3d	B2	9.7552	24.00	0.00
25yr3d	B2	10.0052	24.00	0.00
25yr3d	B2	10.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	10.5052	24.00	0.00
25yr3d	B2	10.7552	24.00	0.00
25yr3d	B2	11.0052	24.00	0.00
25yr3d	B2	11.2552	24.00	0.00
25yr3d	B2	11.5052	24.00	0.00
25yr3d	B2	11.7552	24.00	0.00
25yr3d	B2	12.0052	24.00	0.00
25yr3d	B2	12.2552	24.00	0.00
25yr3d	B2	12.5052	24.00	0.00
25yr3d	B2	12.7552	24.00	0.00
25yr3d	B2	13.0052	24.00	0.00
25yr3d	B2	13.2552	24.00	0.00
25yr3d	B2	13.5052	24.00	0.00
25yr3d	B2	13.7552	24.00	0.00
25yr3d	B2	14.0052	24.00	0.00
25yr3d	B2	14.2552	24.00	0.00
25yr3d	B2	14.5052	24.00	0.00
25yr3d	B2	14.7552	24.00	0.00
25yr3d	B2	15.0052	24.00	0.00
25yr3d	B2	15.2552	24.00	0.00
25yr3d	B2	15.5052	24.00	0.00
25yr3d	B2	15.7552	24.00	0.00
25yr3d	B2	16.0052	24.00	0.00
25yr3d	B2	16.2552	24.00	0.00
25yr3d	B2	16.5052	24.00	0.00
25yr3d	B2	16.7552	24.00	0.00
25yr3d	B2	17.0052	24.00	0.00
25yr3d	B2	17.2552	24.00	0.00
25yr3d	B2	17.5052	24.00	0.00
25yr3d	B2	17.7552	24.00	0.00
25yr3d	B2	18.0052	24.00	0.00
25yr3d	B2	18.2552	24.00	0.00
25yr3d	B2	18.5052	24.00	0.00
25yr3d	B2	18.7552	24.00	0.00
25yr3d	B2	19.0052	24.00	0.00
25yr3d	B2	19.2552	24.00	0.00
25yr3d	B2	19.5052	24.00	0.00
25yr3d	B2	19.7552	24.00	0.00
25yr3d	B2	20.0052	24.00	0.00
25yr3d	B2	20.2552	24.00	0.00
25yr3d	B2	20.5052	24.00	0.00
25yr3d	B2	20.7552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	21.0052	24.00	0.00
25yr3d	B2	21.2552	24.00	0.00
25yr3d	B2	21.5052	24.00	0.00
25yr3d	B2	21.7552	24.00	0.00
25yr3d	B2	22.0052	24.00	0.00
25yr3d	B2	22.2552	24.00	0.00
25yr3d	B2	22.5052	24.00	0.00
25yr3d	B2	22.7552	24.00	0.00
25yr3d	B2	23.0052	24.00	0.00
25yr3d	B2	23.2552	24.00	0.00
25yr3d	B2	23.5052	24.00	0.00
25yr3d	B2	23.7552	24.00	0.00
25yr3d	B2	24.0052	24.00	0.00
25yr3d	B2	24.2552	24.00	0.00
25yr3d	B2	24.5052	24.00	0.00
25yr3d	B2	24.7552	24.00	0.00
25yr3d	B2	25.0052	24.00	0.00
25yr3d	B2	25.2552	24.00	0.00
25yr3d	B2	25.5052	24.00	0.00
25yr3d	B2	25.7552	24.00	0.00
25yr3d	B2	26.0052	24.00	0.00
25yr3d	B2	26.2552	24.00	0.00
25yr3d	B2	26.5052	24.00	0.00
25yr3d	B2	26.7552	24.00	0.00
25yr3d	B2	27.0052	24.00	0.00
25yr3d	B2	27.2552	24.00	0.00
25yr3d	B2	27.5052	24.00	0.00
25yr3d	B2	27.7552	24.00	0.00
25yr3d	B2	28.0052	24.00	0.00
25yr3d	B2	28.2552	24.00	0.00
25yr3d	B2	28.5052	24.00	0.00
25yr3d	B2	28.7552	24.00	0.00
25yr3d	B2	29.0052	24.00	0.00
25yr3d	B2	29.2552	24.00	0.00
25yr3d	B2	29.5052	24.00	0.00
25yr3d	B2	29.7552	24.00	0.00
25yr3d	B2	30.0052	24.00	0.00
25yr3d	B2	30.2552	24.00	0.00
25yr3d	B2	30.5052	24.00	0.00
25yr3d	B2	30.7552	24.00	0.00
25yr3d	B2	31.0052	24.00	0.00
25yr3d	B2	31.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	31.5052	24.00	0.00
25yr3d	B2	31.7552	24.00	0.00
25yr3d	B2	32.0052	24.00	0.00
25yr3d	B2	32.2552	24.00	0.00
25yr3d	B2	32.5052	24.00	0.00
25yr3d	B2	32.7552	24.00	0.00
25yr3d	B2	33.0052	24.01	0.00
25yr3d	B2	33.2552	24.01	0.00
25yr3d	B2	33.5052	24.01	0.00
25yr3d	B2	33.7552	24.01	0.00
25yr3d	B2	34.0052	24.01	0.00
25yr3d	B2	34.2552	24.01	0.00
25yr3d	B2	34.5052	24.01	0.00
25yr3d	B2	34.7552	24.01	0.00
25yr3d	B2	35.0052	24.01	0.00
25yr3d	B2	35.2552	24.01	0.00
25yr3d	B2	35.5052	24.01	0.00
25yr3d	B2	35.7552	24.01	0.00
25yr3d	B2	36.0052	24.01	0.00
25yr3d	B2	36.2552	24.01	0.00
25yr3d	B2	36.5052	24.01	0.00
25yr3d	B2	36.7552	24.01	0.00
25yr3d	B2	37.0052	24.01	0.00
25yr3d	B2	37.2552	24.01	0.00
25yr3d	B2	37.5052	24.01	0.00
25yr3d	B2	37.7552	24.01	0.00
25yr3d	B2	38.0052	24.02	0.00
25yr3d	B2	38.2552	24.02	0.00
25yr3d	B2	38.5052	24.02	0.00
25yr3d	B2	38.7552	24.02	0.00
25yr3d	B2	39.0052	24.02	0.00
25yr3d	B2	39.2552	24.02	0.00
25yr3d	B2	39.5052	24.02	0.00
25yr3d	B2	39.7552	24.02	0.00
25yr3d	B2	40.0052	24.02	0.00
25yr3d	B2	40.2552	24.02	0.00
25yr3d	B2	40.5052	24.02	0.00
25yr3d	B2	40.7552	24.02	0.01
25yr3d	B2	41.0052	24.02	0.01
25yr3d	B2	41.2552	24.03	0.01
25yr3d	B2	41.5052	24.03	0.01
25yr3d	B2	41.7552	24.03	0.01

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	42.0052	24.03	0.01
25yr3d	B2	42.2552	24.03	0.01
25yr3d	B2	42.5052	24.03	0.01
25yr3d	B2	42.7552	24.03	0.01
25yr3d	B2	43.0052	24.03	0.01
25yr3d	B2	43.2552	24.03	0.01
25yr3d	B2	43.5052	24.03	0.01
25yr3d	B2	43.7552	24.04	0.01
25yr3d	B2	44.0052	24.04	0.01
25yr3d	B2	44.2552	24.04	0.01
25yr3d	B2	44.5052	24.04	0.01
25yr3d	B2	44.7552	24.04	0.01
25yr3d	B2	45.0052	24.04	0.02
25yr3d	B2	45.2552	24.04	0.02
25yr3d	B2	45.5052	24.04	0.02
25yr3d	B2	45.7552	24.05	0.02
25yr3d	B2	46.0052	24.05	0.02
25yr3d	B2	46.2552	24.05	0.02
25yr3d	B2	46.5052	24.05	0.02
25yr3d	B2	46.7552	24.05	0.02
25yr3d	B2	47.0052	24.05	0.02
25yr3d	B2	47.2552	24.05	0.02
25yr3d	B2	47.5052	24.06	0.03
25yr3d	B2	47.7552	24.06	0.03
25yr3d	B2	48.0052	24.06	0.03
25yr3d	B2	48.2552	24.06	0.03
25yr3d	B2	48.5052	24.06	0.03
25yr3d	B2	48.7552	24.06	0.03
25yr3d	B2	49.0052	24.07	0.03
25yr3d	B2	49.2552	24.07	0.04
25yr3d	B2	49.5052	24.07	0.04
25yr3d	B2	49.7552	24.07	0.04
25yr3d	B2	50.0052	24.07	0.04
25yr3d	B2	50.2552	24.07	0.04
25yr3d	B2	50.5052	24.08	0.05
25yr3d	B2	50.7552	24.08	0.05
25yr3d	B2	51.0052	24.08	0.05
25yr3d	B2	51.2552	24.08	0.05
25yr3d	B2	51.5052	24.08	0.05
25yr3d	B2	51.7552	24.09	0.06
25yr3d	B2	52.0052	24.09	0.06
25yr3d	B2	52.2552	24.09	0.06

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	52.5052	24.09	0.07
25yr3d	B2	52.7552	24.10	0.07
25yr3d	B2	53.0052	24.10	0.07
25yr3d	B2	53.2552	24.10	0.07
25yr3d	B2	53.5052	24.10	0.08
25yr3d	B2	53.7552	24.11	0.08
25yr3d	B2	54.0052	24.11	0.08
25yr3d	B2	54.2552	24.11	0.09
25yr3d	B2	54.5036	24.12	0.09
25yr3d	B2	54.7536	24.12	0.10
25yr3d	B2	55.0002	24.13	0.10
25yr3d	B2	55.2530	24.13	0.11
25yr3d	B2	55.5053	24.14	0.11
25yr3d	B2	55.7502	24.14	0.11
25yr3d	B2	56.0013	24.15	0.12
25yr3d	B2	56.2520	24.15	0.13
25yr3d	B2	56.5031	24.16	0.13
25yr3d	B2	56.7519	24.16	0.14
25yr3d	B2	57.0016	24.17	0.15
25yr3d	B2	57.2508	24.18	0.15
25yr3d	B2	57.5036	24.19	0.16
25yr3d	B2	57.7505	24.20	0.17
25yr3d	B2	58.0032	24.21	0.18
25yr3d	B2	58.2526	24.22	0.19
25yr3d	B2	58.5033	24.23	0.20
25yr3d	B2	58.7534	24.25	0.21
25yr3d	B2	59.0014	24.26	0.22
25yr3d	B2	59.2509	24.28	0.24
25yr3d	B2	59.5018	24.31	0.25
25yr3d	B2	59.7503	24.35	0.27
25yr3d	B2	60.0002	24.41	0.29
25yr3d	B2	60.2501	24.51	0.32
25yr3d	B2	60.5011	24.62	0.37
25yr3d	B2	60.7510	24.72	0.42
25yr3d	B2	61.0019	24.81	0.48
25yr3d	B2	61.2507	24.90	0.56
25yr3d	B2	61.5007	24.97	0.65
25yr3d	B2	61.7504	25.04	0.75
25yr3d	B2	62.0004	25.10	0.85
25yr3d	B2	62.2533	25.15	0.97
25yr3d	B2	62.5031	25.20	1.09
25yr3d	B2	62.7504	25.24	1.21

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	63.0028	25.28	1.35
25yr3d	B2	63.2555	25.32	1.49
25yr3d	B2	63.5061	25.35	1.63
25yr3d	B2	63.7556	25.39	1.78
25yr3d	B2	64.0051	25.42	1.93
25yr3d	B2	64.2527	25.45	2.09
25yr3d	B2	64.5019	25.48	2.25
25yr3d	B2	64.7519	25.51	2.42
25yr3d	B2	65.0019	25.53	2.59
25yr3d	B2	65.2552	25.56	2.76
25yr3d	B2	65.5011	25.58	2.94
25yr3d	B2	65.7502	25.60	3.12
25yr3d	B2	66.0002	25.62	3.30
25yr3d	B2	66.2544	25.65	3.49
25yr3d	B2	66.5002	25.66	3.67
25yr3d	B2	66.7519	25.68	3.87
25yr3d	B2	67.0019	25.70	4.06
25yr3d	B2	67.2519	25.72	4.26
25yr3d	B2	67.5019	25.74	4.45
25yr3d	B2	67.7519	25.76	4.65
25yr3d	B2	68.0019	25.77	4.86
25yr3d	B2	68.2519	25.79	5.06
25yr3d	B2	68.5019	25.80	5.27
25yr3d	B2	68.7519	25.82	5.48
25yr3d	B2	69.0019	25.83	5.69
25yr3d	B2	69.2519	25.84	5.90
25yr3d	B2	69.5019	25.86	6.11
25yr3d	B2	69.7519	25.87	6.33
25yr3d	B2	70.0019	25.88	6.54
25yr3d	B2	70.2519	25.89	6.76
25yr3d	B2	70.5019	25.90	6.98
25yr3d	B2	70.7519	25.91	7.19
25yr3d	B2	71.0019	25.92	7.41
25yr3d	B2	71.2519	25.93	7.63
25yr3d	B2	71.5019	25.93	7.85
25yr3d	B2	71.7519	25.94	8.08
25yr3d	B2	72.0019	25.95	8.30
25yr3d	B2	76.0019	25.97	11.81
25yr3d	B2	80.0019	25.97	15.11
25yr3d	B2	84.0019	25.96	18.34
25yr3d	B2	88.0019	25.95	21.65
25yr3d	B2	92.0019	25.93	25.03

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	96.0019	25.90	28.45
25yr3d	B2	100.0019	25.87	31.88
25yr3d	B2	104.0019	25.83	35.40
25yr3d	B2	108.0019	25.79	39.04
25yr3d	B2	112.0019	25.74	42.70
25yr3d	B2	116.0019	25.69	46.26
25yr3d	B2	120.0019	25.64	49.67
25yr3d	B2	124.0019	25.59	52.93
25yr3d	B2	128.0019	25.55	56.05
25yr3d	B2	132.0019	25.50	59.03
25yr3d	B2	136.0019	25.45	61.86
25yr3d	B2	140.0019	25.40	64.55
25yr3d	B2	144.0019	25.35	67.10
25yr3d	B2	148.0019	25.31	69.53
25yr3d	B2	152.0019	25.27	71.84
25yr3d	B2	156.0019	25.23	74.06
25yr3d	B2	160.0019	25.19	76.17
25yr3d	B2	164.0019	25.16	78.20
25yr3d	B2	168.0019	25.13	80.14
25yr3d	B2	172.0019	25.10	82.01
25yr3d	B2	176.0019	25.07	83.80
25yr3d	B2	180.0019	25.04	85.52
25yr3d	B2	184.0019	25.02	87.17
25yr3d	B2	188.0019	24.99	88.77
25yr3d	B2	192.0019	24.97	90.30
25yr3d	B2	196.0019	24.94	91.78
25yr3d	B2	200.0019	24.92	93.20
25yr3d	B2	204.0019	24.90	94.57
25yr3d	B2	208.0019	24.88	95.90
25yr3d	B2	212.0019	24.86	97.18
25yr3d	B2	216.0019	24.84	98.41
25yr3d	B2	220.0019	24.82	99.61
25yr3d	B2	224.0019	24.80	100.76
25yr3d	B2	228.0019	24.78	101.88
25yr3d	B2	232.0019	24.76	102.96
25yr3d	B2	236.0019	24.75	104.00
25yr3d	B2	240.0019	24.73	105.01
25yr3d	B2	244.0019	24.72	105.99
25yr3d	B2	248.0019	24.70	106.93
25yr3d	B2	252.0019	24.69	107.85
25yr3d	B2	256.0019	24.67	108.74
25yr3d	B2	260.0019	24.66	109.60

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	264.0019	24.64	110.43
25yr3d	B2	268.0019	24.63	111.24
25yr3d	B2	272.0019	24.62	112.02
25yr3d	B2	276.0019	24.61	112.78
25yr3d	B2	280.0019	24.59	113.52
25yr3d	B2	284.0019	24.58	114.23
25yr3d	B2	288.0019	24.57	114.93
25yr3d	B2	292.0019	24.56	115.60
25yr3d	B2	296.0019	24.55	116.26
25yr3d	B2	300.0019	24.54	116.89
25yr3d	B2	304.0019	24.53	117.51
25yr3d	B2	308.0019	24.52	118.11
25yr3d	B2	312.0019	24.51	118.69
25yr3d	B2	316.0019	24.50	119.25
25yr3d	B2	320.0019	24.49	119.80
25yr3d	B2	324.0019	24.48	120.34
25yr3d	B2	328.0019	24.47	120.86
25yr3d	B2	332.0019	24.46	121.36
25yr3d	B2	336.0019	24.45	121.85
25yr3d	B2	340.0019	24.44	122.32
25yr3d	B2	344.0019	24.44	122.79
25yr3d	B2	348.0019	24.43	123.24
25yr3d	B2	352.0019	24.42	123.67
25yr3d	B2	356.0019	24.41	124.10
25yr3d	B2	360.0019	24.40	124.51

TOTAL SITE

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	25.78	8.6%	0.00	0.00	0.00	25.78
LAKE BANK @ CE + 1	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE TOB	2.28	0.8%	2.28	0.00	0.00	0.00
DRY DETENTION	1.65	0.6%	1.65	0.00	0.00	0.00
PRESERVE	37.83	12.7%	37.83	0.00	0.00	0.00
ROW	32.59	10.9%	13.04	19.55	0.00	0.00
SF	63.92	21.4%	25.57	19.18	19.18	0.00
MF	25.26	8.5%	5.05	10.10	10.10	0.00
GOLF COURSE	96.62	32.3%	93.73	2.90	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	0.5%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	2.3%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**298.86**	**100%**	**185.38**	**55.01**	**32.56**	**25.92**

TOTAL PERVIOUS AREA	185.38	62.0%	
TOTAL IMPERVIOUS AREA	113.49	38.0%	*(INCLUDES WATER SURFACE AREA)*

TOTAL SITE

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:

=	(Average Developed Grade)-(Wet Season Water Table)		
=	27.50	-	24.00
=	3.50	ft.	

Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:

S =	(% of Pervious Area) * (Soil Storage Depth)			
S =	62.0%	x	6.57	in.
S =	**4.07**	**in.**		

Curve Number:

Cn =	1000/(S + 10)
Cn =	**71**

III. SURFACE STORAGE

Basin Area =	298.86	ac.			
Bldg. Coverage Area =	32.56	ac.			
Impervious =	55.01				
Lake Area (at Control)	25.92	ac.	***EL. =***	***24.00***	***(NGVD)***
Preserve Area (at control)	37.83	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less Preserved)=	147.55	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**25.92**	**0.00**	**0.00**	**0.00**	**25.92**	**0.00**
25.00	**28.06**	**0.00**	**0.00**	**0.00**	**28.06**	**26.99**
26.50	**30.34**	**0.00**	**0.00**	**0.00**	**30.34**	**70.79**
27.00	**32.62**	**7.57**	**29.51**	**27.51**	**97.20**	**102.68**
27.50	**32.93**	**15.13**	**59.02**	**49.51**	**156.59**	**166.12**
28.50	**32.93**	**18.92**	**132.79**	**55.01**	**239.65**	**364.25**

IV. WATER QUALITY DETENTION VOLUME

TOTAL SITE

Total Area =	298.86	ac.
Bldg. Coverage =	32.56	ac.
Water Surface Area =	25.92	ac.
Pervious Area =	185.38	ac.

First one (1) Inch of runoff over basin:

= 1" x 298.86 ac. x1'/12" = **24.91** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	240.38	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	55.01	ac.
% Impervious = Impervious Area / Site Area	22.9%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**13.01**	**ac-ft**
Required Water Quality Detention Volume	**24.91**	**ac-ft**
Additional 50% Water Quality Detention Volume	**12.45**	**ac-ft**
Total Required Water Quality Detention Volume	**37.36**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**37.36**	**ac-ft**

Between El. 24.00 and 25.36

WCS Stage-Storage Table

Stage	Site Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	25.92	0.00
25.00	28.06	26.99
26.50	30.34	70.79
27.00	97.20	102.68
27.50	156.59	166.12
28.50	239.65	364.25

Water Quality Min. Weir Elevation = **25.36** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event:

= 55.0 CSM x WCS Area x 1 SM/640 ac.

= **25.68** **cfs**

METHOD OF DETERMINATION:		
Orange River Watershed (Approved ERP Appl # 060503-10)	55.0	CSM

Basin B2

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 1	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE TOB	0.00	0.0%	0.00	0.00	0.00	0.00
DRY DETENTION	0.44	1.4%	0.44	0.00	0.00	0.00
PRESERVE	26.62	82.1%	26.62	0.00	0.00	0.00
ROW	0.00	0.0%	0.00	0.00	0.00	0.00
SF	0.00	0.0%	0.00	0.00	0.00	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	5.38	16.6%	5.22	0.16	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**32.44**	**100%**	**32.28**	**0.16**	**0.00**	**0.00**

TOTAL PERVIOUS AREA	32.28	99.5%
TOTAL IMPERVIOUS AREA	0.16	0.5%

Basin B2

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** in.

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 99.5% x 6.57 in.
S = 6.53 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 60

III. SURFACE STORAGE

Basin Area =	32.44	ac.			
Bldg. Coverage Area =	0.00	ac.			
Dry Detention TOS (at CE + 1)	0.44	ac.	*EL. =*	***25.00***	***(NGVD)***
Dry Detention TOB (at CE + 2)	0.54	ac.	*EL. =*	***26.00***	***(NGVD)***
Golf Course	5.22	ac.			
Preserve Area (at control)	26.62	ac.	*EL. =*	***24.00***	***(NGVD)***
Pervious Area =	32.28	ac.			

Stage-Storage Table

Stage (NGVD)	Dry Detention ac	Preserve Area ac	Golf Course ac	Total Area ac	Total Volume ac-ft
24.00	**0.00**	**26.62**	**0.00**	**26.62**	**0.00**
25.00	**0.44**	**26.62**	**0.00**	**27.06**	**26.84**
26.50	**0.44**	**26.62**	**1.04**	**28.10**	**68.21**
27.00	**0.44**	**26.62**	**2.09**	**29.15**	**82.53**
27.50	**0.44**	**26.62**	**2.61**	**29.67**	**97.23**

IV. WATER QUALITY DETENTION VOLUME
The permitted 2.97 ac-ft of water quality was provided in the existing condition and no modification to the basin is proposed.

V. ALLOWABLE DISCHARGE

BASIN	2	
Contributing Flows		
Basin	1	49.33
Basin	2	32.44
Basin	3	16.53
Basin	4	18.34
Basin	5	131.83
Basin	6	18.20
Total Area =		266.68 ac.

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event:

= 55.0 CSM x WCS Area x 1 SM/640 ac.

= **22.92 cfs**

METHOD OF DETERMINATION:		
55 CSM allowed in Orange River Watershed & Approved ERP Appl # 994015-2	55.0	CSM

VI. WEIR DESIGN

Total Basin

Design Control Structure to Discharge 1/2" in 24 Hours.

Basin Area:	=		=	**266.68 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**11.11 Ac-FT**
½" Discharge Rate:	=		=	**5.72 CFS**
Water Quality Stage:	=		=	**24.56 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**22.22 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation)	=	**24.23 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**24.39 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)	=	**0.39 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 5.72 CFS

Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 7.39 Length of weir **A 1.50 ft wide rectangular weir was used to meet the maximum discharge rate**

Basin B3L3

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.16	7.0%	0.00	0.00	0.00	1.16
LAKE BANK @ CE + 1	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE TOB	0.14	0.8%	0.14	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	2.62	15.8%	1.05	1.57	0.00	0.00
SF	5.83	35.3%	2.33	1.75	1.75	0.00
MF	3.25	19.7%	0.65	1.30	1.30	0.00
GOLF COURSE	3.26	19.7%	3.16	0.10	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.53**	**100%**	**7.61**	**4.72**	**3.05**	**1.16**

TOTAL PERVIOUS AREA	7.61	46.0%	
TOTAL IMPERVIOUS AREA	8.93	54.0%	*(INCLUDES WATER SURFACE AREA)*

Basin B3L3

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:

= (Average Developed Grade)-(Wet Season Water Table)

= 27.50 - 24.00

= 3.50 ft.

Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:

S = (% of Pervious Area) * (Soil Storage Depth)

S = 46.0% x 6.57 in.

S = 3.02 in.

Curve Number:

Cn = 1000/(S + 10)

Cn = 77

III. SURFACE STORAGE

Basin Area =	16.53	ac.			
Bldg. Coverage Area =	3.05	ac.			
Lake Area (at Control)	1.16	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.30	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.44	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.57	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	3.16	ac.			
Pervious Area (less bank & GC) =	4.03	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.16**	**0.00**	**0.00**	**0.00**	**1.16**	**0.00**
25.00	**1.30**	**0.00**	**0.00**	**0.00**	**1.30**	**1.23**
26.50	**1.44**	**0.00**	**0.00**	**0.00**	**1.44**	**3.28**
27.00	**1.57**	**0.63**	**0.81**	**2.36**	**5.37**	**4.98**
27.50	**1.57**	**1.26**	**1.61**	**4.25**	**8.70**	**8.50**
28.50	**1.57**	**1.58**	**3.63**	**4.72**	**11.50**	**18.60**

IV. WATER QUALITY DETENTION VOLUME

BASIN	**3**		
Total Area =		16.53	ac.
Bldg. Coverage =		3.05	ac.
Water Surface Area =		1.16	ac.
Pervious Area =		7.61	ac.

First one (1) Inch of runoff over basin:

= 1" x 16.53 ac. x1'/12" = **1.38** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	12.32	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	4.72	ac.
% Impervious = Impervious Area / Site Area	38.3%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.23**	**ac-ft**
Required Water Quality Detention Volume	**1.38**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.69**	**ac-ft**
Total Required Water Quality Detention Volume	**2.07**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.07**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.16	0.00
25.00	1.30	1.23
26.50	1.44	3.28
27.00	1.57	4.03
27.50	1.57	4.82
28.50	1.57	6.39

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.16 ac + 1.44 ac (Lake 3 and Lake 4) = 2.60 ac, the WQ is provided at 25.76 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin (The required bleed down for B4L4 is accounted for in this basin)

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**34.88 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**1.45 Ac-FT**
½" Discharge Rate:	=		=	**0.75 CFS**
Water Quality Stage:	=		=	**25.76 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)=		**3.12 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation) =		**24.07 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2 =		**24.92 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)=		**0.92 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 0.75 **CFS**

Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 0.27 Length of weir **A 0.27 ft wide rectangular weir was used to meet the maximum bleeder rate**

Basin B4L4

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.44	7.9%	0.00	0.00	0.00	1.44
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	9.88	53.9%	3.95	5.93	0.00	0.00
SF	1.42	7.7%	0.57	0.43	0.43	0.00
MF	2.34	12.8%	0.47	0.94	0.94	0.00
GOLF COURSE	2.87	15.6%	2.78	0.09	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**18.34**	**100%**	**8.16**	**7.38**	**1.36**	**1.44**

TOTAL PERVIOUS AREA	8.16	44.5%	
TOTAL IMPERVIOUS AREA	10.18	55.5%	*(INCLUDES WATER SURFACE AREA)*

Basin B4L4

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:

= (Average Developed Grade)-(Wet Season Water Table)

= 27.50 - 24.00

= 3.50 ft.

Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:

S = (% of Pervious Area) * (Soil Storage Depth)

S = 44.5% x 6.57 in.

S = 2.92 in.

Curve Number:

Cn = 1000/(S + 10)

Cn = 77

III. SURFACE STORAGE

Basin Area =	18.34	ac.			
Bldg. Coverage Area =	1.36	ac.			
Lake Area (at Control)	1.44	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.57	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.70	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.83	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	0.00	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less bank & GC) =	4.99	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.44**	**0.00**	**0.00**	**0.00**	**1.44**	**0.00**
25.00	**1.57**	**0.00**	**0.00**	**0.00**	**1.57**	**1.51**
26.50	**1.70**	**0.00**	**0.00**	**0.00**	**1.70**	**3.96**
27.00	**1.83**	**0.56**	**1.00**	**3.69**	**7.07**	**6.15**
27.50	**1.83**	**1.11**	**2.00**	**6.64**	**11.58**	**10.82**
28.50	**1.83**	**1.39**	**4.49**	**7.38**	**15.09**	**24.15**

IV. WATER QUALITY DETENTION VOLUME

BASIN	**4**	
Total Area =	18.34	ac.
Bldg. Coverage =	1.36	ac.
Water Surface Area =	1.44	ac.
Pervious Area =	8.16	ac.

First one (1) Inch of runoff over basin:

= 1" x 18.34 ac. x1'/12" = **1.53** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	15.54	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	7.38	ac.
% Impervious = Impervious Area / Site Area	47.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.67**	**ac-ft**
Required Water Quality Detention Volume	**1.67**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.84**	**ac-ft**
Total Required Water Quality Detention Volume	**2.51**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.51**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.44	0.00
25.00	1.57	1.51
26.50	1.70	3.96
27.00	1.83	4.84
27.50	1.83	5.76
28.50	1.83	7.59

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.16 ac + 1.44 ac (Lake 3 and Lake 4) = 2.60 ac, the WQ is provided at 25.76 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

The Existing weir, as approved in ERP 36-03765-P (App 990415-2) was utilized.

Basin B5L5

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.61	15.7%	0.00	0.00	0.00	2.61
LAKE BANK @ CE + 1	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE TOB	0.23	1.4%	0.23	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	0.66	4.0%	0.26	0.40	0.00	0.00
SF	2	12.1%	0.80	0.60	0.60	0.00
MF	3.99	24.1%	0.80	1.60	1.60	0.00
GOLF COURSE	6.62	39.9%	6.42	0.20	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.58**	**100%**	**8.98**	**2.79**	**2.20**	**2.61**

TOTAL PERVIOUS AREA	8.98	54.2%	
TOTAL IMPERVIOUS AREA	7.60	45.8%	*(INCLUDES WATER SURFACE AREA)*

Basin B5L5

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 54.2% x 6.57 in.
S = 3.56 In.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	16.58	ac.			
Bldg. Coverage Area =	2.20	ac.			
Lake Area (at Control)	2.61	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.84	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.08	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	3.31	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	6.62	ac.			
Pervious Area (less bank & GC) =	1.86	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.61**	**0.00**	**0.00**	**0.00**	**2.61**	**0.00**
25.50	**2.84**	**0.00**	**0.00**	**0.00**	**2.84**	**2.73**
27.00	**3.08**	**0.00**	**0.00**	**0.00**	**3.08**	**7.17**
27.50	**3.31**	**1.32**	**0.37**	**1.40**	**6.40**	**9.54**
28.00	**3.31**	**2.65**	**0.74**	**2.51**	**9.22**	**13.44**
29.00	**3.31**	**3.31**	**1.68**	**2.79**	**11.09**	**23.60**

Lake B5L6

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.23	9.6%	0.00	0.00	0.00	1.23
LAKE BANK @ CE + 1	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE TOB	0.16	1.2%	0.16	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	1.40	10.9%	0.56	0.84	0.00	0.00
SF	5.62	43.7%	2.25	1.69	1.69	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	4.14	32.2%	4.01	0.12	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**12.86**	**100%**	**7.29**	**2.65**	**1.69**	**1.23**

TOTAL PERVIOUS AREA	7.29	56.7%	
TOTAL IMPERVIOUS AREA	5.57	43.3%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L6

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 56.7% x 6.57 in.
S = 3.72 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 73

III. SURFACE STORAGE

Basin Area =	12.86	ac.			
Bldg. Coverage Area =	1.69	ac.			
Lake Area (at Control)	1.23	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	0.16	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	0.16	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	1.39	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.01	ac.			
Pervious Area (less bank & GC) =	2.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.23**	**0.00**	**0.00**	**0.00**	**1.23**	**0.00**
25.50	**1.31**	**0.00**	**0.00**	**0.00**	**1.31**	**1.27**
27.00	**1.39**	**0.00**	**0.00**	**0.00**	**1.39**	**3.29**
27.50	**1.39**	**0.80**	**0.56**	**1.33**	**4.08**	**4.66**
28.00	**1.39**	**1.61**	**1.12**	**2.39**	**6.50**	**7.30**
29.00	**1.39**	**2.01**	**2.53**	**2.65**	**8.57**	**14.84**

Lake B5L7

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	9.70	20.4%	0.00	0.00	0.00	9.70
LAKE BANK @ CE + 1	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE TOB	0.71	1.5%	0.71	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	3.50	7.4%	1.40	2.10	0.00	0.00
SF	7.01	14.8%	2.80	2.10	2.10	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	18.16	38.2%	17.61	0.54	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	14.7%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**47.51**	**100%**	**25.36**	**7.55**	**4.90**	**9.70**

TOTAL PERVIOUS AREA	25.36	53.4%	
TOTAL IMPERVIOUS AREA	22.15	46.6%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L7

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.4% x 6.57 in.
S = 3.50 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	47.51	ac.			
Bldg. Coverage Area =	4.90	ac.			
Lake Area (at Control)	9.70	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	10.41	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	11.13	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	11.84	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	18.16	ac.			
Pervious Area (less bank & GC) =	5.60	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	9.70	0.00	0.00	0.00	9.70	0.00
25.50	10.41	0.00	0.00	0.00	10.41	10.06
27.00	11.13	0.00	0.00	0.00	11.13	26.21
27.50	11.84	3.63	1.12	3.77	20.37	34.09
28.00	11.84	7.26	2.24	6.79	28.14	46.21
29.00	11.84	9.08	5.04	7.55	33.51	77.04

Lake B5L9

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.49	11.4%	0.00	0.00	0.00	2.49
LAKE BANK @ CE + 1	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE TOB	0.28	1.3%	0.28	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.60	11.9%	1.04	1.56	0.00	0.00
SF	6.92	31.6%	2.77	2.08	2.08	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	9.06	41.3%	8.79	0.27	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**21.92**	**100%**	**13.45**	**3.91**	**2.08**	**2.49**

TOTAL PERVIOUS AREA	13.45	61.3%	
TOTAL IMPERVIOUS AREA	8.47	38.7%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L9

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 61.3% x 6.57 in.
S = 4.03 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 71

III. SURFACE STORAGE

Basin Area =	21.92	ac.			
Bldg. Coverage Area =	2.08	ac.			
Lake Area (at Control)	2.49	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.77	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.05	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	2.77	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	9.06	ac.			
Pervious Area (less bank & GC) =	3.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.49**	**0.00**	**0.00**	**0.00**	**2.49**	**0.00**
25.50	**2.77**	**0.00**	**0.00**	**0.00**	**2.77**	**2.63**
27.00	**3.05**	**0.00**	**0.00**	**0.00**	**3.05**	**7.00**
27.50	**3.05**	**1.81**	**0.76**	**1.95**	**7.58**	**9.66**
28.00	**3.05**	**3.63**	**1.52**	**3.52**	**11.72**	**14.49**
29.00	**3.05**	**4.53**	**3.43**	**3.91**	**14.92**	**27.81**

Lake B5L10

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.42	9.3%	0.00	0.00	0.00	1.42
LAKE BANK @ CE + 1	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE TOB	0.17	1.1%	0.17	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	0.61	4.0%	0.24	0.37	0.00	0.00
SF	1.37	9.0%	0.55	0.41	0.41	0.00
MF	4.10	26.9%	0.82	1.64	1.64	0.00
GOLF COURSE	5.85	38.5%	5.68	0.18	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	9.0%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**15.22**	**100%**	**8.06**	**3.07**	**2.53**	**1.56**

TOTAL PERVIOUS AREA	8.06	53.0%	
TOTAL IMPERVIOUS AREA	7.16	47.0%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L10

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.0% x 6.57 in.
S = 3.48 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	15.22	ac.			
Bldg. Coverage Area =	2.53	ac.			
Lake Area (at Control)	1.56	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.72	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.89	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.92	ac.	***EL. =***	***26.50***	***(NGVD)***
Golf Course	5.68	ac.			
Pervious Area (less bank & GC) =	1.89	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.56**	**0.00**	**0.00**	**0.00**	**1.56**	**0.00**
25.50	**1.72**	**0.00**	**0.00**	**0.00**	**1.72**	**1.64**
27.00	**1.89**	**0.00**	**0.00**	**0.00**	**1.89**	**4.35**
27.50	**1.92**	**1.14**	**0.38**	**1.54**	**4.96**	**6.06**
28.00	**1.92**	**2.27**	**0.75**	**2.76**	**7.71**	**9.23**
29.00	**1.92**	**2.84**	**1.70**	**3.07**	**9.52**	**17.84**

Lake B5L11

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.05	5.9%	0.00	0.00	0.00	1.05
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.94	16.6%	1.18	1.76	0.00	0.00
SF	6.61	37.3%	2.64	1.98	1.98	0.00
MF	2.20	12.4%	0.44	0.88	0.88	0.00
GOLF COURSE	4.55	25.6%	4.41	0.14	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**17.74**	**100%**	**9.06**	**4.76**	**2.86**	**1.05**

TOTAL PERVIOUS AREA	9.06	51.1%	
TOTAL IMPERVIOUS AREA	8.68	48.9%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L11

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 51.1% x 6.57 in.
S = 3.35 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 75

III. SURFACE STORAGE

Basin Area =	17.74	ac.			
Bldg. Coverage Area =	2.86	ac.			
Lake Area (at Control)	1.05	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.18	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.31	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.44	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.41	ac.			
Pervious Area (less bank & GC) =	4.26	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.05**	**0.00**	**0.00**	**0.00**	**1.05**	**0.00**
25.50	**1.18**	**0.00**	**0.00**	**0.00**	**1.18**	**1.12**
27.00	**1.31**	**0.00**	**0.00**	**0.00**	**1.31**	**2.98**
27.50	**1.44**	**0.88**	**0.85**	**2.38**	**5.56**	**4.70**
28.00	**1.44**	**1.76**	**1.70**	**4.29**	**9.20**	**8.39**
29.00	**1.44**	**2.21**	**3.83**	**4.76**	**12.25**	**19.11**

IV. WATER QUALITY DETENTION VOLUME

BASIN **5**

Total Area =	131.83	ac.
Bldg. Coverage =	16.25	ac.
Water Surface Area =	18.64	ac.
Pervious Area =	72.21	ac.

First one (1) inch of runoff over basin:

= 1" x 131.83 ac. x1'/12" = **10.99** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	96.94	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	24.73	ac.
% Impervious = Impervious Area / Site Area	25.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**6.02**	**ac-ft**
Required Water Quality Detention Volume	**10.99**	**ac-ft**
Additional 50% Water Quality Detention Volume	**5.49**	**ac-ft**

Beth Stacy Offsite Imperviousness

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	0.22	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	0.22	ac.
% Impervious = Impervious Area / Site Area	100.0%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**0.05**	**ac-ft**
Total Required Water Quality Detention Volume	**16.52**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**16.52**	**ac-ft**

Between El. 24.00 and 25.35

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Lake Storage Volume
(NGVD)	ac	ac-ft
24.50	18.64	0.00
25.50	20.24	19.44
27.00	21.85	51.01
27.50	22.95	62.20
28.00	22.95	73.68
29.00	22.95	96.63

23 ac-ft of total wq volume is provided

Water Quality Min. Weir Elevation = **25.35** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**131.83 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**5.49 Ac-FT**
½" Discharge Rate:	=		=	**2.83 CFS**
Water Quality Stage:	=		=	**25.67 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**17.51 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	eatment Vol. After 24 Hr. / Total Basin Stora (+ Control Elevation)	=	**24.84 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**25.26 Ft-NAVD**
Average Height During Bleed Down:	=	ıve. Elev. During Bleed Down - Control Elev	=	**0.76 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 2.83 **CFS**

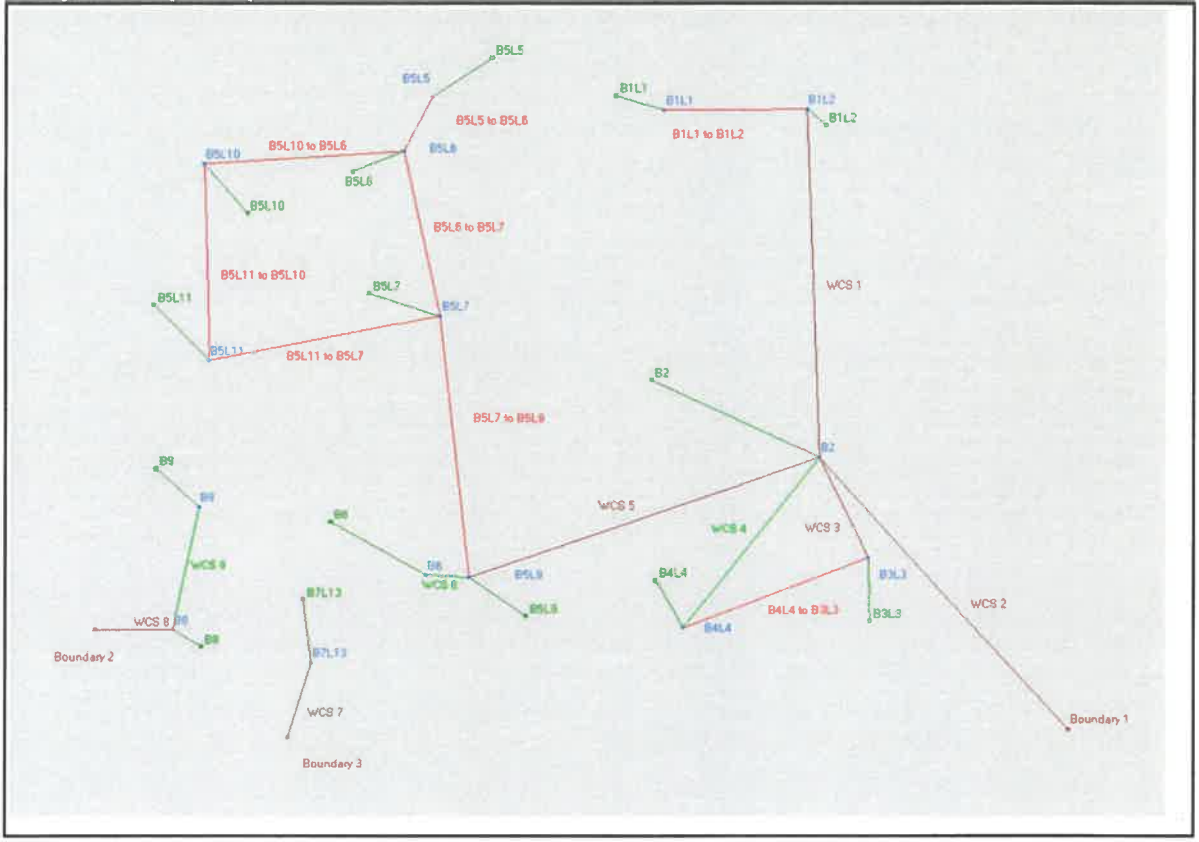
Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 1.37 Length of weir **A 1.33 ft wide rectangular weir was used to meet the maximum bleeder rate**

Background Image: Background



Impervious: Scenario 1 [Set]

Land Cover Zone	% Impervious	% DCIA	% Direct	Ia/S [dec]	Ia Impervious [in]	Ia Pervious [in]
Impervious & Pervious	0.00	0.00	0.00	0.00	0.000	0.000

Curve Number: Scenario 1 [Set]

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 60	60.0
Impervious & Pervious	Calculated CN 63	63.0
Impervious & Pervious	Calculated CN 64	64.0
Impervious & Pervious	Calculated CN 70	70.0
Impervious & Pervious	Calculated CN 71	71.0
Impervious & Pervious	Calculated CN 72	72.0

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 73	73.0
Impervious & Pervious	Calculated CN 74	74.0
Impervious & Pervious	Calculated CN 75	75.0
Impervious & Pervious	Calculated CN 76	76.0
Impervious & Pervious	Calculated CN 77	77.0

Manual Basin: B2

Scenario: Scenario1
Node: B2
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 60.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 32.4400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
32.4400	Impervious & Pervious	Calculated CN 60	

Comment:

Manual Basin: B3L3

Scenario: Scenario1
Node: B3L3
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5300 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5300	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B4L4

Scenario: Scenario1
Node: B4L4

Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 18.3400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
18.3400	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B5L10

Scenario: Scenario1
Node: B5L10
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 15.2200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
15.2200	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L11

Scenario: Scenario1
Node: B5L11
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 17.7400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
17.7400	Impervious & Pervious	Calculated CN 75	

Comment:

Manual Basin: B5L5

Scenario: Scenario1
Node: B5L5
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5800 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5800	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L6

Scenario: Scenario1
Node: B5L6
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 12.8600 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
12.8600	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L7

Scenario: Scenario1
Node: B5L7
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 47.5100 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
47.5100	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L9

Scenario: Scenario1
Node: B5L9
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 21.9200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
21.9200	Impervious & Pervious	Calculated CN 71	

Comment:

Node: B2

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	26.6200	1159567
25.00	27.0600	1178734
26.50	28.1000	1224036
27.00	29.1500	1269774
27.50	29.6700	1292425

Comment:

Node: B3L3

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft

Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.1600	50530
25.00	1.3000	56628
26.50	1.4400	62726
27.00	5.3700	233917
27.50	8.7000	378972
28.50	11.5000	500940

Comment:

Node: B4L4

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.4400	62726
25.00	1.5700	68389
26.50	1.7000	74052
27.00	7.0700	307969
27.50	11.5800	504425
28.50	15.0900	657320

Comment:

Node: B5L10

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.5600	67954
25.50	1.7200	74923
27.00	1.8900	82328
27.50	4.9600	216058
28.00	7.7100	335848
29.00	9.5200	414691

Comment:

Node: B5L11

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.0500	45738
25.50	1.1800	51401
27.00	1.3100	57064
27.50	5.5600	242194
28.00	9.2000	400752
29.00	12.2500	533610

Comment:

Node: B5L5

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.6100	113692
25.50	2.8400	123710
27.00	3.0800	134165
27.50	6.4000	278784
28.00	9.2200	401623
29.00	11.0900	483080

Comment:

Node: B5L6

Scenario: Scenario1
Type: Stage/Area

Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.2300	53579
25.50	1.3100	57064
27.00	1.3900	60548
27.50	4.0800	177725
28.00	6.5000	283140
29.00	8.5700	373309

Comment:

Node: B5L7

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	9.7000	422532
25.50	10.4100	453460
27.00	11.1300	484823
27.50	20.3700	887317
28.00	28.1400	1225778
29.00	33.5100	1459696

Comment:

Node: B5L9

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.4900	108464
25.50	2.7700	120661
27.00	3.0500	132858

Stage [ft]	Area [ac]	Area [ft2]
27.50	7.5800	330185
28.00	11.7200	510523
29.00	14.9200	649915

Comment:

Node: Boundary 1

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 23.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	23.00
0	0	0	60.0000	24.00
0	0	0	80.0000	25.00
0	0	0	100.0000	24.50
0	0	0	115.0000	23.30
0	0	0	336.0000	23.00

Comment:

Pipe Link: B4L4 to B3L3

Scenario: Scenario1
From Node: B3L3
To Node: B4L4
Link Count: 1
Flow Direction: Both
Damping: 0.0000 ft
Length: 731.00 ft
FHWA Code: 0
Entr Loss Coef: 1
Exit Loss Coef: 1
Bend Loss Coef: 0
Bend Location: 0.00 dec
Energy Switch: Energy

Upstream		Downstream	
Invert:	21.00 ft	Invert:	21.00 ft
Manning's N:	0.0130	Manning's N:	0.0130
Geometry: Circular		Geometry: Circular	
Max Depth:	3.00 ft	Max Depth:	3.00 ft
Bottom Clip		Bottom Clip	
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000
Top Clip		Top Clip	
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L10 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.70 ft	Invert:	20.60 ft
From Node:	B5L10	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	450.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L10		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.15 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L10	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	320.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	21.00 ft	Invert:	20.50 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	430.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	1	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			

Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L5 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft
From Node:	B5L5	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	385.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L6 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.40 ft
From Node:	B5L6	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	220.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L7 to B5L9		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft

From Node:	B5L7	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L9	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.50 ft	Max Depth:	2.50 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	585.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Drop Structure Link: WCS 2		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B2	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	Boundary 1	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	65.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Sharp Crested Vertical	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	24.00 ft	Op Table:	
Control Elevation:	24.00 ft	Ref Node:	
Max Depth:	999.00 ft	Discharge Coefficients	
Max Width:	1.50 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600

Orifice Table:

Weir Comment:

Drop Structure Comment:

Drop Structure Link: WCS 3		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B3L3	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B2	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	237.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Sharp Crested Vertical	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	24.00 ft	Ref Node:	
Control Elevation:	24.00 ft	Discharge Coefficients	
Max Depth:	0.25 ft	Weir Default:	3.200
Max Width:	0.27 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component		Bottom Clip	
Weir:	2	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Horizontal		

Geometry Type:	Rectangular
Invert:	25.76 ft
Control Elevation:	25.76 ft
Max Depth:	3.00 ft
Max Width:	5.00 ft
Fillet:	0.00 ft

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	3.200
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Drop Structure Comment:

Weir Link: WCS 4

Scenario:	Scenario1
From Node:	B4L4
To Node:	B2
Link Count:	1
Flow Direction:	Both
Damping:	0.0000 ft
Weir Type:	Broad Crested Vertical
Geometry Type:	Rectangular
Invert:	26.00 ft
Control Elevation:	26.00 ft
Max Depth:	999.00 ft
Max Width:	30.00 ft
Fillet:	0.00 ft

Bottom Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Top Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	2.800
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Comment:

Simulation: 100yr3d

Scenario:	Scenario1
Run Date/Time:	9/18/2025 10:11:53 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	216.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr

Ia/S: 0.20 dec

Smp/Man Basin Rain Opt: Global

Rainfall Name: ~SFWMD-72
Rainfall Amount: 14.20 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 10yr 1d

Scenario: Scenario1
Run Date/Time: 9/23/2025 3:10:07 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec

IA Recovery Time: 12.0000 hr

Ia/S: 0.20 dec

dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FLMOD
		Rainfall Amount:	6.00 in
		Storm Duration:	24.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 25yr3d

Scenario: Scenario1
Run Date/Time: 9/23/2025 3:10:16 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:	
Unit Hydrograph Folder:	

Lookup Tables

Boundary Stage Set:	
Extern Hydrograph Set:	
Curve Number Set:	Scenario 1
Green-Ampt Set:	
Vertical Layers Set:	
Impervious Set:	Scenario 1

Tolerances & Options

Time Marching:	SAOR
Max Iterations:	6
Over-Relax Weight Fact:	0.5 dec
dZ Tolerance:	0.0010 ft
Max dZ:	1.0000 ft
Link Optimizer Tol:	0.0001 ft

IA Recovery Time:	24.0000 hr
Ia/S:	0.20 dec
Smp/Man Basin Rain Opt:	Global
Rainfall Name:	~SFWMD-72
Rainfall Amount:	10.20 in
Storm Duration:	72.0000 hr
Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (1D):	100 ft2
Energy Switch (1D):	Energy

Comment:

Simulation: 5yr1d

Scenario:	Scenario1
Run Date/Time:	9/23/2025 3:10:43 PM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 12.0000 hr

Ia/S: 0.20 dec

Smp/Man Basin Rain Opt: Global

Rainfall Name: ~FLMOD
Rainfall Amount: 5.20 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	10yr 1d	B2	24.84	42.0862
Scenario1	10yr 1d	B3L3	26.07	13.1869
Scenario1	10yr 1d	B4L4	26.09	13.2166
Scenario1	10yr 1d	B5L10	25.86	13.0674
Scenario1	10yr 1d	B5L11	25.87	12.8033
Scenario1	10yr 1d	B5L5	25.73	15.8862
Scenario1	10yr 1d	B5L6	25.72	24.0696
Scenario1	10yr 1d	B5L7	25.71	24.1362
Scenario1	10yr 1d	B5L9	25.64	24.5112
Scenario1	25yr3d	B2	25.98	74.1436
Scenario1	25yr3d	B3L3	26.91	60.4359
Scenario1	25yr3d	B4L4	26.75	60.2153
Scenario1	25yr3d	B5L10	27.32	61.3671
Scenario1	25yr3d	B5L11	27.32	60.8888
Scenario1	25yr3d	B5L5	27.19	64.9794
Scenario1	25yr3d	B5L6	27.16	64.4186
Scenario1	25yr3d	B5L7	27.08	72.0602
Scenario1	25yr3d	B5L9	26.77	72.7019
Scenario1	5yr1d	B2	24.66	50.1783
Scenario1	5yr1d	B3L3	25.94	15.5401
Scenario1	5yr1d	B4L4	25.96	15.5473
Scenario1	5yr1d	B5L10	25.54	13.0172
Scenario1	5yr1d	B5L11	25.54	12.8222
Scenario1	5yr1d	B5L5	25.45	15.5878
Scenario1	5yr1d	B5L6	25.44	24.0950
Scenario1	5yr1d	B5L7	25.44	24.1700
Scenario1	5yr1d	B5L9	25.40	24.4366

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	100yr3d	B2	27.44	216.0017
Scenario1	100yr3d	B3L3	27.44	216.0017
Scenario1	100yr3d	B4L4	27.44	216.0017
Scenario1	100yr3d	B5L10	27.94	64.2212
Scenario1	100yr3d	B5L11	27.93	64.2212
Scenario1	100yr3d	B5L5	27.95	72.0934
Scenario1	100yr3d	B5L6	27.92	72.1017
Scenario1	100yr3d	B5L7	27.86	72.2184
Scenario1	100yr3d	B5L9	27.59	80.0184

Sim	Link Name	Maximum Flow Rate [cfs]	Time to Maximum Flow Rate [hrs]
25yr3d	WCS 2	11.12	109.5011

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	0.0000	24.00	0.00
25yr3d	B2	0.2525	24.00	0.00
25yr3d	B2	0.5010	24.00	0.00
25yr3d	B2	0.7552	24.00	0.00
25yr3d	B2	1.0052	24.00	0.00
25yr3d	B2	1.2552	24.00	0.00
25yr3d	B2	1.5052	24.00	0.00
25yr3d	B2	1.7552	24.00	0.00
25yr3d	B2	2.0052	24.00	0.00
25yr3d	B2	2.2552	24.00	0.00
25yr3d	B2	2.5052	24.00	0.00
25yr3d	B2	2.7552	24.00	0.00
25yr3d	B2	3.0052	24.00	0.00
25yr3d	B2	3.2552	24.00	0.00
25yr3d	B2	3.5052	24.00	0.00
25yr3d	B2	3.7552	24.00	0.00
25yr3d	B2	4.0052	24.00	0.00
25yr3d	B2	4.2552	24.00	0.00
25yr3d	B2	4.5052	24.00	0.00
25yr3d	B2	4.7552	24.00	0.00
25yr3d	B2	5.0052	24.00	0.00
25yr3d	B2	5.2552	24.00	0.00
25yr3d	B2	5.5052	24.00	0.00
25yr3d	B2	5.7552	24.00	0.00
25yr3d	B2	6.0052	24.00	0.00
25yr3d	B2	6.2552	24.00	0.00
25yr3d	B2	6.5052	24.00	0.00
25yr3d	B2	6.7552	24.00	0.00
25yr3d	B2	7.0052	24.00	0.00
25yr3d	B2	7.2552	24.00	0.00
25yr3d	B2	7.5052	24.00	0.00
25yr3d	B2	7.7552	24.00	0.00
25yr3d	B2	8.0052	24.00	0.00
25yr3d	B2	8.2552	24.00	0.00
25yr3d	B2	8.5052	24.00	0.00
25yr3d	B2	8.7552	24.00	0.00
25yr3d	B2	9.0052	24.00	0.00
25yr3d	B2	9.2552	24.00	0.00
25yr3d	B2	9.5052	24.00	0.00
25yr3d	B2	9.7552	24.00	0.00
25yr3d	B2	10.0052	24.00	0.00
25yr3d	B2	10.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	10.5052	24.00	0.00
25yr3d	B2	10.7552	24.00	0.00
25yr3d	B2	11.0052	24.00	0.00
25yr3d	B2	11.2552	24.00	0.00
25yr3d	B2	11.5052	24.00	0.00
25yr3d	B2	11.7552	24.00	0.00
25yr3d	B2	12.0052	24.00	0.00
25yr3d	B2	12.2552	24.00	0.00
25yr3d	B2	12.5052	24.00	0.00
25yr3d	B2	12.7552	24.00	0.00
25yr3d	B2	13.0052	24.00	0.00
25yr3d	B2	13.2552	24.00	0.00
25yr3d	B2	13.5052	24.00	0.00
25yr3d	B2	13.7552	24.00	0.00
25yr3d	B2	14.0052	24.00	0.00
25yr3d	B2	14.2552	24.00	0.00
25yr3d	B2	14.5052	24.00	0.00
25yr3d	B2	14.7552	24.00	0.00
25yr3d	B2	15.0052	24.00	0.00
25yr3d	B2	15.2552	24.00	0.00
25yr3d	B2	15.5052	24.00	0.00
25yr3d	B2	15.7552	24.00	0.00
25yr3d	B2	16.0052	24.00	0.00
25yr3d	B2	16.2552	24.00	0.00
25yr3d	B2	16.5052	24.00	0.00
25yr3d	B2	16.7552	24.00	0.00
25yr3d	B2	17.0052	24.00	0.00
25yr3d	B2	17.2552	24.00	0.00
25yr3d	B2	17.5052	24.00	0.00
25yr3d	B2	17.7552	24.00	0.00
25yr3d	B2	18.0052	24.00	0.00
25yr3d	B2	18.2552	24.00	0.00
25yr3d	B2	18.5052	24.00	0.00
25yr3d	B2	18.7552	24.00	0.00
25yr3d	B2	19.0052	24.00	0.00
25yr3d	B2	19.2552	24.00	0.00
25yr3d	B2	19.5052	24.00	0.00
25yr3d	B2	19.7552	24.00	0.00
25yr3d	B2	20.0052	24.00	0.00
25yr3d	B2	20.2552	24.00	0.00
25yr3d	B2	20.5052	24.00	0.00
25yr3d	B2	20.7552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	21.0052	24.00	0.00
25yr3d	B2	21.2552	24.00	0.00
25yr3d	B2	21.5052	24.00	0.00
25yr3d	B2	21.7552	24.00	0.00
25yr3d	B2	22.0052	24.00	0.00
25yr3d	B2	22.2552	24.00	0.00
25yr3d	B2	22.5052	24.00	0.00
25yr3d	B2	22.7552	24.00	0.00
25yr3d	B2	23.0052	24.00	0.00
25yr3d	B2	23.2552	24.00	0.00
25yr3d	B2	23.5052	24.00	0.00
25yr3d	B2	23.7552	24.00	0.00
25yr3d	B2	24.0052	24.00	0.00
25yr3d	B2	24.2552	24.00	0.00
25yr3d	B2	24.5052	24.00	0.00
25yr3d	B2	24.7552	24.00	0.00
25yr3d	B2	25.0052	24.00	0.00
25yr3d	B2	25.2552	24.00	0.00
25yr3d	B2	25.5052	24.00	0.00
25yr3d	B2	25.7552	24.00	0.00
25yr3d	B2	26.0052	24.00	0.00
25yr3d	B2	26.2552	24.00	0.00
25yr3d	B2	26.5052	24.00	0.00
25yr3d	B2	26.7552	24.00	0.00
25yr3d	B2	27.0052	24.00	0.00
25yr3d	B2	27.2552	24.00	0.00
25yr3d	B2	27.5052	24.00	0.00
25yr3d	B2	27.7552	24.00	0.00
25yr3d	B2	28.0052	24.00	0.00
25yr3d	B2	28.2552	24.00	0.00
25yr3d	B2	28.5052	24.00	0.00
25yr3d	B2	28.7552	24.00	0.00
25yr3d	B2	29.0052	24.00	0.00
25yr3d	B2	29.2552	24.00	0.00
25yr3d	B2	29.5052	24.00	0.00
25yr3d	B2	29.7552	24.00	0.00
25yr3d	B2	30.0052	24.00	0.00
25yr3d	B2	30.2552	24.00	0.00
25yr3d	B2	30.5052	24.00	0.00
25yr3d	B2	30.7552	24.00	0.00
25yr3d	B2	31.0052	24.00	0.00
25yr3d	B2	31.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	31.5052	24.00	0.00
25yr3d	B2	31.7552	24.00	0.00
25yr3d	B2	32.0052	24.00	0.00
25yr3d	B2	32.2552	24.00	0.00
25yr3d	B2	32.5052	24.00	0.00
25yr3d	B2	32.7552	24.00	0.00
25yr3d	B2	33.0052	24.01	0.00
25yr3d	B2	33.2552	24.01	0.00
25yr3d	B2	33.5052	24.01	0.00
25yr3d	B2	33.7552	24.01	0.00
25yr3d	B2	34.0052	24.01	0.00
25yr3d	B2	34.2552	24.01	0.00
25yr3d	B2	34.5052	24.01	0.00
25yr3d	B2	34.7552	24.01	0.00
25yr3d	B2	35.0052	24.01	0.00
25yr3d	B2	35.2552	24.01	0.00
25yr3d	B2	35.5052	24.01	0.00
25yr3d	B2	35.7552	24.01	0.00
25yr3d	B2	36.0052	24.01	0.00
25yr3d	B2	36.2552	24.01	0.00
25yr3d	B2	36.5052	24.01	0.00
25yr3d	B2	36.7552	24.01	0.00
25yr3d	B2	37.0052	24.01	0.00
25yr3d	B2	37.2552	24.01	0.00
25yr3d	B2	37.5052	24.01	0.00
25yr3d	B2	37.7552	24.01	0.00
25yr3d	B2	38.0052	24.02	0.00
25yr3d	B2	38.2552	24.02	0.00
25yr3d	B2	38.5052	24.02	0.00
25yr3d	B2	38.7552	24.02	0.00
25yr3d	B2	39.0052	24.02	0.00
25yr3d	B2	39.2552	24.02	0.00
25yr3d	B2	39.5052	24.02	0.00
25yr3d	B2	39.7552	24.02	0.00
25yr3d	B2	40.0052	24.02	0.00
25yr3d	B2	40.2552	24.02	0.00
25yr3d	B2	40.5052	24.02	0.00
25yr3d	B2	40.7552	24.02	0.01
25yr3d	B2	41.0052	24.02	0.01
25yr3d	B2	41.2552	24.03	0.01
25yr3d	B2	41.5052	24.03	0.01
25yr3d	B2	41.7552	24.03	0.01

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	42.0052	24.03	0.01
25yr3d	B2	42.2552	24.03	0.01
25yr3d	B2	42.5052	24.03	0.01
25yr3d	B2	42.7552	24.03	0.01
25yr3d	B2	43.0052	24.03	0.01
25yr3d	B2	43.2552	24.03	0.01
25yr3d	B2	43.5052	24.03	0.01
25yr3d	B2	43.7552	24.04	0.01
25yr3d	B2	44.0052	24.04	0.01
25yr3d	B2	44.2552	24.04	0.01
25yr3d	B2	44.5052	24.04	0.01
25yr3d	B2	44.7552	24.04	0.01
25yr3d	B2	45.0052	24.04	0.02
25yr3d	B2	45.2552	24.04	0.02
25yr3d	B2	45.5052	24.04	0.02
25yr3d	B2	45.7552	24.05	0.02
25yr3d	B2	46.0052	24.05	0.02
25yr3d	B2	46.2552	24.05	0.02
25yr3d	B2	46.5052	24.05	0.02
25yr3d	B2	46.7552	24.05	0.02
25yr3d	B2	47.0052	24.05	0.02
25yr3d	B2	47.2552	24.05	0.02
25yr3d	B2	47.5052	24.06	0.03
25yr3d	B2	47.7552	24.06	0.03
25yr3d	B2	48.0052	24.06	0.03
25yr3d	B2	48.2552	24.06	0.03
25yr3d	B2	48.5052	24.06	0.03
25yr3d	B2	48.7552	24.06	0.03
25yr3d	B2	49.0052	24.07	0.03
25yr3d	B2	49.2552	24.07	0.04
25yr3d	B2	49.5052	24.07	0.04
25yr3d	B2	49.7552	24.07	0.04
25yr3d	B2	50.0052	24.07	0.04
25yr3d	B2	50.2552	24.07	0.04
25yr3d	B2	50.5052	24.08	0.05
25yr3d	B2	50.7552	24.08	0.05
25yr3d	B2	51.0052	24.08	0.05
25yr3d	B2	51.2552	24.08	0.05
25yr3d	B2	51.5052	24.08	0.05
25yr3d	B2	51.7552	24.09	0.06
25yr3d	B2	52.0052	24.09	0.06
25yr3d	B2	52.2552	24.09	0.06

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	52.5052	24.09	0.07
25yr3d	B2	52.7552	24.10	0.07
25yr3d	B2	53.0052	24.10	0.07
25yr3d	B2	53.2552	24.10	0.07
25yr3d	B2	53.5052	24.10	0.08
25yr3d	B2	53.7552	24.11	0.08
25yr3d	B2	54.0052	24.11	0.08
25yr3d	B2	54.2552	24.11	0.09
25yr3d	B2	54.5036	24.12	0.09
25yr3d	B2	54.7536	24.12	0.10
25yr3d	B2	55.0002	24.13	0.10
25yr3d	B2	55.2530	24.13	0.11
25yr3d	B2	55.5053	24.14	0.11
25yr3d	B2	55.7502	24.14	0.11
25yr3d	B2	56.0013	24.15	0.12
25yr3d	B2	56.2520	24.15	0.13
25yr3d	B2	56.5031	24.16	0.13
25yr3d	B2	56.7519	24.16	0.14
25yr3d	B2	57.0016	24.17	0.15
25yr3d	B2	57.2508	24.18	0.15
25yr3d	B2	57.5036	24.19	0.16
25yr3d	B2	57.7505	24.20	0.17
25yr3d	B2	58.0032	24.21	0.18
25yr3d	B2	58.2526	24.22	0.19
25yr3d	B2	58.5033	24.23	0.20
25yr3d	B2	58.7534	24.25	0.21
25yr3d	B2	59.0014	24.26	0.22
25yr3d	B2	59.2509	24.28	0.24
25yr3d	B2	59.5018	24.31	0.25
25yr3d	B2	59.7503	24.35	0.27
25yr3d	B2	60.0002	24.41	0.29
25yr3d	B2	60.2501	24.51	0.32
25yr3d	B2	60.5011	24.62	0.37
25yr3d	B2	60.7510	24.72	0.42
25yr3d	B2	61.0019	24.81	0.48
25yr3d	B2	61.2507	24.90	0.56
25yr3d	B2	61.5007	24.97	0.65
25yr3d	B2	61.7504	25.04	0.75
25yr3d	B2	62.0004	25.10	0.85
25yr3d	B2	62.2533	25.15	0.97
25yr3d	B2	62.5031	25.20	1.09
25yr3d	B2	62.7504	25.24	1.21

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	63.0028	25.28	1.35
25yr3d	B2	63.2555	25.32	1.49
25yr3d	B2	63.5061	25.35	1.63
25yr3d	B2	63.7556	25.39	1.78
25yr3d	B2	64.0051	25.42	1.93
25yr3d	B2	64.2527	25.45	2.09
25yr3d	B2	64.5019	25.48	2.25
25yr3d	B2	64.7519	25.51	2.42
25yr3d	B2	65.0019	25.53	2.59
25yr3d	B2	65.2552	25.56	2.76
25yr3d	B2	65.5011	25.58	2.94
25yr3d	B2	65.7502	25.60	3.12
25yr3d	B2	66.0002	25.62	3.30
25yr3d	B2	66.2544	25.65	3.49
25yr3d	B2	66.5002	25.66	3.67
25yr3d	B2	66.7519	25.68	3.87
25yr3d	B2	67.0019	25.70	4.06
25yr3d	B2	67.2519	25.72	4.26
25yr3d	B2	67.5019	25.74	4.45
25yr3d	B2	67.7519	25.76	4.65
25yr3d	B2	68.0019	25.77	4.86
25yr3d	B2	68.2519	25.79	5.06
25yr3d	B2	68.5019	25.80	5.27
25yr3d	B2	68.7519	25.82	5.48
25yr3d	B2	69.0019	25.83	5.69
25yr3d	B2	69.2519	25.84	5.90
25yr3d	B2	69.5019	25.86	6.11
25yr3d	B2	69.7519	25.87	6.33
25yr3d	B2	70.0019	25.88	6.54
25yr3d	B2	70.2519	25.89	6.76
25yr3d	B2	70.5019	25.90	6.98
25yr3d	B2	70.7519	25.91	7.19
25yr3d	B2	71.0019	25.92	7.41
25yr3d	B2	71.2519	25.93	7.63
25yr3d	B2	71.5019	25.93	7.85
25yr3d	B2	71.7519	25.94	8.08
25yr3d	B2	72.0019	25.95	8.30
25yr3d	B2	76.0019	25.97	11.81
25yr3d	B2	80.0019	25.97	15.11
25yr3d	B2	84.0019	25.96	18.34
25yr3d	B2	88.0019	25.95	21.65
25yr3d	B2	92.0019	25.93	25.03

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	96.0019	25.90	28.45
25yr3d	B2	100.0019	25.87	31.88
25yr3d	B2	104.0019	25.83	35.40
25yr3d	B2	108.0019	25.79	39.04
25yr3d	B2	112.0019	25.74	42.70
25yr3d	B2	116.0019	25.69	46.26
25yr3d	B2	120.0019	25.64	49.67
25yr3d	B2	124.0019	25.59	52.93
25yr3d	B2	128.0019	25.55	56.05
25yr3d	B2	132.0019	25.50	59.03
25yr3d	B2	136.0019	25.45	61.86
25yr3d	B2	140.0019	25.40	64.55
25yr3d	B2	144.0019	25.35	67.10
25yr3d	B2	148.0019	25.31	69.53
25yr3d	B2	152.0019	25.27	71.84
25yr3d	B2	156.0019	25.23	74.06
25yr3d	B2	160.0019	25.19	76.17
25yr3d	B2	164.0019	25.16	78.20
25yr3d	B2	168.0019	25.13	80.14
25yr3d	B2	172.0019	25.10	82.01
25yr3d	B2	176.0019	25.07	83.80
25yr3d	B2	180.0019	25.04	85.52
25yr3d	B2	184.0019	25.02	87.17
25yr3d	B2	188.0019	24.99	88.77
25yr3d	B2	192.0019	24.97	90.30
25yr3d	B2	196.0019	24.94	91.78
25yr3d	B2	200.0019	24.92	93.20
25yr3d	B2	204.0019	24.90	94.57
25yr3d	B2	208.0019	24.88	95.90
25yr3d	B2	212.0019	24.86	97.18
25yr3d	B2	216.0019	24.84	98.41
25yr3d	B2	220.0019	24.82	99.61
25yr3d	B2	224.0019	24.80	100.76
25yr3d	B2	228.0019	24.78	101.88
25yr3d	B2	232.0019	24.76	102.96
25yr3d	B2	236.0019	24.75	104.00
25yr3d	B2	240.0019	24.73	105.01
25yr3d	B2	244.0019	24.72	105.99
25yr3d	B2	248.0019	24.70	106.93
25yr3d	B2	252.0019	24.69	107.85
25yr3d	B2	256.0019	24.67	108.74
25yr3d	B2	260.0019	24.66	109.60

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	264.0019	24.64	110.43
25yr3d	B2	268.0019	24.63	111.24
25yr3d	B2	272.0019	24.62	112.02
25yr3d	B2	276.0019	24.61	112.78
25yr3d	B2	280.0019	24.59	113.52
25yr3d	B2	284.0019	24.58	114.23
25yr3d	B2	288.0019	24.57	114.93
25yr3d	B2	292.0019	24.56	115.60
25yr3d	B2	296.0019	24.55	116.26
25yr3d	B2	300.0019	24.54	116.89
25yr3d	B2	304.0019	24.53	117.51
25yr3d	B2	308.0019	24.52	118.11
25yr3d	B2	312.0019	24.51	118.69
25yr3d	B2	316.0019	24.50	119.25
25yr3d	B2	320.0019	24.49	119.80
25yr3d	B2	324.0019	24.48	120.34
25yr3d	B2	328.0019	24.47	120.86
25yr3d	B2	332.0019	24.46	121.36
25yr3d	B2	336.0019	24.45	121.85
25yr3d	B2	340.0019	24.44	122.32
25yr3d	B2	344.0019	24.44	122.79
25yr3d	B2	348.0019	24.43	123.24
25yr3d	B2	352.0019	24.42	123.67
25yr3d	B2	356.0019	24.41	124.10
25yr3d	B2	360.0019	24.40	124.51

IBIS LANDING (FKA COPPERHEAD)
WATER MANAGEMENT REPORT

PROJECT INFORMATION

This application is a request for a permit modification to the Ibis Landing Environmental Resource Permit (Permit Number 36-108291-P) for site plan revisions in the southeast corner of the project. Ibis Landing is a partially constructed 298.86-acre residential golf course project formerly known as Copperhead. The site is located along Beth Stacey Blvd, south of SW 23rd Street in Lehigh Acres, Lee County, Florida. The project is being developed by Lennar under the name Ibis Landing.

PROJECT BACKGROUND

On April 13, 2000, Permit Number 36-03765-P (Application Number 990415-2) was approved for construction and operation approval of a surface water management of a residential golf course development located on a 297.8-acre site known as Copperhead. The entire surface water management system, golf course and the first phase of residential development (approximately 100 lots) were constructed in 2001 and placed in service.

A new ERP (Permit Number 36-108291-P) was issued to reauthorize construction of the undeveloped residential areas in September 2023. The residential infrastructure for the western and northeastern areas of the project have been constructed over the last two years. Beth Stacy Blvd (Milwaukee Blvd) will be extended from north to south as part of the Ibis Landing project. This roadway will ultimately be owned by Lee County and will drain to the project's surface water management system.

SURFACE WATER MANAGEMENT SYSTEM

The existing onsite surface water management system consists of 9 primary surface water management basins which use the existing lakes and dry detention areas for treatment and storage. A 10th basin was proposed for the southern (offsite) portion of Beth Stacey Blvd that has not been constructed. The site is surrounded by Lehigh Acres Municipal Services Improvement District (LAMSID) Yellowtail Canal and Yellowtail Canal Extensions on the West, South and East sides. The project discharges to these canals which are part of the Orange River Watershed. The discharge rate is 30.1 cfm which is the allowable rate per LAMSID. No modifications to the previously permitted wetland preserves are proposed.

Stormwater calculations are only being provided for the basins that are being modified (Basins 2-5). No changes to the permitted control elevations are proposed. Per the permit, the eastern portion of the site (modified area) has a control elevation of 24.0' NGVD. The Ibis Landing Surface Water Management System provides the required water quality treatment and attenuation for the 25-year 3-day storm event. Water quality treatment volume is one inch plus an additional 50%. Water quality calculations are provided at the end of the calculations for the basins that discharge offsite (Basins 2).

The eastern portion of the site (Basins 1, 3, 4, 5 and 6) discharge to Basin 2, a 26.62 wetland which discharges to the east to the Yellowtail Canal. This permitted drainage pattern will be maintained. No modification to any weirs that discharge offsite are planned.

PROPOSED PROJECT MODIFICATIONS

This application proposes site plan revisions within Basins 3 and 4 in the southeast corner of the project. The existing lake in Basins 3 (Lake 3) is being reshaped which results in a increase in the lake size of 0.1 acres.

The existing lake in Basin 4 (Lake 4) is being reshaped which results in a increase in the lake size of 0.07 acres. Basins 3 and Basin 4 are also connected to help equalize the stages in these basins. The outfall location in Basin 4 is being maintained and will operate as previously permitted/constructed. The outfall location is Basin 3 is being modified to accommodate the lake reshaping. The bleeder in Basin 3 was sized to account for both Basin 3 and Basin 4 water quality bleed down. Minor revisions to Beth Stacey Blvd are also proposed which consist of providing closed drainage (curb and gutter) for the roadway along with the addition of a second 8' concrete sidewalk along the roadway (which is within Basin 5). The proposed Basin 10, which provided storage for runoff from the southern portion of Beth Stacey Blvd will be eliminated and the roadway will drain to Basins 4 and 5 which increase the sizes of these basins. Approximately 250' offsite roadway will drain to the proposed roadway swales. Compensating water quality storage will be provided for this area of the offsite roadway within Basin 5.

WATER QUALITY

All basins provide the required water quality treatment prior to connecting to the onsite wetland or before discharging offsite. Water quality treatment is provided in the interconnected lakes and dry detention ponds throughout the site. The required water quality volume of 150% of the greater of one inch or 2.5" over the impervious areas is provided. Compensating water quality treatment in the amount of 2.5 inches times the offsite impervious area is provided for the offsite portion of Beth Stacey Blve.

Basin #	Total Area (AC)	Control Elev. (NGVD)	Sub-basins Included	Treatment Provided By	Final Outfall WCS
1	49.33	24.00	B1L1 & B1L2	Wet Detention	WCS 2
2	32.44	24.00	N/A	Dry Detention	WCS 2
3	16.53	24.00	N/A	Wet Detention	WCS 2
4	18.34	24.00	N/A	Wet Detention	WCS 2
5	131.83	24.50	B5L5, B5L6, B5L7, B5L9, B5L10, B5L11	Wet Detention	WCS 2
6	18.20	24.00	N/A	Dry Detention	WCS 2
7	9.40	25.50	N/A	Wet Detention	WCS 7
8	6.11	25.50	N/A	Dry Detention	WCS 8
9	16.68	25.50	N/A	Dry Detention	WCS 8

Basin	CE	Proposed Basin Area	Permitted WQ V (ac-ft)	Proposed Construction WQ V (ac-ft)
B1 (P1-A, P1-B, P2-A, P2-B, P2-C & P2-D)	24.00	49.33	3.81	N/A
B2 (WA)	24.00	32.44	2.97	N/A
B3 (P3)	24.00	16.53	1.82	2.07
B4 (P4)	24.00	18.34	1.76	2.51
B5 (P5, P6, P7, P8-A, P8-B, P9, P10, & P11)	24.50	131.83	23.00	N/A
B6 (WC-A, WC-B, & WC-C)	24.00	18.20	1.75	N/A
B7 (P13)	25.50	9.40	1.18	N/A
B8 (WD)	25.50	6.11	0.93	N/A
B9 (P12)	25.50	16.68	0.38	N/A
	Totals	**298.86**	**37.60**	**4.58**

The proposed project provides 150% of the 1 inch over the project area. 298.86 x 1.5 inches = ***37.36 ac-ft***

The proposed project provides 2.5 inches times the percentage of imperviousness for the offsite improvements. 0.22 ac x 2.5 inches = ***0.05 ac-ft***

Total Required = ***37.41 ac-ft***
Total Provided = ***42.18 ac-ft***

WATER QUANTITY AND MODEL

The project is divided into 9 Basins with Basin 1 and Basin 5 having multiple interconnected lakes/sub-basins. The land use, CN, and stage storage was calculated by sub-basin. An Interconnected Pond Routing model (ICPR 4) was created to simulate the flood routing for the existing SWM system design. The weirs used in the model for Basins 1, 4, 5, 6, & 9 are modeled as they exist. The weir between Lake 3 and the downstream Wetland is proposed to be constructed. This weir will provide pre-treatment for areas that connect to Wetlands. The weirs, for Basins 2, 7, and 8, that discharge offsite will operate as previously permitted/constructed to meet the design parameters of water quality, allowable bleed down, and allowable discharge for SFWMD.

The design elevations are as follows and the flood routing results are included for comparison in the tables on the next page.

	CONDITION	
	Permitted (App#230103-37076)	Proposed (Current)
BASIN	WA	B2
5-YR	24.68	24.66
10-YR	24.86	24.84
25-YR	25.94	25.97
100-YR	27.42	27.43
DISCHARGE	10.98	11.09

	CONDITION	
	Permitted (App#230103-37076)	Proposed (Current)
BASIN	P3	B3
5-YR	25.43	25.78
10-YR	25.61	25.92
25-YR	26.81	26.86
100-YR	27.42	27.43
DISCHARGE	N/A (Internal)	N/A

BASIN	P4	B4
5-YR	25.64	25.79
10-YR	25.97	25.97
25-YR	26.68	26.73
100-YR	27.42	27.43
DISCHARGE	N/A (Internal)	N/A (Internal)

BASIN	P5-P11,P14-P15	B5
5-YR	25.54	25.54
10-YR	25.87	25.87
25-YR	27.32	27.32
100-YR	27.94	27.94
DISCHARGE	N/A (Internal)	N/A

The next page contains a land use comparison between the total permitted land use from Permit No. 36-108291-P application 230103-37076 and the proposed modification.

LAND USE COMPARISON

	Permitted Total	% of Proposed
Basin	B2	
Building	0.00	0.0%
Impervious	0.16	0.5%
Pervious	7.15	22.0%
Dry Detentiion	0.44	1.4%
Wetland	24.69	76.1%
Total	**32.44**	100.0%
Basin	B3	
Building	3.24	19.6%
Impervious	4.59	27.8%
Pervious	7.29	44.1%
Lake	1.41	8.5%
Total	**16.53**	100.0%
Basin	B4	
Building	2.37	13.3%
Impervious	3.74	21.0%
Pervious	10.33	58.0%
Lake	1.37	7.7%
Total	**17.81**	100.0%
Basin	B5	
Building	16.24	12.4%
Impervious	24.18	18.5%
Pervious	71.75	54.8%
Dry Detention	0.07	0.1%
Lake	18.64	14.2%
Total	**130.88**	100.0%
Basin	B10	
Building	0.04	2.7%
Impervious	0.12	8.1%
Pervious	1.11	75.0%
Dry Detention	0.21	14.2%
Total	**1.48**	100.0%

	Proposed Modification Total	% of Permitted
	B2	
	0.00	0.0%
	0.16	0.5%
	7.15	22.0%
	0.44	1.4%
	24.69	76.1%
	32.44	100.0%
	B3	
	3.05	18.5%
	4.71	28.5%
	7.35	44.5%
	1.42	8.6%
	16.53	100.0%
	B4	
	1.36	7.4%
	7.38	40.2%
	8.16	44.5%
	1.44	7.9%
	18.34	100.0%
	P5, P6, P7, P8-A, P8-B, P9, P10, & P11	
	16.25	12.3%
	24.73	18.8%
	72.14	54.7%
	0.07	0.1%
	18.64	14.1%
	131.83	100.0%

Basin 10 has been removed and added to Basins 4 and 5 as shown above.

	Permitted Total	% of Proposed
Impervious		
Building	21.89	11.0%
Impervious	32.79	16.5%
Lake	21.42	10.8%
Total	76.10	38.5%

Proposed Modification Total	% of Permitted
20.66	10.4%
36.98	18.6%
21.50	10.8%
79.14	39.8%

OFFSITE DISCHARGES

CS	Contributing Basins	Contributing Area (AC)	Allowable Discharge* (cfs)	Model Discharge (cfs)
WCS-2	B1, B2, B3, B4, B5, & B6	266.67	12.54	11.10

* Based on 30.1 SCM

The project lies within the Lehigh Acres Municipal Services Improvement District which has an allowable discharge limit of 30.1 CSM. The proposed surface water management system for the 266.67-acre basin has an allowable discharge rate of 12.54 cfs. The total discharge is 11.10 cfs which is less than allowed.

TOTAL SITE

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	26.04	8.7%	0.00	0.00	0.00	26.04
LAKE BANK @ CE + 1	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE TOB	2.28	0.8%	2.28	0.00	0.00	0.00
DRY DETENTION	1.65	0.6%	1.65	0.00	0.00	0.00
PRESERVE	37.83	12.7%	37.83	0.00	0.00	0.00
ROW	32.59	10.9%	13.04	19.55	0.00	0.00
SF	63.92	21.4%	25.57	19.18	19.18	0.00
MF	25.26	8.5%	5.05	10.10	10.10	0.00
GOLF COURSE	96.36	32.2%	93.47	2.89	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	0.5%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	2.3%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**298.86**	**100%**	**185.12**	**55.00**	**32.56**	**26.18**

TOTAL PERVIOUS AREA	185.12	61.9%	
TOTAL IMPERVIOUS AREA	113.74	38.1%	*(INCLUDES WATER SURFACE AREA)*

TOTAL SITE

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 61.9% x 6.57 in.
S = 4.07 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 71

III. SURFACE STORAGE

Basin Area =	298.86	ac.			
Bldg. Coverage Area =	32.56	ac.			
Impervious =	55.00				
Lake Area (at Control)	26.18	ac.	***EL. =***	***24.00***	***(NGVD)***
Preserve Area (at control)	37.83	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less Preserved)=	147.29	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**26.18**	**0.00**	**0.00**	**0.00**	**26.18**	**0.00**
25.00	**28.32**	**0.00**	**0.00**	**0.00**	**28.32**	**27.25**
26.50	**30.60**	**0.00**	**0.00**	**0.00**	**30.60**	**71.44**
27.00	**32.88**	**7.57**	**29.46**	**27.50**	**97.41**	**103.44**
27.50	**32.93**	**15.13**	**58.92**	**49.50**	**156.48**	**166.92**
28.50	**32.93**	**18.92**	**132.57**	**55.00**	**239.41**	**364.86**

IV. WATER QUALITY DETENTION VOLUME

TOTAL SITE

Total Area =	298.86	ac.
Bldg. Coverage =	32.56	ac.
Water Surface Area =	26.18	ac.
Pervious Area =	185.12	ac.

First one (1) Inch of runoff over basin:

= 1" x 298.86 ac. x1'/12" = **24.91** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	240.12	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	55.00	ac.
% Impervious = Impervious Area / Site Area	22.9%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**13.01**	**ac-ft**
Required Water Quality Detention Volume	**24.91**	**ac-ft**
Additional 50% Water Quality Detention Volume	**12.45**	**ac-ft**
Total Required Water Quality Detention Volume	**37.36**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**37.36**	**ac-ft**

Between El. 24.00 and 25.34

WCS Stage-Storage Table

Stage (NGVD)	Site Storage Area ac	Site Storage Volume ac-ft
24.00	26.18	0.00
25.00	28.32	27.25
26.50	30.60	71.44
27.00	97.41	103.44
27.50	156.48	166.92
28.50	239.41	364.86

Water Quality Min. Weir Elevation = **25.34** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event:

= 55.0 CSM x WCS Area x 1 SM/640 ac.

= **25.68** **cfs**

METHOD OF DETERMINATION:		
Orange River Watershed (Approved ERP Appl # 060503-10)	55.0	CSM

Basin B2

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 1	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE TOB	0.00	0.0%	0.00	0.00	0.00	0.00
DRY DETENTION	0.44	1.4%	0.44	0.00	0.00	0.00
PRESERVE	26.62	82.1%	26.62	0.00	0.00	0.00
ROW	0.00	0.0%	0.00	0.00	0.00	0.00
SF	0.00	0.0%	0.00	0.00	0.00	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	5.38	16.6%	5.22	0.16	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**32.44**	**100%**	**32.28**	**0.16**	**0.00**	**0.00**

TOTAL PERVIOUS AREA	32.28	99.5%
TOTAL IMPERVIOUS AREA	0.16	0.5%

Basin B2

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 99.5% x 6.57 in.
S = 6.53 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 60

III. SURFACE STORAGE

Basin Area =	32.44	ac.			
Bldg. Coverage Area =	0.00	ac.			
Dry Detention TOS (at CE + 1)	0.44	ac.	***EL. =***	***25.00***	***(NGVD)***
Dry Detention TOB (at CE + 2)	0.54	ac.	***EL. =***	***26.00***	***(NGVD)***
Golf Course	5.22	ac.			
Preserve Area (at control)	26.62	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area =	32.28	ac.			

Stage-Storage Table

Stage (NGVD)	Dry Detention ac	Preserve Area ac	Golf Course ac	Total Area ac	Total Volume ac-ft
24.00	**0.00**	**26.62**	**0.00**	**26.62**	**0.00**
25.00	**0.44**	**26.62**	**0.00**	**27.06**	**26.84**
26.50	**0.44**	**26.62**	**1.04**	**28.10**	**68.21**
27.00	**0.44**	**26.62**	**2.09**	**29.15**	**82.53**
27.50	**0.44**	**26.62**	**2.61**	**29.67**	**97.23**

IV. WATER QUALITY DETENTION VOLUME
The permitted 2.97 ac-ft of water quality was provided in the existing condition and no modification to the basin is proposed.

V. ALLOWABLE DISCHARGE

BASIN	**2**	
Contributing Flows		
Basin	1	49.33
Basin	2	32.44
Basin	3	16.53
Basin	4	18.34
Basin	5	131.83
Basin	6	18.20
Total Area =		266.68 ac.

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event:

= 55.0 CSM x WCS Area x 1 SM/640 ac.

= **22.92** **cfs**

METHOD OF DETERMINATION:		
55 CSM allowed in Orange River Watershed & Approved ERP Appl # 994015-2	55.0	CSM

VI. WEIR DESIGN

Total Basin

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**266.68 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**11.11 Ac-FT**
½" Discharge Rate:	=		=	**5.72 CFS**
Water Quality Stage:	=		=	**24.56 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**22.22 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation)	=	**24.23 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**24.39 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)	=	**0.39 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q **=** 5.72 **CFS**

Weir Length (L)

H **=** height above weir

$L = Q/(3.13)(H^{1.5})$

L= 7.42 Length of weir **A 1.50 ft wide rectangular weir was used to meet the maximum discharge rate**

Basin B3L3

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.42	8.6%	0.00	0.00	0.00	1.42
LAKE BANK @ CE + 1	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE TOB	0.14	0.8%	0.14	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	2.62	15.8%	1.05	1.57	0.00	0.00
SF	5.83	35.3%	2.33	1.75	1.75	0.00
MF	3.25	19.7%	0.65	1.30	1.30	0.00
GOLF COURSE	3.00	18.1%	2.91	0.09	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.53**	**100%**	**7.35**	**4.71**	**3.05**	**1.42**

TOTAL PERVIOUS AREA	7.35	44.5%	
TOTAL IMPERVIOUS AREA	9.18	55.5%	*(INCLUDES WATER SURFACE AREA)*

Basin B3L3

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 44.5% x 6.57 in.
S = 2.92 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 77

III. SURFACE STORAGE

Basin Area =	16.53	ac.			
Bldg. Coverage Area =	3.05	ac.			
Lake Area (at Control)	1.42	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.56	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.70	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.83	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	2.91	ac.			
Pervious Area (less bank & GC) =	4.03	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.42**	**0.00**	**0.00**	**0.00**	**1.42**	**0.00**
25.00	**1.56**	**0.00**	**0.00**	**0.00**	**1.56**	**1.49**
26.50	**1.70**	**0.00**	**0.00**	**0.00**	**1.70**	**3.93**
27.00	**1.83**	**0.58**	**0.81**	**2.36**	**5.58**	**5.75**
27.50	**1.83**	**1.16**	**1.61**	**4.24**	**8.85**	**9.35**
28.50	**1.83**	**1.46**	**3.63**	**4.71**	**11.63**	**19.59**

IV. WATER QUALITY DETENTION VOLUME

BASIN 3

Total Area =	16.53	ac.
Bldg. Coverage =	3.05	ac.
Water Surface Area =	1.42	ac.
Pervious Area =	7.35	ac.

First one (1) Inch of runoff over basin:

= 1" x 16.53 ac. x1'/12" = **1.38** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	12.06	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	4.71	ac.
% Impervious = Impervious Area / Site Area	39.0%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.23**	**ac-ft**
Required Water Quality Detention Volume	**1.38**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.69**	**ac-ft**
Total Required Water Quality Detention Volume	**2.07**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.07**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.42	0.00
25.00	1.56	1.49
26.50	1.70	3.93
27.00	1.83	4.81
27.50	1.83	5.73
28.50	1.83	7.56

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.42 ac + 1.44 ac (Lake 3 and Lake 4) = 2.86 ac, the WQ is provided at 25.60 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin (The required bleed down for B4L4 is accounted for in this basin)

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**34.88 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**1.45 Ac-FT**
½" Discharge Rate:	=		=	**0.75 CFS**
Water Quality Stage:	=		=	**25.60 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)=		**3.12 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation) =		**24.07 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2 =		**24.84 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)=		**0.84 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q **=** 0.75 **CFS**

Weir Length (L)

H **=** height above weir

$L = Q/(3.13)(H^{1.5})$

L= 0.31 Length of weir **A 0.31 ft wide rectangular weir was used to meet the maximum bleeder rate**

Basin B4L4

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.44	7.9%	0.00	0.00	0.00	1.44
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	9.88	53.9%	3.95	5.93	0.00	0.00
SF	1.42	7.7%	0.57	0.43	0.43	0.00
MF	2.34	12.8%	0.47	0.94	0.94	0.00
GOLF COURSE	2.87	15.6%	2.78	0.09	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**18.34**	**100%**	**8.16**	**7.38**	**1.36**	**1.44**

TOTAL PERVIOUS AREA	8.16	44.5%	
TOTAL IMPERVIOUS AREA	10.18	55.5%	*(INCLUDES WATER SURFACE AREA)*

Basin B4L4

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 44.5% x 6.57 in.
S = 2.92 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 77

III. SURFACE STORAGE

Basin Area =	18.34	ac.			
Bldg. Coverage Area =	1.36	ac.			
Lake Area (at Control)	1.44	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.57	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.70	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.83	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	0.00	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less bank & GC) =	4.99	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.44**	**0.00**	**0.00**	**0.00**	**1.44**	**0.00**
25.00	**1.57**	**0.00**	**0.00**	**0.00**	**1.57**	**1.51**
26.50	**1.70**	**0.00**	**0.00**	**0.00**	**1.70**	**3.96**
27.00	**1.83**	**0.56**	**1.00**	**3.69**	**7.07**	**6.15**
27.50	**1.83**	**1.11**	**2.00**	**6.64**	**11.58**	**10.82**
28.50	**1.83**	**1.39**	**4.49**	**7.38**	**15.09**	**24.15**

IV. WATER QUALITY DETENTION VOLUME

BASIN 4

Total Area =	18.34	ac.
Bldg. Coverage =	1.36	ac.
Water Surface Area =	1.44	ac.
Pervious Area =	8.16	ac.

First one (1) Inch of runoff over basin:

= 1" x 18.34 ac. x1'/12" = **1.53** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	15.54	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	7.38	ac.
% Impervious = Impervious Area / Site Area	47.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.67**	**ac-ft**
Required Water Quality Detention Volume	**1.67**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.84**	**ac-ft**
Total Required Water Quality Detention Volume	**2.51**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.51**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.44	0.00
25.00	1.57	1.51
26.50	1.70	3.96
27.00	1.83	4.84
27.50	1.83	5.76
28.50	1.83	7.59

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.16 ac + 1.44 ac (Lake 3 and Lake 4) = 2.60 ac, the WQ is provided at 25.76 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

The Existing weir, as approved in ERP 36-03765-P (App 990415-2) was utilized.

Basin B5L5

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.61	15.7%	0.00	0.00	0.00	2.61
LAKE BANK @ CE + 1	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE TOB	0.23	1.4%	0.23	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	0.66	4.0%	0.26	0.40	0.00	0.00
SF	2	12.1%	0.80	0.60	0.60	0.00
MF	3.99	24.1%	0.80	1.60	1.60	0.00
GOLF COURSE	6.62	39.9%	6.42	0.20	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.58**	**100%**	**8.98**	**2.79**	**2.20**	**2.61**

TOTAL PERVIOUS AREA 8.98 54.2%
TOTAL IMPERVIOUS AREA 7.60 45.8% *(INCLUDES WATER SURFACE AREA)*

Basin B5L5

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 54.2% x 6.57 in.
S = 3.56 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	16.58	ac.			
Bldg. Coverage Area =	2.20	ac.			
Lake Area (at Control)	2.61	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.84	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.08	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	3.31	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	6.62	ac.			
Pervious Area (less bank & GC) =	1.86	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.61**	**0.00**	**0.00**	**0.00**	**2.61**	**0.00**
25.50	**2.84**	**0.00**	**0.00**	**0.00**	**2.84**	**2.73**
27.00	**3.08**	**0.00**	**0.00**	**0.00**	**3.08**	**7.17**
27.50	**3.31**	**1.32**	**0.37**	**1.40**	**6.40**	**9.54**
28.00	**3.31**	**2.65**	**0.74**	**2.51**	**9.22**	**13.44**
29.00	**3.31**	**3.31**	**1.68**	**2.79**	**11.09**	**23.60**

Lake B5L6

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.23	9.6%	0.00	0.00	0.00	1.23
LAKE BANK @ CE + 1	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE TOB	0.16	1.2%	0.16	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	1.40	10.9%	0.56	0.84	0.00	0.00
SF	5.62	43.7%	2.25	1.69	1.69	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	4.14	32.2%	4.01	0.12	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**12.86**	**100%**	**7.29**	**2.65**	**1.69**	**1.23**

TOTAL PERVIOUS AREA	7.29	56.7%	
TOTAL IMPERVIOUS AREA	5.57	43.3%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L6

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 56.7% x 6.57 in.
S = 3.72 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 73

III. SURFACE STORAGE

Basin Area =	12.86	ac.			
Bldg. Coverage Area =	1.69	ac.			
Lake Area (at Control)	1.23	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	0.16	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	0.16	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	1.39	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.01	ac.			
Pervious Area (less bank & GC) =	2.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.23**	**0.00**	**0.00**	**0.00**	**1.23**	**0.00**
25.50	**1.31**	**0.00**	**0.00**	**0.00**	**1.31**	**1.27**
27.00	**1.39**	**0.00**	**0.00**	**0.00**	**1.39**	**3.29**
27.50	**1.39**	**0.80**	**0.56**	**1.33**	**4.08**	**4.66**
28.00	**1.39**	**1.61**	**1.12**	**2.39**	**6.50**	**7.30**
29.00	**1.39**	**2.01**	**2.53**	**2.65**	**8.57**	**14.84**

Lake B5L7

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	9.70	20.4%	0.00	0.00	0.00	9.70
LAKE BANK @ CE + 1	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE TOB	0.71	1.5%	0.71	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	3.50	7.4%	1.40	2.10	0.00	0.00
SF	7.01	14.8%	2.80	2.10	2.10	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	18.16	38.2%	17.61	0.54	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	14.7%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**47.51**	**100%**	**25.36**	**7.55**	**4.90**	**9.70**

TOTAL PERVIOUS AREA	25.36	53.4%	
TOTAL IMPERVIOUS AREA	22.15	46.6%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L7

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.4% x 6.57 in.
S = 3.50 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	47.51	ac.			
Bldg. Coverage Area =	4.90	ac.			
Lake Area (at Control)	9.70	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	10.41	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	11.13	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	11.84	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	18.16	ac.			
Pervious Area (less bank & GC) =	5.60	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**9.70**	**0.00**	**0.00**	**0.00**	**9.70**	**0.00**
25.50	**10.41**	**0.00**	**0.00**	**0.00**	**10.41**	**10.06**
27.00	**11.13**	**0.00**	**0.00**	**0.00**	**11.13**	**26.21**
27.50	**11.84**	**3.63**	**1.12**	**3.77**	**20.37**	**34.09**
28.00	**11.84**	**7.26**	**2.24**	**6.79**	**28.14**	**46.21**
29.00	**11.84**	**9.08**	**5.04**	**7.55**	**33.51**	**77.04**

Lake B5L9

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.49	11.4%	0.00	0.00	0.00	2.49
LAKE BANK @ CE + 1	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE TOB	0.28	1.3%	0.28	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.60	11.9%	1.04	1.56	0.00	0.00
SF	6.92	31.6%	2.77	2.08	2.08	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	9.06	41.3%	8.79	0.27	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**21.92**	**100%**	**13.45**	**3.91**	**2.08**	**2.49**

TOTAL PERVIOUS AREA 13.45 61.3%
TOTAL IMPERVIOUS AREA 8.47 38.7% *(INCLUDES WATER SURFACE AREA)*

Lake B5L9

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 61.3% x 6.57 in.
S = 4.03 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 71

III. SURFACE STORAGE

Basin Area =	21.92	ac.			
Bldg. Coverage Area =	2.08	ac.			
Lake Area (at Control)	2.49	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.77	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.05	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	2.77	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	9.06	ac.			
Pervious Area (less bank & GC) =	3.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.49**	**0.00**	**0.00**	**0.00**	**2.49**	**0.00**
25.50	**2.77**	**0.00**	**0.00**	**0.00**	**2.77**	**2.63**
27.00	**3.05**	**0.00**	**0.00**	**0.00**	**3.05**	**7.00**
27.50	**3.05**	**1.81**	**0.76**	**1.95**	**7.58**	**9.66**
28.00	**3.05**	**3.63**	**1.52**	**3.52**	**11.72**	**14.49**
29.00	**3.05**	**4.53**	**3.43**	**3.91**	**14.92**	**27.81**

Lake B5L10

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.42	9.3%	0.00	0.00	0.00	1.42
LAKE BANK @ CE + 1	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE TOB	0.17	1.1%	0.17	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	0.61	4.0%	0.24	0.37	0.00	0.00
SF	1.37	9.0%	0.55	0.41	0.41	0.00
MF	4.10	26.9%	0.82	1.64	1.64	0.00
GOLF COURSE	5.85	38.5%	5.68	0.18	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	9.0%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**15.22**	**100%**	**8.06**	**3.07**	**2.53**	**1.56**

TOTAL PERVIOUS AREA 8.06 53.0%
TOTAL IMPERVIOUS AREA 7.16 47.0% *(INCLUDES WATER SURFACE AREA)*

Lake B5L10

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.0% x 6.57 in.
S = 3.48 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	15.22	ac.			
Bldg. Coverage Area =	2.53	ac.			
Lake Area (at Control)	1.56	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.72	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.89	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.92	ac.	***EL. =***	***26.50***	***(NGVD)***
Golf Course	5.68	ac.			
Pervious Area (less bank & GC) =	1.89	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.56**	**0.00**	**0.00**	**0.00**	**1.56**	**0.00**
25.50	**1.72**	**0.00**	**0.00**	**0.00**	**1.72**	**1.64**
27.00	**1.89**	**0.00**	**0.00**	**0.00**	**1.89**	**4.35**
27.50	**1.92**	**1.14**	**0.38**	**1.54**	**4.96**	**6.06**
28.00	**1.92**	**2.27**	**0.75**	**2.76**	**7.71**	**9.23**
29.00	**1.92**	**2.84**	**1.70**	**3.07**	**9.52**	**17.84**

Lake B5L11

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.05	5.9%	0.00	0.00	0.00	1.05
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.94	16.6%	1.18	1.76	0.00	0.00
SF	6.61	37.3%	2.64	1.98	1.98	0.00
MF	2.20	12.4%	0.44	0.88	0.88	0.00
GOLF COURSE	4.55	25.6%	4.41	0.14	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**17.74**	**100%**	**9.06**	**4.76**	**2.86**	**1.05**

TOTAL PERVIOUS AREA 9.06 51.1%
TOTAL IMPERVIOUS AREA 8.68 48.9% *(INCLUDES WATER SURFACE AREA)*

Lake B5L11

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 51.1% x 6.57 in.
S = 3.35 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 75

III. SURFACE STORAGE

Basin Area =	17.74	ac.			
Bldg. Coverage Area =	2.86	ac.			
Lake Area (at Control)	1.05	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.18	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.31	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.44	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.41	ac.			
Pervious Area (less bank & GC) =	4.26	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.05**	**0.00**	**0.00**	**0.00**	**1.05**	**0.00**
25.50	**1.18**	**0.00**	**0.00**	**0.00**	**1.18**	**1.12**
27.00	**1.31**	**0.00**	**0.00**	**0.00**	**1.31**	**2.98**
27.50	**1.44**	**0.88**	**0.85**	**2.38**	**5.56**	**4.70**
28.00	**1.44**	**1.76**	**1.70**	**4.29**	**9.20**	**8.39**
29.00	**1.44**	**2.21**	**3.83**	**4.76**	**12.25**	**19.11**

IV. WATER QUALITY DETENTION VOLUME

BASIN 5

Total Area =	131.83	ac.
Bldg. Coverage =	16.25	ac.
Water Surface Area =	18.64	ac.
Pervious Area =	72.21	ac.

First one (1) Inch of runoff over basin:
= 1" x 131.83 ac. x1'/12" = **10.99** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	96.94	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	24.73	ac.
% Impervious = Impervious Area / Site Area	25.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**6.02**	**ac-ft**
Required Water Quality Detention Volume	**10.99**	**ac-ft**
Additional 50% Water Quality Detention Volume	**5.49**	**ac-ft**

Beth Stacy Offsite Imperviousness
2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	0.22	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	0.22	ac.
% Impervious = Impervious Area / Site Area	100.0%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**0.05**	**ac-ft**
Total Required Water Quality Detention Volume	**16.52**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**16.52**	**ac-ft**

Between El. 24.00 and 25.35

Stage-Storage Table for Water Quality Calculations

Stage (NGVD)	Lake Storage Area ac	Lake Storage Volume ac-ft
24.50	18.64	0.00
25.50	20.24	19.44
27.00	21.85	51.01
27.50	22.95	62.20
28.00	22.95	73.68
29.00	22.95	96.63

23 ac-ft of total wq volume is provided

Water Quality Min. Weir Elevation = **25.35** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin
Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**131.83 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**5.49 Ac-FT**
½" Discharge Rate:	=		=	**2.83 CFS**
Water Quality Stage:	=		=	**25.67 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**17.51 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	eatment Vol. After 24 Hr. / Total Basin Storag (+ Control Elevation)	=	**24.84 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**25.26 Ft-NAVD**
Average Height During Bleed Down:	=	∖ve. Elev. During Bleed Down - Control Elev.	=	**0.76 Ft**

Size Bleeder:
Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q **=** 2.83 **CFS**

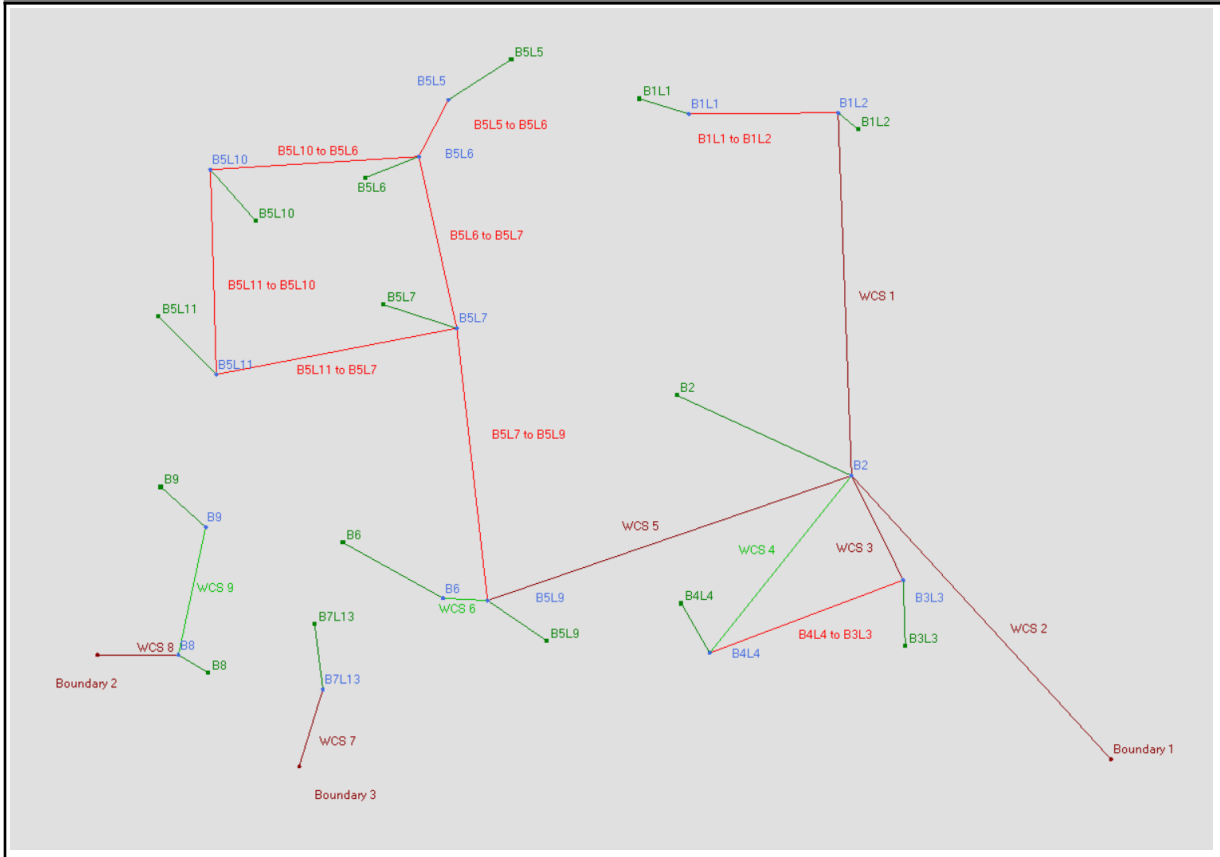
Weir Length (L)

H **=** height above weir

$L= Q/(3.13)(H^{1.5})$

L= 1.37 Length of weir **A 1.33 ft wide rectangular weir was used to meet the maximum bleeder rate**

Background Image: Background



Impervious: Scenario 1 [Set]

Land Cover Zone	% Impervious	% DCIA	% Direct	Ia/S [dec]	Ia Impervious [in]	Ia Pervious [in]
Impervious & Pervious	0.00	0.00	0.00	0.00	0.000	0.000

Curve Number: Scenario 1 [Set]

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 60	60.0
Impervious & Pervious	Calculated CN 63	63.0
Impervious & Pervious	Calculated CN 64	64.0
Impervious & Pervious	Calculated CN 70	70.0
Impervious & Pervious	Calculated CN 71	71.0
Impervious & Pervious	Calculated CN 72	72.0

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 73	73.0
Impervious & Pervious	Calculated CN 74	74.0
Impervious & Pervious	Calculated CN 75	75.0
Impervious & Pervious	Calculated CN 76	76.0
Impervious & Pervious	Calculated CN 77	77.0

Manual Basin: B2

Scenario: Scenario1
Node: B2
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 60.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 32.4400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
32.4400	Impervious & Pervious	Calculated CN 60	

Comment:

Manual Basin: B3L3

Scenario: Scenario1
Node: B3L3
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5300 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5300	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B4L4

Scenario: Scenario1
Node: B4L4

Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 18.3400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
18.3400	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B5L10

Scenario: Scenario1
Node: B5L10
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 15.2200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
15.2200	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L11

Scenario: Scenario1
Node: B5L11
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 17.7400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
17.7400	Impervious & Pervious	Calculated CN 75	

Comment:

Manual Basin: B5L5

Scenario: Scenario1
Node: B5L5
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5800 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5800	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L6

Scenario: Scenario1
Node: B5L6
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 12.8600 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
12.8600	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L7

Scenario: Scenario1
Node: B5L7
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 47.5100 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
47.5100	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L9

Scenario: Scenario1
Node: B5L9
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 21.9200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
21.9200	Impervious & Pervious	Calculated CN 71	

Comment:

Node: B2

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	26.6200	1159567
25.00	27.0600	1178734
26.50	28.1000	1224036
27.00	29.1500	1269774
27.50	29.6700	1292425

Comment:

Node: B3L3

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft

Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.4200	61855
25.00	1.5600	67954
26.50	1.7000	74052
27.00	5.5800	243065
27.50	8.8500	385506
28.50	11.6300	506603

Comment:

Node: B4L4

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.4400	62726
25.00	1.5700	68389
26.50	1.7000	74052
27.00	7.0700	307969
27.50	11.5800	504425
28.50	15.0900	657320

Comment:

Node: B5L10

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.5600	67954
25.50	1.7200	74923
27.00	1.8900	82328
27.50	4.9600	216058
28.00	7.7100	335848
29.00	9.5200	414691

Comment:

Node: B5L11

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.0500	45738
25.50	1.1800	51401
27.00	1.3100	57064
27.50	5.5600	242194
28.00	9.2000	400752
29.00	12.2500	533610

Comment:

Node: B5L5

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.6100	113692
25.50	2.8400	123710
27.00	3.0800	134165
27.50	6.4000	278784
28.00	9.2200	401623
29.00	11.0900	483080

Comment:

Node: B5L6

Scenario: Scenario1
Type: Stage/Area

Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.2300	53579
25.50	1.3100	57064
27.00	1.3900	60548
27.50	4.0800	177725
28.00	6.5000	283140
29.00	8.5700	373309

Comment:

Node: B5L7

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	9.7000	422532
25.50	10.4100	453460
27.00	11.1300	484823
27.50	20.3700	887317
28.00	28.1400	1225778
29.00	33.5100	1459696

Comment:

Node: B5L9

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.4900	108464
25.50	2.7700	120661
27.00	3.0500	132858

Stage [ft]	Area [ac]	Area [ft2]
27.50	7.5800	330185
28.00	11.7200	510523
29.00	14.9200	649915

Comment:

Node: Boundary 1

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 23.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	23.00
0	0	0	60.0000	24.00
0	0	0	80.0000	25.00
0	0	0	100.0000	24.50
0	0	0	115.0000	23.30
0	0	0	336.0000	23.00

Comment:

Pipe Link: B4L4 to B3L3

Scenario: Scenario1
From Node: B3L3
To Node: B4L4
Link Count: 1
Flow Direction: Both
Damping: 0.0000 ft
Length: 731.00 ft
FHWA Code: 0
Entr Loss Coef: 1
Exit Loss Coef: 1
Bend Loss Coef: 0
Bend Location: 0.00 dec
Energy Switch: Energy

Upstream		Downstream	
Invert:	21.00 ft	Invert:	21.00 ft
Manning's N:	0.0130	Manning's N:	0.0130
Geometry: Circular		Geometry: Circular	
Max Depth:	3.00 ft	Max Depth:	3.00 ft
Bottom Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000
Top Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L10 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.70 ft	Invert:	20.60 ft
From Node:	B5L10	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	450.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L10		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.15 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L10	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	320.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	21.00 ft	Invert:	20.50 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	430.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	1	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			

Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L5 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft
From Node:	B5L5	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	385.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L6 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.40 ft
From Node:	B5L6	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	220.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L7 to B5L9		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft

From Node:	B5L7	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L9	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.50 ft	Max Depth:	2.50 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	585.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Drop Structure Link: WCS 2		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B2	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	Boundary 1	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	65.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Sharp Crested Vertical	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	24.00 ft	Op Table:	
Control Elevation:	24.00 ft	Ref Node:	
Max Depth:	999.00 ft	Discharge Coefficients	
Max Width:	1.50 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600

Orifice Table:

Weir Comment:

Drop Structure Comment:

Drop Structure Link: WCS 3		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B3L3	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B2	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	237.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Sharp Crested Vertical	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	24.00 ft	Ref Node:	
Control Elevation:	24.00 ft	Discharge Coefficients	
Max Depth:	0.25 ft	Weir Default:	3.200
Max Width:	0.31 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component		Bottom Clip	
Weir:	2	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Horizontal		

Geometry Type:	Rectangular
Invert:	25.60 ft
Control Elevation:	25.60 ft
Max Depth:	3.00 ft
Max Width:	5.00 ft
Fillet:	0.00 ft

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	3.200
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Drop Structure Comment:

Weir Link: WCS 4

Scenario:	Scenario1
From Node:	B4L4
To Node:	B2
Link Count:	1
Flow Direction:	Both
Damping:	0.0000 ft
Weir Type:	Broad Crested Vertical
Geometry Type:	Rectangular
Invert:	26.00 ft
Control Elevation:	26.00 ft
Max Depth:	999.00 ft
Max Width:	30.00 ft
Fillet:	0.00 ft

Bottom Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Top Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	2.800
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Comment:

Simulation: 100yr3d

Scenario:	Scenario1
Run Date/Time:	11/26/2025 8:38:35 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	216.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1
Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~SFWMD-72
Rainfall Amount: 14.20 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 10yr 1d

Scenario: Scenario1
Run Date/Time: 11/26/2025 9:15:25 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1
Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
IA Recovery Time: 12.0000 hr
Ia/S: 0.20 dec

dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FLMOD
		Rainfall Amount:	6.00 in
		Storm Duration:	24.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 25yr3d

Scenario: Scenario1
Run Date/Time: 11/26/2025 9:15:34 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources		Lookup Tables	
Rainfall Folder:		Boundary Stage Set:	
		Extern Hydrograph Set:	
Unit Hydrograph Folder:		Curve Number Set:	Scenario 1
		Green-Ampt Set:	
		Vertical Layers Set:	
		Impervious Set:	Scenario 1

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~SFWMD-72
		Rainfall Amount:	10.20 in
		Storm Duration:	72.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 5yr1d

Scenario:	Scenario1
Run Date/Time:	11/26/2025 9:16:05 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1
Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 12.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FLMOD
Rainfall Amount: 5.20 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	10yr 1d	B2	24.84	42.0421
Scenario1	10yr 1d	B3L3	25.92	13.5713
Scenario1	10yr 1d	B4L4	25.97	13.6057
Scenario1	10yr 1d	B5L10	25.86	13.0673
Scenario1	10yr 1d	B5L11	25.87	12.8036
Scenario1	10yr 1d	B5L5	25.73	15.8838
Scenario1	10yr 1d	B5L6	25.72	24.0671
Scenario1	10yr 1d	B5L7	25.71	24.1421
Scenario1	10yr 1d	B5L9	25.64	24.5088
Scenario1	25yr3d	B2	25.97	74.3888
Scenario1	25yr3d	B3L3	26.86	60.4576
Scenario1	25yr3d	B4L4	26.73	60.2311
Scenario1	25yr3d	B5L10	27.32	61.3670
Scenario1	25yr3d	B5L11	27.32	60.8877
Scenario1	25yr3d	B5L5	27.19	64.9738
Scenario1	25yr3d	B5L6	27.16	64.4155
Scenario1	25yr3d	B5L7	27.08	72.0555
Scenario1	25yr3d	B5L9	26.77	72.6888
Scenario1	5yr1d	B2	24.66	50.1984
Scenario1	5yr1d	B3L3	25.78	15.5401
Scenario1	5yr1d	B4L4	25.79	15.5401
Scenario1	5yr1d	B5L10	25.54	13.0176
Scenario1	5yr1d	B5L11	25.54	12.8223
Scenario1	5yr1d	B5L5	25.45	15.5867
Scenario1	5yr1d	B5L6	25.44	24.0901
Scenario1	5yr1d	B5L7	25.44	24.1651
Scenario1	5yr1d	B5L9	25.40	24.4401

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	100yr3d	B2	27.43	216.0068
Scenario1	100yr3d	B3L3	27.43	216.0068
Scenario1	100yr3d	B4L4	27.43	216.0068
Scenario1	100yr3d	B5L10	27.94	64.2225
Scenario1	100yr3d	B5L11	27.93	64.2225
Scenario1	100yr3d	B5L5	27.94	72.0985
Scenario1	100yr3d	B5L6	27.92	72.1068
Scenario1	100yr3d	B5L7	27.86	72.2151
Scenario1	100yr3d	B5L9	27.59	79.9485

Sim	Link Name	Maximum Flow Rate [cfs]	Time to Maximum Flow Rate [hrs]
25yr3d	WCS 2	11.10	109.5305

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	0.0000	24.00	0.00
25yr3d	B2	0.2525	24.00	0.00
25yr3d	B2	0.5010	24.00	0.00
25yr3d	B2	0.7552	24.00	0.00
25yr3d	B2	1.0052	24.00	0.00
25yr3d	B2	1.2552	24.00	0.00
25yr3d	B2	1.5052	24.00	0.00
25yr3d	B2	1.7552	24.00	0.00
25yr3d	B2	2.0052	24.00	0.00
25yr3d	B2	2.2552	24.00	0.00
25yr3d	B2	2.5052	24.00	0.00
25yr3d	B2	2.7552	24.00	0.00
25yr3d	B2	3.0052	24.00	0.00
25yr3d	B2	3.2552	24.00	0.00
25yr3d	B2	3.5052	24.00	0.00
25yr3d	B2	3.7552	24.00	0.00
25yr3d	B2	4.0052	24.00	0.00
25yr3d	B2	4.2552	24.00	0.00
25yr3d	B2	4.5052	24.00	0.00
25yr3d	B2	4.7552	24.00	0.00
25yr3d	B2	5.0052	24.00	0.00
25yr3d	B2	5.2552	24.00	0.00
25yr3d	B2	5.5052	24.00	0.00
25yr3d	B2	5.7552	24.00	0.00
25yr3d	B2	6.0052	24.00	0.00
25yr3d	B2	6.2552	24.00	0.00
25yr3d	B2	6.5052	24.00	0.00
25yr3d	B2	6.7552	24.00	0.00
25yr3d	B2	7.0052	24.00	0.00
25yr3d	B2	7.2552	24.00	0.00
25yr3d	B2	7.5052	24.00	0.00
25yr3d	B2	7.7552	24.00	0.00
25yr3d	B2	8.0052	24.00	0.00
25yr3d	B2	8.2552	24.00	0.00
25yr3d	B2	8.5052	24.00	0.00
25yr3d	B2	8.7552	24.00	0.00
25yr3d	B2	9.0052	24.00	0.00
25yr3d	B2	9.2552	24.00	0.00
25yr3d	B2	9.5052	24.00	0.00
25yr3d	B2	9.7552	24.00	0.00
25yr3d	B2	10.0052	24.00	0.00
25yr3d	B2	10.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	10.5052	24.00	0.00
25yr3d	B2	10.7552	24.00	0.00
25yr3d	B2	11.0052	24.00	0.00
25yr3d	B2	11.2552	24.00	0.00
25yr3d	B2	11.5052	24.00	0.00
25yr3d	B2	11.7552	24.00	0.00
25yr3d	B2	12.0052	24.00	0.00
25yr3d	B2	12.2552	24.00	0.00
25yr3d	B2	12.5052	24.00	0.00
25yr3d	B2	12.7552	24.00	0.00
25yr3d	B2	13.0052	24.00	0.00
25yr3d	B2	13.2552	24.00	0.00
25yr3d	B2	13.5052	24.00	0.00
25yr3d	B2	13.7552	24.00	0.00
25yr3d	B2	14.0052	24.00	0.00
25yr3d	B2	14.2552	24.00	0.00
25yr3d	B2	14.5052	24.00	0.00
25yr3d	B2	14.7552	24.00	0.00
25yr3d	B2	15.0052	24.00	0.00
25yr3d	B2	15.2552	24.00	0.00
25yr3d	B2	15.5052	24.00	0.00
25yr3d	B2	15.7552	24.00	0.00
25yr3d	B2	16.0052	24.00	0.00
25yr3d	B2	16.2552	24.00	0.00
25yr3d	B2	16.5052	24.00	0.00
25yr3d	B2	16.7552	24.00	0.00
25yr3d	B2	17.0052	24.00	0.00
25yr3d	B2	17.2552	24.00	0.00
25yr3d	B2	17.5052	24.00	0.00
25yr3d	B2	17.7552	24.00	0.00
25yr3d	B2	18.0052	24.00	0.00
25yr3d	B2	18.2552	24.00	0.00
25yr3d	B2	18.5052	24.00	0.00
25yr3d	B2	18.7552	24.00	0.00
25yr3d	B2	19.0052	24.00	0.00
25yr3d	B2	19.2552	24.00	0.00
25yr3d	B2	19.5052	24.00	0.00
25yr3d	B2	19.7552	24.00	0.00
25yr3d	B2	20.0052	24.00	0.00
25yr3d	B2	20.2552	24.00	0.00
25yr3d	B2	20.5052	24.00	0.00
25yr3d	B2	20.7552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	21.0052	24.00	0.00
25yr3d	B2	21.2552	24.00	0.00
25yr3d	B2	21.5052	24.00	0.00
25yr3d	B2	21.7552	24.00	0.00
25yr3d	B2	22.0052	24.00	0.00
25yr3d	B2	22.2552	24.00	0.00
25yr3d	B2	22.5052	24.00	0.00
25yr3d	B2	22.7552	24.00	0.00
25yr3d	B2	23.0052	24.00	0.00
25yr3d	B2	23.2552	24.00	0.00
25yr3d	B2	23.5052	24.00	0.00
25yr3d	B2	23.7552	24.00	0.00
25yr3d	B2	24.0052	24.00	0.00
25yr3d	B2	24.2552	24.00	0.00
25yr3d	B2	24.5052	24.00	0.00
25yr3d	B2	24.7552	24.00	0.00
25yr3d	B2	25.0052	24.00	0.00
25yr3d	B2	25.2552	24.00	0.00
25yr3d	B2	25.5052	24.00	0.00
25yr3d	B2	25.7552	24.00	0.00
25yr3d	B2	26.0052	24.00	0.00
25yr3d	B2	26.2552	24.00	0.00
25yr3d	B2	26.5052	24.00	0.00
25yr3d	B2	26.7552	24.00	0.00
25yr3d	B2	27.0052	24.00	0.00
25yr3d	B2	27.2552	24.00	0.00
25yr3d	B2	27.5052	24.00	0.00
25yr3d	B2	27.7552	24.00	0.00
25yr3d	B2	28.0052	24.00	0.00
25yr3d	B2	28.2552	24.00	0.00
25yr3d	B2	28.5052	24.00	0.00
25yr3d	B2	28.7552	24.00	0.00
25yr3d	B2	29.0052	24.00	0.00
25yr3d	B2	29.2552	24.00	0.00
25yr3d	B2	29.5052	24.00	0.00
25yr3d	B2	29.7552	24.00	0.00
25yr3d	B2	30.0052	24.00	0.00
25yr3d	B2	30.2552	24.00	0.00
25yr3d	B2	30.5052	24.00	0.00
25yr3d	B2	30.7552	24.00	0.00
25yr3d	B2	31.0052	24.00	0.00
25yr3d	B2	31.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	31.5052	24.00	0.00
25yr3d	B2	31.7552	24.00	0.00
25yr3d	B2	32.0052	24.00	0.00
25yr3d	B2	32.2552	24.00	0.00
25yr3d	B2	32.5052	24.00	0.00
25yr3d	B2	32.7552	24.00	0.00
25yr3d	B2	33.0052	24.01	0.00
25yr3d	B2	33.2552	24.01	0.00
25yr3d	B2	33.5052	24.01	0.00
25yr3d	B2	33.7552	24.01	0.00
25yr3d	B2	34.0052	24.01	0.00
25yr3d	B2	34.2552	24.01	0.00
25yr3d	B2	34.5052	24.01	0.00
25yr3d	B2	34.7552	24.01	0.00
25yr3d	B2	35.0052	24.01	0.00
25yr3d	B2	35.2552	24.01	0.00
25yr3d	B2	35.5052	24.01	0.00
25yr3d	B2	35.7552	24.01	0.00
25yr3d	B2	36.0052	24.01	0.00
25yr3d	B2	36.2552	24.01	0.00
25yr3d	B2	36.5052	24.01	0.00
25yr3d	B2	36.7552	24.01	0.00
25yr3d	B2	37.0052	24.01	0.00
25yr3d	B2	37.2552	24.01	0.00
25yr3d	B2	37.5052	24.01	0.00
25yr3d	B2	37.7552	24.01	0.00
25yr3d	B2	38.0052	24.02	0.00
25yr3d	B2	38.2552	24.02	0.00
25yr3d	B2	38.5052	24.02	0.00
25yr3d	B2	38.7552	24.02	0.00
25yr3d	B2	39.0052	24.02	0.00
25yr3d	B2	39.2552	24.02	0.00
25yr3d	B2	39.5052	24.02	0.00
25yr3d	B2	39.7552	24.02	0.00
25yr3d	B2	40.0052	24.02	0.00
25yr3d	B2	40.2552	24.02	0.00
25yr3d	B2	40.5052	24.02	0.00
25yr3d	B2	40.7552	24.02	0.01
25yr3d	B2	41.0052	24.03	0.01
25yr3d	B2	41.2552	24.03	0.01
25yr3d	B2	41.5052	24.03	0.01
25yr3d	B2	41.7552	24.03	0.01

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	42.0052	24.03	0.01
25yr3d	B2	42.2552	24.03	0.01
25yr3d	B2	42.5052	24.03	0.01
25yr3d	B2	42.7552	24.03	0.01
25yr3d	B2	43.0052	24.03	0.01
25yr3d	B2	43.2552	24.03	0.01
25yr3d	B2	43.5052	24.04	0.01
25yr3d	B2	43.7552	24.04	0.01
25yr3d	B2	44.0052	24.04	0.01
25yr3d	B2	44.2552	24.04	0.01
25yr3d	B2	44.5052	24.04	0.01
25yr3d	B2	44.7552	24.04	0.01
25yr3d	B2	45.0052	24.04	0.02
25yr3d	B2	45.2552	24.04	0.02
25yr3d	B2	45.5052	24.04	0.02
25yr3d	B2	45.7552	24.05	0.02
25yr3d	B2	46.0052	24.05	0.02
25yr3d	B2	46.2552	24.05	0.02
25yr3d	B2	46.5052	24.05	0.02
25yr3d	B2	46.7552	24.05	0.02
25yr3d	B2	47.0052	24.05	0.02
25yr3d	B2	47.2552	24.05	0.02
25yr3d	B2	47.5052	24.06	0.03
25yr3d	B2	47.7552	24.06	0.03
25yr3d	B2	48.0052	24.06	0.03
25yr3d	B2	48.2552	24.06	0.03
25yr3d	B2	48.5052	24.06	0.03
25yr3d	B2	48.7552	24.06	0.03
25yr3d	B2	49.0052	24.07	0.03
25yr3d	B2	49.2552	24.07	0.04
25yr3d	B2	49.5052	24.07	0.04
25yr3d	B2	49.7552	24.07	0.04
25yr3d	B2	50.0052	24.07	0.04
25yr3d	B2	50.2552	24.07	0.04
25yr3d	B2	50.5052	24.08	0.05
25yr3d	B2	50.7552	24.08	0.05
25yr3d	B2	51.0052	24.08	0.05
25yr3d	B2	51.2552	24.08	0.05
25yr3d	B2	51.5052	24.09	0.06
25yr3d	B2	51.7552	24.09	0.06
25yr3d	B2	52.0052	24.09	0.06
25yr3d	B2	52.2552	24.09	0.06

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	52.5052	24.09	0.07
25yr3d	B2	52.7552	24.10	0.07
25yr3d	B2	53.0052	24.10	0.07
25yr3d	B2	53.2552	24.10	0.08
25yr3d	B2	53.5052	24.11	0.08
25yr3d	B2	53.7552	24.11	0.08
25yr3d	B2	54.0052	24.11	0.09
25yr3d	B2	54.2552	24.12	0.09
25yr3d	B2	54.5052	24.12	0.09
25yr3d	B2	54.7552	24.12	0.10
25yr3d	B2	55.0052	24.13	0.10
25yr3d	B2	55.2552	24.13	0.11
25yr3d	B2	55.5018	24.14	0.11
25yr3d	B2	55.7558	24.14	0.12
25yr3d	B2	56.0026	24.15	0.12
25yr3d	B2	56.2538	24.15	0.13
25yr3d	B2	56.5046	24.16	0.13
25yr3d	B2	56.7501	24.17	0.14
25yr3d	B2	57.0025	24.17	0.15
25yr3d	B2	57.2523	24.18	0.15
25yr3d	B2	57.5033	24.19	0.16
25yr3d	B2	57.7501	24.20	0.17
25yr3d	B2	58.0004	24.21	0.18
25yr3d	B2	58.2513	24.22	0.19
25yr3d	B2	58.5054	24.23	0.20
25yr3d	B2	58.7500	24.25	0.21
25yr3d	B2	59.0013	24.26	0.22
25yr3d	B2	59.2518	24.28	0.24
25yr3d	B2	59.5015	24.31	0.25
25yr3d	B2	59.7502	24.34	0.27
25yr3d	B2	60.0002	24.40	0.29
25yr3d	B2	60.2505	24.49	0.32
25yr3d	B2	60.5010	24.60	0.36
25yr3d	B2	60.7507	24.70	0.42
25yr3d	B2	61.0019	24.80	0.48
25yr3d	B2	61.2501	24.88	0.55
25yr3d	B2	61.5004	24.95	0.64
25yr3d	B2	61.7516	25.02	0.73
25yr3d	B2	62.0019	25.08	0.84
25yr3d	B2	62.2532	25.13	0.95
25yr3d	B2	62.5000	25.18	1.06
25yr3d	B2	62.7516	25.22	1.19

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	63.0011	25.26	1.32
25yr3d	B2	63.2556	25.30	1.46
25yr3d	B2	63.5040	25.34	1.60
25yr3d	B2	63.7529	25.37	1.74
25yr3d	B2	64.0034	25.41	1.90
25yr3d	B2	64.2538	25.44	2.05
25yr3d	B2	64.5055	25.47	2.21
25yr3d	B2	64.7530	25.50	2.38
25yr3d	B2	65.0005	25.52	2.54
25yr3d	B2	65.2505	25.55	2.72
25yr3d	B2	65.5063	25.57	2.90
25yr3d	B2	65.7522	25.60	3.07
25yr3d	B2	66.0055	25.62	3.26
25yr3d	B2	66.2555	25.64	3.44
25yr3d	B2	66.5055	25.66	3.63
25yr3d	B2	66.7555	25.68	3.82
25yr3d	B2	67.0055	25.70	4.01
25yr3d	B2	67.2555	25.72	4.21
25yr3d	B2	67.5055	25.73	4.41
25yr3d	B2	67.7555	25.75	4.61
25yr3d	B2	68.0055	25.77	4.81
25yr3d	B2	68.2555	25.78	5.01
25yr3d	B2	68.5055	25.80	5.22
25yr3d	B2	68.7555	25.81	5.42
25yr3d	B2	69.0055	25.82	5.63
25yr3d	B2	69.2555	25.84	5.84
25yr3d	B2	69.5055	25.85	6.06
25yr3d	B2	69.7555	25.86	6.27
25yr3d	B2	70.0055	25.87	6.48
25yr3d	B2	70.2555	25.88	6.70
25yr3d	B2	70.5055	25.89	6.92
25yr3d	B2	70.7555	25.90	7.13
25yr3d	B2	71.0055	25.91	7.35
25yr3d	B2	71.2555	25.92	7.57
25yr3d	B2	71.5055	25.92	7.79
25yr3d	B2	71.7555	25.93	8.01
25yr3d	B2	72.0055	25.94	8.23
25yr3d	B2	76.0055	25.96	11.71
25yr3d	B2	80.0055	25.96	14.99
25yr3d	B2	84.0055	25.96	18.19
25yr3d	B2	88.0055	25.94	21.49
25yr3d	B2	92.0055	25.92	24.85

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	96.0055	25.90	28.25
25yr3d	B2	100.0055	25.87	31.67
25yr3d	B2	104.0055	25.83	35.18
25yr3d	B2	108.0055	25.78	38.81
25yr3d	B2	112.0055	25.74	42.47
25yr3d	B2	116.0055	25.69	46.03
25yr3d	B2	120.0055	25.64	49.45
25yr3d	B2	124.0055	25.60	52.72
25yr3d	B2	128.0055	25.55	55.86
25yr3d	B2	132.0055	25.50	58.85
25yr3d	B2	136.0055	25.45	61.69
25yr3d	B2	140.0055	25.40	64.38
25yr3d	B2	144.0055	25.35	66.94
25yr3d	B2	148.0055	25.31	69.38
25yr3d	B2	152.0055	25.27	71.70
25yr3d	B2	156.0055	25.23	73.91
25yr3d	B2	160.0055	25.19	76.03
25yr3d	B2	164.0055	25.16	78.06
25yr3d	B2	168.0055	25.13	80.00
25yr3d	B2	172.0055	25.10	81.87
25yr3d	B2	176.0055	25.07	83.66
25yr3d	B2	180.0055	25.04	85.38
25yr3d	B2	184.0055	25.02	87.04
25yr3d	B2	188.0055	24.99	88.64
25yr3d	B2	192.0055	24.97	90.17
25yr3d	B2	196.0055	24.94	91.65
25yr3d	B2	200.0055	24.92	93.08
25yr3d	B2	204.0055	24.90	94.45
25yr3d	B2	208.0055	24.88	95.78
25yr3d	B2	212.0055	24.86	97.06
25yr3d	B2	216.0055	24.84	98.30
25yr3d	B2	220.0055	24.82	99.49
25yr3d	B2	224.0055	24.80	100.65
25yr3d	B2	228.0055	24.78	101.76
25yr3d	B2	232.0055	24.77	102.84
25yr3d	B2	236.0055	24.75	103.89
25yr3d	B2	240.0055	24.73	104.90
25yr3d	B2	244.0055	24.72	105.88
25yr3d	B2	248.0055	24.70	106.83
25yr3d	B2	252.0055	24.69	107.75
25yr3d	B2	256.0055	24.67	108.64
25yr3d	B2	260.0055	24.66	109.50

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	264.0055	24.65	110.33
25yr3d	B2	268.0055	24.63	111.14
25yr3d	B2	272.0055	24.62	111.93
25yr3d	B2	276.0055	24.61	112.69
25yr3d	B2	280.0055	24.59	113.43
25yr3d	B2	284.0055	24.58	114.15
25yr3d	B2	288.0055	24.57	114.84
25yr3d	B2	292.0055	24.56	115.52
25yr3d	B2	296.0055	24.55	116.17
25yr3d	B2	300.0055	24.54	116.81
25yr3d	B2	304.0055	24.53	117.43
25yr3d	B2	308.0055	24.52	118.03
25yr3d	B2	312.0055	24.51	118.61
25yr3d	B2	316.0055	24.50	119.18
25yr3d	B2	320.0055	24.49	119.73
25yr3d	B2	324.0055	24.48	120.27
25yr3d	B2	328.0055	24.47	120.78
25yr3d	B2	332.0055	24.46	121.29
25yr3d	B2	336.0055	24.45	121.78
25yr3d	B2	340.0055	24.44	122.26
25yr3d	B2	344.0055	24.44	122.72
25yr3d	B2	348.0055	24.43	123.17
25yr3d	B2	352.0055	24.42	123.61
25yr3d	B2	356.0055	24.41	124.04
25yr3d	B2	360.0055	24.40	124.45

PART IV #23-07

Recommended Action

Engineering Review:

By: _____ 3/3/2026 **Approved _X_ Denied ___**

District Field Review:

By: _____ Approved ___ Denied ___

District Manager: _____ Date: _____

Comments:

The referenced permit modification application resubmission by Atwell LLC proposes revisions to previously approved Permit No. 23-07 for a multi-acre residential development, Ibis Landing (FKA Copperhead). The southeastern portion of the project has been modified to include changes to the site including drainage infrastructure. AIM Engineering & Surveying, Inc. has reviewed the submitted documents and recommends approval with the following stipulations:

1. Sheet 11 depicting the modifications and revisions to section Q-Q should be provided.

2. The permittee shall submit signed and sealed record drawings within thirty (30) days of project completion. The drawings must include sufficient detail to demonstrate that the permitted facilities have been constructed in substantial compliance with LA-MSID–approved plans and design.

3. Prior to commencing construction, the permittee shall provide LA-MSID with copies of all applicable permits from other governing agencies.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: PER #25-40 (M1) Blackstone Corp Park Minor Modification

Meeting Date: March 16, 2026

Agenda Item # 2D

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve PER #25-40 (M1) Blackstone Corp Park Minor Modification, with any listed stipulations.

What the action accomplishes: Allows minor changes to the existing permit #25-40 approved April 21, 2025.

Fiscal Impact: Permit Fees: $500.00

Staff comments: Revised the lot sizes and number of lots in the southeast corner and changed ownership.

Staff review and recommended approval:

[x] **MANAGER** [ ] **ASSISTANT DISTRICT MGR**

[x] **ATTORNEY** [ ] **FIELD DIRECTOR**

[ ] **ENGINEER** [ ] **FINANCE DIRECTOR**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

LEHIGH ACRES - MUNICIPAL SERVICES IMPROVEMENT DISTRICT PERMIT

THIS PERMIT is granted this 16th day of March, 2026, by Lehigh Acres Municipal Services Improvement District (hereinafter referred to as "DISTRICT"), 601 East County Lane, Lehigh Acres, Florida 33936, to MNL Fort Myers Blackstone, LLC, 6562 University Pkwy, Suite 210, Sarasota, FL 34240, hereinafter referred to as the "Permittee") as a non-exclusive permit for a proposed corporate park on 61.78 acres located at 336 Blackstone Drive, with an on-site storm water management system and outfall to Gecko Canal (57-20-3), in Lehigh Acres, FL.

W I T N E S S E T H:

1. In the event the DISTRICT is not the fee title owner of the real property (the "Property") to which this Permit is applicable, the Permittee agrees, prior to commencement, to obtain all necessary consents from the fee title owners of the Property.

2. This Permit does not constitute a waiver of the necessity by the Permittee to obtain such other necessary and appropriate permits required by other governmental bodies or agencies; therefore the Permittee, prior to commencement, is required to obtain any and all other applicable federal, state, and local permits required in connection with Permittee's use of the Property and agrees that at all times it will comply with the requirements of all federal, state and local laws, ordinances, rules and regulations applicable or pertaining to the use of the Property by the Permittee.

3. Permittee understands and agrees that the use of the Property pursuant to this Permit is subordinate to the rights and interests of the DISTRICT and to the extent applicable that of the fee title owner of the Property. Further, Permittee does hereby stipulate that the Permittee is not relying upon any representations whatsoever by the DISTRICT regarding the DISTRICT'S right, title, interest, or ownership as to the Property for which this Permit is applicable.

4. The Permittee, by accepting this permit, specifically agrees to allow authorized District Personnel, upon presentation of credentials or other documents as may be required by law, access to the premises, at reasonable times, where the permitted activity is located or conducted for the purpose of:
    a. Having access to and copying any records that must be kept under the conditions of the permit:
    b. Reviewing the facility, equipment, practices, or operations regulated or required under this permit; and
    c. Sampling or monitoring any substances or parameters at any location reasonably necessary to assure compliance with this permit or District rules.

5. The DISTRICT specifically reserves the right to maintain its facilities located over, under or upon the Property; to make improvements; add additional facilities; maintain, construct or alter its roads; maintain any facilities, devices, or improvements on the Property which aid in or are necessary to the DISTRICT'S operation; and the right to enter upon the Property at all times for such purposes. Permittee understands that in the exercise of such rights and interest, the DISTRICT may require Permittee, at no cost to DISTRICT, to relocate, alter, or remove the Permittee's facilities and equipment or other improvements made by Permittee pursuant to this Permit which interfere with or prevent the DISTRICT, in the DISTRICT'S opinion, from properly and faithfully constructing, improving and maintaining its facilities. If the Permittee should fail to relocate, alter, or remove the Permittee's facilities, equipment or other improvements made by the Permittee pursuant to this Permit within a reasonable time period following receipt of notice from the DISTRICT requesting same, then in that event the DISTRICT shall have the right to enter upon the lands and make such relocation, alterations or removal of the Permittee's facilities, equipment/or other improvements at no cost to DISTRICT.

6. Permittee agrees that it will not use the Property in any manner which interferes with the DISTRICT'S use of the Property or causes a hazardous condition to exist.

7. The DISTRICT assumes no responsibility for the ownership, operation and/or maintenance of the Permittee's facilities as permitted herein.

8. The Permittee agrees to backfill any excavation it makes within the Property and to repair or replace (i) any below-grade authorized facilities and features located therein, including but not limited to: water, sewer, stormwater or irrigation facilities, structures or appurtenances and (ii) as to at or above-grade facilities and features, this requirement shall only be applicable to grasses and sod, sidewalks, and roadway improvements (the term roadway improvements shall include but not be limited to curbing, roadway base, sub grade and asphaltic or concrete surfaces) which are removed or damaged as a direct result of said excavation. Such repair or replacement shall substantially restore these features to their condition as it existed immediately prior to the event precipitating the excavation.

9. All below-grade Permittee installations, including water and sewer lines, whether sub aqueous or not, together with appurtenant facilities, shall be located underground and have a minimum ground cover of thirty inches (30") below original design profile grade, including District waterways and roadways.

10. All Permittee activity related to sub aqueous utility, water and/or sewer lines, including appurtenant facilities, shall be conducted in such fashion that the primary functional purpose of the canal or lake body facility, specifically flood control, within which the activity is taking place is not adversely affected. More specifically, sub aqueous construction shall conform to the criteria given in the Lehigh Acres Municipal Services Improvement District (Lehigh Acres – MSID) Administrative Policy and Procedures.

11. Permittee shall, at no expense to DISTRICT and within a reasonable time following notice, adjust the positions and elevations of its facilities as may be required in connection with future improvements to, or construction of, works of the DISTRICT.

12. Permittee does hereby indemnify and hold harmless the DISTRICT, its Board of Commissioners, officers, personnel, and it's Engineers against any claims, losses, damages (including consequential), expenses, or legal fees that might arise out or result from the implementation of the proposed project by the Permittee or that may be caused by a failure of DISTRICT facilities or property.

13. The Permittee shall be obligated at the time of submission of its Permit application, to provide a detail of any other existing improvements located over, under, upon or through the Property and the Permittee's accompanying plans and specifications shall clearly indicate how the Permittee will comply with required clearances between the requested permitted activity and existing improvements located over, under and upon the Property, including compliance with other permitted users duly adopted and published standards.

14. Permittee shall reimburse the DISTRICT for its legal, engineering and other expenses incurred as a result of the implementation of the proposed project.

15. If Permittee should violate any of the terms or conditions of the Permit and shall not correct or remedy same within ten (10) days of receiving written notice of said violation from the DISTRICT, then in that event, the DISTRICT may, at its option, revoke, cancel and terminate this Permit.

16. This Permit may not be assigned by the Permittee without the prior written approval of the DISTRICT.

17. Unless otherwise adjusted, modified or terminated as set forth hereinabove, including the Conditions

attached hereto, this Permit shall continue in full force and effect so long as Permittee complies with the terms of this Permit.

18. In any litigation, including breach, enforcement or interpretation arising out of this Permit, the prevailing party to this Permit shall be entitled to recover reasonable attorney's fees, costs and expenses related to said action from the non-prevailing party.

19. That attached hereto are Supplemental Conditions to this Permit which are incorporated herein and made a part hereof.

LEHIGH ACRES – MUNICIPAL SERVICES IMPROVEMENT DISTRICT

BY: _____

NAME TYPED: David E. Lindsay

TITLE: District Manager

SUPPLEMENTAL CONDITIONS TO PERMIT NO. 25-40 (M1)

A. GENERAL CONDITIONS

1. Notification shall be given by the Permittee to the District Engineer, AIM Engineering, at 239-332-4569 forty-eight (48) hours prior to commencement of the permitted activity. The District Engineer shall establish points of construction, if applicable, which will require inspection by the District Engineer prior to construction of the permitted activity. When the Permittee considers the permitted activity complete, the Permittee shall notify the District Engineer and schedule an on-site final inspection which shall be held in the presence of a representative of the Permittee and the District Engineer.

2. The permitted activity shall be implemented, constructed and/or installed in full accordance with the approved plans and specifications within six (6) full calendar years from the date of permit approval. Deviations from the approved plans or specifications shall be coordinated with and approved by the District Engineer prior to implementation of any such deviation.

3. When working in a District road right-of-way, not more than one-half (½) of the road or street shall be closed and traffic shall be controlled so as to provide for the safety and minimum hindrance to the public. All traffic control operations shall conform to the most current issue of the Florida Department of Transportation publication "Manual on Traffic Controls and safe Practices for Street and Highway Construction, Maintenance and Utility Operations."

4. The District Engineer shall be the final authority as to the quality and quantity of the material and works required to satisfy the terms and conditions of this Permit as they relate to District improvements and facilities.

5. Upon completion of the permitted activity and after its final review and acceptance by the District Engineer THE PERMITTEE SHALL DELIVER TO THE DISTRICT OFFICE ONE COMPLETE SET OF THE FINAL "RECORD DRAWINGS." FAILURE TO PROVIDE THE FINAL RECORD DRAWINGS MAY RESULT IN THE REVOCATION, CANCELLATION AND TERMINATION OF THIS PERMIT.

6. The above Conditions shall be continuing obligations of the Permittee and shall be complied with in the event of any subsequent maintenance, repair, replacement or modification of the permitted improvement.

Stipulations:

1). Permit #25-40 (M1) is approved according to the revised master site and drainage plans signed/sealed February 9th, 2026.

2). Maintenance of the entire master on-site storm water management system and lakes are the responsibility of the Permittee.

3). Any disturbed canal right-of-way must be restored with sod and maintained until fully established. Seeding will not be permitted.

4). An "As-Built" survey showing the completed improvements (including perimeter berm, pipe inverts, finished floor elevations and all drainage structure elevations) must be submitted to Lehigh Acres - MSID within 30 days following completion of the improvements.

5). Each development parcel will need to submit their site plans and drainage calculations for individual drainage permits from LAMSID, in compliance with the overall revised master plan being approved under this permit modification.

6). Permittee shall obtain any other permits required by EPA, FDEP, SFWMD, FDOT and Lee County prior to starting construction and submit a copy to LA-MSID.

PART I

DATE SUBMITTED: 02 - 10 - 26

PERMIT #: 25 - 40 (M1)

APPLICATION FOR A PERMIT

***Note**: The review of permit applications will be conducted per the Lehigh Acres Municipal Services Improvement District's Administrative Policy and Procedures

AGENT NAME: Atwell, LLC **PHONE #:** (239)939-5490

PROJECT NAME: Blackstone Corporate Park

TOTAL ACREAGE: 61.78 ACRES

Property Address (STRAP associated with this address will be used as the primary STRAP for this project):					
Mailing Address:	336 Blackstone Drive				
City:	Fort Myers	**State:**	Florida	**Zip:**	33913

STRAP NUMBERS (*attach additional sheet if necessary*):

2	0	-	4	5	-	2	7	-	7	2	-	0	0	0	5	1	-	0	0	0	0
2	0	-	4	5	-	2	7	-	5	5	-	0	0	0	4	4	-	0	0	0	0
2	0	-	4	5	-	2	7	-	6	3	-	0	0	2	3	9	-	0	0	9	1
2	0	-	4	5	-	2	7	-	6	3	-	0	0	2	3	9	-	0	1	0	0
		-			-			-			-						-				

Brief description of proposed project:

Application for: Minor Modification to existing LAMSID Permit #25-40 for the backbone infrastructure with roads and stormwater system to support future commercial development.

Is the project proposed to be developed in phases? ☐ **Yes** ☒ **No**

Estimated # of Phases:	N/A
***Permit Fee:**	$ 500.00 (Minor Modification Fee) **to be verified by staff)**

* See Administrative Policy and Procedures Sections 4.15 – 4.17 for current Permit Fee Amount.

PART II

OWNER/APPLICANT/ DEVELOPER INFORMATION

A.

Name of Applicant	CNL Fort Myers Blackstone, LLC						
Mailing Address:	**Street:**	6562 University Pkwy, Suite 210					
City:	Sarasota	**State:**	FL			**Zip:**	34240
Phone No.	**Area Code:**	941	**Number:**	454 - 2714		**Ext.**	
Fax No.	**Area Code:**		**Number:**	-			
E-mail Address:	dmoyer@castoinfo.com						

B. Relationship of applicant to property*:

☒ **Owner*** ☐ **Trustee*** ☐ **Option holder*** ☐ **Lessee***

☐ **Contract Purchaser*** ☐ **Other (indicate)*** _____

C.

Name of Developer:	CNL Fort Myers Blackstone, LLC						
Mailing Address:	**Street:**	6562 University Pkwy, Suite 210					
City:	Sarasota	**State:**	FL			**Zip:**	34240
Phone No.	**Area Code:**	(941)	**Number:**	454 - 2714		**Ext.**	
Fax No.	**Area Code:**	( )	**Number:**	-			
E-mail Address:	dmoyer@castoinfo.com						

D.

Name of Authorized Agent(s):	Rachel Tracey, P.E. - Atwell, LLC						
Mailing Address:	**Street:**	10511 Six Mile Cypress Pkwy, Suite 101					
City:	Fort Myers	**State:**	Florida			**Zip:**	33966
Phone No.	**Area Code:**	(239)	**Number:**	939 - 5490		**Ext.**	
Fax No.	**Area Code:**	( )	**Number:**	-			
E-mail Address:	rtracey@atwell.com						

E.

Name of Property Owner:	Same as Applicant						
Mailing Address:	**Street:**						
City:		**State:**				**Zip:**	
Phone No.	**Area Code:**	( )	**Number:**	-		**Ext.**	
Fax No.	**Area Code:**	( )	**Number:**	-			
E-mail Address:							

*A disclosure of the name of all persons or entities having an ownership interest in the property is required (attached).

PART III

ADDITIONAL STRAP NUMBERS:
20-45-27-55-00044.0000, 20-45-27-63-00239.0090, 20-45-27-63-00239.0100, 20-45-27-71-00050.0000

DISCLOSURE OF OWNERSHIP INTEREST FORM FORM:

PRIMARY STRAP NUMBER: 20 - 45 - 27 - 72 - 00051 - 0000

1. If the property is owned in the fee simple by an **INDIVIDUAL**, tenancy by the entirety, tenancy in common, or joint tenancy, list all parties with an ownership interest as well as the percentage of such interest.

Name and Address	Percentage of Ownership

2. If the property is owned by a **CORPORATION**, list the officers and stockholders and the percentage of stock owned by each.

Name, Address, Office	Percentage of Stock

3. If the property is in the name of a **TRUSTEE**, list the beneficiaries of the trust with percentages of interest.

Name and Address	Percentage of Interest

4. If the property is in the name of a **GENERAL PARTNERSHIP OR LIMITED PARTNERSHIP**, list the names of general and limited partners.

Name and Address	Percentage of Interest

5. If there is a **CONTACT FOR PURCHASE**, whether contingent on this application Or not, and whether a Corporation, Trustee, or Partnership, list the names of the contract purchasers below, including the officers, stockholders, beneficiaries, or partners.

Name and Address	Percentage of Interest

Date of Contract (mm/dd/yy): / /

6. If any contingency clause or contract terms involve additional parties, list all individuals or officers, if a corporation, partnership, or trust.

Name and Address	Percentage of Interest

For any changes of ownership of changes in contracts for purchase subsequent to the date of the application, but prior to the date of final certificate for compliance, a supplemental disclosure of interest must be filed.

The above is a full disclosure of all parties of interest in this application, to the best of my knowledge and belief.

Michael Chadwick
SIGNATURE OF APPLICANT

Manager, CNL Fort Myers Blackstone, LLC
PRINTED OR TYPED NAME OF APPLICANT

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this 26th day of January (m), 2026(y), by Michael Chadwick who is personally know to me or has produced _____ as identification.

Notary Public State of Florida
Erica McMillin
My Commission HH 452659
Expires 10/9/2027

SIGNATURE OF NOTARY PUBLIC
Erica McMillin
PRINTED NAME OF NOTARY PUBLIC

PART VI

SUBMITTALS

NO. OF COPIES		SUBMITTAL DOCUMENT
SUB'D	REQ'D	
		ITEMS REQUIRED FOR INITIAL SUBMITTAL
2	2	**Construction/Development Plans** signed and sealed by a Licensed Florida Professional Engineer including: 1. Location map showing the location of the proposed project in relation to adjacent streets and LA-MSID facilities. 2. Details of proposed works that fully communicate the work and/or use desired (i.e. Existing Conditions Survey, Proposed Site Plan, Paving Grading & Drainage Plan, Cross Sections, Element Details, and Erosion Control Plan sheets). 3. Sufficient survey data to determine elevations, property and ROW lines, limits of existing and proposed conditions, and notation of utilized vertical datum (NAVD/NGVD).
2	2	**Stormwater Drainage Calculations** signed and sealed by a Licensed Florida Professional Engineer including, but not limited to, computer stormwater routing modeling data (ICPR4, Cascade, etc.) to verify compliance with the Design Criteria (input summary data below): **Water Quality Volume (Required):** See SWM Report which includes nutrient analysis based on new SFWMD (ACRE-FT) **Water Quality Volume (Provided):** performance standards (ACRE-FT) **Control Elevation:** 29.22 (FT-NGVD) **Minimum Road/Parking Elevation:** 31.89 (FT-NGVD) *(above the 10-year, 1-day design storm peak stage elevation)* **Minimum Perimeter Berm Elevation:** 32.92 (FT-NGVD) *(above the 25-year, 3-day design storm peak stage elevation)* **Minimum Finished Floor Elevation:** 33.52 (FT-NGVD) *(above the 100-year, 3-day design storm peak stage elevation and previously experienced high-water levels external to project)*
2	2	**Flash Drive / CD** containing plans in both AutoCAD (.dwg file drawn in "State Plane Coordinates") and PDF formats.
2	2	**Permit Application** (PARTS I & II)
2	2	**Disclosure of Owner Form (signed and notarized)** (PART III)
2	2	**Professional Consultant Form & Submittals** (PARTS V & VI)
2	2	**Copy of Approved FDEP Notice of Intent (NOI)** for all sites one (1) acre and greater in size.
2	2	**Copy of approved SWFMD permit**, for all sites required to get a permit, **or FDEP "10/2 Self-Certification"** approval.
2	1	**Check for appropriate Permit Fees** made out to LA-MSID.

PART V

PROFESSIONAL CONSULTANT FORM

ATTORNEY

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

ENGINEER

Name:	Rachel Tracey, P.E., Atwell, LLC						
Mailing Address:	10511 Six Mile Cypress Pkwy., Suite 101						
City:	Fort Myers	State:		Florida		Zip:	33966
Business No.	Area Code:	(239)	Number:	939-5490		Ext.	
Other Contact:							

OTHER REPRESENTATIVE/PROFESSIONAL

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

Ent	Acct No	Invoice	Date	Special Handling	Reference	Deposit Codes	Net
755008	16601	CR016893BK26	1/29/2026		LAMSID APP FEE		500.00

Permit # 25-40 (MH)

COPY

RECEIVED
FEB 10 2026
LEHIGH ACRES
MUNICIPAL SERVICES
IMPROVEMENT DISTRICT

						Check Amount
Payor:	CNL FORT MYERS BLACKSTONE, LLC	Vendor ID	Date	Check No.		
Payee:	LEHIGH ACRES MUNICIPAL SERVICES	601008	1/30/2026	000006		500.00

Retain this statement for your records

VERIFY THE AUTHENTICITY OF THIS MULTI-TONE SECURITY DOCUMENT. CHECK BACKGROUND AREA CHANGES COLOR GRADUALLY FROM TOP TO BOTTOM.

CNL FORT MYERS BLACKSTONE, LLC
250 CIVIC CENTER DRIVE, SUITE 500
COLUMBUS, OH 43215

THE HUNTINGTON NATIONAL BANK
41 South High Street (HC0840)
Columbus, OH 43287

25-2
440

Details on back. Security Features Included

Date	Check No.	Check Amount
1/30/2026	000006	500.00

Five Hundred AND 00/100 Dollars

Pay to the order of:

VOID IF NOT CASHED WITHIN 90 DAYS WITHIN DATE OF ISSUE

LEHIGH ACRES MUNICIPAL SERVICES
IMPROVEMENT DISTRICT
601 EAST COUNTY LANE
LEHIGH ACRES, FL 33936

John D Carr

⑈000006⑈ ⑆044000241⑆ 018943051433⑈

ATWELL

February 10, 2026

Lehigh Acres Municipal Services Improvement District
601 East County Lane
Lehigh Acres, Florida 33936

Reference: **Blackstone Corporate Park**
Minor Modification to LAMSID Permit #25-40

To Whom It May Concern,

The above-mentioned project proposes a minor modification to existing LAMSID permit #25-40 for Blackstone Corporate Park. Please see enclosed updated plans and report. The site has been modified as the previous box store tenant for parcel 2 is no longer going at this site. The previous 10.55-acre parcel two has been adjusted for additional lake with smaller proposed parcels 2, 16, and 17 for future commercial development.

The site provides additional attenuation over the previous submitted and decreases the previous requested impervious area.

Attached please find the following items being submitted for review:

One (1) of each of the following items is included with this submittal:
1. Check in the amount of $500.00 for the Minor Modification Application Fee
2. CD with PDF and AutoCAD file of Engineering Plans-

Three (3) copies of the following items are included with this submittal:
1. Copy of Existing LAMSID 25-40 Approval
2. Cover Letter
3. Revised Stormwater Management Report
4. Revised Application for a Permit
5. Revised LAMSID Signatures
6. Engineering Plans

If you have any questions regarding this submittal, please feel free to contact me at our Fort Myers office.

Sincerely,
ATWELL, LLC

Rachel Tracey, P.E.
Senior Project Engineer
rtracey@atwell.com

Cc: Megan Knight, mknight@atwell.com

RECEIVED
FEB 10 2026
LEHIGH ACRES
MUNICIPAL SERVICES
IMPROVEMENT DISTRICT

March 3, 2026

Lehigh Acres Municipal Services Improvement District
601 East County Lane
Lehigh Acres, Florida 33936

Reference: **Blackstone Corporate Park**
Minor Modification to LAMSID Permit #25-40
Change of Ownership

To Whom It May Concern,

Please accept this letter and the attached documents as our formal request to change the permittee for the above-referenced permit to the following:

Original Permittee / Previous Owner:

MCIN Bell, LLC
5571 Halifax Avenue
Fort Myers, FL 33912

New Permittee/ New Property Owner:

CNL Fort Myers Blackstone, LLC
6562 University Pkwy, Suite 210
Sarasota, Florida 34240

Enclosed please find the following documents in support of this request:

1. Warranty Deed
2. Property Data Page
3. Evidence of Authority (SunBiz Information) showing signature authority

If you have any questions regarding this submittal, please feel free to contact me at our Fort Myers office.

Sincerely,
ATWELL, LLC

Rachel Tracey, P.E.
Senior Project Engineer
rtracey@atwell.com

Cc: Megan Knight, mknight@atwell.com

Kevin C. Karnes, Lee County Clerk of the Circuit Court & Comptroller
INSTR# 2026000005125, DocType D, Pages 3, Recorded 1/8/2026 at 12:23 PM, DeputyClerk WMILLER
Rec Fees: $27.00 Deed Doc: $99,050.00 ERECORD

Prepared by and Return to:
Matthew Staggs, Esq.
Blalock Walters, P.A.
802 11th Street West
Bradenton, Florida 34205
File: 41852.202

DOCUMENTARY STAMPS IN
THE AMOUNT OF $99,050.00
ARE AFFIXED HERETO.

SPECIAL WARRANTY DEED

This **Special Warranty Deed** made this 7th day of January, 2026, by **MCIN Bell, LLC, a Florida limited liability company**, whose address is 12320 ITEC Park Drive, #405, Fort Myers, Florida 33913, hereinafter called the "Grantor", and **CNL Fort Myers Blackstone, LLC, a Florida limited liability company**, whose address is 6562 University Pkwy., Suite 210, Sarasota, Florida 34240, hereinafter called the "Grantee", for the Grantor's undivided interest in the property described below:

(Whenever used herein the term "Grantor" and "Grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts, and trustees).

WITNESSETH, that the Grantor, for and in consideration of the sum of Ten 00/100 Dollars ($10.00) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, alienates, remises, releases, conveys, and confirms unto the Grantee, all of its interest in the following described property situated in Lee County, Florida (the "Property") to wit:

See Exhibit "A" attached hereto.

SUBJECT TO taxes for the year of 2026 and subsequent years, and zoning and other prohibitions and regulations imposed by governmental authorities.

TOGETHER with all the tenements, hereditaments, and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD the same in fee simple forever.

AND the Grantor does hereby covenant with the Grantee that at the time of the delivery of this special warranty deed, the Property is free and clear from all encumbrances made by the Grantor except the above noted, and that the Grantor will warrant and defend the same against all lawful claims and demands of all persons claiming by, through, or under the Grantor, but against none other.

[Signature Page to Follow]

IN WITNESS WHEREOF, the Grantor has signed and sealed these presents the day and year first above written.

Signed, sealed, and delivered in our presence:

MCIN Bell, LLC,
a Florida limited liability company

By: _____
Ronald E. Inge, as a Manager

Dominique Eidem

Print Name: Dominique Eidem

Address: 12800 University Dr, 350
Fort Myers FL 33907

Carrie L Moore

Print Name: CARRIE L. MOORE

Address: 12800 University Dr. #350
Fort Myers, FL 33907

STATE OF Florida
COUNTY OF LEE

The foregoing instrument was acknowledged, sworn, and subscribed before me, by means of ☒ physical presence or ☐ online notarization, this 6th day of January, 2026, by Ronald E. Inge, as a Manager of MCIN Bell, LLC, a Florida limited liability company, on behalf of the limited liability company,

☐ who is personally known to me,

☒ who has produced FL-DL as identification, and who have acknowledged before me that he executed the same freely and voluntarily for the purposes therein expressed.

My Commission Expires:

Notary Public State of Florida
Carrie L Moore
My Commission HH 460934
Expires 1/21/2028

Carrie L Moore
Signature
Print Name: CARRIE L. MOORE
NOTARY PUBLIC - STATE OF FL

EXHIBIT "A"
Legal Description of the Property

Parcel 1:

Parcel 51, Units 72 and 73 of Mirror Lakes, Sections 19 and 20, Township 45 South, Range 27 East, Lehigh Acres, according to the map or plat thereof as recorded in Plat Book 27, Pages 159 through 160, of the Public Records of Lee County, Florida.

Parcel 2:

Lot 9, Block 239, Unit 63, Mirror Lakes, Section 20, Township 45 South, Range 27 East, Lehigh Acres, according to the map or plat thereof, as recorded in Plat Book 27, Page(s) 150, of the Public Records of Lee County, Florida.

Parcel 3:

Lot 10, Block 239, Unit 63, Mirror Lakes, Section 20, Township 45 South, Range 27 East, Lehigh Acres, according to the map or plat thereof, as recorded in Plat Book 27, Page(s) 150, of the Public Records of Lee County, Florida.

Parcel 4:

Parcel 44, Plat of Unit 55, Mirror Lakes, Section 20, Township 45 South, Range 27 East, Lehigh Acres, according to the map or plat thereof, as recorded in Plat Book 27, Page 142, of the Public Records of Lee County, Florida.

Detail by Entity Name

Florida Limited Liability Company
CNL FORT MYERS BLACKSTONE, LLC

Filing Information

Document Number	L25000295563
FEI/EIN Number	NONE
Date Filed	06/25/2025
Effective Date	06/23/2025
State	FL
Status	ACTIVE
Last Event	LC STMNT OF RA/RO CHG
Event Date Filed	10/06/2025
Event Effective Date	NONE

Principal Address

6562 UNIVERSITY PARKWAY
STE 210
SARASOTA, FL 34240 UN

Mailing Address

250 CIVIC CENTER DRIVE
STE 500
COLUMBUS, OH 43215 UN

Registered Agent Name & Address

CORPORATION SERVICE COMPANY
1201 HAYS STREET
TALLAHASSEE, FL 32301-2525

Name Changed: 10/06/2025

Address Changed: 10/06/2025

Authorized Person(s) Detail

Name & Address

Title MGR

CASTO NET LEASE PROPERTIES, LLC

6562 UNIVERSITY PARKWAY, STE 210
SARASOTA, FL 34240 UN

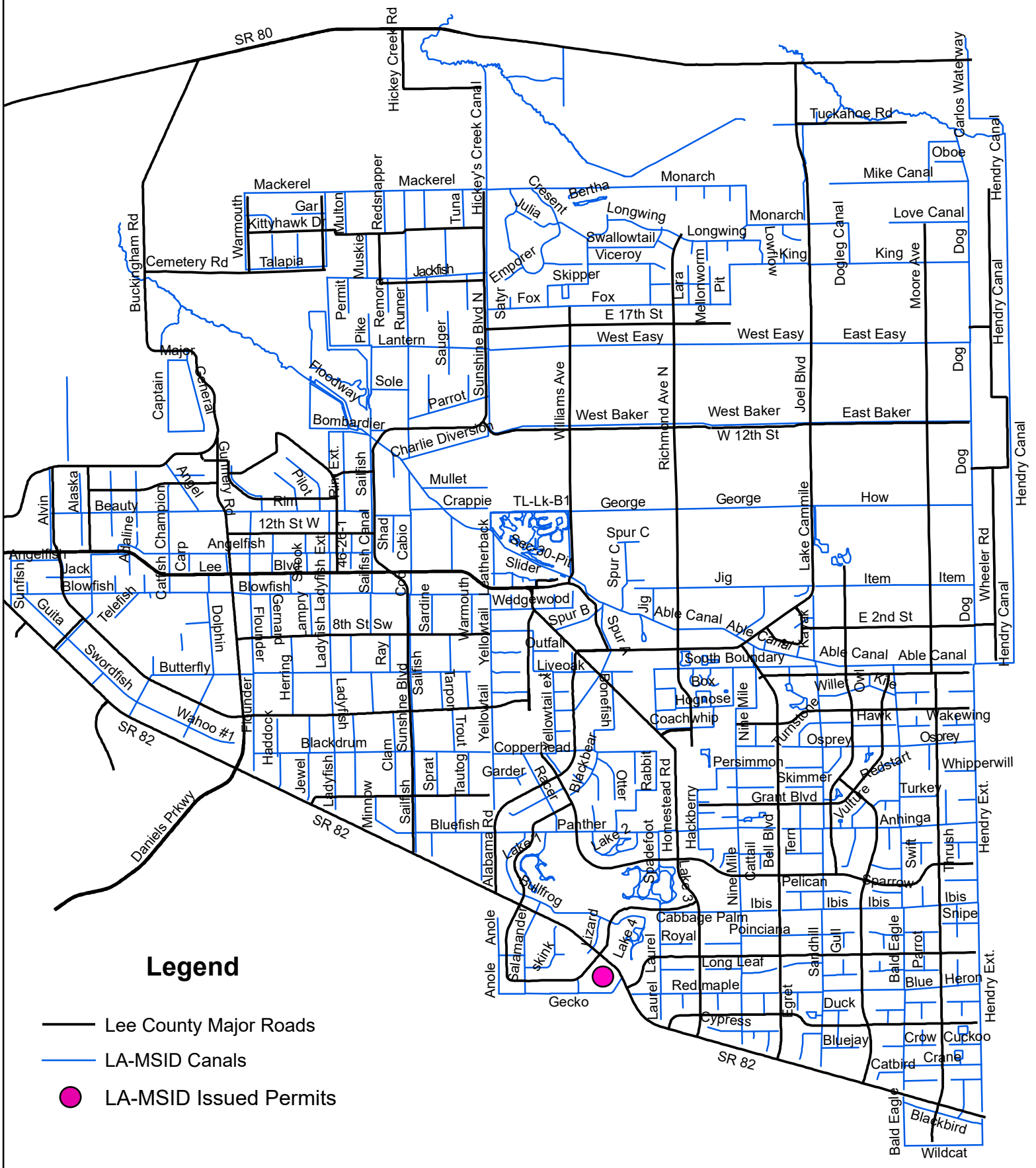
Annual Reports

No Annual Reports Filed

Document Images

10/06/2025 -- CORLCRACHG	View image in PDF format
06/25/2025 -- Florida Limited Liability	View image in PDF format

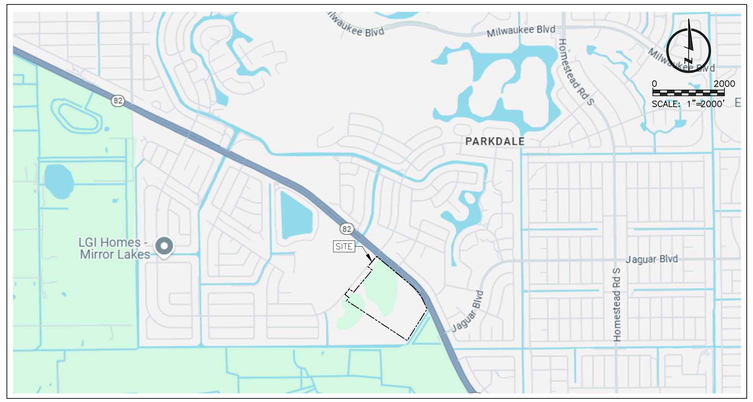
Permit # 2025-40 (M1)



Legend

- Lee County Major Roads
- LA-MSID Canals
- LA-MSID Issued Permits

BLACKSTONE CORPORATE PARK



GENERAL LOCATION MAP



LOCATION MAP

SECTION 20

TOWNSHIP 45 SOUTH

RANGE 27 EAST

LEE COUNTY, FLORIDA

SITE INFORMATION

PROJECT AREA: 61.78 AC

PARCEL STRAP NO: 20-45-27-72-00051.0000, 20-45-27-55-00044.0000
20-45-27-63-00239.0100, 20-45-27-63-00239.0090

PARCEL ADDRESS: ACCESS UNDETERMINED
LEHIGH ACRES, FL 33913

CURRENT ZONING: MPD

PROPOSED USE: COMMERCIAL

PREPARED FOR:

CASTO
c/o MCIN BELL, L.L.C.

5571 HALIFAX AVENUE
FORT MYERS, FLORIDA 33912

UTILITY SERVICE PROVIDERS

WATER:	
FLORIDA GOVERNMENTAL UTILITY AUTHORITY	9400 SOUTHPARK CENTER LOOP, SUITE 400 ORLANDO, FLORIDA 32819 PHONE: (407) 629-6900
SEWER:	
FLORIDA GOVERNMENTAL UTILITY AUTHORITY	9400 SOUTHPARK CENTER LOOP, SUITE 400 ORLANDO, FLORIDA 32819 PHONE: (407) 629-6900
STORMWATER:	
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT	601 EAST COUNTY LANE LEHIGH ACRES, FLORIDA 33936 PHONE: (239) 368-0044
ELECTRIC:	
LEE COUNTY ELECTRIC COOPERATIVE, INC.	P.O. BOX 3455 NORTH FORT MYERS, FLORIDA 33918 PHONE: (239) 656-2300
PHONE:	
LUMEN TECHNOLOGIES	2820 CARGO STREET FORT MYERS, FLORIDA 33916 PHONE: (239) 233-2686
CABLE:	
COMCAST	12641 CORPORATE LAKES DRIVE FORT MYERS, FLORIDA 33913 PHONE: (239) 986-7043
FIRE PROTECTION:	
LEHIGH ACRES FIRE CONTROL AND RESCUE DISTRICT	11 HOMESTEAD ROAD S. LEHIGH ACRES, FLORIDA 33936 PHONE: (239) 303-5300
SOLID WASTE DISPOSAL:	
WASTE PRO USA	13110 RICKENBACKER PARKWAY FORT MYERS, FLORIDA 33913 PHONE: (239) 337-0800

PROJECT CONTACTS

CIVIL ENGINEER:	
ATWELL, L.L.C. RACHEL J. TRACEY, P.E., PROJECT MANAGER	10511 SIX MILE CYPRESS PARKWAY, SUITE 101 FORT MYERS, FLORIDA 33966 PHONE: (239) 939-5490
SURVEYOR:	
ATWELL, L.L.C. ALLEN M. VOSE III, P.S.M.	10511 SIX MILE CYPRESS PARKWAY, SUITE 101 FORT MYERS, FLORIDA 33966 PHONE: (239) 939-5490
LANDSCAPE ARCHITECT:	
DAVID M. JONES, JR. AND ASSOCIATES, INC. GREGORY J. DISERIO, R.L.A.	2221 McGREGOR BOULEVARD FORT MYERS, FLORIDA 33901 PHONE: (239) 337-5525
TRAFFIC ENGINEER:	
TR TRANSPORTATION CONSULTANTS, INC. TED TREESH	2726 OAK RIDGE COURT, SUITE 503 FORT MYERS, FLORIDA 33901 PHONE: (239) 278-3090

SHEET LIST TABLE

#	SHEET TITLE
01	COVER SHEET
02	EXISTING CONDITIONS & DEMOLITION PLAN
03	BEST MANAGEMENT PRACTICE PLAN & DETAILS
04	GENERAL NOTES
05	PROJECT NOTES
06	MASTER SITE PLAN
07	MASTER DRAINAGE PLAN
08	PAVING, GRADING & DRAINAGE PLAN 1
09	PAVING, GRADING & DRAINAGE PLAN 2
10	PAVING, GRADING & DRAINAGE PLAN 3
11	TYPICAL CROSS SECTIONS 1
12	TYPICAL CROSS SECTIONS 2
13	PAVING, GRADING & DRAINAGE DETAILS 1
14	PAVING, GRADING & DRAINAGE DETAILS 2
15	PRESERVE PLAN
~~16~~	~~MASTER UTILITY PLAN~~
~~17~~	~~PLAN & PROFILE 1~~
~~18~~	~~PLAN & PROFILE 2~~
~~19~~	~~PLAN & PROFILE 3~~
~~20~~	~~PLAN & PROFILE 4~~
~~21~~	~~PLAN & PROFILE 5~~
~~22~~	~~PLAN & PROFILE 6~~
~~23~~	~~PLAN & PROFILE 7~~
~~24~~	~~LIFT STATION DETAILS 1~~
~~25~~	~~LIFT STATION DETAILS 2~~
~~26~~	~~LIFT STATION DETAILS 3~~
~~27~~	~~UTILITY DETAILS 1~~
~~28~~	~~UTILITY DETAILS 2~~
~~29~~	~~UTILITY DETAILS 3~~
~~30~~	~~UTILITY DETAILS 4~~

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

FGUA PROJECT NUMBER: 24-088-LED

ALL ELEVATIONS ON CIVIL ENGINEERING PLANS REFERENCE: NAVD'88

NO.	DATE	REVISION DESCRIPTION	BY

RACHEL J. TRACEY, P.E.
P.E. LIC. #65322

J:\Atwell\Jobs\24xx\2405360B - fort myers blackstone\engineering\dev plans\01 COVER SHEET.dwg, 2/17/2025 11:44:15 AM, VIVIAN ZALEWSKA, ATWELL, LLC

x:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\02 EXISTING CONDITIONS & DEMOLITION PLAN.dwg, 2/17/2025 11:44:45 AM, VIVIAN ZALESANSKI, ATWELL, LLC



EXISTING CONDITIONS & DEMOLITION PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:

CASTO

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

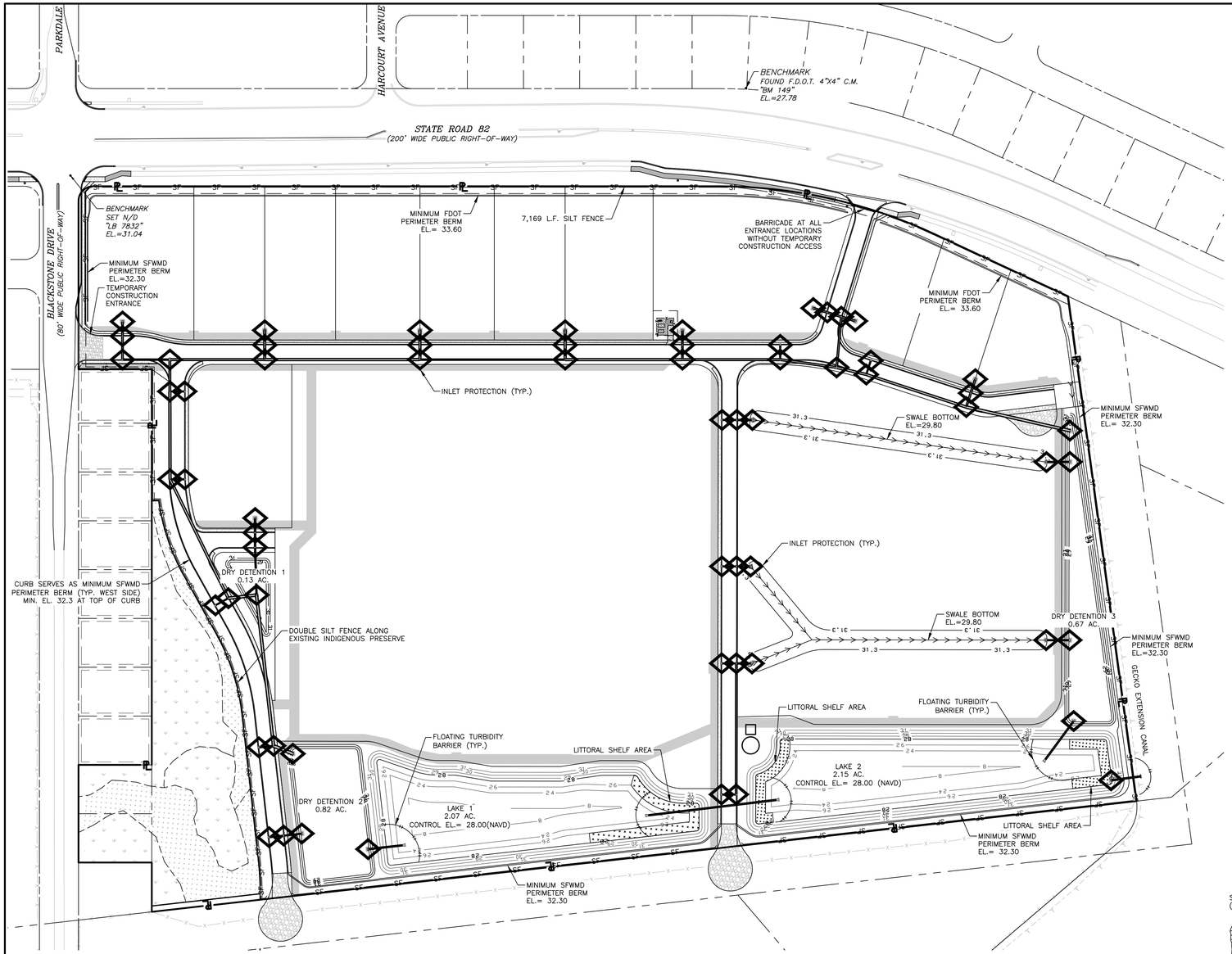
ALL UTILITIES ARE SHOWN IN APPROXIMATE LOCATIONS AND ARE TO BE FIELD VERIFIED BY CONTRACTOR PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

RACHEL L. TRACEY P.E.
P.E. LIC. #83327

DRAFTED BY: VZ
DESIGNED BY: RLT
REVIEWED BY: SWA
PROJECT NUMBER: 24003608
COMPLETION DATE: 01/31/2025
SURVEY DATE: N/A
BOOK AND PAGE:

SHEET 02

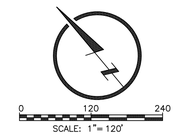


LEGEND:

= STAKED SEDIMENT AT INLETS

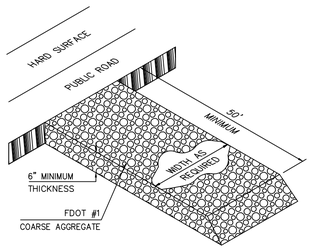
SF = STAKED SILT FENCE

= TURBIDITY BARRIER

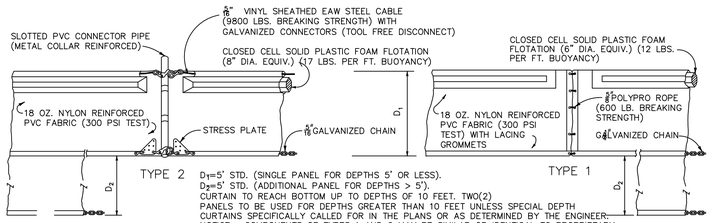


GENERAL NOTES:

1. PRIOR TO COMMENCEMENT OF CONSTRUCTION ACTIVITY, APPROPRIATE EROSION CONTROL DEVICES SHALL BE INSTALLED TO CONTROL AND REDUCE SOIL EROSION AND SEDIMENT TRANSPORT TO OFF SITE AREAS. THE CONTRACTOR SHALL MAINTAIN THESE DEVICES THROUGHOUT THE DURATION OF CONSTRUCTION. ALL DEVICES SHALL REMAIN IN PLACE UNTIL THE SURROUNDING AREAS ARE ESTABLISHED.
2. THE FOLLOWING MINIMUM REQUIREMENTS ARE RECOMMENDED: (REFERENCE FLORIDA DEVELOPMENT MANUAL, FDER, PPS 6-301 TO 6-500).
   - A) BMP 1.01 – TEMPORARY GRAVEL CONSTRUCTION ENTRANCE
   - B) BMP 1.05 – STRAW BALE BARRIER
   - C) BMP 1.06 – SILT FENCE
   - D) BMP 1.08 – STORM INLET DRAIN PROTECTION
3. THESE BEST MANAGEMENT PRACTICES (BMP) ARE TYPICAL OF THE REQUIREMENTS FOR SOIL EROSION CONTROL PER DIVISION 3.7 OF THE LEE COUNTY LAND DEVELOPMENT CODE. THEY MAY NOT CONSTITUTE COMPLETE REQUIREMENTS FOR COMPLIANCE WITH REGULATORY AGENCIES AND SPECIFIC PERMIT CONDITIONS.

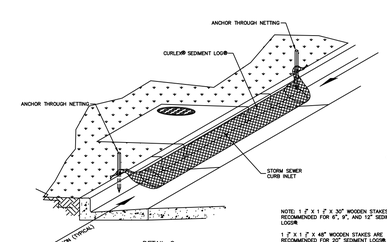


GRAVEL CONSTRUCTION ENTRANCE
Not To Scale



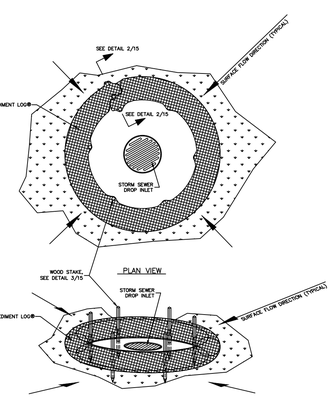
D_1=5' STD. (SINGLE PANEL FOR DEPTHS 5' OR LESS).
D_2=5' STD. (ADDITIONAL PANEL FOR DEPTHS > 5').
CURTAIN TO REACH BOTTOM UP TO DEPTHS OF 10 FEET. TWO(2) PANELS TO BE USED FOR DEPTHS GREATER THAN 10 FEET UNLESS SPECIAL DEPTH CURTAINS SPECIFICALLY CALLED FOR IN THE PLANS OR AS DETERMINED BY THE ENGINEER.
NOTICE: COMPONENTS OF TYPES 1 AND 2 MAY BE SIMILAR OR IDENTICAL TO PROPRIETARY DESIGNS. ANY INFRINGEMENT ON THE PROPRIETARY RIGHTS OF THE DESIGNER SHALL BE THE SOLE RESPONSIBILITY OF THE USER. SUBSTITUTIONS FOR TYPES 1 AND 2 SHALL BE AS APPROVED BY THE ENGINEER.

FLOATING TURBIDITY BARRIER DETAIL



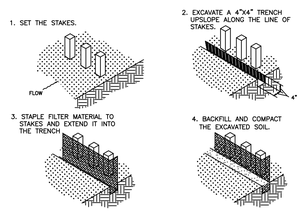
DETAIL 2
CURLEX® SEDIMENT LOGS®
CURB INLET PROTECTION
NO SCALE

NOTE: 1 2" X 1 2" X 30" WOODEN STAKES ARE RECOMMENDED FOR 6", 9", AND 12" SEDIMENT LOGS®.
1 2" X 1 2" X 48" WOODEN STAKES ARE RECOMMENDED FOR 20" SEDIMENT LOGS®.

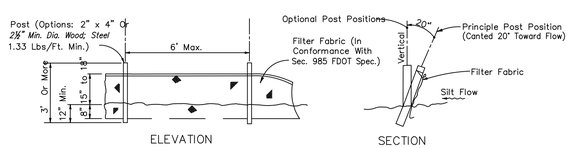


DETAIL 1
CURLEX® SEDIMENT LOGS®
DROP INLET PROTECTION
NO SCALE

CURLEX SEDIMENT LOGS
CURB INLET PROTECTION



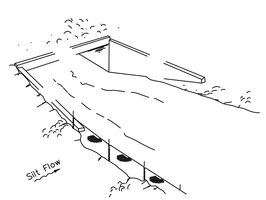
CONSTRUCTION OF A FILTER BARRIER
NTS



ELEVATION SECTION

Note: Silt Fence to be paid for under the contract unit price for Staked Silt Fence (LF).

TYPE III SILT FENCE
Not To Scale

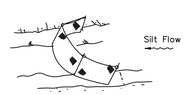


Type III Silt Fence



Type III Silt Fence Protection
Around Ditch Bottom Inlets.

Do not deploy in a manner that silt fences will act as a dam across permanent flowing watercourses. Silt fences are to be used at upland locations and turbidity barriers used at permanent bodies of water.



Note: Spacing for Type III Silt Fence to be in accordance with Chart I, Sheet 1 of 3 and ditch installations at drainage structures Sheet 2 of 3.

Type III Silt Fence

SILT FENCE APPLICATIONS
Not To Scale

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BEST MANAGEMENT PRACTICE PLAN & DETAILS

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL UTILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

RACHEL J. TRACEY, P.E.

P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 03

K:\Atwell\Jobs\24xx\24003608 - Fort Myers Blackstone\Engineering\Dev plans\03 BEST MANAGEMENT PRACTICE PLAN & DETAILS.dwg, 2/17/2025 11:44:58 AM, VIVIAN ZALEWSKI, ATWELL, LLC

PRE-BID SUBMITTAL:

THE CONTRACTOR SHALL VISIT THE PROJECT SITE PRIOR TO BIDDING TO FAMILIARIZE HIMSELF WITH THE CONDITIONS FOR CONSTRUCTION ACTIVITIES. THE CONTRACTOR SHALL OBTAIN FROM THE OWNER A WRITTEN LIST OF ALL PERMITS AND COPIES THEREOF, AND CAREFULLY REVIEW ALL PLANS, SPECIFICATIONS, AND PERMITS PREVIOUSLY SECURED ON BEHALF OF THE OWNER. IN CASE OF ANY DISCREPANCY EITHER IN PERMIT DOCUMENTS, PLANS, DRAWINGS, OR SPECIFICATIONS, THE CONTRACTOR MUST PROMPTLY SUBMIT A "WRITTEN CLARIFICATION REQUEST" TO THE OWNER, WHO WILL PROMPTLY FORWARD THE REQUEST TO THE ENGINEER WHO WILL MAKE A DETERMINATION IN WRITING. THE CONTRACTOR MUST VERIFY EXISTING FACILITY INFORMATION, AND ALL DESIGN/PERMIT DATA REQUIRED FOR WORK THAT IS TO CONNECT WITH EXISTING FACILITIES. ANY DISCREPANCIES BETWEEN THE CONTRACT REQUIREMENTS AND THE EXISTING CONDITIONS MUST BE REPORTED TO THE OWNER IN WRITING, FOR AN ENGINEERING DETERMINATION. ANY FUTURE ADJUSTMENT DUE TO FAILURE BY THE CONTRACTOR TO IDENTIFY THE RELATED DISCREPANCY, WILL BE AT THE CONTRACTOR'S EXPENSE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ANY LICENSES, ADDITIONAL PERMITS, AND FOR COMPLYING WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL LAWS, CODES, AND REGULATIONS IN CONNECTION WITH THE PERFORMANCE OF THE WORK.

CONSTRUCTION SAFETY AND LIABILITY:

THE CONTRACTOR MUST TAKE PROPER SAFETY AND HEALTH PRECAUTIONS TO PROTECT THE WORK, THE WORKERS, THE PUBLIC, AND THE PROPERTY OF OTHERS. THE CONTRACTOR IS RESPONSIBLE ALSO FOR ALL MATERIALS DELIVERED AND WORK PERFORMED UNTIL COMPLETION AND ALL ACCEPTANCES HAVE BEEN OBTAINED. THE CONTRACTOR SHALL MAINTAIN TRAFFIC DURING CONSTRUCTION IN ACCORDANCE WITH THE STATE OF FLORIDA ROADWAY AND TRAFFIC DESIGN STANDARDS (LATEST EDITION). THE CONTRACTOR IS RESPONSIBLE FOR ALL DAMAGE TO PERSONS OR PROPERTY THAT OCCURS AS A RESULT OF THE CONTRACTOR'S NEGLIGENCE. THE CONTRACTOR MUST SAVE HARMLESS AND INDEMNIFY THE OWNER AND THE ENGINEER OF RECORD, ITS OFFICERS, REPRESENTATIVES AND EMPLOYEES FROM ALL CLAIMS, LOSS, DAMAGE, ACTIONS, CAUSES OF ACTION, AND/OR EXPENSES RESULTING FROM, BROUGHT FOR, OR ON ACCOUNT OF ANY PERSONAL INJURY OR PROPERTY DAMAGE RECEIVED OR SUSTAINED BY ANY PERSONS OR PROPERTY GROWING OUT OF OCCURRING, OR ATTRIBUTABLE TO ANY ACT OR OMISSION UNDER OR RELATING TO THE CONTRACT, RESULTING IN WHOLE OR IN PART FROM THE NEGLIGENT ACTS OR OMISSIONS OF THE CONTRACTOR, ANY SUBCONTRACTOR, OR ANY EMPLOYEE, AGENT, OR REPRESENTATIVE OF THE CONTRACTOR OR ANY SUBCONTRACTOR.

PRE-CONSTRUCTION:

THE OWNER SHALL SECURE PRIOR TO CONSTRUCTION A PROFESSIONAL LAND SURVEYOR TO PERFORM AN "AS-BUILT" SURVEY OF ALL COMPLETED IMPROVEMENTS. THE OWNER SHALL ALSO SECURE PRIOR TO CONSTRUCTION A PROFESSIONAL ENGINEER TO PROVIDE THE PROPER INSPECTION SERVICES NEEDED IN ORDER TO CERTIFY TO ALL APPLICABLE REGULATORY AGENCIES THAT THE IMPROVEMENTS WERE CONSTRUCTED IN SUBSTANTIAL COMPLIANCE WITH ALL APPLICABLE PERMITS AND APPROVALS. THE OWNER SHALL COORDINATE A PRE-CONSTRUCTION MEETING WITH THE ENGINEER, SURVEYOR, CONTRACTOR, TESTING LAB, UTILITY COMPANIES, AND APPROPRIATE REGULATORY AGENCIES. THE CONTRACTOR SHALL PROVIDE A SHOP DRAWING SUBMISSION SCHEDULE FOR ALL PROJECT MATERIALS AND COMPONENTS. THE CONTRACTOR SHALL NOT INITIATE CONSTRUCTION OF ANY PORTION OF THE IMPROVEMENTS UNTIL THE SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED FOR THAT PORTION BY THE ENGINEER. THE OWNER, CONTRACTOR, ENGINEER AND UTILITY COMPANY SHALL ALSO DISCUSS ALL DOCUMENTATION REQUIRED FOR CONTRIBUTED FACILITIES TRANSFER FROM THE OWNER/DEVELOPER TO THE UTILITY COMPANY UPON PROJECT COMPLETION, UNLESS OTHERWISE SPECIFIED BY THE UTILITY COMPANY, THE FOLLOWING DOCUMENTS SHALL BE PROVIDED:

DOCUMENTS REQUIRED FOR CONTRIBUTED FACILITIES FROM DEVELOPER TO UTILITY COMPANY:

1. UTILITY EASEMENT(S): MUST BE RECORDED AT COUNTY CLERK OF COURT OFFICE BEFORE SUBMITTING TO UTILITY.
2. EASEMENT ACKNOWLEDGEMENT.
3. AFFIDAVIT.
4. RELEASE OF LIEN.
5. BILL OF SALE.
6. ASSIGNMENT OF RIGHTS UNDER UTILITY AGREEMENT: WHEN PROPERTY HAS BEEN TRANSFERRED TO A NEW OWNER.
7. RECORD DRAWINGS (AS-BUILT): MUST BE SIGNED AND SEALED BY ENGINEER OF RECORD, SUBMIT TWO REPRODUCIBLE COPIES OF PRINTS.
8. DETAILED COST OF CONSTRUCTION: MUST INCLUDE INDIVIDUAL ITEMS OR APPURTENANCES, UNIT COST AND TOTAL COST OF EACH. DO NOT INCLUDE WATER SERVICE LINES OR SEWER LATERALS.
9. DESCRIPTION OF FACILITIES: A SHORT EXPLANATION DEPICTING WHAT HAS BEEN CONSTRUCTED.
10. D.E.P. APPLICATION(S).
11. INSPECTION REPORT(S).
12. PRESSURE TEST REPORT(S).
13. INFILTRATION-EXFILTRATION TEST REPORT(S), INCLUDING VIDEO TAPES AND LAMPING REPORTS.
14. LIFT STATION INSPECTION (START-UP) REPORT(S) AND EQUIPMENT SHOP DRAWINGS.
15. BACTERIOLOGICAL TEST REPORT(S).
16. ENGINEER'S CERTIFICATE OF SUBSTANTIAL COMPLETION TO D.E.P.
17. SYSTEM(S) ACCEPTANCE LETTER(S) FROM D.E.P.

NOTES: ITEM 1-6 & 8 TO BE SUPPLIED BY OWNER. ITEMS 7 & 9-17 TO BE SUPPLIED TO ENGINEER OF RECORD.

UNLESS OTHERWISE SPECIFIED BY THE UTILITY, THE CONTRACTOR SHALL NOTIFY THE SUPERINTENDENTS OF THE WATER, GAS, SEWER, TELEPHONE, AND POWER COMPANIES, 10 DAYS IN ADVANCE, THAT THE CONTRACTOR INTENDS TO START WORK IN A SPECIFIC AREA. THE OWNER AND ENGINEER DISCLAIM ANY RESPONSIBILITY FOR THE SUPPORT AND PROTECTION OF SEWERS, DRAINS, WATER LINES, GAS LINES, CONDUITS OF ANY KIND, UTILITIES OR OTHER STRUCTURES OWNED BY THE CITY, COUNTY, STATE OR BY PRIVATE OR PUBLIC UTILITIES LEGALLY OCCUPYING ANY STREET, ALLEY, PUBLIC PLACE, RIGHT-OF-WAY, OR EASEMENT.

PROJECT SIGN:

THE CONTRACTOR SHALL PROVIDE AND MAINTAIN A CONSTRUCTION PROJECT SIGN AT A LOCATION DIRECTED BY THE OWNER. ATWELL, L.L.C. SHALL PROVIDE A SEPARATE SIGN FOR INSTALLATION BY THE CONTRACTOR AT THIS LOCATION. THESE SIGNS SHALL BE ERECTED WITHIN 15 DAYS AFTER RECEIVING A NOTICE TO PROCEED. UPON PROJECT COMPLETION, THE CONTRACTOR SHALL REMOVE THESE SIGNS AND RETURN TO ATWELL, L.L.C. THEIR SIGN.

ENVIRONMENTAL PROTECTION DURING CONSTRUCTION:

PROTECTION OF LAND RESOURCES – EXCEPT IN AREAS IDENTIFIED ON THE PLANS TO BE CLEARED, THE CONTRACTOR MUST NOT DEFACE, INJURE, OR DESTROY TREES OR SHRUBS OR REMOVE OR CUT THEM WITHOUT WRITTEN AUTHORIZATION FROM THE OWNER. IN THE ABSENCE OF A CLEARING PLAN, AREAS SHOWN FOR IMPROVEMENTS SHALL BE CLEARED UNLESS NOTED OTHERWISE.

PROTECTION OF WATER RESOURCES – IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO INVESTIGATE AND COMPLY WITH ALL APPLICABLE FEDERAL, STATE, REGIONAL COUNTY AND MUNICIPAL LAWS CONCERNING POLLUTION OF WATER RESOURCES. ALL WORK MUST BE PERFORMED IN SUCH A MANNER THAT OBJECTIONABLE CONDITIONS WILL NOT BE CREATED IN PUBLIC WATERS RUNNING THROUGH OR ADJACENT TO THE PROJECT AREA.

1. EROSION AND SEDIMENT CONTROL – ALL PRACTICAL AND NECESSARY EFFORT SHOULD BE TAKEN DURING CONSTRUCTION TO CONTROL AND PREVENT EROSION AND THE TRANSPORT OF SEDIMENT TO SURFACE DRAINS, SURFACE WATER, OR ONTO OTHER PROPERTY BY ANY OR ALL OF THE FOLLOWING METHODS:
   A. STORMWATER FACILITIES ARE TO BE BUILT AS EARLY IN THE CONSTRUCTION PHASE AS POSSIBLE TO ENSURE THE TREATMENT OF STORMWATER RUNOFF. TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES, HOWEVER, SUCH AS BERMS, SEDIMENT BASINS, GRASSING, SODDING, SAND BAGGING, BALED HAY OR STRAW, FLOATING SILT BARRIERS, STACKED SILT BARRIERS, ETC. MUST BE PROVIDED AND MAINTAINED UNTIL THE PERMANENT FACILITIES ARE COMPLETED AND OPERATIONAL.
   B. REVEGETATION AND STABILIZATION OF DISTURBED GROUND SURFACES SHOULD BE ACCOMPLISHED AS SOON AS POSSIBLE.
   C. FULL COMPACTION OF ANY FILL MATERIAL PLACED AROUND NEWLY INSTALLED STRUCTURES.
   D. PROHIBIT THE USE OF ANY CONSTRUCTION EQUIPMENT THAT LEAKS EXCESSIVE AMOUNTS OF FUEL OIL, OR HYDRAULIC FLUID.
2. ALL DISTURBED AREAS SHALL BE GRADED FOR POSITIVE DRAINAGE, EXCEPT RETENTION AREAS, AND SHALL BE STABILIZED BY SODDING, EXCEPT WHERE SEEDING AND MULCHING ARE CALLED FOR ON THE PLANS. THE LATEST VERSION OF THE F.D.O.T. ROAD AND BRIDGE SPECIFICATIONS SHALL BE USED UNLESS MORE RESTRICTIVE LOCAL SPECIFICATIONS EXIST.

   CONTRACTOR RESPONSIBLE FOR STABILIZING AND MAINTAINING SLOPES AND SOD THROUGHOUT CONSTRUCTION UNTIL SUCH TIME AS APPROVED BY THE ENGINEER.

PROTECTION OF FISH AND WILDLIFE:

THE CONTRACTOR MUST AT ALL TIMES PERFORM ALL WORK IN A WAY AND TAKE SUCH STEPS AS REQUIRED TO PREVENT ANY INTERFERENCE WITH OR DISTURBANCE TO FISH AND WILDLIFE. THE CONTRACTOR SHALL NOT ALTER WATER FLOWS OR OTHERWISE DISTURB NATIVE HABITATS AND JURISDICTIONAL WETLANDS LOCATED WITHIN AND / OR ADJACENT TO THE PROJECT AREA.

RECORDING AND PRESERVING HISTORICAL AND ARCHEOLOGICAL FINDS:

ALL ITEMS HAVING ANY APPARENT HISTORICAL OR ARCHEOLOGICAL INTEREST THAT ARE DISCOVERED IN THE COURSE OF ANY CONSTRUCTION ACTIVITIES MUST BE CAREFULLY PRESERVED. THE CONTRACTOR MUST LEAVE THE ARCHEOLOGICAL FIND UNDISTURBED AND MUST IMMEDIATELY REPORT THE FIND TO THE OWNER SO THAT THE PROPER AUTHORITY MAY BE NOTIFIED.

EARTHWORK:

1. GENERAL

1.01 SUBMITTALS

A. EROSION AND CONTROL MEASURES
B. COMPACTION TESTS
C. SOIL CLASSIFICATION TESTS
D. PRESERVATION PLANS

1.02 SITE EXAMINATION

A. CONTRACTORS, BEFORE SUBMITTING BIDS, SHALL INFORM THEMSELVES AS TO LOCATION AND NATURE OF THE WORK, CHARACTER OF EQUIPMENT AND FACILITIES NEEDED FOR PERFORMANCE OF THE WORK, GENERAL AND LOCAL CONDITIONS PREVAILING AT THE SITE, AND OTHER MATTERS WHICH MAY IN ANY WAY, AFFECT THE WORK UNDER CONTRACT.
B. EXAMINE SOURCES OF INFORMATION CONCERNING GROUND WATER LEVEL, WHETHER SURFACE OR SUBSURFACE. EACH BIDDER TO DRAW HIS OWN CONCLUSION CONCERNING GROUND WATER LEVELS AND HOW WATER AFFECTS HIS WORK.

1.03 SUBSURFACE INVESTIGATIONS

A. SUBSURFACE DATA, INCLUDING GROUND WATER ELEVATIONS OR CONDITIONS, IF SHOWN ON THE DRAWINGS OR ATTACHED TO THESE SPECIFICATIONS, ARE PRESENTED ONLY AS INFORMATION THAT IS AVAILABLE WHICH INDICATED CERTAIN CONDITIONS FOUND AND LIMITED TO THE EXACT LOCATIONS. SHALL NOT BE INTERPRETED AS AN INDICATION OF CONDITIONS THAT MAY ACTUALLY BE DEVELOPED THROUGH THE PERIOD OF CONSTRUCTION. BIDDERS SHALL EXAMINE THE SITE OF THE WORK AND MAKE THEIR OWN DETERMINATION OF THE CHARACTER OF MATERIALS AND THE CONDITIONS TO BE ENCOUNTERED ON THE WORK, AND THEIR PROPOSAL SHALL BE BASED UPON THEIR OWN INVESTIGATIONS. THE OWNER AND ENGINEER SHALL NOT BE HELD RESPONSIBLE FOR VARIATIONS FOUND TO EXIST BETWEEN THE ATTACHED DATA ABOVE REFERRED TO AND ACTUAL FIELD CONDITIONS THAT DEVELOP THROUGH THE PERIOD OF CONSTRUCTION.
B. WHERE EXISTING GRADES, UTILITY LINES AND SUBSTRUCTURES ARE SHOWN ON THE DRAWINGS, THE OWNER AND ENGINEER ASSUME NO RESPONSIBILITY FOR CORRECTNESS OF EXISTING CONDITIONS INDICATED. THE CONTRACTOR SHALL ASCERTAIN EXACT LOCATIONS OF UTILITIES AND SUBSTRUCTURES THAT MAY BE AFFECTED BY THIS PROJECT, AND SHALL BE RESPONSIBLE FOR ANY DAMAGE OR INJURY THAT MAY RESULT FROM WORKING ON OR NEAR THOSE UTILITIES, SUBSTRUCTURES WHICH ARE NOT TO BE REMOVED OR DEMOLISHED.
C. THE CONTRACTOR SHALL MAKE HIS OWN DEDUCTIONS OF THE SUBSURFACE CONDITIONS WHICH MAY AFFECT METHODS OR COST OF CONSTRUCTION AND HE AGREES THAT THE CONTRACTOR WILL MAKE NO CLAIM FOR DAMAGES OR OTHER COMPENSATION EXCEPT SUCH AS ARE PROVIDED FOR IN THE AGREEMENT, SHOULD THE CONDITIONS FOUND FIND CONDITIONS DURING THE PROGRESS OF THE WORK DIFFERENT FROM THOSE AS CALCULATED OR ANTICIPATED BY THE CONTRACTOR.

1.04 BENCHMARKS AND MONUMENTS

A. MAINTAIN CAREFULLY EXISTING BENCH MARKS, MONUMENTS, AND OTHER REFERENCE POINTS IF DISTURBED OR DESTROYED, REPLACE AS DIRECTED.

1.05 JOB CONDITIONS

A. CONDITION OF PREMISES: ACCEPT SITE AS FOUND AND EXCAVATE, FILL, COMPACT, AND BACKFILL SITE AS HEREINAFTER SPECIFIED.
B. PROTECTION
   a. EXISTING STRUCTURES AND PROPERTY: TAKE PRECAUTIONS TO GUARD AGAINST MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES AND FACILITIES; PROVIDE AND PLACE BRACING OR SHORING AS NECESSARY OR PROPER IN CONNECTION THEREWITH; BE RESPONSIBLE FOR SAFETY AND SUPPORT OF SUCH STRUCTURES; BE LIABLE FOR ANY MOVEMENT OR SETTLEMENT, ANY DAMAGE OR INJURY CAUSED THEREBY OR RESULTING THEREFROM. IF AT ANY SAFETY OR ANY ADJACENT STRUCTURES APPEARS TO BE ENDANGERED, CEASE OPERATIONS, TAKE PRECAUTIONS TO SUPPORT SUCH STRUCTURES AND NOTIFY THE OWNER. RESUME OPERATIONS ONLY AFTER PERMISSION HAS BEEN CHANGED BY THE OWNER.
   b. SIDEWALKS AND STREETS: TAKE PRECAUTIONS TO GUARD AGAINST MOVEMENT, SETTLEMENT OR COLLAPSE OF ANY SIDEWALKS, CURBS OR STREETS ADJACENT OR ADJOINING SITE; BE LIABLE FOR ANY SUCH MOVEMENT, SETTLEMENT OR COLLAPSE; REPAIR PROMPTLY SUCH DAMAGE WHEN SO ORDERED; INSTALL SUCH SHORING, INCLUDING SHEET PILING, AS MAY BE REQUIRED DURING EXCAVATION, TO PROTECT BANKS, ADJACENT PAVING, STRUCTURES AND UTILITIES.
   c. RESPONSIBILITY: BE RESPONSIBLE FOR ANY DAMAGE TO EXISTING STRUCTURES OR TO EQUIPMENT AND FURNISHINGS HOUSED THEREIN WHICH ARE DUE DIRECTLY OR INDIRECTLY TO CONSTRUCTION OPERATIONS; REPAIR OR REPLACE AS MAY BE NECESSITATED BY SITE GRADING OR LOCATION OF NEW BUILDING. USE EVERY POSSIBLE PRECAUTION TO PREVENT INJURIES TO LANDSCAPING, DRIVES, CURBS AND WALKS ON OR ADJACENT TO SITE OF THE WORK AND REPLACE, AT NO EXPENSE TO OWNER, ANY OF SUCH DESTROYED.

2. EXECUTION

2.01 GENERAL

A. ACCOMPLISH IN A MANNER THAT PROVIDES FOR THE SAFETY OF THE PUBLIC AND WORKMEN AND PROVIDE FOR THE PROTECTION OF ALL PROPERTY.
B. CONSTRUCTION: DO NOT CLOSE, OBSTRUCT OR STORE MATERIAL OR EQUIPMENT IN STREETS, SIDEWALKS, ALLEYS OR PASSAGEWAYS WITHOUT A PERMIT IN ACCORDANCE WITH LOCAL ORDINANCES, REGULATIONS AND CODES.
C. INTERFERENCE: CONDUCT OPERATIONS WITH MINIMUM INTERFERENCE WITH ROADS, STREETS, DRIVEWAYS, ALLEYS, SIDEWALKS AND OTHER FACILITIES.
D. PNEUMATIC TOOLS: WORK WITH PNEUMATIC OR VIBRATORY TOOLS WILL BE PERMITTED ONLY IN A MANNER WHICH CAUSES NO RELATED DAMAGES.
E. REMOVAL: UNLESS OTHERWISE NOTED OR SPECIFIED TO BE RELOCATED OR STORED, ALL MATERIALS REMOVED BECOME THE PROPERTY OF THE CONTRACTOR AND ARE TO BE REMOVED COMPLETELY AWAY FROM THE SITE BY THE CONTRACTOR. DO NOT STORE OR PERMIT DEBRIS TO ACCUMULATE ON THE SITE.
F. TEMPORARY STRUCTURES: REMOVE ALL TEMPORARY STRUCTURES WHEN THEY ARE NO LONGER REQUIRED.
G. REPAIR: CLEAN UP, REPAIR OR REPLACE AT NO COST TO OWNER ALL PROPERTY DAMAGED BY REASON OF REQUIRED WORK. ALL PATCHWORK SHALL MATCH EXISTING AND BE PERFORMED IN A NEAT AND WORKMANLIKE MANNER BY CRAFTSMEN SKILLED IN THE TRADE INVOLVED. IN NEWLY GRADED AREAS TAKE EVERY PRECAUTION AND TEMPORARY MEASURE NECESSARY, TO PREVENT DAMAGE FROM EROSION OF FRESHLY GRADED AREA, WHERE ANY SETTLEMENT MAY OCCUR PRIOR TO ACCEPTANCE OF THE WORK, REPAIR AND RE-ESTABLISH GRADES TO THE REQUIRED ELEVATIONS AND SLOPES AT NO ADDITIONAL COST TO THE OWNER. THIS APPLIES TO DAMAGE TO THE NEWLY GRADED AREAS WITHIN THE CONSTRUCTION LIMITS AND DAMAGE TO ADJACENT PROPERTIES BY ERODED MATERIAL.

2.02 LOCATIONS AND ELEVATIONS

A. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SURVEYS, MEASUREMENTS AND LAYOUTS REQUIRED FOR PROPER EXECUTION OF THE WORK. LAY OUT LINES AND GRADES FROM EXISTING SURVEY CONTROL SYSTEM AND AS SHOWN ON DRAWINGS.

2.03 CLEARING AND GRUBBING

A. WITHIN LIMITS OF AREA DESIGNATED FOR GRADING AND SITE CONSTRUCTION WORK, REMOVE TREES, BRUSH, STUMPS, WOOD, DEBRIS AND OTHER DELETERIOUS MATERIALS NOT REQUIRED TO REMAIN AS PART OF FINISHED WORK.
B. REMOVE ALL GRASS, PLANTS, VEGETATION AND ORGANIC MATERIAL FROM SAME AREA.

2.04 STRIPPING

A. STRIP ALL TOPSOIL ORGANIC MATERIAL SURFACE LITTER, RUBBLE AND OVERBURDEN FOR ENTIRE DEPTH OF ROOT SYSTEM OF GRASS OR OTHER VEGETATION OVER THE LIMITS OF CONSTRUCTION.
B. STOCKPILE TOPSOIL ON SITE WHERE DIRECTED.

2.05 EXCAVATION

A. BEGIN EXCAVATION AFTER STRIPPING, CLEARING AND GRUBBING WHERE APPLICABLE, HAS BEEN COMPLETED.
B. EXCAVATE TO GRADES REQUIRED TO ACCOMMODATE THE PROPOSED CONSTRUCTION. DEWATER AS NEEDED.
C. REMOVE 'UNSATISFACTORY MATERIALS' ENCOUNTERED FROM THE BUILDING AREAS, AND OTHER NON-LANDSCAPED AREAS.
D. EXCAVATE IN SUCH A MANNER THAT QUICK AND EFFICIENT DRAINAGE OF STORMWATER WILL BE AFFECTED.
E. CLASSIFY EXCAVATED MATERIALS AND STOCKPILE SEPARATELY SUITABLE SOILS FOR USE AS BACKFILL MATERIALS. IF SUFFICIENT QUANTITIES OF EXCAVATED MATERIALS MEETING REQUIREMENTS FOR BACKFILL ARE NOT AVAILABLE ON SITE, PROVIDE MATERIALS MEETING THESE REQUIREMENTS.
F. STOCKPILE EXCAVATED MATERIAL SUITABLE FOR USE AS FILL AND BACKFILL.

2.06 FILLING, BACKFILLING AND COMPACTING

A. THE WORK CONSISTS OF COMPACTION OF EXISTING EARTH (EXCLUDE ROCK), SURFACES AFTER EXCAVATION, FILLING AND COMPACTION OF SAID AREA TO LEVELS REQUIRED WITH SUITABLE BACKFILL MATERIAL.
B. MATERIALS: 'SATISFACTORY FILL MATERIALS' AASHTO CLASSIFICATION A-3 OR BETTER SHALL BE USED IN FILLS AND BACKFILLS.
C. FILLING AND BACKFILLING: PLACE 'SATISFACTORY FILL MATERIAL' IN HORIZONTAL LAYERS NOT EXCEEDING 6 INCHES IN LOOSE DEPTH. COMPACT AS SPECIFIED HEREIN NO MATERIAL SHALL BE PLACED ON SURFACES THAT ARE MUDDY.

EARTHWORK (CON'T):

D. COMPACTION: COMPACTION SHALL BE WITH EQUIPMENT SUITED TO SOIL BEING COMPACTED. MOISTEN OR AERATE MATERIAL AS NECESSARY TO PROVIDE MOISTURE CONTENT THAT WILL READILY FACILITATE OBTAINING SPECIFIED COMPACTION WITH EQUIPMENT USED. COMPACT EACH LAYER TO NOT LESS THAN PERCENTAGE OF MAXIMUM DENSITY SPECIFIED BELOW DETERMINED IN ACCORDANCE WITH AASHTO T-180. INSURE THAT THE COMPACTION OF PREVIOUSLY PREPARED FILL AREAS HAS BEEN MAINTAINED PRIOR PLACING NEW LAYERS.
E. RECONDITIONING OF SUBGRADE: WHERE APPROVED COMPACTED SUBGRADES ARE DISTURBED BY THE CONTRACTOR'S SUBSEQUENT OPERATIONS OF ADVERSE WEATHER SUBGRADE SHALL BE SCARIFIED AND COMPACTED AS SPECIFIED HEREIN BEFORE TO REQUIRED DENSITY PRIOR TO FURTHER CONSTRUCTION THEREON. RE-COMPACTION OVER UNDERGROUND UTILITIES SHALL BE BY POWER-DRIVEN HAND TAMPERS.
F. COMPACTION REQUIREMENTS
   a. FILL UNDER LAWNS AND PLANTED: 95%
   b. BELOW SLABS ON GRADE AND CONCRETE WALKS: 98%
   c. UNDER PAVING PARKING AREAS: 98%

2.07 TESTING

A. THE CONTRACTOR WILL PROVIDE THE SERVICES OF A TESTING LABORATORY TO PERFORM SPECIFIED TESTS, INSPECTIONS, INSTRUMENTATION AND INSPECTION OF WORK.
B. TESTS OF MATERIALS SHALL BE AS FOLLOWS:
   a. SOIL CLASSIFICATION: ONE TEST FROM EACH TYPE OF MATERIAL ENCOUNTERED AND / OR PROPOSED TO BE USED.
   b. LABORATORY TESTS FOR MOISTURE-CONTEST AND DENSITY ACCORDING TO AASHTO T-180: ONE TEST FOR EACH MATERIAL ENCOUNTERED AND / OR PROPOSED TO BE USED.
   c. FIELD TESTS FOR MOISTURE CONTEST AND DENSITY: ONE TEST PER LAYER OF FILL PER 5,000 SQUARE FEET OF AREA.

SUPPLEMENTAL SPECIFICATIONS:

1. GENERAL

THE CONTRACTOR SHALL BECOME FAMILIAR WITH AND ADHERE TO THE SPECIFICATIONS AND STANDARDS OF THE UTILITY COMPANIES WHICH ARE SERVING THE PROJECT SITE. THE CONTRACTOR SHALL BECOME FAMILIAR WITH AND COMPLY WITH ALL SITE DEVELOPMENT STANDARDS AND CODES OF THE REGULATORY AGENCIES ASSOCIATED WITH THIS PROJECT. THE LATEST VERSION OF THE FLORIDA DEPARTMENT OF TRANSPORTATION ROAD AND BRIDGE CONSTRUCTION SPECIFICATIONS AND THE LATEST FLORIDA DEPARTMENT OF TRANSPORTATION ROADWAY AND TRAFFIC DESIGN STANDARDS SHALL BE INCLUDED WITHIN THE PROJECT SPECIFICATIONS. UNLESS OTHERWISE NOTED, EITHER ON THE PLANS OR WITHIN THE SPECIFICATIONS, THE APPLICABLE SECTIONS OF THE FLORIDA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS AND THE LATEST FLORIDA DEPARTMENT OF TRANSPORTATION ROADWAY AND TRAFFIC DESIGN STANDARDS SHALL APPLY INCLUDING REFERENCES THEREIN. THE GENERAL DESCRIPTION OF THE NATURE OF THE WORK SHALL BE SUFFICIENT CORRELATION TO THE FLORIDA DEPARTMENT OF TRANSPORTATION SPECIFICATIONS. EXACT ITEM DESCRIPTION IS NOT REQUIRED. IN THE EVENT THERE ARE CONFLICTS BETWEEN SPECIFICATIONS OR REQUIREMENTS, THE MOST RESTRICTIVE (CONSERVATIVE) SPECIFICATION OR REQUIREMENT SHALL BE USED.

2. POTABLE WATER DISTRIBUTION/ WASTEWATER COLLECTION INSTALLATION

UNLESS OTHERWISE NOTED ON THE PLANS, THE STANDARDS AND SPECIFICATIONS OF THE ASSOCIATED UTILITY COMPANY SERVING THE PROJECT SITE SHALL BE ADHERED TO FOR ALL MATERIALS, INSTALLATION, TESTING, AND CERTIFICATION ACTIVITIES FOR ALL PRIME STATIONS, MAIN LINES, SERVICES, AND APPURTENANCES. IF STANDARDS AND SPECIFICATIONS ARE NOT AVAILABLE, THE CONSTRUCTION SHALL CONFORM WITH THE LATEST STANDARDS AND SPECIFICATIONS ADOPTED BY THE UTILITIES, LOCAL GOVERNMENTAL REGULATIONS, OR THE MANUFACTURERS RECOMMENDED INSTALLATION PROCEDURES, WHICHEVER IS SPECIFICALLY THE MOST RESTRICTIVE. A COPY OF THE UTILITIES SPECIFICATIONS CAN BE REVIEWED AT THE OFFICE OF ATWELL, L.L.C.

3. STORMWATER PIPE INSTALLATION AND MISCELLANEOUS EXCAVATIONS

UNLESS OTHERWISE NOTED ON THE PLANS OR SPECIFICATIONS, THE CONTRACTOR SHALL PERFORM THE EXCAVATION, BEDDING, JOINTS, AND BACKFILLING OPERATIONS IN ACCORDANCE WITH THE APPLICABLE WATER/ WASTEWATER INSTALLATION SPECIFICATIONS, LOCAL GOVERNMENTAL REGULATIONS OR STANDARDS, F.D.O.T. STANDARDS AND SPECIFICATIONS OR MANUFACTURER'S RECOMMENDED INSTALLATION PROCEDURES, WHICHEVER IS SPECIFICALLY THE MOST RESTRICTIVE.

4. UNSUITABLE MATERIALS

IF UNSUITABLE MATERIAL IS ENCOUNTERED WITHIN THE ROADWAY AREA AND / OR UTILITY AREAS IT SHALL BE REMOVED TO A DEPTH OF THREE (3) FEET BELOW THE SUB-BASE OR TRENCH BOTTOM AND SHALL BE BACKFILLED WITH THE AASHTO A-3 MATERIAL OR BETTER WITH PLACEMENT AND COMPACTION METHODS IN ACCORDANCE WITH THE LATEST EDITION OF THE FLORIDA DEPARTMENT OF TRANSPORTATION'S STANDARD SPECIFICATIONS OR AS OTHERWISE NOTED ON THE PLANS. UNSUITABLE MATERIALS SHALL BE REMOVED FROM SITE, UNLESS THE ENGINEER APPROVES USE WITHIN LANDSCAPED AREAS.

DEWATERING:

1.01 GENERAL

A. DEWATERING CONSISTS OF PERFORMING ALL WORK NECESSARY TO REMOVE SURFACE WATER AND / OR CONTROL THE GROUND WATER LEVELS AND HYDROSTATIC PRESSURES IN ORDER TO PERMIT ALL EXCAVATION AND CONSTRUCTION UNDER THIS CONTRACT TO BE PERFORMED IN THE DRY.
B. WORK OF THIS SECTION INCLUDES INSTALLATION, OPERATIONS, MAINTENANCE, SUPERVISION, SUPPLY, DISMANTLING, AND REMOVAL FROM THE SITE OF THE DEWATERING EQUIPMENT.
C. THE CONTRACTOR MUST FAMILIARIZE HIMSELF WITH THE POTENTIAL FOR EXCESSIVE RAINFALL, THE GROUND CONDITIONS, AND THE GROUND WATER CONDITIONS, GROUND WATER ELEVATION CAN FLUCTUATE. IT IS ANTICIPATED THAT ANY EXCAVATIONS MAY ENCOUNTER THE GROUND WATER TABLE.
D. DRAINAGE OF THE SITE: AT ALL TIMES THE CONTRACTOR SHALL MAINTAIN AND OPERATE ADEQUATE SURFACE AND SUBSURFACE DRAINAGE METHODS IN ORDER TO KEEP THE CONSTRUCTION SITE DRY AND IN SUCH CONDITION THAT PLACEMENT AND COMPACTION OF FILL MAY PROCEED UNHINDERED BY SATURATION OF THE AREA DURING CONSTRUCTION, THE SURFACE OF THE BACKFILL AREA SHALL BE LEFT IN SUCH CONDITION THAT PRECIPITATION AND / OR SURFACE WATER WILL RUN OFF WITHOUT PONDING.

1.02 METHOD

A. THE CONTROL OF ALL SURFACE AND SUBSURFACE WATER IS PART OF THE DEWATERING REQUIREMENTS, MAINTAIN ADEQUATE CONTROL SO THAT THE STABILITY OF EXCAVATED AND CONSTRUCTION SLOPES IS NOT ADVERSELY AFFECTED BY WATER, THAT EROSION IS CONTROLLED AND THAT THE FLOODING OR DAMAGE TO STRUCTURES OR DAMAGE TO STRUCTURES DOES NOT OCCUR, DRAIN SURFACE WATER AWAY FROM THE EXCAVATION.
B. DISPOSE OF ALL WATER REMOVED FROM THE EXCAVATION IN A MANNER THAT WILL NOT ENDANGER PUBLIC HEALTH, PROPERTY, OR PORTIONS OF THE WORK UNDER CONSTRUCTION OR COMPLETED. DISPOSE OF WATER IN A MANNER THAT WILL CAUSE NO INCONVENIENCE WHATSOEVER TO THE OWNER OR TO OTHERS ENGAGED IN WORK AT THE SITE.
C. DISPOSE OF WATER RESULTING FROM DEWATERING OPERATIONS IN ACCORDANCE WITH CITY, COUNTY, STATE AND FEDERAL REGULATIONS.
D. CONDUCT OPERATIONS SO THAT STORMWATER RUNOFF, SEDIMENT IS NOT DISCHARGED TO THE ADJACENT WATER BODIES, SEWERS, STREETS AND ADJACENT PROPERTIES.
E. DEWATERING SYSTEM SHALL BE SO DESIGNED AS TO PREVENT REMOVAL OF SOIL FINES FROM THE SITE DURING THE DEWATERING OPERATION.

PORTLAND CEMENT CONCRETE PAVING:

1.01 QUALITY ASSURANCE

A. COMPLY WITH ACI STANDARDS RECOMMENDED PRACTICES FOR CONSTRUCTION OF CONCRETE PAVEMENTS AND CONCRETE BASES (ACI 325.9R, LATEST EDITION)

1.02 REFERENCE STANDARDS

A. THE FOLLOWING REFERENCE STANDARDS OF THE ISSUES LISTED BELOW BUT REFERRED TO THEREAFTER BY BASIC DESIGNATION ONLY, FORM A PART OF THIS SPECIFICATION TO THE EXTENT INDICATED BY THE REFERENCES THERETO. TESTS SHALL BE PERFORMED IN ACCORDANCE WITH HEREINAFTER SPECIFIED STANDARDS.
   a. AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM)
   b. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS

LAND USE (ON-SITE):

USE:		
PAVEMENT/ CONCRETE	4.47 AC	7.2%
WETLAND PRESERVE	2.17 AC	3.5%
UPLAND PRESERVE	1.27 AC	2.1%
DRY DETENTION (AT BOTTOM)	1.62 AC	2.6%
LAKES (AT CONTROL)	4.22 AC	6.8%
PERVIOUS	48.03 AC	77.8%
TOTAL	61.78 AC	100.0%

NOTES:

1. PARCELS 1 AND 2 MAXIMUM 90% IMPERVIOUS.
2. PARCELS 3 THROUGH 14 MAXIMUM 80% IMPERVIOUS.
3. OFF-SITE PRESERVE 5.83 AC.

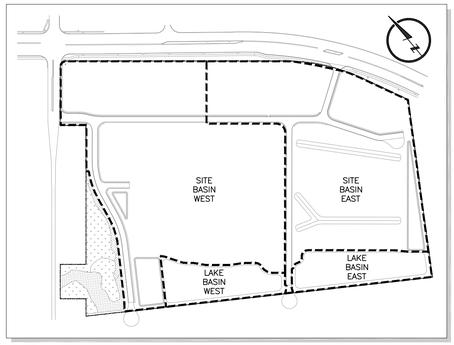
SURFACE WATER MANAGEMENT SUMMARY:

	SITE BASIN WEST (FT-NAVD)	SITE BASIN EAST (FT-NAVD)	LAKE BASIN WEST (FT-NAVD)	LAKE BASIN EAST (FT-NAVD)
SFWMD 5 YEAR, 1 DAY	31.47	31.35	31.35	31.34
SFWMD 10 YEAR, 1 DAY	31.66	31.63	31.63	31.62
SFWMD 25 YEAR, 3 DAY	32.23	32.22	32.22	32.21
SFWMD 25 YEAR, 3 DAY BACK TO BACK	32.25	32.24	32.24	32.23
SFWMD 100 YEAR, ZERO DISCHARGE	32.77	32.77	32.77	32.77
FDOT 100 YEAR CRITICAL	32.53	32.53	32.53	32.53
MINIMUM ROAD	31.66	31.66	31.66	31.66
SFWMD MINIMUM PERIMETER BERM	32.3	32.3	32.3	32.3
FDOT MINIMUM PERIMETER BERM – GREEN SPACE BERM (1 FT ABOVE FDOT 100 YEAR CRITICAL)	33.6	33.6	33.6	33.6
FDOT MINIMUM PERIMITER BERM – HARDENED STRUCTURE (6" ABOVE 100 YEAR FDOT 100 YEAR CRITICAL)	33.03	33.03	33.03	33.03
MINIMUM FINISHED FLOOR ELEVATION	32.77	32.77	32.77	32.77

FEMA: ZONE X PER PANEL 12071C0475F

PROPERTY DATA:

EXISTING ZONING DESIGNATION:	Z-23-040, BLACKSTONE CORPORATE PARK MPD, COMPANION ADD2024-00219
EXISTING LAND USE:	VACANT
PROPOSED USE:	COMMERICAL
FLUM DESIGNATION:	URBAN COMMUNITY, INDUSTRIAL DEVELOPMENT, AND WETLANDS
PLANNING COMMUNITY:	LEHIGH ACRES, 17
LEE PLAN CONSISTENCY:	THE PROPOSED IMPROVEMENTS AND USES ARE CONSISTENT WITH THE LEE PLAN, Z-23-040, PROPOSED ADD2024-00219, AND THE LDC.
DEVELOPMENT SCHEDULE: (PER Z-23-040 AND ADD2024-00219)	MAXIMUM 500,000 SF COMMERICAL RETAIL USE



BASIN MAP
N.T.S.

SITE DEVELOPMENT REGULATIONS: (LEE COUNTY RESOLUTION Z-23-040)

MINIMUM LOT SIZE:	10,000 SQUARE FEET
MINIMUM LOT WIDTH:	100 FEET
MINIMUM LOT DEPTH:	100 FEET
DEVELOPMENT SETBACKS:	
STREET:	20 FEET
SIDE:	10 FEET
REAR:	10 FEET
WATERBODY:	25 FEET
PRESERVE:	20 FEET (30 FEET FOR FIRE DEPENDENT INDIGENOUS PLANT COMMUNITIES)
BUILDING SEPARATION:	ONE HALF THE SUM OF THE HEIGHTS OF BOTH BUILDINGS, OR 20 FEET WHICHEVER IS GREATER.
MINIMUM DEVELOPMENT PERIMETER:	25 FEET (INDUSTRIAL AREAS) 15 FEET (COMMERCIAL AREAS)
MAXIMUM BUILDING HEIGHT:	65 FEET
MINIMUM OPEN SPACE:	30%

LANDSCAPE BUFFERS: (LEE COUNTY ZONING ORD. Z-23-030, MCP)

DIRECTION	BUFFER REQUIRED	BUFFER PROVIDED
NORTHEAST (ABUTTING SR82)	15' TYPE D	15' TYPE D
NORTHWEST (BLACKSTONE DRIVE)	15' TYPE D	15' TYPE D
SOUTHWEST/ WEST (ABUTTING PARCELS 20-45-27-63-00239.0010, 20-45-27-63-00239.0020, AND PORTION OF 20-45-27-63-00239.0030)	5' TYPE A	5' TYPE A
WEST (ABUTTING PRESERVE)	NO BUFFER REQUIRED	NO BUFFER REQUIRED
EAST (ABUTTING LAMSID CANAL)	NO BUFFER REQUIRED	NO BUFFER REQUIRED
SOUTH	NO BUFFER REQUIRED	NO BUFFER REQUIRED
INTERNAL BUFFERS	NO BUFFER REQUIRED PER LDC SECTION 33-1405 FOR COMMERCIAL PROJECTS PART OF MIXED USE DEVELOPMENT BETWEEN INTERNAL USES	NO BUFFER REQUIRED

OPEN SPACE AND INDIGENOUS PRESERVE CALCULATIONS (PER ADD2024-00219):

REQUIRED OPEN SPACE:
COMMERCIAL USE AREA A: 61.78 AC x 30% = 18.53 AC
REQUIRED INDIGENOUS (50% OF REQUIRED OPEN SPACE):
COMMERCIAL USE AREA A: 18.53 AC x 50% = 9.27 AC

PROVIDED INDIGENOUS PRESERVE AREAS:
9.27 AC (3.44 AC ON SITE AND 5.83 AC OFF SITE, SEE PRESERVE PLAN)

ON SITE:		OFFSITE:	
AREA 1:	0.36 AC	AREA 6:	0.51 AC
AREA 2:	1.24 AC	AREA 7:	3.96 AC
AREA 3:	0.53 AC	AREA 8:	1.08 AC
AREA 4:	0.91 AC	PORTION OF AREA 9:	0.17 AC
AREA 5:	0.40 AC	AREA 10:	0.03 AC
TOTAL:	3.44 AC	AREA 11:	0.08 AC
		TOTAL:	5.83 AC

PROVIDED OPEN SPACE:

LAKE (AT CONTROL ELEVATION):	4.22 AC
DRY DETENTION (AT DETENTION BOTTOM):	1.62 AC
COMMERCIAL PARCELS 1-14 @ 10% EA:	4.23 AC
INDIGENOUS PRESERVE AREAS:	9.27 AC
TOTAL:	19.34 AC

GENERAL NOTES:

1. IRRIGATION TO BE PROVIDED BY WELL.
2. BLACKSTONE CORPORATE PARK PROPERTY OWNERS ASSOCIATION SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF THE WATER MANAGEMENT SYSTEM, OPEN SPACE, COMMON AREAS, AND PRIVATE STREETS.
3. REFERENCE FDOT ACCESS AND DRAINAGE EXCEPTION PERMIT AND ASSOCIATED PLANS FOR WORK IN THE FDOT RIGHT OF WAY.
4. FOR SIDEWALK IN LEE COUNTY ROW (LEEDOT), SECTION 10-296 TABLE 3 GOVERNS FOR SIDEWALK CONSTRUCTION. SEE DETAIL SHEET FOR FURTHER INFORMATION.
5. SIDEWALK AND CURB REPLACEMENT LIMITS MUST EXTEND TO THE NEAREST CONSTRUCTION JOINT TO PREVENT SMALL SECTIONS OF EXISTING PANELS/CURB FROM BEING LEFT IN PLACE.
6. FOR PAVEMENT WITHIN THE LEE COUNTY ROW (LEEDOT), SECTION 10-296 TABLE 3 GOVERNS.

FGUA GENERAL NOTES:

1. ALL CONSTRUCTION SHALL MEET LEE COUNTY STANDARDS AND SPECIFICATIONS AS APPLICABLE.
2. AN FGUA REPRESENTATIVE IS TO BE PRESENT DURING CONNECTIONS TO EXISTING FACILITIES WITH 72-HOUR NOTIFICATION.
3. ALL UTILITIES WITHIN THE PROPERTY LINES WILL BE OWNED AND MAINTAINED BY THE PROPERTY OWNER.
4. ANY UTILITIES (PIPES, VALVES, AND ASSOCIATED HARDWARE) TO BE CONVEYED TO FGUA WILL BE LOCATED WITHIN AN EXISTING PUBLIC UTILITY EASEMENT, RIGHT-OF-WAY (ROW) AND/OR A DEDICATED FGUA UTILITY EASEMENT.
5. ANY REAL PROPERTY (LIFT STATIONS, PUMP HOUSES, WELLS, ETC.) WILL NOT BE LOCATED WITHIN THE ROW AND WILL BE CONVEYED TO FGUA VIA A SPECIFIC WARRANTY DEED.
6. LOW PRESSURE AIR TEST, AND VIDEO INSPECTION ARE REQUIRED FOR THE GRAVITY COLLECTION SYSTEM – WATER SYSTEMS ARE TESTED TO A MINIMUM OF 150PSI FOR TWO-HOUR DURATION AS PER AWWA C 600 STANDARDS. FIRE SYSTEMS ARE NOT THE FGUA RESPONSIBILITY.
7. INSTALL 10-GAUGE TRACER WIRE ON ALL MAIN PIPELINES. WIRE TO EXTEND INTO EACH VALVE BOX WITH SUFFICIENT EXCESS TO BRING WIRE THREE FEET ABOVE FINISH GRADE.
8. ALL WATER AND/OR WASTEWATER LINES PASSING UNDER PAVED AREAS WILL BE CONSTRUCTED WITH A CASING AND SHALL APPLY WITH LOCAL OR STATE DOT STANDARDS.
9. A REDUCED PRESSURE ZONE (RPZ), BACKFLOW PREVENTION ASSEMBLY (BFP) IS REQUIRED. ACCEPTABLE MODELS FEBCO 825Y, WATTS 909, WILKINS 975XL, OR CONBRACO 400.
10. FULL CIRCLE STAINLESS STEEL TAPPING SADDLE SLEEVE WITH RESILIENT SEATED TAPPING VALVE FOR PROPOSED WATER AND FORCE MAIN CONNECTIONS TO BE USED.
11. WATER TAPPING SADDLES TO BE DOUBLE STRAP BRASS OR DUCTILE IRON OR EPOXY COATED STEEL WITH STAINLESS STEEL HARDWARE. TIE STRAPS AND BOLTS SHALL BE CORROSION RESISTANT ALLOY STEEL.
12. ALL FIRE FLOW LINES WILL RUN SEPARATE FROM THE DOMESTIC WATER MAINS AND HAVE A FIRE DETECTION METER AND A FIRE LINE BACKFLOW PREVENTION ASSEMBLY (BFP) ASSEMBLY SHALL INCLUDE A LOW FLOW BY-PASS METER W/ RPZ.
13. MANHOLE FRAMES & COVERS, AND CO CAPS ARE TO BE WATER TIGHT, IF NEEDED EXISTING MANHOLE TOPS ARE TO BE ADJUSTED TO PROPOSED GRADE.
14. INFLOW PROTECTION DISHES ARE REQUIRED FOR ALL MANHOLES; STAINLESS STEEL IN PAVED AREAS, ABS IN UNPAVED AREAS.
15. A CLEANOUT SHALL BE PROVIDED AT THE ROW/PL FOR ALL SERVICE CONNECTIONS
16. THE FGUA SHALL OWN AND MAINTAIN ALL WATER SERVICE LINES UP TO THE POINT OF CONNECTION. All WATER MAINS SHALL BE A MINIMUM OF FOUR INCHES (4") IN DIAMETER AND ALL SUCH WATER MAINS SHALL BE CONSTRUCTED IN DEDICATED RIGHTS-OF-WAY OR APPROPRIATE UTILITY EASEMENTS DEDICATED TO THE FGUA.
17. ALL SEWER FORCE MAIN LINES AND GRAVITY SERVICE LINES ACCEPTED MUST BE AT LEAST FOUR INCHES (4") IN DIAMETER AND EIGHT INCHES (8"), RESPECTIVELY, EXCEPT LATERAL SEWER SERVICE LINES WHICH SHALL BE NO LESS THAN SIX INCHES (6") IN DIAMETER AND ACCEPTED BY THE FGUA TO A CLEANOUT LOCATED ON THE EDGE OF THE RIGHT-OF-WAY OR AN EASEMENT.
18. A BALL CHECK VALVE FOLLOWED BY A PLUG VALVE WILL BE INSTALLED ON FORCE MAINS LOCATED IN PUE OR R/W AT THE PROPERTY LINE DISCHARGING FROM PRIVATELY OWNED LIFT STATIONS CONNECTING INTO THE UTILITY MAINS
19. CONNECTION TO EXISTING MANHOLES AND/OR LIFT STATIONS SHALL BE BY CORE BORE WITH KOR-N-SEAL CONNECTORS.
20. ALL CONNECTIONS WILL BE MADE AS HOT TAPS. ANY EXCEPTIONS TO THIS WILL REQUIRE FGUA APPROVAL. ALL HOT TAP CONNECTIONS SHALL BE PERFORMED BY A LICENSED, UNDERGROUND CONTRACTOR, WITH ADVANCE NOTIFICATION (72 HOURS) TO THE FGUA INSPECTOR.
21. BACKFLOW PREVENTION DEVICES SHALL BE PRIVATELY OWNED AND MAINTAINED. BFPDS SHOULD BE LOCATED OUTSIDE OF ANY FGUA EASEMENT OR ROW/PUBLIC EASEMENT. IT IS THE SOLE RESPONSIBILITY OF THE PROPERTY OWNER TO MAINTAIN THE DEVICE AND PROVIDE ANNUAL TESTING CERTIFICATIONS TO THE FGUA.

Lehigh Acres Fire Control and Rescue District
Life Safety Division

Office Address:
636 Thomas Sherwin Ave S
Lehigh Acres, FL
Phone: 239-303-5335
Email: LifeSafety@Lehighfd.com

Mailing Address:
11 Homestead Rd S
Lehigh Acres, FL 33936
Phone: 239-303-5300
Fax: 239-369-2436
www.Lehighfd.com

Fire Flow Test Report

Fire Inspector: J.Nickell Date: 10/25/2024 Time: 14:16:00 PM

Requested by Firm: Respec Name: Dan Mumford, E.I
Address: 14122 Jackson St Ste 1 Email Address:
City, State, Zip Code: Fort Myers, FL 33901
Phone: 239-418-0691 Project Name: Blackstone Corprate Park

Static / Residual Pressure Hydrant: LEH-00792 - 336 Black Stone Dr
Flow Hydrant: LEH-00796 - 837 Lockman Dr



Static Pressure 55 PSI Test Duration: 6 Minutes
Residual Pressure 40 PSI Estimated Water Used: 6956 Gallons

	Outlet No. 1		Outlet No. 2	
Inside DIameter of Hydrant Outlet =	2.5	Inches	2.5	Inches
Discharge Coefficient of Hydrant Outlet =	0.9		0.9	
Pitot or Pressure Gauge Reading =	35	PSI		PSI

Min Fire Flow Pressure = 20 PSI Test Flow = 992.68 GPM 0.00 GPM
Combined Flow of Outlets No. 1 & 2 = 992.68 GPM

Available Fire Flow @ 20 PSI = 1,568.62 GPM

Coefficients: 0.7 - Squared Butt / 0.8 - Smooth Squared Butt / 0.9 - Smooth and Round Butt Page 1 of 2

FIRE FLOW

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.936.5490

PROJECT NOTES

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE AT THE TIME. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND NUMBER OF LINES ARE CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

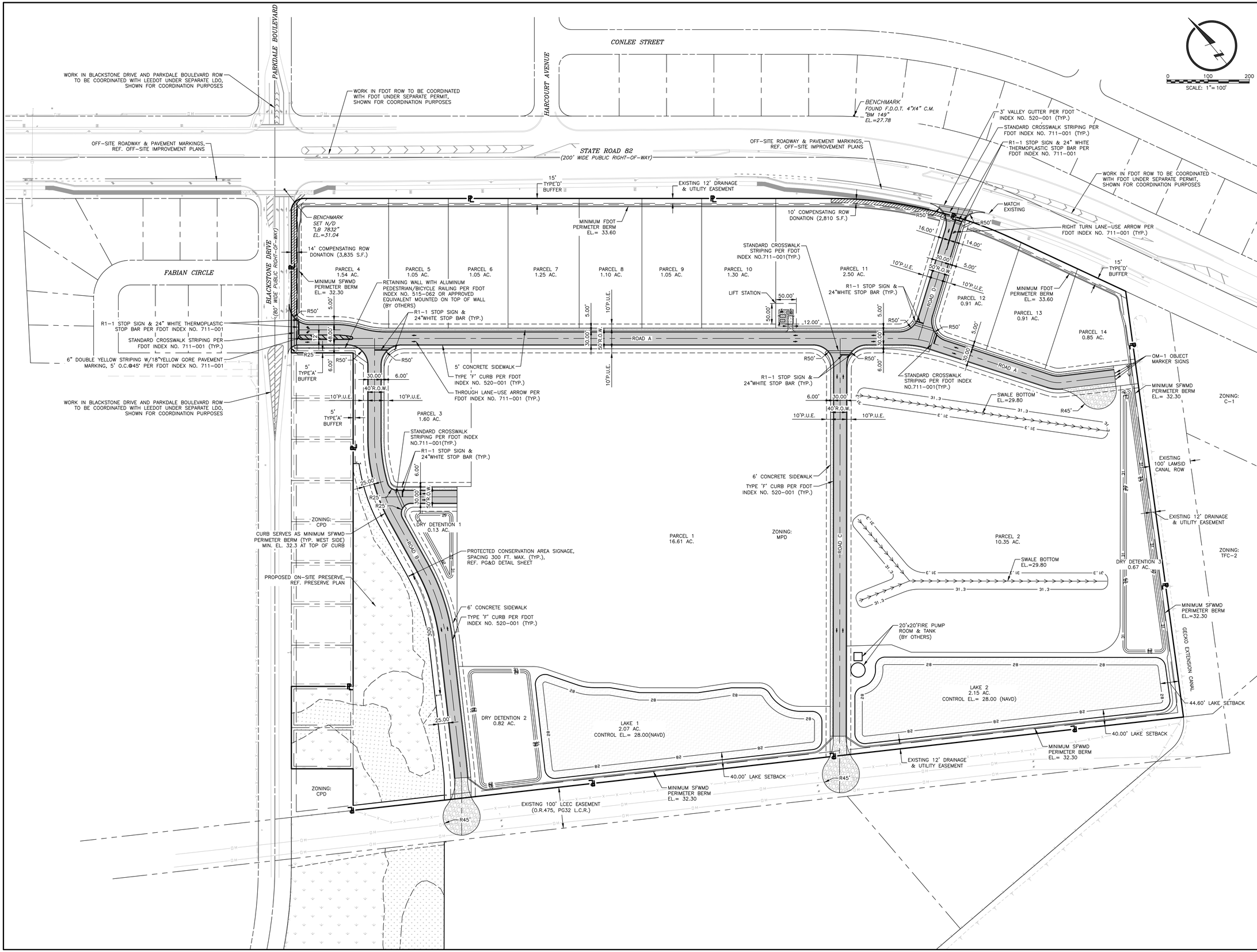
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA

PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88

BOOK AND PAGE:

SHEET 05

c:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\05 PROJECT NOTES.dwg, 2/17/2025 11:45:11 AM, KIANA ZALEWSKI, ATWELL, LLC



ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

MASTER SITE PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:

CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THE CONTRACTOR SHALL VERIFY ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

RACHEL J. TRACEY, P.E.

P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 06

x:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\06 MASTER SITE PLAN.dwg, 2/17/2025 11:45:21 AM, VINAY ZALEAMOR, ATWELL, LLC

STATE ROAD 82
(200' WIDE PUBLIC RIGHT-OF-WAY)

BLACKSTONE DRIVE
(80' WIDE PUBLIC RIGHT-OF-WAY)

SCALE: 1"= 80'

BENCHMARK
SET N/D
"LB 7832"
EL.=31.04

MINIMUM SFWMD PERIMETER BERM EL.= 32.30

OVERALL BASIN AREA

MINIMUM FDOT PERIMETER BERM EL.= 33.60

S-01 CATCH BASIN RIM EL.= 30.80 I.E. (30" SW)= 24.94

S-02 TYPE 9 INLET RIM EL.= 32.10 I.E. (30" SW)= 24.94 I.E. (30" NE)= 24.94

S-03 TYPE 9 INLET RIM EL.= 32.10 I.E. (30" NE)= 24.94 I.E. (24" SE)= 24.94

S-04 JUNCTION BOX RIM EL.= 32.90 I.E. (24" NW)= 24.68 I.E. (36" SE)= 23.20 I.E. (36" SW)= 23.20

S-05 CATCH BASIN RIM EL.= 30.80 I.E. (38" SW)= 25.81

S-06 TYPE 9 INLET RIM EL.= 32.20 I.E. (38" NE)= 25.81 I.E. (38" SW)= 25.81

S-07 TYPE 9 INLET RIM EL.= 32.20 I.E. (36" NW)= 23.20 I.E. (38" NE)= 25.81 I.E. (45" SE)= 23.00

S-08 CATCH BASIN RIM EL.= 30.80 I.E. (38" SW)= 24.50

S-09 TYPE 9 INLET RIM EL.= 32.20 I.E. (38" NE)= 24.50 I.E. (38" SW)= 24.50

S-10 TYPE 9 INLET RIM EL.= 32.20 I.E. (45" NW)= 23.00 I.E. (38" NE)= 24.50 I.E. (24" SE)= 24.68

S-11 CATCH BASIN RIM EL.= 30.80 I.E. (38" SW)= 24.30

S-12 TYPE 9 INLET RIM EL.= 32.00 I.E. (38" NE)= 24.30 I.E. (38" SW)= 24.30

S-13 TYPE 9 INLET RIM EL.= 32.00 I.E. (30" NE)= 24.30 I.E. (24" NW)= 24.68 I.E. (45" SE)= 23.73

S-14 TYPE 9 INLET RIM EL.= 32.00 I.E. (30" SW)= 24.85 I.E. (30" NE)= 24.85

S-15 TYPE 9 INLET RIM EL.= 32.00 I.E. (45" NW)= 23.73 I.E. (30" NE)= 24.85 I.E. (36" SE)= 23.73

S-16 TYPE 9 INLET RIM EL.= 32.00 I.E. (18" SW)= 27.80

S-59 CATCH BASIN RIM EL.= 30.80 I.E. (30" SW)= 24.85

S-18 CATCH BASIN RIM EL.= 30.80 I.E. (30" SE)= 26.57

S-19 TYPE 9 INLET RIM EL.= 32.00 I.E. (30" NW)= 26.57 I.E. (30" SE)= 26.57

S-20 TYPE 9 INLET RIM EL.= 32.00 I.E. (30" NW)= 26.57 I.E. (30" SE)= 26.57 I.E. (30" SW)= 26.57

S-21 CATCH BASIN RIM EL.= 30.80 I.E. (30" NW)= 26.57

S-23 TYPE 9 INLET RIM EL.= 32.00 I.E. (18" SW)= 27.80

S-26 CATCH BASIN RIM EL.= 30.80 I.E. (30" SW)= 27.62

S-17 TYPE 9 INLET RIM EL.= 32.00 I.E. (36" NW)= 23.73 I.E. (18" NE)= 27.65 I.E. (36" SE)= 23.73

S-24 TYPE 9 INLET RIM EL.= 32.00 I.E. (42" NW)= 23.73 I.E. (18" NE)= 27.65 I.E. (42" SE)= 23.73

S-60 'BUBBLE UP' CATCH BASIN RIM EL.= 29.80 I.E. (24" NW)= 26.55

S-28 TYPE 9 INLET RIM EL.= 32.00 I.E. (42" NW)= 23.73 I.E. (30" NE)= 27.62 I.E. (53" SE)= 22.45

S-27 TYPE 9 INLET RIM EL.= 32.00 I.E. (30" NE)= 27.62 I.E. (30" SW)= 27.62

S-63 'BUBBLE UP' CATCH BASIN RIM EL.= 29.80 I.E. (24" SE)= 23.55

S-22 JUNCTION BOX RIM EL.= 33.00 I.E. (36" NW)= 23.73 I.E. (30" NE)= 26.57 I.E. (42" SE)= 23.73

S-29 'BUBBLE UP' CATCH BASIN RIM EL.= 30.00 I.E. (53" NW)= 22.45
12"HIGHx42"WIDE SLOT I.E. (E)= 29.00
12"HIGHx36"WIDE SLOT I.E. (N)= 29.00
12"HIGHx42"WIDE SLOT I.E. (S)= 29.00

S-64 'BUBBLE UP' CATCH BASIN RIM EL.= 29.00 I.E. (24" NW)= 23.55

S-66 'BUBBLE UP' CATCH BASIN RIM EL.= 29.00 I.E. (24" NW)= 23.55

S-65 'BUBBLE UP' CATCH BASIN RIM EL.= 29.80 I.E. (24" SE)= 23.55

S-62 'BUBBLE UP' CATCH BASIN RIM EL.= 29.80 I.E. (24" NW)= 23.70

S-61 'BUBBLE UP' CATCH BASIN RIM EL.= 29.80 I.E. (24" NW)= 26.55

S-47 TYPE 9 INLET RIM EL.= 32.10 I.E. (18" SE)= 28.00

S-48 TYPE 9 INLET RIM EL.= 32.10 I.E. (18" NW)= 27.85 I.E. (24" SE)= 26.55

S-49 TYPE 9 INLET RIM EL.= 32.20 I.E. (18" SE)= 28.00

S-50 TYPE 9 INLET RIM EL.= 32.20 I.E. (18" NW)= 27.85 I.E. (24" SE)= 26.55

S-52 TYPE 9 INLET RIM EL.= 32.00 I.E. (18" NW)= 27.85 I.E. (24" SE)= 23.70 I.E. (30" SW)= 23.70

S-51 TYPE 9 INLET RIM EL.= 32.00 I.E. (18" SE)= 28.00

S-53 TYPE 9 INLET RIM EL.= 32.00 I.E. (18" SE)= 28.20

S-54 TYPE 9 INLET RIM EL.= 32.00 I.E. (18" NW)= 28.20 I.E. (30" NE)= 23.70

S-55 F.E.S. I.E. (24" SE)= 19.50

S-56 F.E.S. I.E. (24" NW)= 19.50

S-57 F.E.S. I.E. (24" E)= 19.50

S-58 HEADWALL I.E. (24" NW)= 25.50
REF. FDOT INDEX NO. 430-030, STRAIGHT CONCRETE ENDWALL

S-30 TYPE 9 INLET RIM EL.= 32.10 I.E. (36" NE)= 23.20 I.E. (18" SE)= 27.75 I.E. (42" SW)= 23.20

S-31 TYPE 9 INLET RIM EL.= 32.10 I.E. (18" NW)= 27.90

S-32 TYPE 9 INLET RIM EL.= 32.00 I.E. (42" NE)= 23.20 I.E. (18" SE)= 27.55 I.E. (42" S)= 23.20

S-33 TYPE 9 INLET RIM EL.= 32.00 I.E. (18" NW)= 27.70

S-34 TYPE 9 INLET RIM EL.= 31.80 I.E. (18" E)= 27.60

S-35 TYPE 9 INLET RIM EL.= 31.80 I.E. (42" N)= 23.20 I.E. (18" W)= 27.45 I.E. (48" SE)= 23.20

S-36 'BUBBLE UP' CATCH BASIN RIM EL.= 30.00 I.E. (48" NW)= 23.20 I.E. (24" NE)= 24.05 I.E. (48" SW)= 23.20

S-37 CATCH BASIN RIM EL.= 30.80 I.E. (24" SW)= 24.05

S-38 TYPE 9 INLET RIM EL.= 31.80 I.E. (24" NE)= 24.05 I.E. (24" SW)= 24.05

S-39 TYPE 9 INLET RIM EL.= 31.80 I.E. (24" NE)= 24.05 I.E. (24" SW)= 24.05

S-40 TYPE 9 INLET RIM EL.= 31.65 I.E. (18" SE)= 27.40

S-41 TYPE 9 INLET RIM EL.= 31.65 I.E. (60" SE)= 22.17 I.E. (48" NE)= 23.20 I.E. (18" NW)= 27.25

S-42 'BUBBLE UP' CATCH BASIN RIM EL.= 30.00 I.E. (60" NW)= 22.17
12"HIGHx42"WIDE SLOT I.E. (E)= 29.00
12"HIGHx36"WIDE SLOT I.E. (N)= 29.00
12"HIGHx42"WIDE SLOT I.E. (S)= 29.00

S-43 TYPE 9 INLET RIM EL.= 31.70 I.E. (18" SE)= 28.00

S-44 TYPE 9 INLET RIM EL.= 31.70 I.E. (18" NW)= 27.85 I.E. (18" SE)= 24.09

S-45 'BUBBLE UP' CATCH BASIN RIM EL.= 30.00 I.E. (18" NW)= 24.09
12"HIGHx24"WIDE SLOT I.E. (E)= 29.00

S-46 F.E.S. I.E. (24" NW)= 19.50

CURB SERVES AS MINIMUM SFWMD PERIMETER BERM (TYP. WEST SIDE) MIN. EL. 32.3 AT TOP OF CURB

DRY DETENTION 1
0.13 AC.

DRY DETENTION 2
0.82 AC.

DRY DETENTION 3
0.67 AC.

LAKE 1
2.07 AC.
CONTROL EL.= 28.00(NAVD)

LAKE 2
2.15 AC.
CONTROL EL.= 28.00 (NAVD)

SITE BASIN WEST

SITE BASIN EAST

LAKE BASIN WEST

LAKE BASIN EAST

20' LAKE MAINTENANCE EASEMENT

LITTORAL SHELF AREA

SWALE BOTTOM EL.=29.80

RIPRAP SUMP (TYP.)

GECKO EXTENSION CANAL

CS-DD 1 REF. PG&D DETAIL SHEET

CS-DD 2 REF. PG&D DETAIL SHEET

CS-1 REF. PG&D DETAIL SHEET

19"X30" ERCP
24"X38" ERCP
29"X45" ERCP
38"X60" ERCP
34"X53" ERCP
24"RCP
18"RCP
36"RCP
42"RCP
48"RCP
19"X30" ERCP
24"ADS HP

NOTES:

1. PARCEL 1 REQUIRED TO CONVEY STORMWATER TO DRY DETENTION 1 AND/OR 2 FOR DRY PRE-TREATMENT.

2. PARCEL 2 REQUIRED TO CONVEY STORMWATER TO DRY DETENTION 3 FOR PRE-TREATMENT. PARCEL 2 REQUIRED TO CONVEY STORMWATER FROM S-60, 61 AND 62 THROUGH SITE TO DRY DETENTION 3 AT BUILDOUT.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

MASTER DRAINAGE PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE, EXPRESSED OR IMPLIED, THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

RACHEL J. TRACEY, P.E.

P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 07

s:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\07 MASTER DRAINAGE PLAN.dwg, 2/17/2025 11:45:34 AM, VIVIAN ZALEZNIKOW, ATWELL, LLC

BENCHMARK
SET N/D
"LB 7832"
EL.=31.04

BLACKSTONE DRIVE
(80' WIDE PUBLIC RIGHT-OF-WAY)

MINIMUM SFWMD
PERIMETER BERM
EL.= 32.30

MINIMUM FDOT
PERIMETER BERM
EL.= 33.60

FILL PARCELS TO
31.30 FT-NAVD

FILL PARCEL TO
31.30 FT-NAVD

FILL PARCEL TO
31.30 FT-NAVD

DRY DETENTION 1
0.13 AC.

CURB SERVES AS MINIMUM SFWMD
PERIMETER BERM (TYP. WEST SIDE)
MIN. EL. 32.3 AT TOP OF CURB

MATCHLINE SHEET 09

MATCHLINE SHEET 10: LAKE 1

0 50 100
SCALE: 1"= 50'

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PAVING, GRADING & DRAINAGE PLAN 1

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811
ALL UTILITIES ARE SHOWN BASED UPON THE INFORMATION AVAILABLE TO THE CORPORATION. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, TYPE AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.
Know what's below.
Call before you dig.

NOT RELEASED FOR CONSTRUCTION

RACHEL J. TRACEY, P.E.
P.E. LIC. #65322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 08

n:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\DR PAVING, GRADING & DRAINAGE PLAN 1.dwg, 2/17/2025 11:45:48 AM, VIVIAN ZALESKOVA, ATWELL, LLC

MATCHLINE SHEET 08

MINIMUM FDOT PERIMETER BERM EL.= 33.60

FILL PARCELS TO 31.30 FT-NAVD

STATE ROAD 82 (200' WIDE PUBLIC RIGHT-OF-WAY)

SCALE: 1"= 50'

MINIMUM FDOT PERIMETER BERM EL.= 33.60

FILL PARCELS TO 31.30 FT-NAVD

TOP OF BANK

MINIMUM SFWMD PERIMETER BERM EL.= 32.30

SWALE BOTTOM EL.=29.80

RIPRAP SUMP (TYP.)

FILL PARCEL TO 31.30 FT-NAVD

FILL PARCEL TO 31.30 FT-NAVD

SWALE BOTTOM EL.=29.80

DRY DETENTION 3 0.67 AC.

RIPRAP SUMP (TYP.)

20' LAKE MAINTENANCE EASEMENT

GECKO EXTENSION CANAL

MATCHLINE SHEET 10: LAKE 2

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PAVING, GRADING & DRAINAGE PLAN 2

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO ATWELL. HOWEVER, THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

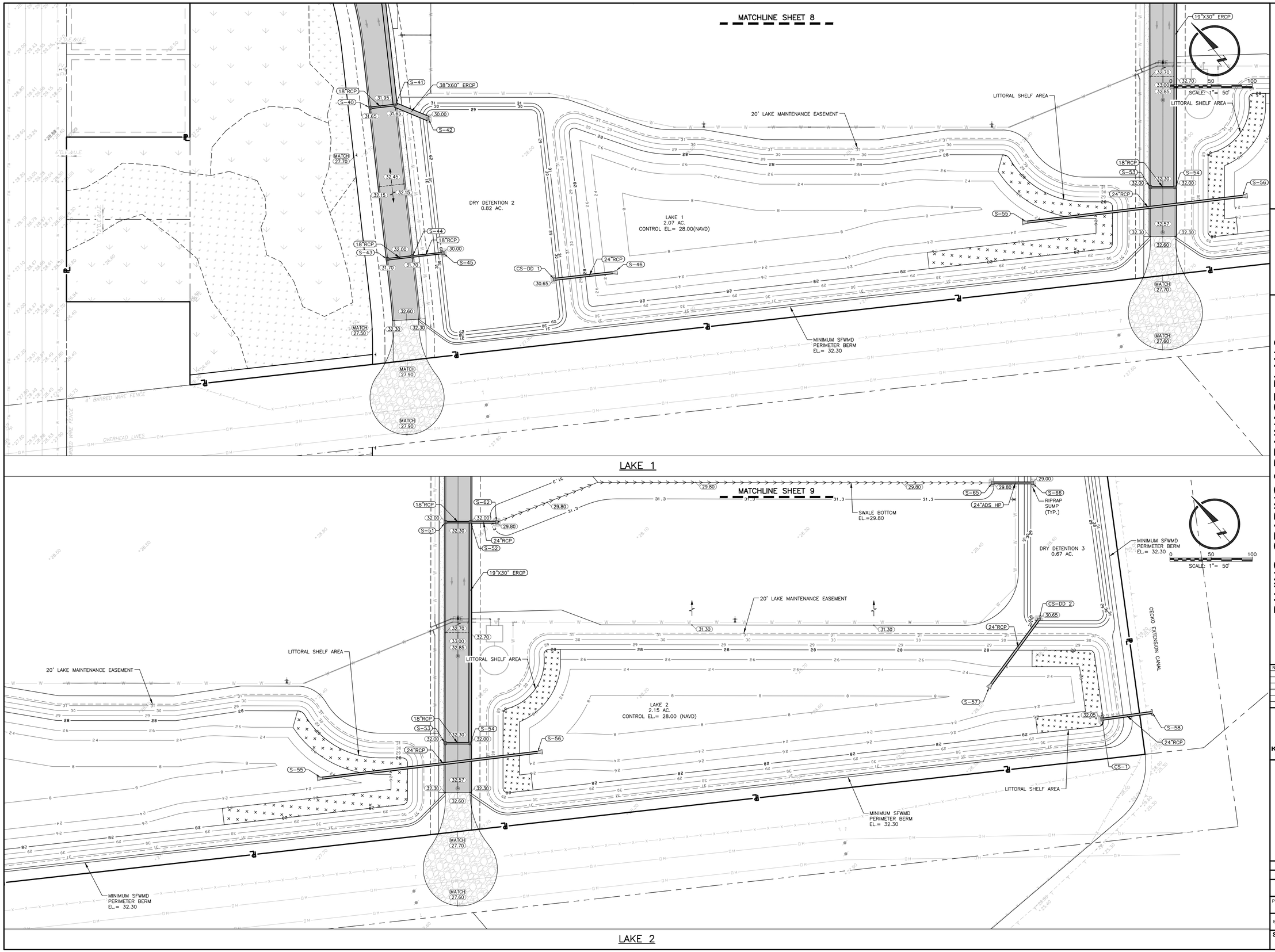
RACHEL J. TRACEY, P.E.

P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 09

c:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\09 PAVING, GRADING & DRAINAGE PLAN 2.dwg, 2/17/2025 11:46:01 AM, VIVIAN ZALESKOVA, ATWELL, LLC



ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PAVING, GRADING & DRAINAGE PLAN 3

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

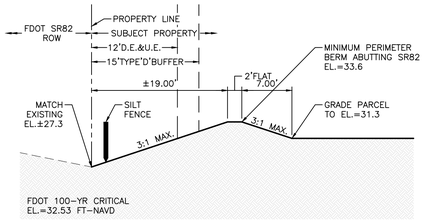
NOT RELEASED FOR CONSTRUCTION

RACHEL J. TRACEY, P.E.

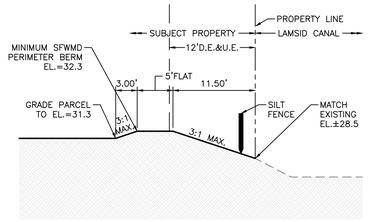
P.E. LIC. #85322

DRAFTED BY: VZ	DESIGNED BY: RJT	REVIEWED BY: DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

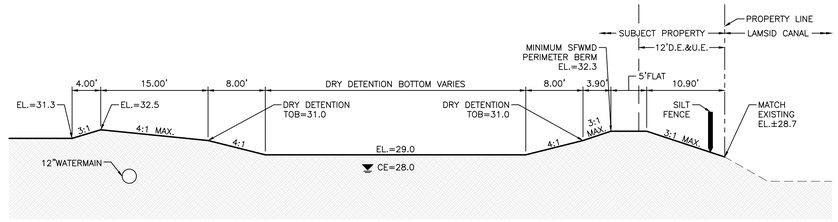
SHEET 10



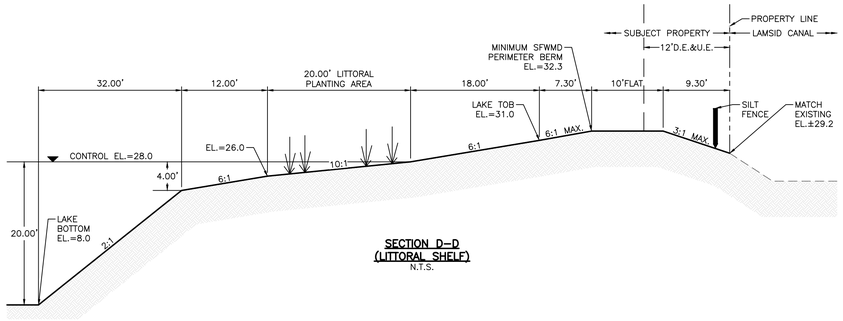
SECTION A–A
N.T.S.



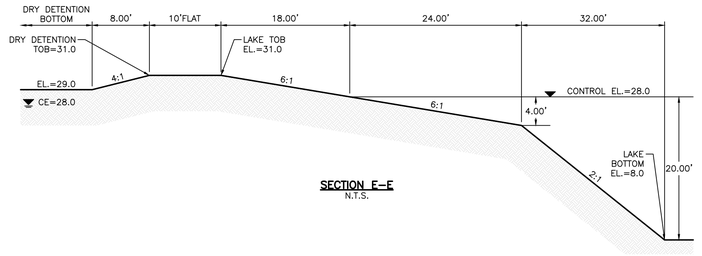
SECTION B–B
N.T.S.



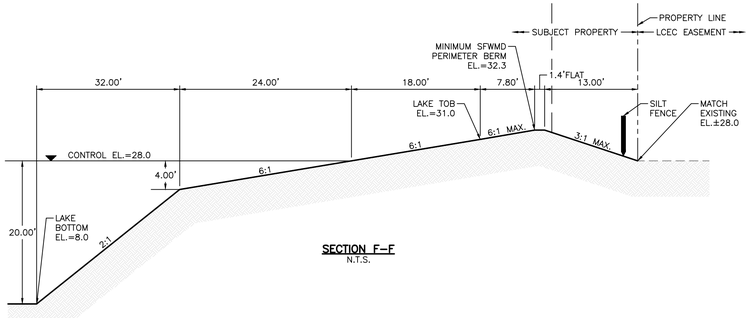
SECTION C–C
N.T.S.



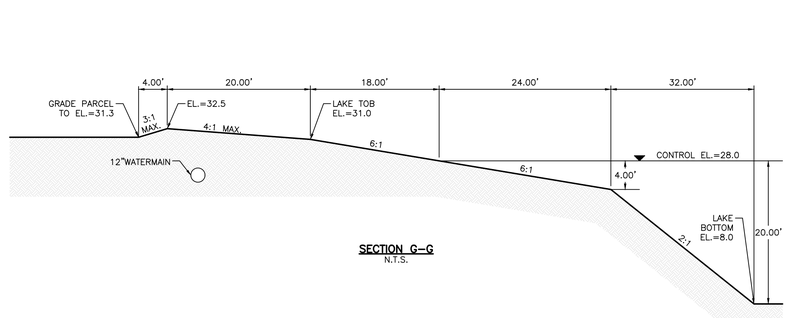
SECTION D–D
(LITTORAL SHELF)
N.T.S.



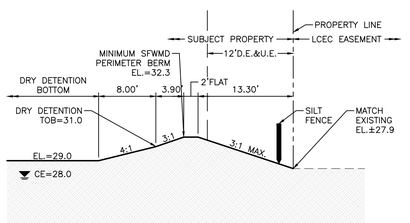
SECTION E–E
N.T.S.



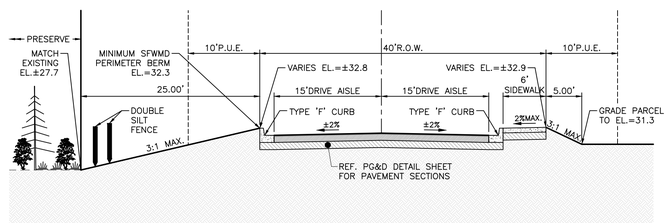
SECTION F–F
N.T.S.



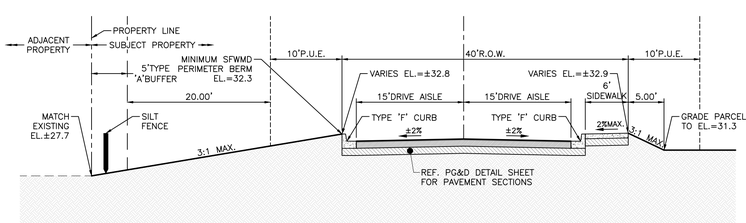
SECTION G–G
N.T.S.



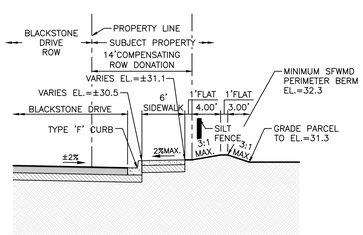
SECTION H–H
N.T.S.



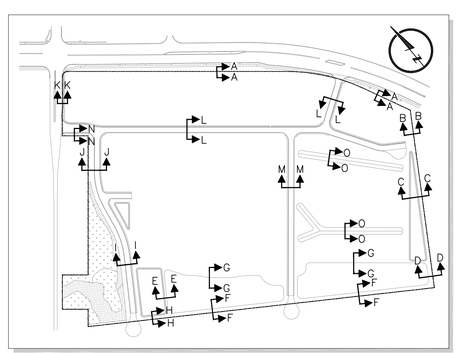
SECTION I–I
N.T.S.



SECTION J–J
N.T.S.



SECTION K–K
N.T.S.



KEY MAP
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

TYPICAL CROSS SECTIONS 1

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's **below.** **Call** before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL UTILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

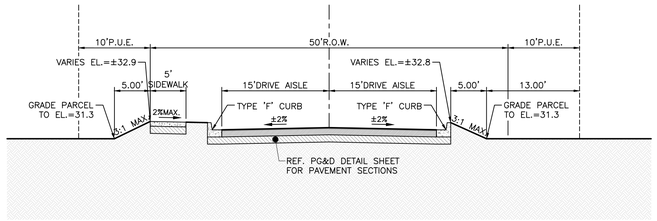
RACHEL J. TRACEY, P.E.

P.E. LIC. #85322

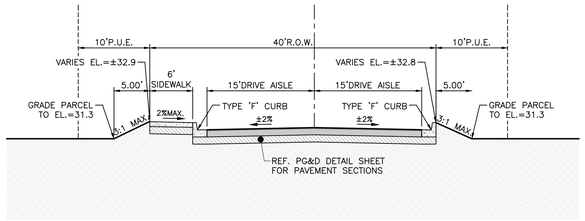
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET
11

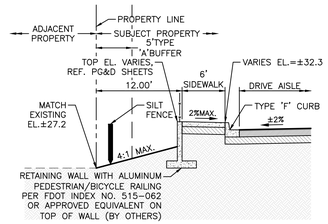
K:\Atwell\Jobs\24xx\24003608 - Fort Myers Blackstone\Engineering\Dev plans\11 TYPICAL CROSS SECTIONS.dwg, 2/17/2025 11:46:28 AM, IVANA ZALESANSKI, ATWELL, LLC



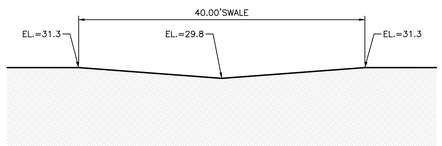
SECTION L–L
N.T.S.



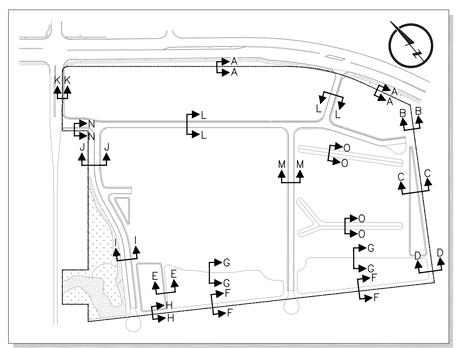
SECTION M–M
N.T.S.



SECTION N–N
N.T.S.



SECTION O–O
N.T.S.



KEY MAP
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

TYPICAL CROSS SECTIONS 2

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

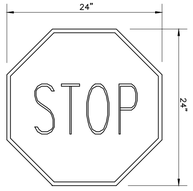
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

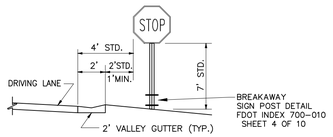
RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88
BOOK AND PAGE:		

SHEET 12



24" R1-1 STOP SIGN
N.T.S.

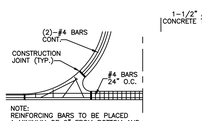


TYPICAL SIGN PLACEMENT
N.T.S.

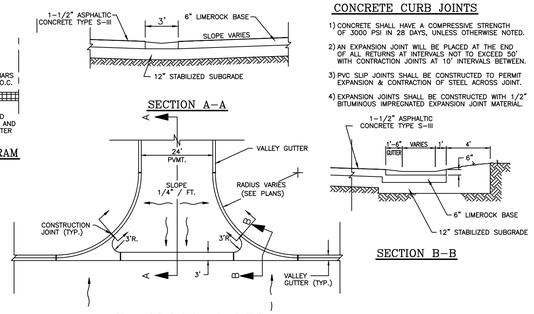
PRESERVE
PROTECTED AREA
NO MOWING
NO VEGETATION REMOVAL
NO FILLING
NO DUMPING

PRESERVE SIGN

PRESERVE SIGNS ARE TO BE A MAXIMUM SIZE OF 2 SQUARE FEET AND A MAXIMUM HEIGHT OF 4 FEET. SPACING OF SIGNS SHOWN ON PLAN IS EVERY 300 FEET MAXIMUM.



REINFORCING DIAGRAM



VALLEY CROSSING DETAIL
N.T.S.

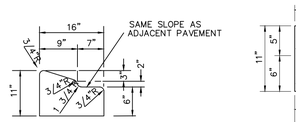
CONCRETE CURB JOINTS

1) CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI IN 28 DAYS, UNLESS OTHERWISE NOTED.

2) AN EXPANSION JOINT WILL BE PLACED AT THE END OF ALL RETURNS AT INTERVALS NOT TO EXCEED 50' WITH CONTRACTION JOINTS AT 10' INTERVALS BETWEEN.

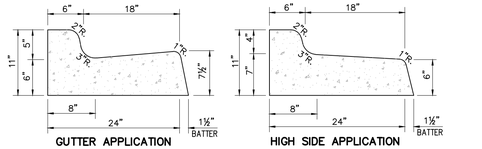
3) PVC SLIP JOINTS SHALL BE CONSTRUCTED TO PERMIT EXPANSION & CONTRACTION OF STEEL ACROSS JOINT.

4) EXPANSION JOINTS SHALL BE CONSTRUCTED WITH 1/2" BITUMINOUS IMPREGNATED EXPANSION JOINT MATERIAL.



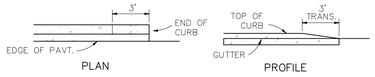
NOTES:
1. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH)
2. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 520.

TYPE 'A' CURB
N.T.S.



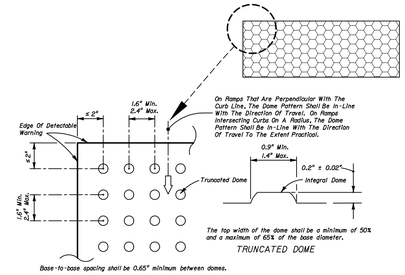
NOTES:
1. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 P.S.I. MINIMUM STRENGTH)
2. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 520.
3. WHEN USED ON HIGH SIDE OF PAVEMENTS, THE CROSS SLOPE OF THE GUTTER SHALL SPILL OUT WITH 1" FALL AS SHOWN.

TYPE "F" CURB
N.T.S.



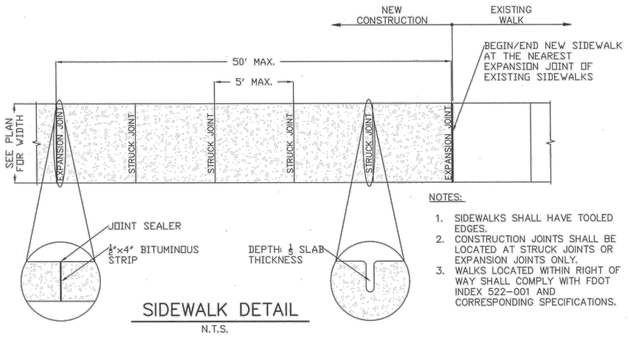
STRAIGHT END

CURB & GUTTER ENDING
N.T.S.



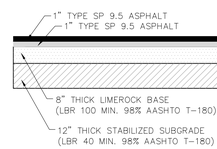
All Sidewalk Curb Ramps Shall Have Detectable Warning Surfaces That Extend The Full Width Of The Ramp And In The Direction Of Travel 24 inches (600 mm) From The Back Of Curb.

DETECTABLE WARNING PAD DETAIL
N.T.S.

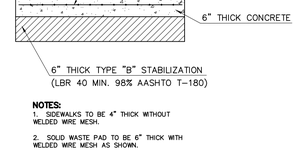


SIDEWALK DETAIL
N.T.S.

NOTES:
1. SIDEWALKS SHALL HAVE TOOLED EDGES.
2. CONSTRUCTION JOINTS SHALL BE LOCATED AT STRUCK JOINTS OR EXPANSION JOINTS ONLY.
3. WALKS LOCATED WITHIN RIGHT OF WAY SHALL COMPLY WITH FDOT INDEX 522-001 AND CORRESPONDING SPECIFICATIONS.

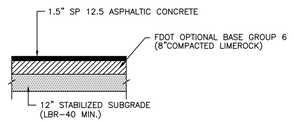


ASPHALT SECTION
N.T.S.

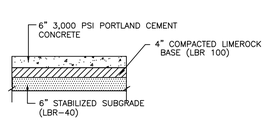


NOTES:
1. SIDEWALKS TO BE 4" THICK WITHOUT WELDED WIRE MESH.
2. SOLID WASTE PAD TO BE 6" THICK WITH WELDED WIRE MESH AS SHOWN.

SIDEWALK CONCRETE SECTION
N.T.S.



TYPICAL PAVEMENT SECTION (OFF-SITE)
LEEDOT R/W
N.T.S.



TYPICAL SIDEWALK SECTION (OFF-SITE)
LEEDOT R/W
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PAVING, GRADING & DRAINAGE DETAILS 1

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION AND/OR THE SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

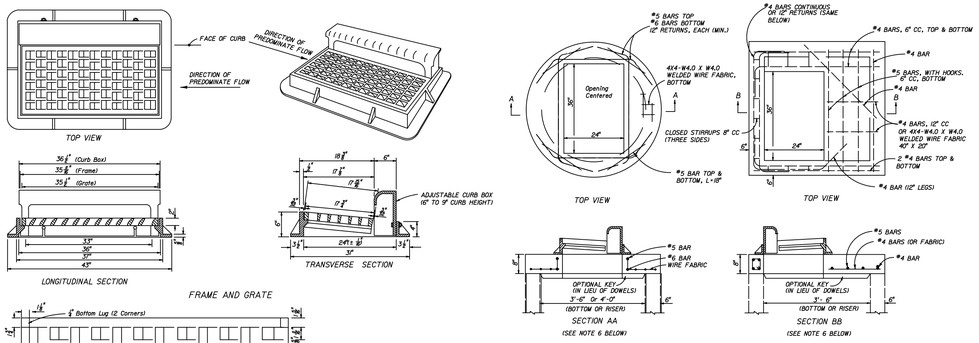
RACHEL J. TRACEY, P.E.

P.E. LIC. #65322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET
13

x:\Atwell\Jobs\24xx\24003608 - fort myers boca\on\engineering\dwg plans\13 PAVING, GRADING & DRAINAGE DETAILS.dwg, 2/17/2025 11:46:35 AM, IVAN ZALESANSKI, ATWELL, LLC



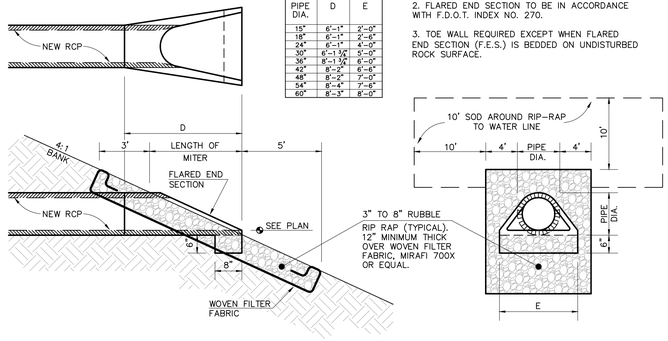
GENERAL NOTES

1. THIS INLET IS PRIMARILY INTENDED FOR LOCATIONS WITH LIGHT TO MODERATE FLOWS WHERE RIGHT OF WAY DOES NOT PERMIT THE USE OF THROATED CURB INLETS TYPES I THROUGH 6. THE TYPICAL APPLICATION IS ON CURB RETURNS TO CITY STREETS. THE INLET GRATE IS SUITABLE FOR PEDESTRIAN AND BICYCLE TRAFFIC.
2. THIS INLET TO BE LOCATED IN VERTICAL FACED CURBS SUCH AS CURB AND GUTTER TYPE F. GRATE SHALL BE ORIENTED WITH VANES DIRECTED TOWARD PREDOMINATE FLOW. INLET TO BE LOCATED OUTSIDE PEDESTRIAN CROSSWALK WHERE PRACTICAL.
3. FOR STRUCTURE BOTTOMS SEE INDEX NO. 200. FOR SUPPLEMENTAL DETAILS SEE INDEX NO. 201.
4. ALL STEEL IN SLAB TOPS SHALL HAVE MINIMUM COVER UNLESS OTHERWISE SHOWN. TOPS SHALL BE EITHER CAST-IN-PLACE OR PRECAST CONCRETE.
5. FOR ALTERNATE B APPLICATIONS, TOP SLAB OPENINGS SHALL BE PLACED SUCH THAT 2 EDGES OF INLET FRAME WILL BE LOCATED DIRECTLY ABOVE BOTTOM WALL OR RISER WALL.
6. WHEN USED ON A STRUCTURE WITH DIMENSIONS LARGER THAN THOSE DETAILED ABOVE AND RISERS ARE NOT APPLIED, THE TOP SLAB SHALL BE CONSTRUCTED USING INDEX 200 WITH THE SLAB OPENING ADJUST TO 24"x 36". THE "SPECIAL TOP SLAB" ON INDEX 200 IS NOT PERMITTED.
7. FRAME MAY BE ADJUSTED WITH ONE TO SIX COURSES OF BRICK.
8. INLET AND GRATE DETAIL SHOWN IS NEENAH R-3067-L. VANED GRATES WITH APPROXIMATELY EQUAL OPENINGS WILL BE PERMITTED THAT SATISFY AASHTO H-20 LOADING. INLET AND GRATE SHALL BE CLASS 30 CASTINGS IN ACCORDANCE WITH ASTM A 48. GRATES SHALL BE REVERSIBLE, RIGHT OR LEFT.

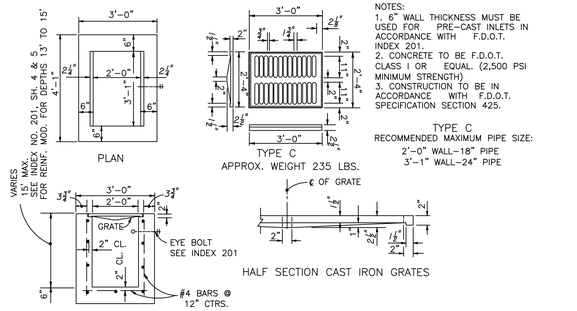
INLET TOPS TYPE 9
N.T.S.

TABLE		
PIPE DIA.	D	E
15"	6'-1"	3'-0"
18"	6'-1"	3'-6"
24"	6'-1"	4'-0"
30"	6'-1 1/2"	5'-0"
36"	8'-1 1/2"	6'-0"
42"	8'-2"	6'-6"
48"	8'-2"	7'-0"
54"	8'-4"	7'-6"
60"	8'-3"	8'-0"

NOTES:
1. TYPICAL ALL PIPES INTO LAKES.
2. FLARED END SECTION TO BE IN ACCORDANCE WITH F.D.O.T. INDEX NO. 270.
3. TOE WALL REQUIRED EXCEPT WHEN FLARED END SECTION (F.E.S.) IS BEDDED ON UNDISTURBED ROCK SURFACE.



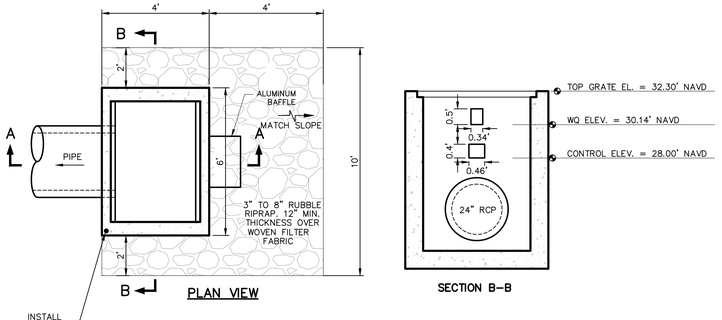
FLARED END SECTION PIPE TREATMENT
N.T.S.



NOTES:
1. 6" WALL THICKNESS MUST BE USED FOR PRE-CAST INLETS IN ACCORDANCE WITH F.D.O.T. INDEX 201.
2. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH)
3. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 425.

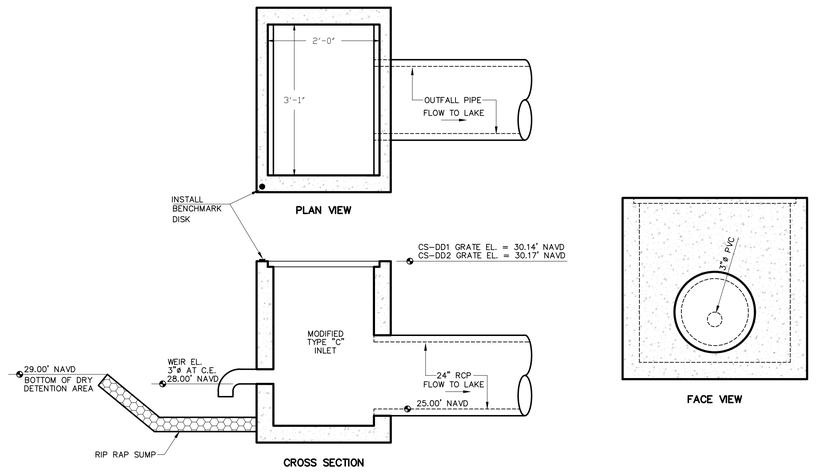
TYPE C
RECOMMENDED MAXIMUM PIPE SIZE:
2'-0" WALL-18" PIPE
3'-1" WALL-24" PIPE

INLET "C" DETAIL
N.T.S.



NOTE:
FURTHER DESIGN OF TYPE "E" INLET SEE FDOT INDEX NO.425-052

WATER CONTROL STRUCTURE DETAIL
CS-1
N.T.S.



WATER CONTROL STRUCTURE DETAIL
CS-DD1 & CS-DD2
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PAVING, GRADING & DRAINAGE DETAILS 2

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE. THE CONTRACTOR IS THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE FOR EACH IS ACCURATE. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

RACHEL J. TRACEY, P.E.
P.E. LIC. #65332

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 14

c:\Users\vzeh\24hrs\24003608 - fort myers blackstone\engineering\dev plans\13 PAVING, GRADING & DRAINAGE DETAILS.dwg, 2/17/2025 11:46:37 AM, VIVIAN ZALESKOSKI, ATWELL, LLC



ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PRESERVE PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CASTO

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

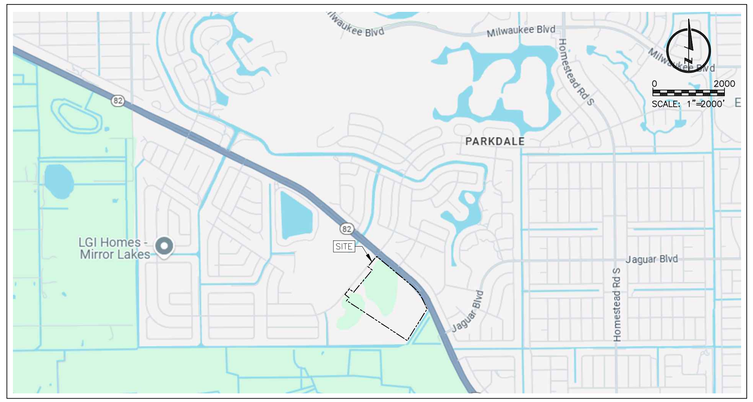
RACHEL J. TRACEY, P.E.

P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 15

BLACKSTONE CORPORATE PARK



GENERAL LOCATION MAP



LOCATION MAP

SECTION 20

TOWNSHIP 45 SOUTH

RANGE 27 EAST

LEE COUNTY, FLORIDA

SITE INFORMATION

PROJECT AREA: 61.78 AC

PARCEL STRAP NO: 20-45-27-72-00051.0000, 20-45-27-55-00044.0000
20-45-27-63-00239.0100, 20-45-27-63-00239.0090

PARCEL ADDRESS: ACCESS UNDETERMINED
LEHIGH ACRES, FL 33913

CURRENT ZONING: MPD

PROPOSED USE: COMMERCIAL

PREPARED FOR:

CNL FORT MYERS BLACKSTONE, LLC

6562 UNIVERSITY PARKWAY, SUITE 210
SARASOTA, FLORIDA 34240

UTILITY SERVICE PROVIDERS

WATER:	
FLORIDA GOVERNMENTAL UTILITY AUTHORITY	9400 SOUTHPARK CENTER LOOP, SUITE 400 ORLANDO, FLORIDA 32819 PHONE: (407) 629-6900
SEWER:	
FLORIDA GOVERNMENTAL UTILITY AUTHORITY	9400 SOUTHPARK CENTER LOOP, SUITE 400 ORLANDO, FLORIDA 32819 PHONE: (407) 629-6900
STORMWATER:	
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT	601 EAST COUNTY LANE LEHIGH ACRES, FLORIDA 33936 PHONE: (239) 368-0044
ELECTRIC:	
LEE COUNTY ELECTRIC COOPERATIVE, INC.	P.O. BOX 3455 NORTH FORT MYERS, FLORIDA 33918 PHONE: (239) 656-2300
PHONE:	
LUMEN TECHNOLOGIES	2820 CARGO STREET FORT MYERS, FLORIDA 33916 PHONE: (239) 233-2686
CABLE:	
COMCAST	12641 CORPORATE LAKES DRIVE FORT MYERS, FLORIDA 33913 PHONE: (239) 986-7043
FIRE PROTECTION:	
LEHIGH ACRES FIRE CONTROL AND RESCUE DISTRICT	11 HOMESTEAD ROAD S. LEHIGH ACRES, FLORIDA 33936 PHONE: (239) 303-5300
SOLID WASTE DISPOSAL:	
WASTE PRO USA	13110 RICKENBACKER PARKWAY FORT MYERS, FLORIDA 33913 PHONE: (239) 337-0800

PROJECT CONTACTS

CIVIL ENGINEER:	
ATWELL, L.L.C. RACHEL J. TRACEY, P.E., PROJECT MANAGER	10511 SIX MILE CYPRESS PARKWAY, SUITE 101 FORT MYERS, FLORIDA 33966 PHONE: (239) 939-5490
SURVEYOR:	
ATWELL, L.L.C. ALLEN M. VOSE III, P.S.M.	10511 SIX MILE CYPRESS PARKWAY, SUITE 101 FORT MYERS, FLORIDA 33966 PHONE: (239) 939-5490
LANDSCAPE ARCHITECT:	
DAVID M. JONES, JR. AND ASSOCIATES, INC. GREGORY J. DISERIO, R.L.A.	2221 McGREGOR BOULEVARD FORT MYERS, FLORIDA 33901 PHONE: (239) 337-5525
TRAFFIC ENGINEER:	
TR TRANSPORTATION CONSULTANTS, INC. TED TREESH	2726 OAK RIDGE COURT, SUITE 503 FORT MYERS, FLORIDA 33901 PHONE: (239) 278-3090

#	SHEET TITLE
01	COVER SHEET
02	EXISTING CONDITIONS & DEMOLITION PLAN
03	BEST MANAGEMENT PRACTICE PLAN & DETAILS
04	GENERAL NOTES
05	PROJECT NOTES
06	MASTER SITE PLAN
07	MASTER DRAINAGE PLAN
08	PAVING, GRADING & DRAINAGE PLAN 1
09	PAVING, GRADING & DRAINAGE PLAN 2
10	PAVING, GRADING & DRAINAGE PLAN 3
11	TYPICAL CROSS SECTIONS 1
12	TYPICAL CROSS SECTIONS 2
13	PAVING, GRADING & DRAINAGE DETAILS 1
14	PAVING, GRADING & DRAINAGE DETAILS 2
15	PRESERVE PLAN
16	MASTER UTILITY PLAN
17	PLAN & PROFILE 1
18	PLAN & PROFILE 2
19	PLAN & PROFILE 3
20	PLAN & PROFILE 4
21	PLAN & PROFILE 5
22	PLAN & PROFILE 6
23	LIFT STATION DETAILS 1
24	LIFT STATION DETAILS 2
25	LIFT STATION DETAILS 3
26	UTILITY DETAILS 1
27	UTILITY DETAILS 2
28	UTILITY DETAILS 3
29	UTILITY DETAILS 4
30	OFF-SITE ROADWAY IMPROVEMENT PLAN
31	OFF-SITE ROADWAY IMPROVEMENT PLAN
32	OFF-SITE ROADWAY IMPROVEMENT PLAN
33	OFF-SITE ROADWAY SECTIONS 1
34	OFF-SITE ROADWAY SECTIONS 2
35	OFF-SITE ROADWAY SECTIONS 3
36	OFF-SITE ROADWAY SECTIONS 4
37	OFF-SITE ROADWAY SECTIONS 5
38	OFF-SITE ROADWAY AUTOTURN ANALYSES

FGUA PROJECT NUMBER: 24-088-LED

FDOT PERMIT NUMBERS:
ACCESS PERMIT #2025-A-192-00019
DRAINAGE PERMIT #2025-D-192-00029

ALL ELEVATIONS ON CIVIL ENGINEERING PLANS REFERENCE: NAVD'88

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	REVISED PER COUNTY & FGUA COMMENTS	VZ
2	10/17/2025	REVISED PER COUNTY COMMENTS	VZ
3	02/06/2026	REVISED PER COUNTY COMMENTS	VZ

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

RACHEL J. TRACEY, P.E.
P.E. LIC. #65322

c:\Atwell\Jobs\24xx\24003606 - fort myers blackstone\engineering\dev plans\01 COVER SHEET.dwg, 2/9/2026 11:43:53 AM, VIVIAN ZALTZMAN, ATWELL, LLC

s:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\02 EXISTING CONDITIONS & DEMOLITION PLAN.dwg, 2/9/2026 11:44:33 AM, VIVIAN ZALESANSKI, ATWELL, LLC



NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THE LOCATIONS AND SIZES OF THE FACILITIES ARE SHOWN ON THE PLANS AS THE SUBJECT FACILITY. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICES ONLY PRIOR TO CONSTRUCTION.

EXISTING CONDITIONS & DEMOLITION PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

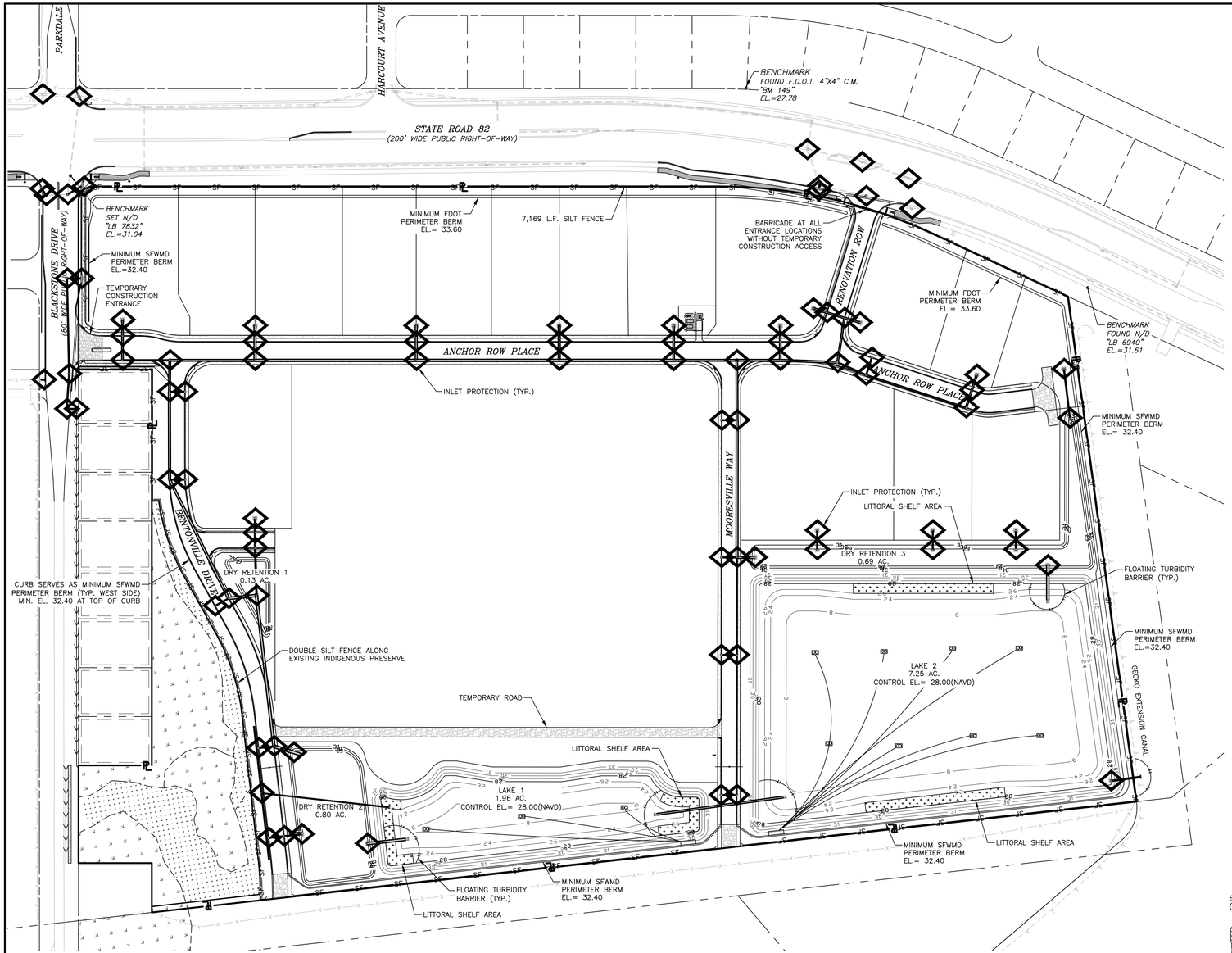
PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

RACHEL J. TRACEY, P.E.
P.E. LIC. #8302

DRAFTED BY: VZ	DESIGNED BY: RJT	REVIEWED BY: DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 02



LEGEND:

= STAKED SEDIMENT AT INLETS

—SF— = STAKED SILT FENCE

= TURBIDITY BARRIER

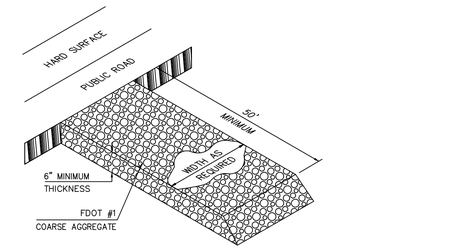
0 120 240
SCALE: 1"=120'

GENERAL NOTES:

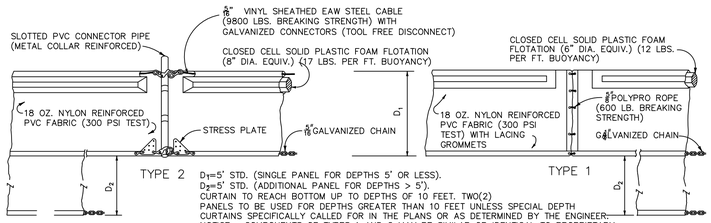
1. PRIOR TO COMMENCEMENT OF CONSTRUCTION ACTIVITY, APPROPRIATE EROSION CONTROL DEVICES SHALL BE INSTALLED TO CONTROL AND REDUCE SOIL EROSION AND SEDIMENT TRANSPORT TO OFF SITE AREAS. THE CONTRACTOR SHALL MAINTAIN THESE DEVICES THROUGHOUT THE DURATION OF CONSTRUCTION. ALL DEVICES SHALL REMAIN IN PLACE UNTIL THE SURROUNDING AREAS ARE ESTABLISHED.

2. THE FOLLOWING MINIMUM REQUIREMENTS ARE RECOMMENDED: (REFERENCE FLORIDA DEVELOPMENT MANUAL, FDER, PPS 6-301 TO 6-500).
   - A) BMP 1.01 – TEMPORARY GRAVEL CONSTRUCTION ENTRANCE
   - B) BMP 1.05 – STRAW BALE BARRIER
   - C) BMP 1.06 – SILT FENCE
   - D) BMP 1.08 – STORM INLET DRAIN PROTECTION

3. THESE BEST MANAGEMENT PRACTICES (BMP) ARE TYPICAL OF THE REQUIREMENTS FOR SOIL EROSION CONTROL PER DIVISION 3.7 OF THE LEE COUNTY LAND DEVELOPMENT CODE. THEY MAY NOT CONSTITUTE COMPLETE REQUIREMENTS FOR COMPLIANCE WITH REGULATORY AGENCIES AND SPECIFIC PERMIT CONDITIONS.

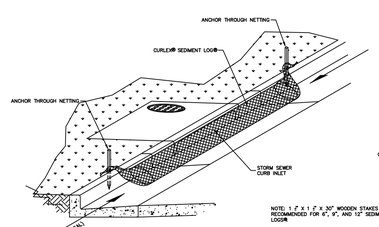


GRAVEL CONSTRUCTION ENTRANCE
Not To Scale

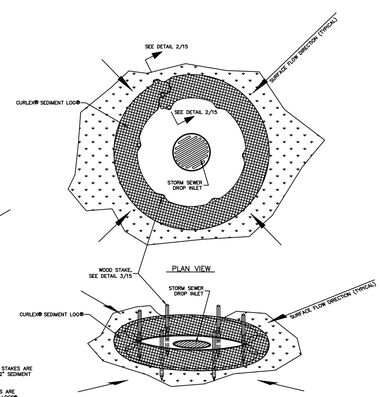


D1=5' STD. (SINGLE PANEL FOR DEPTHS 5' OR LESS).
D2=5' STD. (ADDITIONAL PANEL FOR DEPTHS > 5').
CURTAIN TO REACH BOTTOM UP TO DEPTHS OF 10 FEET. TWO(2) PANELS TO BE USED FOR DEPTHS GREATER THAN 10 FEET UNLESS SPECIAL DEPTH CURTAINS SPECIFICALLY CALLED FOR IN THE PLANS OR AS DETERMINED BY THE ENGINEER.
NOTICE: COMPONENTS OF TYPES 1 AND 2 MAY BE SIMILAR OR IDENTICAL TO PROPRIETARY DESIGNS. ANY INFRINGEMENT ON THE PROPRIETARY RIGHTS OF THE DESIGNER SHALL BE THE SOLE RESPONSIBILITY OF THE USER. SUBSTITUTIONS FOR TYPES 1 AND 2 SHALL BE AS APPROVED BY THE ENGINEER.

FLOATING TURBIDITY BARRIER DETAIL



DETAIL 2
CURLEX® SEDIMENT LOGS®
CURB INLET PROTECTION
NO SCALE

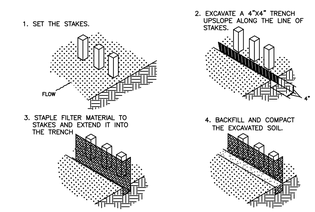


NOTE: 1.5" X 1.5" X 30" WOODEN STAKES ARE RECOMMENDED FOR 6", 9", AND 12" SEDIMENT LOGS®

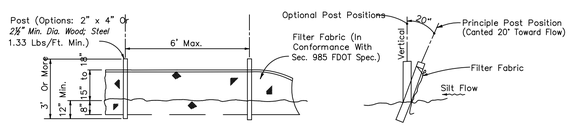
1.5" X 1.5" X 48" WOODEN STAKES ARE RECOMMENDED FOR 20" SEDIMENT LOGS®

DETAIL 1
CURLEX® SEDIMENT LOGS®
DROP INLET PROTECTION
NO SCALE

CURLEX SEDIMENT LOGS
CURB INLET PROTECTION

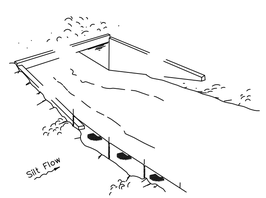


CONSTRUCTION OF A FILTER BARRIER
NTS

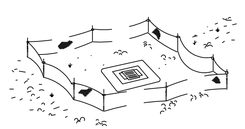


Note: Silt Fence to be paid for under the contract unit price for Staked Silt Fence (LF).

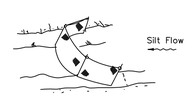
TYPE III SILT FENCE
Not To Scale



Type III Silt Fence



Type III Silt Fence Protection
Around Ditch Bottom Inlets.



Note: Spacing for Type III Silt Fence to be in accordance with Chart I, Sheet 1 of 3 and ditch installations at drainage structures Sheet 2 of 3.

Type III Silt Fence

Do not deploy in a manner that silt fences will act as a dam across permanent flowing watercourses. Silt fences are to be used at upland locations and turbidity barriers used at permanent bodies of water.

SILT FENCE APPLICATIONS
Not To Scale

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.936.5490

BEST MANAGEMENT PRACTICE PLAN & DETAILS

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND CONFIGURATION SHOWN IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 03

K:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\03 BEST MANAGEMENT PRACTICE PLAN & DETAILS.dwg, 2/6/2026 11:44:51 AM, KHAN ZAIZANGIR, ATWELL, LLC

PRE-BID SUBMITTAL:

THE CONTRACTOR SHALL VISIT THE PROJECT SITE PRIOR TO BIDDING TO FAMILIARIZE HIMSELF WITH THE CONDITIONS FOR CONSTRUCTION ACTIVITIES. THE CONTRACTOR SHALL OBTAIN FROM THE OWNER A WRITTEN LIST OF ALL PERMITS AND COPIES THEREOF, AND CAREFULLY REVIEW ALL PLANS, SPECIFICATIONS, AND PERMITS PREVIOUSLY SECURED ON BEHALF OF THE OWNER. IN CASE OF ANY DISCREPANCY EITHER IN PERMIT DOCUMENTS, PLANS, DRAWINGS, OR SPECIFICATIONS, THE CONTRACTOR MUST PROMPTLY SUBMIT A "WRITTEN CLARIFICATION REQUEST" TO THE OWNER, WHO WILL PROMPTLY FORWARD THE REQUEST TO THE ENGINEER WHO WILL MAKE A DETERMINATION IN WRITING. THE CONTRACTOR MUST VERIFY EXISTING FACILITY INFORMATION, AND ALL DESIGN/PERMIT DATA REQUIRED FOR WORK THAT IS TO CONNECT WITH EXISTING FACILITIES. ANY DISCREPANCIES BETWEEN THE CONTRACT REQUIREMENTS AND THE EXISTING CONDITIONS MUST BE REPORTED TO THE OWNER IN WRITING, FOR AN ENGINEERING DETERMINATION. ANY FUTURE ADJUSTMENT DUE TO FAILURE BY THE CONTRACTOR TO IDENTIFY THE RELATED DISCREPANCY, WILL BE AT THE CONTRACTOR'S EXPENSE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ANY LICENSES, ADDITIONAL PERMITS, AND FOR COMPLYING WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL LAWS, CODES, AND REGULATIONS IN CONNECTION WITH THE PERFORMANCE OF THE WORK.

CONSTRUCTION SAFETY AND LIABILITY:

THE CONTRACTOR MUST TAKE PROPER SAFETY AND HEALTH PRECAUTIONS TO PROTECT THE WORK, THE WORKERS, THE PUBLIC, AND THE PROPERTY OF OTHERS. THE CONTRACTOR IS RESPONSIBLE ALSO FOR ALL MATERIALS DELIVERED AND WORK PERFORMED UNTIL COMPLETION AND ALL ACCEPTANCES HAVE BEEN OBTAINED. THE CONTRACTOR SHALL MAINTAIN TRAFFIC DURING CONSTRUCTION IN ACCORDANCE WITH THE STATE OF FLORIDA ROADWAY AND TRAFFIC DESIGN STANDARDS (LATEST EDITION). THE CONTRACTOR IS RESPONSIBLE FOR ALL DAMAGE TO PERSONS OR PROPERTY THAT OCCURS AS A RESULT OF THE CONTRACTOR'S NEGLIGENCE. THE CONTRACTOR MUST SAVE HARMLESS AND INDEMNIFY THE OWNER AND THE ENGINEER OF RECORD, ITS OFFICERS, REPRESENTATIVES AND EMPLOYEES FROM ALL CLAIMS, LOSS, DAMAGE, ACTIONS, CAUSES OF ACTION, AND/OR EXPENSES RESULTING FROM, BROUGHT FOR, OR ON ACCOUNT OF ANY PERSONAL INJURY OR PROPERTY DAMAGE RECEIVED OR SUSTAINED BY ANY PERSONS OR PROPERTY GROWING OUT OF OCCURRING, OR ATTRIBUTABLE TO ANY ACT OR OMISSION UNDER OR RELATED TO THE CONTRACT, RESULTING IN WHOLE OR IN PART FROM THE NEGLIGENT ACTS OR OMISSIONS OF THE CONTRACTOR, ANY SUBCONTRACTOR, OR ANY EMPLOYEE, AGENT, OR REPRESENTATIVE OF THE CONTRACTOR OR ANY SUBCONTRACTOR.

PRE-CONSTRUCTION:

THE OWNER SHALL SECURE PRIOR TO CONSTRUCTION A PROFESSIONAL LAND SURVEYOR TO PERFORM AN "AS-BUILT" SURVEY OF ALL COMPLETED IMPROVEMENTS. THE OWNER SHALL ALSO SECURE PRIOR TO CONSTRUCTION A PROFESSIONAL ENGINEER TO PROVIDE THE APPROPRIATE SERVICES NEEDED IN ORDER TO CERTIFY TO ALL APPLICABLE REGULATORY AGENCIES THAT THE IMPROVEMENTS WERE CONSTRUCTED IN SUBSTANTIAL COMPLIANCE WITH ALL APPLICABLE PERMITS AND APPROVALS. THE OWNER SHALL COORDINATE A PRE-CONSTRUCTION MEETING WITH THE ENGINEER, SURVEYOR, CONTRACTOR, TESTING LAB, UTILITY COMPANIES, AND APPROPRIATE REGULATORY AGENCIES. THE CONTRACTOR SHALL PROVIDE A SHOP DRAWING SUBMISSION SCHEDULE FOR ALL PROJECT MATERIALS AND COMPONENTS. THE CONTRACTOR SHALL NOT INITIATE CONSTRUCTION OF ANY PORTION OF THE IMPROVEMENTS UNTIL THE SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED FOR THAT PORTION BY THE ENGINEER. THE OWNER, CONTRACTOR, ENGINEER AND UTILITY COMPANY SHALL ALSO DISCUSS ALL DOCUMENTATION REQUIRED FOR CONTRIBUTED FACILITIES TRANSFER FROM THE OWNER/DEVELOPER TO THE UTILITY COMPANY UPON PROJECT COMPLETION, UNLESS OTHERWISE SPECIFIED BY THE UTILITY COMPANY, THE FOLLOWING DOCUMENTS SHALL BE PROVIDED:

DOCUMENTS REQUIRED FOR CONTRIBUTED FACILITIES FROM DEVELOPER TO UTILITY COMPANY:

1. UTILITY EASEMENT(S): MUST BE RECORDED AT COUNTY CLERK OF COURT OFFICE BEFORE SUBMITTING TO UTILITY.
2. EASEMENT ACKNOWLEDGEMENT.
3. AFFIDAVIT.
4. RELEASE OF LIEN.
5. BILL OF SALE.
6. ASSIGNMENT OF RIGHTS UNDER UTILITY AGREEMENT: WHEN PROPERTY HAS BEEN TRANSFERRED TO A NEW OWNER.
7. RECORD DRAWINGS (AS-BUILT): MUST BE SIGNED AND SEALED BY ENGINEER OF RECORD, SUBMIT TWO REPRODUCIBLE COPIES OF PRINTS.
8. DETAILED COST OF CONSTRUCTION: MUST INCLUDE INDIVIDUAL ITEMS OR APPURTENANCES, UNIT COST AND TOTAL COST OF EACH. DO NOT INCLUDE WATER SERVICE LINES OR SEWER LATERALS.
9. DESCRIPTION OF FACILITIES: A SHORT EXPLANATION DEPICTING WHAT HAS BEEN CONSTRUCTED.
10. D.E.P. APPLICATION(S).
11. INSPECTION REPORT(S).
12. PRESSURE TEST REPORT(S).
13. INFILTRATION-EXFILTRATION TEST REPORT(S), INCLUDING VIDEO TAPES AND LAMPING REPORTS.
14. LIFT STATION INSPECTION (START-UP) REPORT(S) AND EQUIPMENT SHOP DRAWINGS.
15. BACTERIOLOGICAL TEST REPORT(S).
16. ENGINEER'S CERTIFICATE OF SUBSTANTIAL COMPLETION TO D.E.P.
17. SYSTEM(S) ACCEPTANCE LETTER(S) FROM D.E.P.

NOTES: ITEM 1-6 & 8 TO BE SUPPLIED BY OWNER. ITEMS 7 & 9-17 TO BE SUPPLIED TO ENGINEER OF RECORD.

UNLESS OTHERWISE SPECIFIED BY THE UTILITY, THE CONTRACTOR SHALL NOTIFY THE SUPERINTENDENTS OF THE WATER, GAS, SEWER, TELEPHONE, AND POWER COMPANIES, 10 DAYS IN ADVANCE, THAT THE CONTRACTOR INTENDS TO START WORK IN A SPECIFIC AREA. THE OWNER AND ENGINEER DISCLAIM ANY RESPONSIBILITY FOR THE SUPPORT AND PROTECTION OF SEWERS, DRAINS, WATER LINES, GAS LINES, CONDUITS OF ANY KIND, UTILITIES OR OTHER STRUCTURES OWNED BY THE CITY, COUNTY, STATE OR BY PRIVATE OR PUBLIC UTILITIES LEGALLY OCCUPYING ANY STREET, ALLEY, PUBLIC PLACE, RIGHT-OF-WAY, OR EASEMENT.

PROJECT SIGN:

THE CONTRACTOR SHALL PROVIDE AND MAINTAIN A CONSTRUCTION PROJECT SIGN AT A LOCATION DIRECTED BY THE OWNER. ATWELL, L.L.C. SHALL PROVIDE A SEPARATE SIGN FOR INSTALLATION BY THE CONTRACTOR AT THIS LOCATION. THESE SIGNS SHALL BE ERECTED WITHIN 15 DAYS AFTER RECEIVING A NOTICE TO PROCEED. UPON PROJECT COMPLETION, THE CONTRACTOR SHALL REMOVE THESE SIGNS AND RETURN TO ATWELL, L.L.C. THEIR SIGN.

ENVIRONMENTAL PROTECTION DURING CONSTRUCTION:

PROTECTION OF LAND RESOURCES – EXCEPT IN AREAS IDENTIFIED ON THE PLANS TO BE CLEARED, THE CONTRACTOR MUST NOT DEFACE, INJURE, OR DESTROY TREES OR SHRUBS OR REMOVE OR CUT THEM WITHOUT WRITTEN AUTHORIZATION FROM THE OWNER. IN THE ABSENCE OF A CLEARING PLAN, AREAS SHOWN FOR IMPROVEMENTS SHALL BE CLEARED UNLESS NOTED OTHERWISE.

PROTECTION OF WATER RESOURCES – IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO INVESTIGATE AND COMPLY WITH ALL APPLICABLE FEDERAL, STATE, REGIONAL COUNTY AND MUNICIPAL LAWS CONCERNING POLLUTION OF WATER RESOURCES. ALL WORK MUST BE PERFORMED IN SUCH A MANNER THAT OBJECTIONABLE CONDITIONS WILL NOT BE CREATED IN PUBLIC WATERS RUNNING THROUGH OR ADJACENT TO THE PROJECT AREA.

1. EROSION AND SEDIMENT CONTROL – ALL PRACTICAL AND NECESSARY EFFORT SHOULD BE TAKEN DURING CONSTRUCTION TO CONTROL AND PREVENT EROSION AND THE TRANSPORT OF SEDIMENT TO SURFACE DRAINS, SURFACE WATER, OR ONTO OTHER PROPERTY BY ANY OR ALL OF THE FOLLOWING METHODS:
   A. STORMWATER FACILITIES ARE TO BE BUILT AS EARLY IN THE CONSTRUCTION PHASE AS POSSIBLE TO ENSURE THE TREATMENT OF STORMWATER RUNOFF. TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES, HOWEVER, SUCH AS BERMS, SEDIMENT BASINS, GRASSING, SODDING, SAND BAGGING, BALED HAY OR STRAW, FLOATING SILT BARRIERS, STACKED SILT BARRIERS, ETC. MUST BE PROVIDED AND MAINTAINED UNTIL THE PERMANENT FACILITIES ARE COMPLETED AND OPERATIONAL.
   B. REVEGETATION AND STABILIZATION OF DISTURBED GROUND SURFACES SHOULD BE ACCOMPLISHED AS SOON AS POSSIBLE.
   C. FULL COMPACTION OF ANY FILL MATERIAL PLACED AROUND NEWLY INSTALLED STRUCTURES.
   D. PROHIBIT THE USE OF ANY CONSTRUCTION EQUIPMENT THAT LEAKS EXCESSIVE AMOUNTS OF FUEL OIL, OR HYDRAULIC FLUID.
2. ALL DISTURBED AREAS SHALL BE GRADED FOR POSITIVE DRAINAGE, EXCEPT RETENTION AREAS, AND SHALL BE STABILIZED BY SODDING, EXCEPT WHERE SEEDING AND MULCHING ARE CALLED FOR ON THE PLANS. THE LATEST VERSION OF THE F.D.O.T. ROAD AND BRIDGE SPECIFICATIONS SHALL BE USED UNLESS MORE RESTRICTIVE LOCAL SPECIFICATIONS EXIST.

   CONTRACTOR RESPONSIBLE FOR STABILIZING AND MAINTAINING SLOPES AND SOD THROUGHOUT CONSTRUCTION UNTIL SUCH TIME AS APPROVED BY THE ENGINEER.

PROTECTION OF FISH AND WILDLIFE:

THE CONTRACTOR MUST AT ALL TIMES PERFORM ALL WORK IN A WAY AND TAKE SUCH STEPS AS REQUIRED TO PREVENT ANY INTERFERENCE WITH OR DISTURBANCE TO FISH AND WILDLIFE. THE CONTRACTOR SHALL NOT ALTER WATER FLOWS OR OTHERWISE DISTURB NATIVE HABITATS AND JURISDICTIONAL WETLANDS LOCATED WITHIN AND / OR ADJACENT TO THE PROJECT AREA.

RECORDING AND PRESERVING HISTORICAL AND ARCHEOLOGICAL FINDS:

ALL ITEMS HAVING ANY APPARENT HISTORICAL OR ARCHEOLOGICAL INTEREST THAT ARE DISCOVERED IN THE COURSE OF ANY CONSTRUCTION ACTIVITIES MUST BE CAREFULLY PRESERVED. THE CONTRACTOR MUST LEAVE THE ARCHEOLOGICAL FIND UNDISTURBED AND MUST IMMEDIATELY REPORT THE FIND TO THE OWNER SO THAT THE PROPER AUTHORITY MAY BE NOTIFIED.

EARTHWORK:

1. GENERAL

1.01 SUBMITTALS

A. EROSION AND CONTROL MEASURES
B. COMPACTION TESTS
C. SOIL CLASSIFICATION TESTS
D. PRESERVATION PLANS

1.02 SITE EXAMINATION

A. CONTRACTORS, BEFORE SUBMITTING BIDS, SHALL INFORM THEMSELVES AS TO LOCATION AND NATURE OF THE WORK, CHARACTER OF EQUIPMENT AND FACILITIES NEEDED FOR PERFORMANCE OF THE WORK, GENERAL AND LOCAL CONDITIONS PREVAILING AT THE SITE, AND OTHER MATTERS WHICH MAY IN ANY WAY, AFFECT THE WORK UNDER CONTRACT.
B. EXAMINE SOURCES OF INFORMATION CONCERNING GROUND WATER LEVEL WHETHER SURFACE OR SUBSURFACE. EACH BIDDER TO DRAW HIS OWN CONCLUSION CONCERNING GROUND WATER LEVELS AND HOW WATER AFFECTS HIS WORK.

1.03 SUBSURFACE INVESTIGATIONS

A. SUBSURFACE DATA, INCLUDING GROUND WATER ELEVATIONS OR CONDITIONS, IF SHOWN ON THE DRAWINGS OR ATTACHED TO THESE SPECIFICATIONS, ARE PRESENTED ONLY AS INFORMATION THAT IS AVAILABLE WHICH INDICATED CERTAIN CONDITIONS FOUND AND LIMITED TO THE EXACT LOCATIONS. SHALL NOT BE INTERPRETED AS AN INDICATION OF CONDITIONS THAT MAY ACTUALLY BE DEVELOPED THROUGH THE PERIOD OF CONSTRUCTION. BIDDERS SHALL EXAMINE THE SITE OF THE WORK AND MAKE THEIR OWN DETERMINATION OF THE CHARACTER OF MATERIALS AND THE CONDITIONS TO BE ENCOUNTERED ON THE WORK, AND THEIR PROPOSAL SHALL BE BASED UPON THEIR OWN INVESTIGATIONS. THE OWNER AND ENGINEER SHALL NOT BE HELD RESPONSIBLE FOR VARIATIONS FOUND TO EXIST BETWEEN THE ATTACHED DATA ABOVE REFERRED TO AND ACTUAL FIELD CONDITIONS THAT DEVELOP THROUGH THE PERIOD OF CONSTRUCTION.
B. WHERE EXISTING GRADES, UTILITY LINES AND SUBSTRUCTURES ARE SHOWN ON THE DRAWINGS, THE OWNER AND ENGINEER ASSUME NO RESPONSIBILITY FOR CORRECTNESS OF EXISTING CONDITIONS INDICATED. THE CONTRACTOR SHALL ASCERTAIN EXACT LOCATIONS OF UTILITIES AND SUBSTRUCTURES THAT MAY BE AFFECTED BY THIS PROJECT, AND SHALL BE RESPONSIBLE FOR ANY DAMAGE OR INJURY THAT MAY RESULT FROM WORKING ON OR NEAR THOSE UTILITIES, SUBSTRUCTURES WHICH ARE NOT TO BE REMOVED OR DEMOLISHED.
C. THE CONTRACTOR SHALL MAKE HIS OWN DEDUCTIONS OF THE SUBSURFACE CONDITIONS WHICH MAY AFFECT METHODS OR COST OF CONSTRUCTION AND HE AGREES THAT THE CONTRACTOR WILL MAKE NO CLAIM FOR DAMAGES OR OTHER COMPENSATION EXCEPT SUCH AS ARE PROVIDED FOR IN THE AGREEMENT, SHOULD THE CONTRACTOR FIND CONDITIONS DURING THE PROGRESS OF THE WORK DIFFERENT FROM THOSE AS CALCULATED OR ANTICIPATED BY THE CONTRACTOR.

1.04 BENCHMARKS AND MONUMENTS

A. MAINTAIN CAREFULLY EXISTING BENCH MARKS, MONUMENTS, AND OTHER REFERENCE POINTS IF DISTURBED OR DESTROYED, REPLACE AS DIRECTED.

1.05 JOB CONDITIONS

A. CONDITION OF PREMISES: ACCEPT SITE AS FOUND AND EXCAVATE, FILL, COMPACT, AND BACKFILL SITE AS HEREINAFTER SPECIFIED.
B. PROTECTION
   a. EXISTING STRUCTURES AND PROPERTY: TAKE PRECAUTIONS TO GUARD AGAINST MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES AND FACILITIES; PROVIDE AND PLACE BRACING OR SHORING AS NECESSARY OR PROPER IN CONNECTION THEREWITH; BE RESPONSIBLE FOR SAFETY AND SUPPORT OF SUCH STRUCTURES; BE LIABLE FOR ANY MOVEMENT OR SETTLEMENT, ANY DAMAGE OR INJURY CAUSED THEREBY OR RESULTING THEREFROM. IF AT ANY SAFETY OR ANY ADJACENT STRUCTURES APPEARS TO BE ENDANGERED, CEASE OPERATIONS, TAKE PRECAUTIONS TO SUPPORT SUCH STRUCTURES AND NOTIFY THE OWNER. RESUME OPERATIONS ONLY AFTER PERMISSION HAS BEEN CHANGED BY THE OWNER.
   b. SIDEWALKS AND STREETS: TAKE PRECAUTIONS TO GUARD AGAINST MOVEMENT, SETTLEMENT OR COLLAPSE OF ANY SIDEWALKS, CURBS OR STREET PAVEMENTS ON ADJOINING SITE; BE LIABLE FOR ANY SUCH MOVEMENT, SETTLEMENT OR COLLAPSE; REPAIR PROMPTLY SUCH DAMAGE WHEN SO ORDERED; INSTALL SUCH SHORING, INCLUDING SHEET PILING, AS MAY BE REQUIRED DURING EXCAVATION, TO PROTECT BANKS, ADJACENT PAVING, STRUCTURES AND UTILITIES.
   c. RESPONSIBILITY: BE RESPONSIBLE FOR ANY DAMAGE TO EXISTING STRUCTURES OR TO EQUIPMENT AND FURNISHINGS HOUSED THEREIN WHICH ARE DUE DIRECTLY OR INDIRECTLY TO CONSTRUCTION OPERATIONS. REPAIR OR REPLACE SUCH ITEMS AS NECESSITATED BY SITE GRADING OR LOCATION OF NEW BUILDING. USE EVERY POSSIBLE PRECAUTION TO PREVENT INJURIES TO LANDSCAPING, DRIVES, CURBS AND WALKS ON OR ADJACENT TO SITE OF THE WORK AND REPLACE, AT NO EXPENSE TO OWNER, ANY OF SUCH DESTROYED.

2. EXECUTION

2.01 GENERAL

A. ACCOMPLISH IN A MANNER THAT PROVIDES FOR THE SAFETY OF THE PUBLIC AND WORKMEN AND PROVIDE FOR THE PROTECTION OF ALL PROPERTY.
B. CONSTRUCTION: DO NOT CLOSE, OBSTRUCT OR STORE MATERIAL OR EQUIPMENT IN STREETS, SIDEWALKS, ALLEYS OR PASSAGEWAYS WITHOUT A PERMIT IN ACCORDANCE WITH LOCAL ORDINANCES, REGULATIONS AND CODES.
C. INTERFERENCE: CONDUCT OPERATIONS WITH MINIMUM INTERFERENCE WITH ROADS, STREETS, DRIVEWAYS, ALLEYS, SIDEWALKS AND OTHER FACILITIES.
D. PNEUMATIC TOOLS: WORK WITH PNEUMATIC OR VIBRATORY TOOLS WILL BE PERMITTED ONLY IN A MANNER WHICH CAUSES NO RELATED DAMAGES.
E. REMOVAL: UNLESS OTHERWISE NOTED OR SPECIFIED TO BE RELOCATED OR STORED, ALL MATERIALS REMOVED BECOME THE PROPERTY OF THE CONTRACTOR AND ARE TO BE REMOVED COMPLETELY AWAY FROM THE SITE BY THE CONTRACTOR. DO NOT STORE OR PERMIT DEBRIS TO ACCUMULATE ON THE SITE.
F. TEMPORARY STRUCTURES: REMOVE ALL TEMPORARY STRUCTURES WHEN THEY ARE NO LONGER REQUIRED.
G. REPAIR: CLEAN UP, REPAIR OR REPLACE AT NO COST TO OWNER ALL PROPERTY DAMAGED BY REASON OF REQUIRED WORK. ALL PATCHWORK SHALL MATCH EXISTING AND BE PERFORMED IN A NEAT AND WORKMANLIKE MANNER BY CRAFTSMEN SKILLED IN THE TRADE INVOLVED. IN NEWLY GRADED AREAS TAKE EVERY PRECAUTION AND TEMPORARY MEASURE NECESSARY, TO PREVENT DAMAGE FROM EROSION OF FRESHLY GRADED AREA, WHERE ANY SETTLEMENT MAY OCCUR PRIOR TO ACCEPTANCE OF THE WORK, REPAIR AND RE-ESTABLISH GRADES TO THE REQUIRED ELEVATIONS AND SLOPES AT NO ADDITIONAL COST TO THE OWNER. THIS APPLIES TO DAMAGE TO THE NEWLY GRADED AREAS WITHIN THE CONSTRUCTION LIMITS AND DAMAGE TO ADJACENT PROPERTIES BY ERODED MATERIAL.

2.02 LOCATIONS AND ELEVATIONS

A. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SURVEYS, MEASUREMENTS AND LAYOUTS REQUIRED FOR PROPER EXECUTION OF THE WORK. LAY OUT LINES AND GRADES FROM EXISTING SURVEY CONTROL SYSTEM AND AS SHOWN ON DRAWINGS.

2.03 CLEARING AND GRUBBING

A. WITHIN LIMITS OF AREA DESIGNATED FOR GRADING AND SITE CONSTRUCTION WORK, REMOVE TREES, BRUSH, STUMPS, WOOD, DEBRIS AND OTHER DELETERIOUS MATERIALS NOT REQUIRED TO REMAIN AS PART OF FINISHED WORK.
B. REMOVE ALL GRASS, PLANTS, VEGETATION AND ORGANIC MATERIAL FROM SAME AREA.

2.04 STRIPPING

A. STRIP ALL TOPSOIL ORGANIC MATERIAL SURFACE LITTER, RUBBLE AND OVERBURDEN FOR ENTIRE DEPTH OF ROOT SYSTEM OF GRASS OR OTHER VEGETATION OVER THE LIMITS OF CONSTRUCTION.
B. STOCKPILE TOPSOIL ON SITE WHERE DIRECTED.

2.05 EXCAVATION

A. BEGIN EXCAVATION AFTER STRIPPING, CLEARING AND GRUBBING WHERE APPLICABLE, HAS BEEN COMPLETED.
B. EXCAVATE TO GRADES REQUIRED TO ACCOMMODATE THE PROPOSED CONSTRUCTION. DEWATER AS NEEDED.
C. REMOVE 'UNSATISFACTORY MATERIALS' ENCOUNTERED FROM THE BUILDING AREAS, AND OTHER NON-LANDSCAPED AREAS.
D. EXCAVATE IN SUCH A MANNER THAT QUICK AND EFFICIENT DRAINAGE OF STORMWATER WILL BE AFFECTED.
E. CLASSIFY EXCAVATED MATERIALS AND STOCKPILE SEPARATELY SUITABLE SOILS FOR USE AS BACKFILL MATERIALS. IF SUFFICIENT QUANTITIES OF EXCAVATED MATERIALS MEETING REQUIREMENTS FOR BACKFILL ARE NOT AVAILABLE ON SITE, PROVIDE MATERIALS MEETING THESE REQUIREMENTS.
F. STOCKPILE EXCAVATED MATERIAL SUITABLE FOR USE AS FILL AND BACKFILL.

2.06 FILLING, BACKFILLING AND COMPACTING

A. THE WORK CONSISTS OF COMPACTION OF EXISTING EARTH (EXCLUDE ROCK), SURFACES AFTER EXCAVATION, FILLING AND COMPACTION OF SAID AREA TO LEVELS REQUIRED WITH SUITABLE BACKFILL MATERIAL.
B. MATERIALS: 'SATISFACTORY FILL MATERIALS' AASHTO CLASSIFICATION A-3 OR BETTER SHALL BE USED IN FILLS AND BACKFILLS.
C. FILLING AND BACKFILLING: PLACE 'SATISFACTORY FILL MATERIAL' IN HORIZONTAL LAYERS NOT EXCEEDING 6 INCHES IN LOOSE DEPTH. COMPACT AS SPECIFIED HEREIN NO MATERIAL SHALL BE PLACED ON SURFACES THAT ARE MUDDY.

EARTHWORK (CON'T):

D. COMPACTION: COMPACTION SHALL BE WITH EQUIPMENT SUITED TO SOIL BEING COMPACTED. MOISTEN OR AERATE MATERIAL AS NECESSARY TO PROVIDE MOISTURE CONTENT THAT WILL READILY FACILITATE OBTAINING SPECIFIED COMPACTION WITH EQUIPMENT USED. COMPACT EACH LAYER TO NOT LESS THAN PERCENTAGE OF MAXIMUM DENSITY SPECIFIED BELOW DETERMINED IN ACCORDANCE WITH AASHTO T-180. INSURE THAT THE COMPACTION OF PREVIOUSLY PREPARED FILL AREAS HAS BEEN MAINTAINED PRIOR TO PLACING NEW LAYERS.
E. RECONDITIONING OF SUBGRADE: WHERE APPROVED COMPACTED SUBGRADES ARE DISTURBED BY THE CONTRACTOR'S SUBSEQUENT OPERATIONS OF ADVERSE WEATHER SUBGRADE SHALL BE SCARIFIED AND COMPACTED AS SPECIFIED HEREIN BEFORE TO REQUIRED DENSITY PRIOR TO FURTHER CONSTRUCTION THEREON. RE-COMPACTION OVER UNDERGROUND UTILITIES SHALL BE BY POWER-DRIVEN HAND TAMPERS.
F. COMPACTION REQUIREMENTS
   a. FILL UNDER LAWNS AND PLANTED: 95%
   b. BELOW SLABS ON GRADE AND CONCRETE WALKS: 98%
   c. UNDER PAVING PARKING AREAS: 98%

2.07 TESTING

A. THE CONTRACTOR WILL PROVIDE THE SERVICES OF A TESTING LABORATORY TO PERFORM SPECIFIED TESTS, INSPECTIONS, INSTRUMENTATION AND INSPECTION OF WORK.
B. TESTS OF MATERIALS SHALL BE AS FOLLOWS:
   a. SOIL CLASSIFICATION: ONE TEST FROM EACH TYPE OF MATERIAL ENCOUNTERED AND / OR PROPOSED TO BE USED.
   b. LABORATORY TESTS FOR MOISTURE-CONTEST AND DENSITY ACCORDING TO AASHTO T-180: ONE TEST FOR EACH MATERIAL ENCOUNTERED AND / OR PROPOSED TO BE USED.
   c. FIELD TESTS FOR MOISTURE CONTEST AND DENSITY: ONE TEST PER LAYER OF FILL PER 5,000 SQUARE FEET OF AREA.

SUPPLEMENTAL SPECIFICATIONS:

1. GENERAL

THE CONTRACTOR SHALL BECOME FAMILIAR WITH AND ADHERE TO THE SPECIFICATIONS AND STANDARDS OF THE UTILITY COMPANIES WHICH ARE SERVING THE PROJECT SITE. THE CONTRACTOR SHALL BECOME FAMILIAR WITH AND COMPLY WITH ALL SITE DEVELOPMENT STANDARDS AND CODES OF THE REGULATORY AGENCIES ASSOCIATED WITH THIS PROJECT. THE LATEST VERSION OF THE FLORIDA DEPARTMENT OF TRANSPORTATION ROAD AND BRIDGE CONSTRUCTION SPECIFICATIONS AND THE LATEST FLORIDA DEPARTMENT OF TRANSPORTATION ROADWAY AND TRAFFIC DESIGN STANDARDS SHALL BE INCLUDED WITHIN THE PROJECT SPECIFICATIONS. UNLESS OTHERWISE NOTED, EITHER ON THE PLANS OR WITHIN THE SPECIFICATIONS, THE APPLICABLE SECTIONS OF THE FLORIDA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS AND THE LATEST FLORIDA DEPARTMENT OF TRANSPORTATION ROADWAY AND TRAFFIC DESIGN STANDARDS SHALL APPLY INCLUDING REFERENCES THEREIN. THE GENERAL DESCRIPTION OF THE NATURE OF THE WORK SHALL BE SUFFICIENT CORRELATION TO THE FLORIDA DEPARTMENT OF TRANSPORTATION SPECIFICATIONS. EACH ITEM DESCRIBED IN A CONTRACTOR'S EXPENSE.

2. POTABLE WATER DISTRIBUTION/ WASTEWATER COLLECTION INSTALLATION

UNLESS OTHERWISE NOTED ON THE PLANS, THE STANDARDS AND SPECIFICATIONS OF THE ASSOCIATED UTILITY COMPANY SERVING THE PROJECT SITE SHALL BE ADHERED TO FOR ALL MATERIALS, INSTALLATION, TESTING, AND CERTIFICATION ACTIVITIES FOR ALL PRIME STATIONS, MAIN LINES, SERVICES, AND APPURTENANCES. IF STANDARDS AND SPECIFICATIONS ARE NOT AVAILABLE, THE CONTRACTOR SHALL CONFORM WITH THE LATEST STANDARDS AND SPECIFICATIONS ADOPTED BY THE UTILITIES, LOCAL GOVERNMENTAL REGULATIONS, OR THE MANUFACTURERS RECOMMENDED INSTALLATION PROCEDURES, WHICHEVER IS SPECIFICALLY THE MOST RESTRICTIVE. A COPY OF THE UTILITIES SPECIFICATIONS CAN BE REVIEWED AT THE OFFICE OF ATWELL, L.L.C.

3. STORMWATER PIPE INSTALLATION AND MISCELLANEOUS EXCAVATIONS

UNLESS OTHERWISE NOTED ON THE PLANS OR SPECIFICATIONS, THE CONTRACTOR SHALL PERFORM THE EXCAVATION, BEDDING, JOINTS, AND BACKFILLING OPERATIONS IN ACCORDANCE WITH THE APPLICABLE WATER/ WASTEWATER INSTALLATION SPECIFICATIONS, LOCAL GOVERNMENTAL REGULATIONS OR STANDARDS, F.D.O.T. STANDARDS AND SPECIFICATIONS OR MANUFACTURER'S RECOMMENDED INSTALLATION PROCEDURES, WHICHEVER IS SPECIFICALLY THE MOST RESTRICTIVE.

4. UNSUITABLE MATERIALS

IF UNSUITABLE MATERIAL IS ENCOUNTERED WITHIN THE ROADWAY AREA AND / OR UTILITY AREAS IT SHALL BE REMOVED TO A DEPTH OF THREE (3) FEET BELOW THE SUB-BASE OR TRENCH BOTTOM AND SHALL BE BACKFILLED WITH THE AASHTO A-3 MATERIAL OR BETTER WITH PLACEMENT AND COMPACTION METHODS IN ACCORDANCE WITH THE LATEST EDITION OF THE FLORIDA DEPARTMENT OF TRANSPORTATION'S STANDARD SPECIFICATIONS OR AS OTHERWISE NOTED ON THE PLANS. UNSUITABLE MATERIALS SHALL BE REMOVED FROM SITE, UNLESS THE ENGINEER APPROVES USE WITHIN LANDSCAPED AREAS.

DEWATERING:

1.01 GENERAL

A. DEWATERING CONSISTS OF PERFORMING ALL WORK NECESSARY TO REMOVE SURFACE WATER AND / OR CONTROL THE GROUND WATER LEVELS AND HYDROSTATIC PRESSURES IN ORDER TO PERMIT ALL EXCAVATION AND CONSTRUCTION UNDER THIS CONTRACT TO BE PERFORMED IN THE DRY.
B. WORK OF THIS SECTION INCLUDES INSTALLATION, OPERATIONS, MAINTENANCE, SUPERVISION, SUPPLY, DISMANTLING, AND REMOVAL FROM THE SITE OF THE DEWATERING EQUIPMENT.
C. THE CONTRACTOR MUST FAMILIARIZE HIMSELF WITH THE POTENTIAL FOR EXCESSIVE RAINFALL, THE GROUND CONDITIONS, AND THE GROUND WATER CONDITIONS, GROUND WATER ELEVATION CAN FLUCTUATE. IT IS ANTICIPATED THAT ANY EXCAVATIONS MAY ENCOUNTER THE GROUND WATER TABLE.
D. DRAINAGE OF THE SITE: AT ALL TIMES THE CONTRACTOR SHALL MAINTAIN AND OPERATE ADEQUATE SURFACE AND SUBSURFACE DRAINAGE METHODS IN ORDER TO KEEP THE CONSTRUCTION SITE DRY AND IN SUCH CONDITION THAT PLACEMENT AND COMPACTION OF FILL MAY PROCEED UNHINDERED BY SATURATION OF THE AREA DURING CONSTRUCTION, THE SURFACE OF THE BACKFILL AREA SHALL BE LEFT IN SUCH CONDITION THAT PRECIPITATION AND / OR SURFACE WATER WILL RUN OFF WITHOUT PONDING.

1.02 METHOD

A. THE CONTROL OF ALL SURFACE AND SUBSURFACE WATER IS PART OF THE DEWATERING REQUIREMENTS, MAINTAIN ADEQUATE CONTROL SO THAT THE STABILITY OF EXCAVATED AND CONSTRUCTION SLOPES IS NOT ADVERSELY AFFECTED BY WATER, THAT EROSION IS CONTROLLED, THAT THE FLOODING OR DAMAGE TO STRUCTURES OR DAMAGE TO STRUCTURES DOES NOT OCCUR, DRAIN SURFACE WATER AWAY FROM THE EXCAVATION.
B. DISPOSE OF ALL WATER REMOVED FROM THE EXCAVATION IN A MANNER THAT WILL NOT ENDANGER PUBLIC HEALTH, PROPERTY, OR PORTIONS OF THE WORK UNDER CONSTRUCTION OR COMPLETED. DISPOSE OF WATER IN A MANNER THAT WILL CAUSE NO INCONVENIENCE WHATSOEVER TO THE OWNER OR TO OTHERS ENGAGED IN WORK AT THE SITE.
C. DISPOSE OF WATER RESULTING FROM DEWATERING OPERATIONS IN ACCORDANCE WITH CITY, COUNTY, STATE AND FEDERAL REGULATIONS.
D. CONDUCT OPERATIONS SO THAT STORMWATER RUNOFF, SEDIMENT IS NOT DISCHARGED TO THE ADJACENT WATER BODIES, SEWERS, STREETS AND ADJACENT PROPERTIES.
E. DEWATERING SYSTEM SHALL BE SO DESIGNED AS TO PREVENT REMOVAL OF SOIL FINES FROM THE SITE DURING THE DEWATERING OPERATION.

PORTLAND CEMENT CONCRETE PAVING:

1.01 QUALITY ASSURANCE

A. COMPLY WITH ACI STANDARDS RECOMMENDED PRACTICES FOR CONSTRUCTION OF CONCRETE PAVEMENTS AND CONCRETE BASES (ACI 325.9R, LATEST EDITION)

1.02 REFERENCE STANDARDS

A. THE FOLLOWING REFERENCE STANDARDS OF THE ISSUES LISTED BELOW BUT REFERRED TO THEREAFTER BY BASIC DESIGNATION ONLY, FORM A PART OF THIS SPECIFICATION TO THE EXTENT INDICATED BY THE REFERENCES THERETO. TESTS SHALL BE PERFORMED IN ACCORDANCE WITH HEREINAFTER SPECIFIED STANDARDS.
   a. AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM)
   b. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) STANDARD.
   c. FLORIDA DEPARTMENT OF TRANSPORTATION (FDOT) STANDARDS AND SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION--SECTION

LAND USE (ON-SITE):

USE:		
PAVEMENT/ SIDEWALK/ CONCRETE	5.15 AC	8.3%
WETLAND PRESERVE	2.23 AC	3.6%
UPLAND	1.37 AC	2.2%
DRY RETENTION (AT BOTTOM)	1.62 AC	2.6%
LAKES (AT CONTROL)	9.21 AC	14.9%
PERVIOUS	42.20 AC	68.4%
TOTAL	61.78 AC	100.0%

NOTES:
1. PARCEL 1 MAXIMUM 90% IMPERVIOUS.
2. PARCELS 2 THROUGH 17 MAXIMUM 80% IMPERVIOUS.
3. OFF-SITE PRESERVE 5.83 AC.

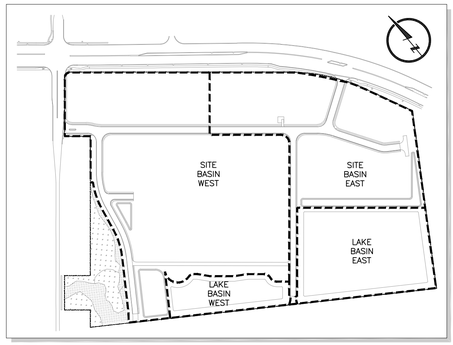
SURFACE WATER MANAGEMENT SUMMARY:

	SITE BASIN WEST (FT-NAVD)	SITE BASIN EAST (FT-NAVD)	LAKE BASIN WEST (FT-NAVD)	LAKE BASIN EAST (FT-NAVD)
SFWMD 5 YEAR, 1 DAY	30.36	30.23	30.23	30.22
SFWMD 10 YEAR, 1 DAY	30.67	30.64	30.64	30.64
SFWMD 25 YEAR, 3 DAY	31.60	31.59	31.59	31.59
SFWMD 25 YEAR, 3 DAY BACK TO BACK	31.65	31.64	31.64	31.64
SFWMD 100 YEAR, ZERO DISCHARGE	32.30	32.30	32.30	32.30
FDOT 100 YEAR CRITICAL	31.95	31.95	31.95	31.95
MINIMUM ROAD	30.67	30.67	30.67	30.67
SFWMD MINIMUM PERIMETER BERM	31.7	31.7	31.7	31.7
FDOT MINIMUM PERIMETER BERM – GREEN SPACE BERM (1 FT ABOVE FDOT 100 YEAR CRITICAL)	33.0	33.0	33.0	33.0
FDOT MINIMUM PERIMITER BERM – HARDENED STRUCTURE (6" ABOVE 100 YEAR FDOT 100 YEAR CRITICAL)	32.5	32.5	32.5	32.5
MINIMUM FINISHED FLOOR ELEVATION	32.30	32.30	32.30	32.30

FEMA: ZONE X PER PANEL 12071C0475F

PROPERTY DATA:

EXISTING ZONING DESIGNATION:	Z-23-040, BLACKSTONE CORPORATE PARK MPD, COMPANION ADD2024-00219
EXISTING LAND USE:	VACANT
PROPOSED USE:	COMMERCIAL
FLUM DESIGNATION:	URBAN COMMUNITY, INDUSTRIAL DEVELOPMENT, AND WETLANDS
PLANNING COMMUNITY:	LEHIGH ACRES, 17
LEE PLAN CONSISTENCY:	THE PROPOSED IMPROVEMENTS AND USES ARE CONSISTENT WITH THE LEE PLAN, Z-23-040, ADD2024-00219, ADD2025-00199 AND THE LDC.
DEVELOPMENT SCHEDULE: (PER Z-23-040 AND ADD2024-00219)	MAXIMUM 500,000 SF COMMERCIAL RETAIL USE



BASIN MAP
N.T.S.

SITE DEVELOPMENT REGULATIONS: (LEE COUNTY RESOLUTION Z-23-040)

MINIMUM LOT SIZE:	10,000 SQUARE FEET
MINIMUM LOT WIDTH:	100 FEET
MINIMUM LOT DEPTH:	100 FEET
DEVELOPMENT SETBACKS:	
STREET:	20 FEET
SIDE:	10 FEET
REAR:	10 FEET
WATERBODY:	25 FEET
PRESERVE:	20 FEET (30 FEET FOR FIRE DEPENDENT INDIGENOUS PLANT COMMUNITIES)
BUILDING SEPARATION:	ONE HALF THE SUM OF THE HEIGHTS OF BOTH BUILDINGS, OR 20 FEET WHICHEVER IS GREATER.
MINIMUM DEVELOPMENT PERIMETER:	25 FEET (INDUSTRIAL AREAS) 15 FEET (COMMERCIAL AREAS)
MAXIMUM BUILDING HEIGHT:	65 FEET
MINIMUM OPEN SPACE:	30%

NRCS SOIL DATA:

MAP UNIT SYMBOL	SOIL TYPE	HYDRAULIC RATING	AREA	PERCENT
14	VALKARIA FINE SAND	A/D	8.00	13.0%
27 & 132	POMPANO FINE SAND	A/D	6.51	10.5%
28	IMMOKALEE SAND	B/D	47.16	76.3%
129	PINEDA FINE SAND	A/D	0.10	0.2%

LANDSCAPE BUFFERS: (LEE COUNTY ZONING ORD. Z-23-030, MCP)

DIRECTION	BUFFER REQUIRED	BUFFER PROVIDED
NORTHEAST (ABUTTING SR82)	15' TYPE D	15' TYPE D
NORTHWEST (BLACKSTONE DRIVE)	15' TYPE D	15' TYPE D
SOUTHWEST/ WEST (ABUTTING PARCELS 20-45-27-63-00239.0010, 20-45-27-63-00239.0020, AND PORTION OF 20-45-27-63-00239.0030)	5' TYPE A	5' TYPE A
WEST (ABUTTING PRESERVE)	NO BUFFER REQUIRED	NO BUFFER REQUIRED
EAST (ABUTTING LAMSID CANAL)	NO BUFFER REQUIRED	NO BUFFER REQUIRED
SOUTH	NO BUFFER REQUIRED	NO BUFFER REQUIRED
INTERNAL BUFFERS	NO BUFFER REQUIRED PER LDC SECTION 33-1405 FOR COMMERCIAL PROJECTS PART OF MIXED USE DEVELOPMENT BETWEEN INTERNAL USES	NO BUFFER REQUIRED

IRRIGATION NOTE:

IRRIGATION WILL BE PROVIDED BY WELL.

UTILITY NOTES:

1. WATER – 6-INCH WATER LINE EXTENDED TO 5 FEET WITHIN THE PROPERTY AND CAPPED. BUYER TO UTILIZE STUBOUT FOR POTABLE AND FIRE NEEDS. BUYER RESPONSIBLE FOR THE INSTALLATION OF WATER METER AND BACKFLOW PREVENTORS.
2. SANITARY SEWER – 6-INCH SANITARY SEWER LINE EXTEND TO 5 FEET WITHIN THE PROPERTY AND CAPPED.

OPEN SPACE AND INDIGENOUS PRESERVE CALCULATIONS (PER ADD2024-00219):

REQUIRED OPEN SPACE:
COMMERCIAL USE AREA A: 61.78 AC x 30% = 18.53 AC
REQUIRED INDIGENOUS (50% OF REQUIRED OPEN SPACE):
COMMERCIAL USE AREA A: 18.53 AC x 50% = 9.27 AC

PROVIDED INDIGENOUS PRESERVE AREAS:
9.27 AC (3.44 AC ON SITE AND 5.83 AC OFF SITE, SEE PRESERVE PLAN)

ON SITE:		OFFSITE:	
AREA 1:	0.36 AC	AREA 6:	0.51 AC
AREA 2:	1.24 AC	AREA 7:	3.96 AC
AREA 3:	0.53 AC	AREA 8:	1.08 AC
AREA 4:	0.91 AC	PORTION OF AREA 9:	0.17 AC
AREA 5:	0.40 AC	AREA 10:	0.03 AC
TOTAL:	3.44 AC	AREA 11:	0.08 AC
		TOTAL:	5.83 AC

PROVIDED OPEN SPACE:

LAKE (AT CONTROL ELEVATION):	9.21 AC
DRY RETENTION (AT RETENTION BOTTOM):	1.62 AC
COMMERCIAL PARCELS 1-17 @ 10% EA:	3.59 AC
INDIGENOUS PRESERVE AREAS:	9.27 AC
TOTAL:	23.69 AC

PLANNING DISTRICT 17 COMMERCIAL ALLOCATION:

ALLOCATION	EXISTING DEVELOPMENT	REMAINING ACRES
900	393	507

BLACKSTONE CORPORATE PARK MPD DEVELOPMENT SUMMARY:

PARCEL IDENTIFIER	PROJECT NAME	DO NUMBER	DO STATUS	COMMERCIAL DEVELOPMENT AREA
LOT 1	BIG BOX RETAIL	DOS2025-00077	IN REVIEW	189,434 SF
LOT 12	FL FUEL STATION #3915	DOS2025-00100	IN REVIEW	1,618 SF
TOTAL TO DATE				191,052 SF

GENERAL NOTES:

1. IRRIGATION TO BE PROVIDED BY WELL.
2. BLACKSTONE CORPORATE PARK PROPERTY OWNERS ASSOCIATION SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF THE WATER MANAGEMENT SYSTEM, OPEN SPACE, COMMON AREAS, AND PRIVATE STREETS.
3. REFERENCE FDOT ACCESS AND DRAINAGE EXCEPTION PERMIT AND ASSOCIATED PLANS FOR WORK IN THE FDOT RIGHT OF WAY.
4. FOR SIDEWALK IN LEE COUNTY ROW (LEEDOT), SECTION 10-296 TABLE 3 GOVERNS FOR SIDEWALK CONSTRUCTION. SEE DETAIL SHEET FOR FURTHER INFORMATION.
5. SIDEWALK AND CURB REPLACEMENT LIMITS MUST EXTEND TO THE NEAREST CONSTRUCTION JOINT TO PREVENT SMALL SECTIONS OF EXISTING PANELS/CURB FROM BEING LEFT IN PLACE.
6. FOR PAVEMENT WITHIN THE LEE COUNTY ROW (LEEDOT), SECTION 10-296 TABLE 3 GOVERNS.
7. AFTER THE LOTS ARE FILLED AND GRADED, THE SITE IS TO BE STABILIZED WITH SEED AND MULCH.
8. THROAT INLETS TO BE FDOT TYPE 9 CURB INLET TOPS WITH HEAVY DUTY GRATE PER FDOT INDEX NO.425-024. THROAT INLET STRUCTURES TO UTILIZE USF 5130 FRAME AND HOOD WITH 6168 GRATE.
9. ALL ACCESSIBLE PATHWAYS, ACCESS AISLES, ACCESSIBLE MANEUVERING SPACES AND PARKING SPACES TO CONFORM TO THE FLORIDA ACCESSIBILITY CODE. CONTRACTOR TO VERIFY ACCESSIBILITY AND COORDINATE WITH OWNER AND ENGINEER OF ANY DISCREPANCIES.
10. MINIMUM 3' LANDING REQUIRED AT TOP OF CURB RAMP WITH LESS THAN 2% SLOPE IN ALL DIRECTIONS PER FLORIDA ACCESSIBILITY CODE 406 (TYPICAL ALL CURB RAMPS).
11. RAMP SLOPE TO BE LESS THAN 5% RUNNING SLOPE AND LESS THAN 2% CROSS SLOPE PER FLORIDA ACCESSIBILITY CODE 406.

FGUA GENERAL NOTES:

1. ALL CONSTRUCTION SHALL MEET LEE COUNTY STANDARDS AND SPECIFICATIONS AS APPLICABLE.
2. AN FGUA REPRESENTATIVE IS TO BE PRESENT DURING CONNECTIONS TO EXISTING FACILITIES WITH 72-HOUR NOTIFICATION.
3. ALL UTILITIES WITHIN THE PROPERTY LINES WILL BE OWNED AND MAINTAINED BY THE PROPERTY OWNER.
4. ANY UTILITIES (PIPES, VALVES, AND ASSOCIATED HARDWARE) TO BE CONVEYED TO FGUA WILL BE LOCATED WITHIN AN EXISTING PUBLIC UTILITY EASEMENT, RIGHT-OF-WAY (ROW) AND/OR A DEDICATED FGUA UTILITY EASEMENT.
5. ANY REAL PROPERTY (LIFT STATIONS, PUMP HOUSES, WELLS, ETC.) WILL NOT BE LOCATED WITHIN THE ROW AND WILL BE CONVEYED TO FGUA VIA A SPECIFIC WARRANTY DEED.
6. LOW PRESSURE AIR TEST, AND VIDEO INSPECTION ARE REQUIRED FOR THE GRAVITY COLLECTION SYSTEM – WATER SYSTEMS ARE TESTED TO A MINIMUM OF 150PSI FOR TWO-HOUR DURATION AS PER AWWA C 600 STANDARDS. FIRE SYSTEMS ARE NOT THE FGUA RESPONSIBILITY.
7. INSTALL 10-GAUGE TRACER WIRE ON ALL MAIN PIPELINES. WIRE TO EXTEND INTO EACH VALVE BOX WITH SUFFICIENT EXCESS TO BRING WIRE THREE FEET ABOVE FINISH GRADE.
8. ALL WATER AND/OR WASTEWATER LINES PASSING UNDER PAVED AREAS WILL BE CONSTRUCTED WITH A CASING AND SHALL APPLY WITH LOCAL OR STATE DOT STANDARDS.
9. A REDUCED PRESSURE ZONE (RPZ), BACKFLOW PREVENTION ASSEMBLY (BFP) IS REQUIRED. ACCEPTABLE MODELS FEBCO 825Y, WATTS 909, WILKINS 975XL, OR CONBRACO 400.
10. FULL CIRCLE STAINLESS STEEL TAPPING SADDLE SLEEVE WITH RESILIENT SEATED TAPPING VALVE FOR PROPOSED WATER AND FORCE MAIN CONNECTIONS TO BE USED.
11. WATER TAPPING SADDLES TO BE DOUBLE STRAP BRASS OR DUCTILE IRON OR EPOXY COATED STEEL WITH STAINLESS STEEL HARDWARE. TIE STRAPS AND BOLTS SHALL BE CORROSION RESISTANT ALLOY STEEL.
12. ALL FIRE FLOW LINES WILL RUN SEPARATE FROM THE DOMESTIC WATER MAINS AND HAVE A FIRE DETECTION METER AND A FIRE LINE BACKFLOW PREVENTION ASSEMBLY (BFP) ASSEMBLY SHALL INCLUDE A LOW FLOW BY-PASS METER W/ RPZ.
13. MANHOLE FRAMES & COVERS, AND CO CAPS ARE TO BE WATER TIGHT, IF NEEDED EXISTING MANHOLE TOPS ARE TO BE ADJUSTED TO PROPOSED GRADE.
14. INFLOW PROTECTION DISHES ARE REQUIRED FOR ALL MANHOLES; STAINLESS STEEL IN PAVED AREAS, ABS IN UNPAVED AREAS.
15. A CLEANOUT SHALL BE PROVIDED AT THE ROW/PL FOR ALL SERVICE CONNECTIONS
16. THE FGUA SHALL OWN AND MAINTAIN ALL WATER SERVICE LINES UP TO THE POINT OF CONNECTION. All WATER MAINS SHALL BE A MINIMUM OF FOUR INCHES (4") IN DIAMETER AND ALL SUCH WATER MAINS SHALL BE CONSTRUCTED IN DEDICATED RIGHTS-OF-WAY OR APPROPRIATE UTILITY EASEMENTS DEDICATED TO THE FGUA.
17. ALL SEWER FORCE MAIN LINES AND GRAVITY SERVICE LINES ACCEPTED MUST BE AT LEAST FOUR INCHES (4") IN DIAMETER AND EIGHT INCHES (8"), RESPECTIVELY, EXCEPT LATERAL SEWER SERVICE LINES WHICH SHALL BE NO LESS THAN SIX INCHES (6") IN DIAMETER AND ACCEPTED BY THE FGUA TO A CLEANOUT LOCATED ON THE EDGE OF THE RIGHT-OF-WAY OR AN EASEMENT.
18. A BALL CHECK VALVE FOLLOWED BY A PLUG VALVE WILL BE INSTALLED ON FORCE MAINS LOCATED IN PUE OR R/W AT THE PROPERTY LINE DISCHARGING FROM PRIVATELY OWNED LIFT STATIONS CONNECTING INTO THE UTILITY MAINS
19. CONNECTION TO EXISTING MANHOLES AND/OR LIFT STATIONS SHALL BE BY CORE BORE WITH KOR-N-SEAL CONNECTORS.
20. ALL CONNECTIONS WILL BE MADE AS HOT TAPS. ANY EXCEPTIONS TO THIS WILL REQUIRE FGUA APPROVAL. ALL HOT TAP CONNECTIONS SHALL BE PERFORMED BY A LICENSED, UNDERGROUND CONTRACTOR, WITH ADVANCE NOTIFICATION (72 HOURS) TO THE FGUA INSPECTOR.
21. BACKFLOW PREVENTION DEVICES SHALL BE PRIVATELY OWNED AND MAINTAINED. BFPDS SHOULD BE LOCATED OUTSIDE OF ANY FGUA EASEMENT OR ROW/PUBLIC EASEMENT. IT IS THE SOLE RESPONSIBILITY OF THE PROPERTY OWNER TO MAINTAIN THE DEVICE AND PROVIDE ANNUAL TESTING CERTIFICATIONS TO THE FGUA.

Lehigh Acres Fire Control and Rescue District
Life Safety Division

Office Address:
636 Thomas Sherwin Ave S
Lehigh Acres, FL
Phone: 239-303-5335
Email: LifeSafety@Lehighfd.com

Mailing Address:
11 Homestead Rd S
Lehigh Acres, FL 33936
Phone: 239-303-5300
Fax: 239-369-2436
www.Lehighfd.com

Fire Flow Test Report

Fire Inspector: J.Nickell Date: 10/25/2024 Time: 14:16:00 PM

Requested by Firm: Respec Name: Dan Mumford, E.I
Address: 14122 Jackson St Ste 1 Email Address:
City, State, Zip Code: Fort Myers, FL 33901
Phone: 239-418-0691 Project Name: Blackstone Corprate Park

Static / Residual Pressure Hydrant: LEH-00792 - 336 Black Stone Dr
Flow Hydrant: LEH-00796 - 837 Lockman Dr



Static Pressure	55 PSI	Test Duration:	6 Minutes
Residual Pressure	40 PSI	Estimated Water Used:	6956 Gallons

	Outlet No. 1		Outlet No. 2	
Inside Dlameter of Hydrant Outlet =	2.5	Inches	2.5	Inches
Discharge Coefficient of Hydrant Outlet =	0.9		0.9	
Pitot or Pressure Gauge Reading =	35	PSI		PSI

Min Fire Flow Pressure = 20 PSI Test Flow = 992.68 GPM 0.00 GPM
Combined Flow of Outlets No. 1 & 2 = 992.68 GPM

Available Fire Flow @ 20 PSI = 1,568.62 GPM

Coefficients: 0.7 - Squared Butt / 0.8 - Smooth Squared Butt / 0.9 - Smooth and Round Butt Page 1 of 2

FIRE FLOW

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PROJECT NOTES

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ
2	10/17/2025	PER COUNTY COMMENTS	VZ

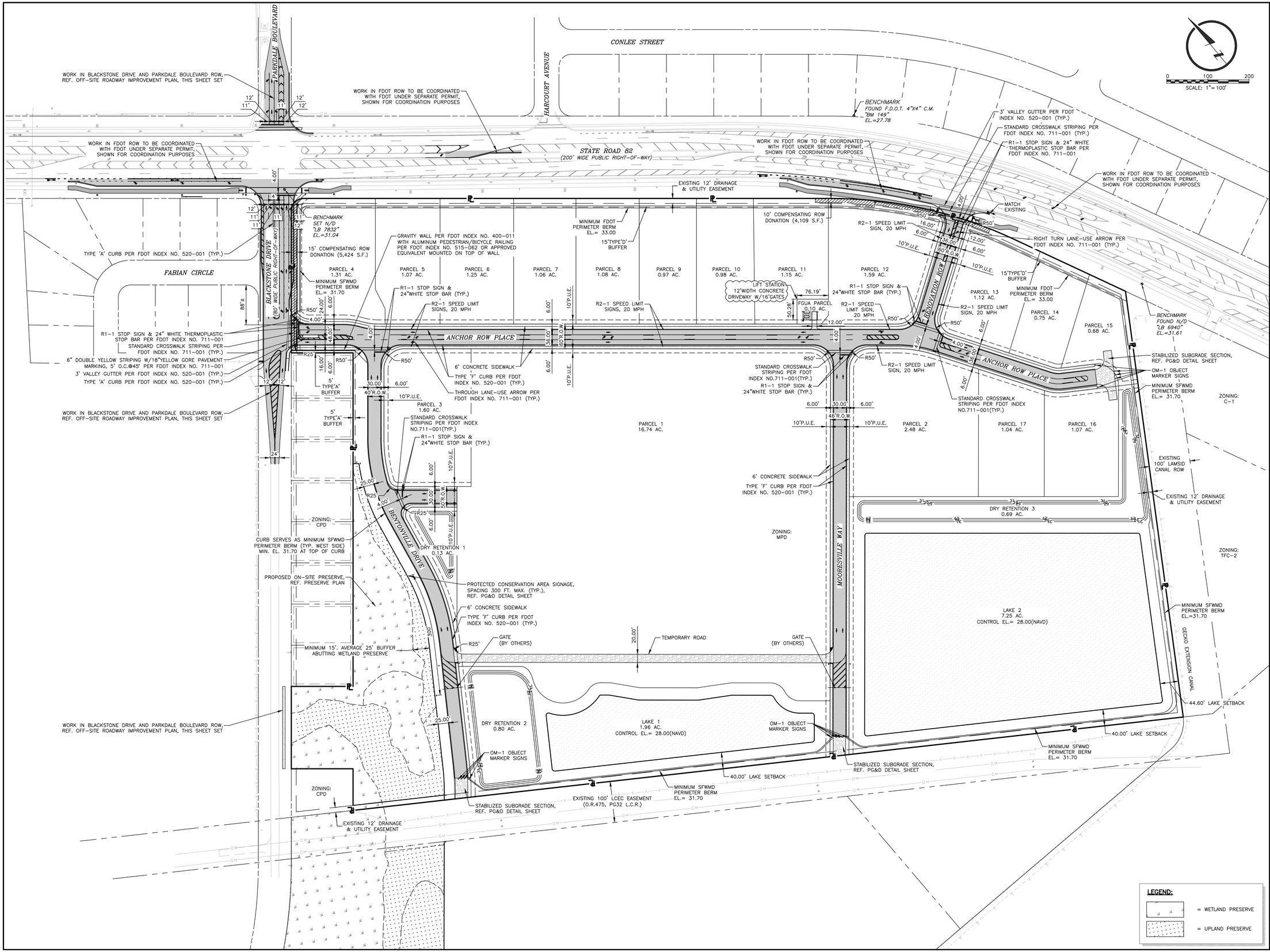
811 ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN, OR THAT THE LOCATION AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING UTILITIES PRIOR TO CONSTRUCTION.
Know what's below. Call before you dig.

RACHEL J. TRACEY, P.E.
P.E. LIC. #65322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 05

n:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\Engineering\dev plans\05 PROJECT NOTES.dwg, 2/9/2026 11:45:08 AM, VIVIAN ZALEMSKI, ATWELL, LLC



LEGEND:

= WETLAND PRESERVE

= UPLAND PRESERVE

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.936.5490

MASTER SITE PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:

CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ
2	10/17/2025	PER COUNTY COMMENTS	VZ
3	02/06/2026	PER COUNTY COMMENTS	VZ

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN ACCORDING TO INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND CONDITION OF THOSE SHOWN ARE CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

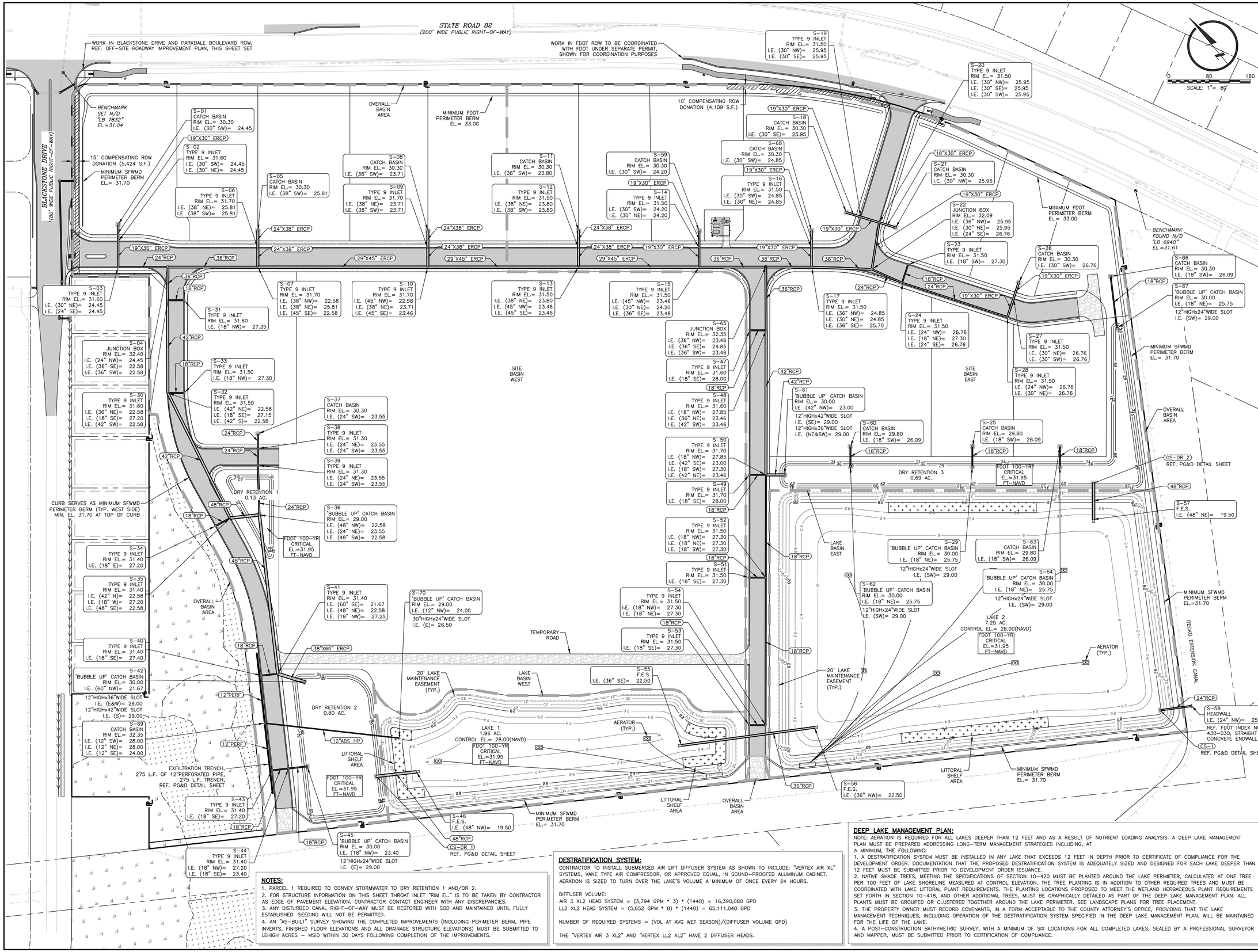
RACHEL J. TRACEY, P.E.

P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 06

K:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\06 MASTER SITE PLAN.dwg, 2/9/2026 11:45:27 AM, VIVIAN ZALESAKOVA, ATWELL, LLC



NOTES:

1. PARCEL 1 REQUIRED TO CONVEY STORMWATER TO DRY RETENTION 1 AND/OR 2.
2. FOR STRUCTURE INFORMATION ON THIS SHEET THROAT INLET "RIM EL." IS TO BE TAKEN BY CONTRACTOR AS EDGE OF PAVEMENT ELEVATION. CONTRACTOR CONTACT ENGINEER WITH ANY DISCREPANCIES.
3. ANY DISTURBED CANAL RIGHT-OF-WAY MUST BE RESTORED WITH SOD AND MAINTAINED UNTIL FULLY ESTABLISHED. SEEDING WILL NOT BE PERMITTED.
4. AN "AS-BUILT" SURVEY SHOWING THE COMPLETED IMPROVEMENTS (INCLUDING PERIMETER BERM, PIPE INVERTS, FINISHED FLOOR ELEVATIONS AND ALL DRAINAGE STRUCTURE ELEVATIONS) MUST BE SUBMITTED TO LEHIGH ACRES – MSID WITHIN 30 DAYS FOLLOWING COMPLETION OF THE IMPROVEMENTS.

DESTRATIFICATION SYSTEM:

CONTRACTOR TO INSTALL SUBMERGED AIR LIFT DIFFUSER SYSTEM AS SHOWN TO INCLUDE: "VERTEX AIR XL" SYSTEMS, VANE TYPE AIR COMPRESSOR, OR APPROVED EQUAL, IN SOUND-PROOFED ALUMINUM CABINET. AERATION IS SIZED TO TURN OVER THE LAKE'S VOLUME A MINIMUM OF ONCE EVERY 24 HOURS.

DIFFUSER VOLUME:
AIR 3 XL2 HEAD SYSTEM = (3,794 GPM * 3) * (1440) = 16,390,080 GPD
LL2 XL2 HEAD SYSTEM = (5,652 GPM * 8) * (1440) = 65,111,040 GPD

NUMBER OF REQUIRED SYSTEMS = (VOL AT AVG WET SEASON)/(DIFFUSER VOLUME GPD)

THE "VERTEX AIR 3 XL2" AND "VERTEX LL2 XL2" HAVE 2 DIFFUSER HEADS.

DEEP LAKE MANAGEMENT PLAN:

NOTE: AERATION IS REQUIRED FOR ALL LAKES DEEPER THAN 12 FEET AND AS A RESULT OF NUTRIENT LOADING ANALYSIS. A DEEP LAKE MANAGEMENT PLAN MUST BE PREPARED ADDRESSING LONG-TERM MANAGEMENT STRATEGIES INCLUDING, AT A MINIMUM, THE FOLLOWING:

1. A DESTRATIFICATION SYSTEM MUST BE INSTALLED IN ANY LAKE THAT EXCEEDS 12 FEET IN DEPTH PRIOR TO CERTIFICATE OF COMPLIANCE FOR THE DEVELOPMENT ORDER. DOCUMENTATION THAT THE PROPOSED DESTRATIFICATION SYSTEM IS ADEQUATELY SIZED AND DESIGNED FOR EACH LAKE DEEPER THAN 12 FEET MUST BE SUBMITTED PRIOR TO DEVELOPMENT ORDER ISSUANCE.
2. NATIVE SHADE TREES, MEETING THE SPECIFICATIONS OF SECTION 10-420 MUST BE PLANTED AROUND THE LAKE PERIMETER, CALCULATED AT ONE TREE PER 100 FEET OF LAKE SHORELINE MEASURED AT CONTROL ELEVATION. THE TREE PLANTING IS IN ADDITION TO OTHER REQUIRED TREES AND MUST BE COORDINATED WITH LAKE LITTORAL PLANT REQUIREMENTS. THE PLANTING LOCATIONS PROPOSED TO MEET THE WETLAND HERBACEOUS PLANT REQUIREMENTS SET FORTH IN SECTION 10-418, AND OTHER ADDITIONAL TREES, MUST BE GRAPHICALLY DETAILED AS PART OF THE DEEP LAKE MANAGEMENT PLAN. ALL PLANTS MUST BE GROUPED OR CLUSTERED TOGETHER AROUND THE LAKE PERIMETER. SEE LANDSCAPE PLANS FOR TREE PLACEMENT.
3. THE PROPERTY OWNER MUST RECORD COVENANTS, IN A FORM ACCEPTABLE TO THE COUNTY ATTORNEY'S OFFICE, PROVIDING THAT THE LAKE MANAGEMENT TECHNIQUES, INCLUDING OPERATION OF THE DESTRATIFICATION SYSTEM SPECIFIED IN THE DEEP LAKE MANAGEMENT PLAN, WILL BE MAINTAINED FOR THE LIFE OF THE LAKE.
4. A POST-CONSTRUCTION BATHYMETRIC SURVEY, WITH A MINIMUM OF SIX LOCATIONS FOR ALL COMPLETED LAKES, SEALED BY A PROFESSIONAL SURVEYOR AND MAPPER, MUST BE SUBMITTED PRIOR TO CERTIFICATION OF COMPLIANCE.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

MASTER DRAINAGE PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

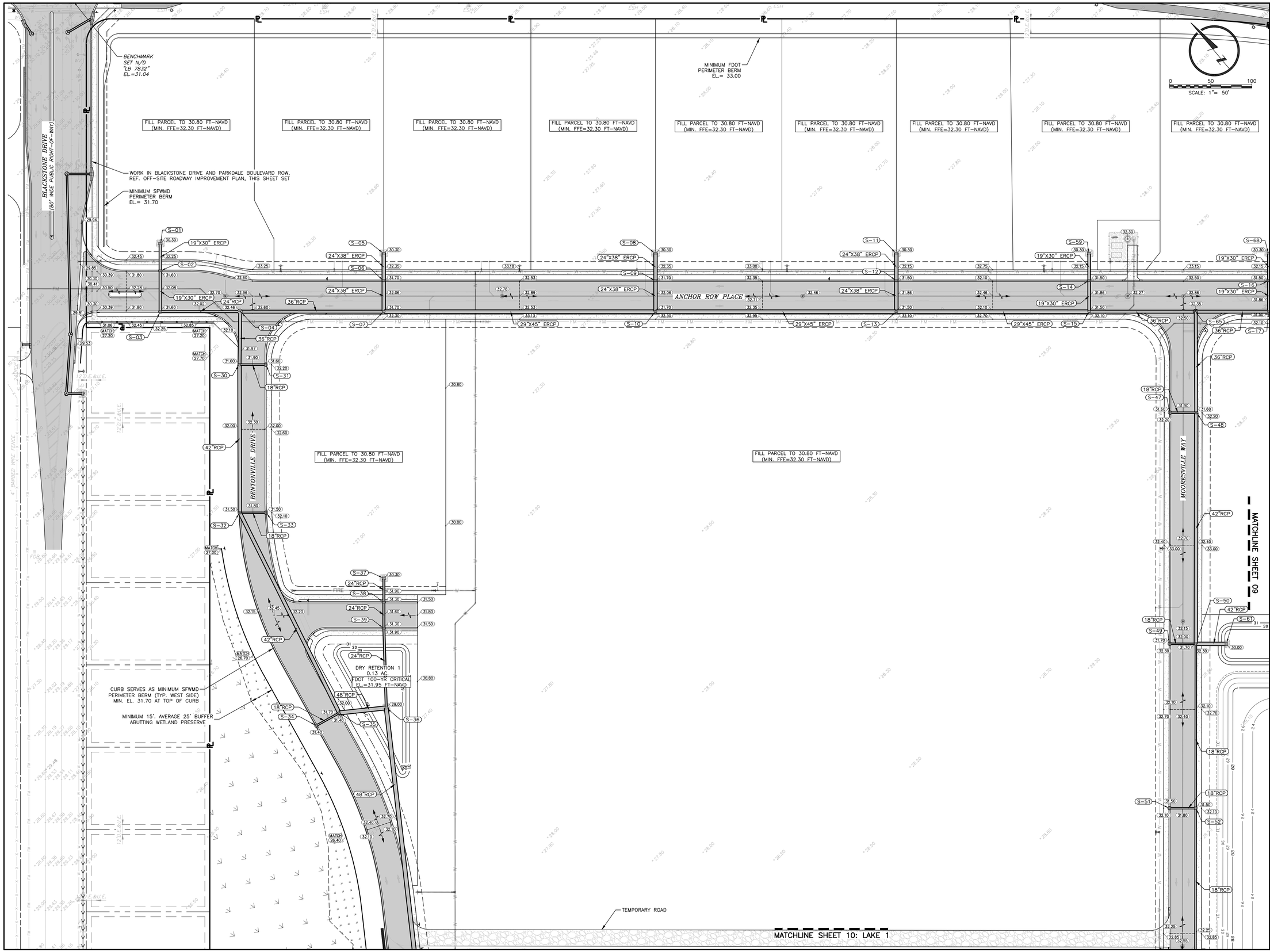
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #65332

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 07

K:\Atwell\24xx\24003608 - Fort Myers Blackstone\engineering\dwg plans\07 MASTER DRAINAGE PLAN.dwg, 7/9/2025 11:45:43 AM, IVAN ZALESNIKOV, ATWELL, LLC



ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PAVING, GRADING & DRAINAGE PLAN 1

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 08

MATCHLINE SHEET 08

MINIMUM FDOT PERIMETER BERM EL.= 33.00

FILL PARCEL TO 30.80 FT-NAVD (MIN. FFE=32.30 FT-NAVD)

WORK IN FDOT ROW TO BE COORDINATED WITH FDOT UNDER SEPARATE PERMIT, SHOWN FOR COORDINATION PURPOSES

STATE ROAD 82 (200' WIDE PUBLIC RIGHT-OF-WAY)

SCALE: 1"= 50'

RENOVATION ROW

MINIMUM FDOT PERIMETER BERM ON PAVEMENT EL.= 32.50

BENCHMARK FOUND N/D "LB 6940" EL.=31.61

ANCHOR ROW PLACE

MOORESVILLE WAY

MINIMUM SFWMD PERIMETER BERM EL.= 31.70

DRY RETENTION 3
0.69 AC.
FDOT 100-YR CRITICAL
EL.=31.95 FT-NAVD

LAKE 2
7.25 AC.
CONTROL EL.= 28.00(NAVD)
FDOT 100-YR CRITICAL
EL.=31.95 FT-NAVD

LITTORAL SHELF AREA

TEMPORARY ROAD

GECKO EXTENSION CANAL

MATCHLINE SHEET 10: LAKE 2

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PAVING, GRADING & DRAINAGE PLAN 2

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE AT THE TIME. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE SHOWN ARE CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

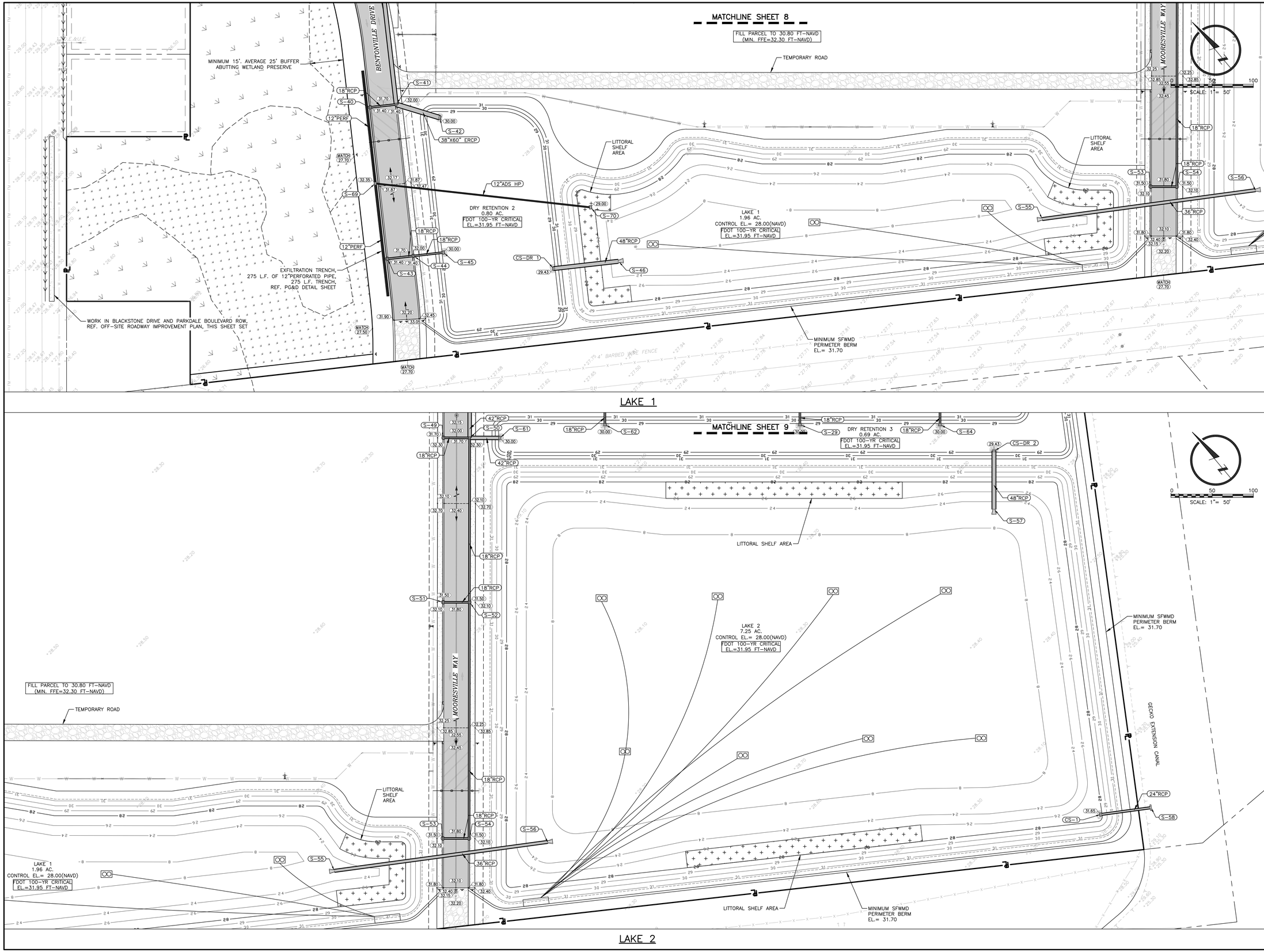
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA

PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88

BOOK AND PAGE:

SHEET 09

E:\Revit\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\09 PAVING, GRADING & DRAINAGE PLAN 2.dwg, 2/3/2026 11:46:28 AM, VIVIAN ZALESKASKI, ATWELL, LLC



ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PAVING, GRADING & DRAINAGE PLAN 3

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

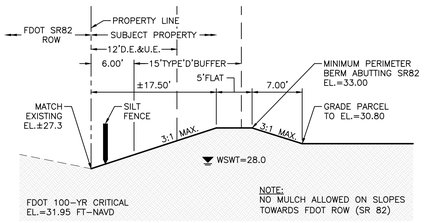
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.

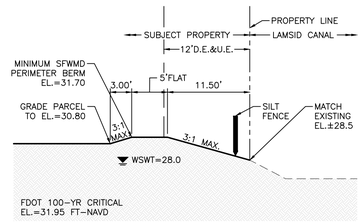
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88
BOOK AND PAGE:		

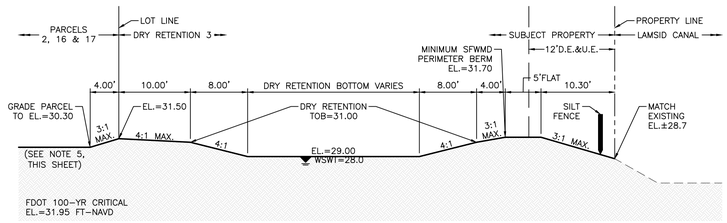
SHEET 10



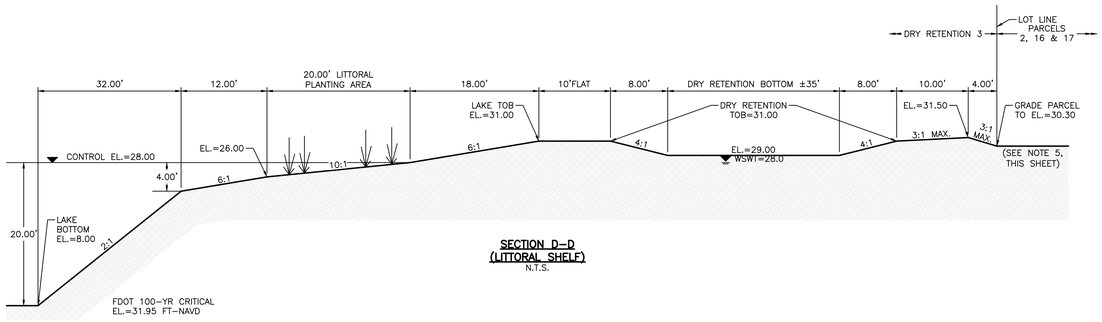
SECTION A–A
N.T.S.



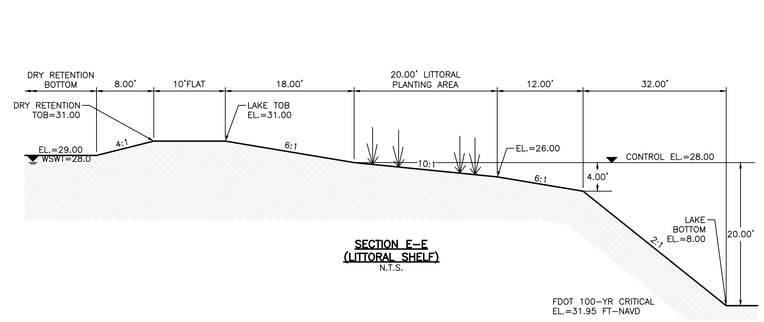
SECTION B–B
N.T.S.



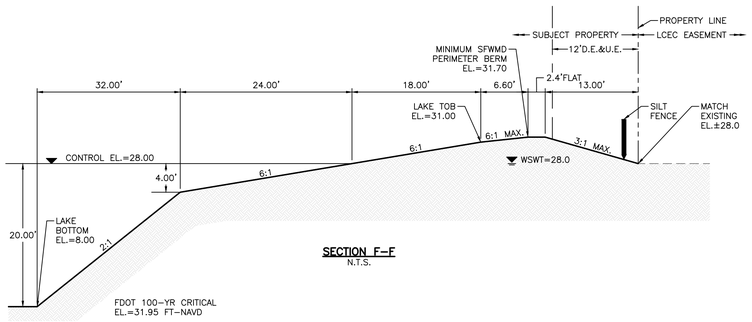
SECTION C–C
N.T.S.



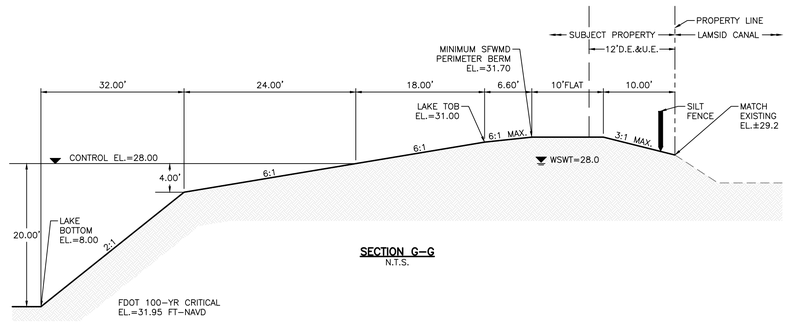
SECTION D–D
(LITTORAL SHELF)
N.T.S.



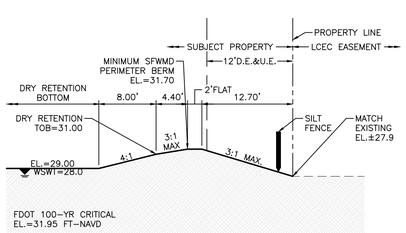
SECTION E–E
(LITTORAL SHELF)
N.T.S.



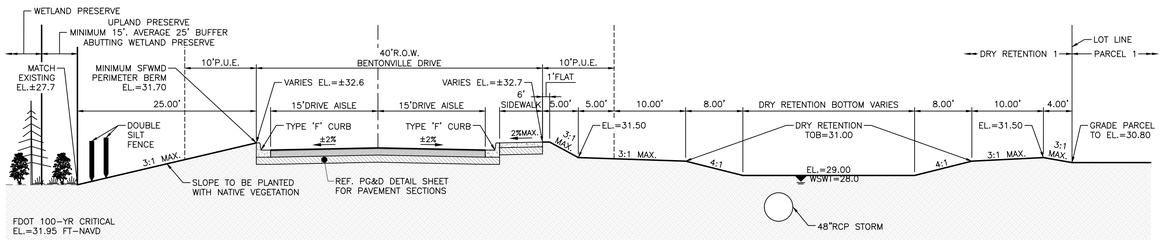
SECTION F–F
N.T.S.



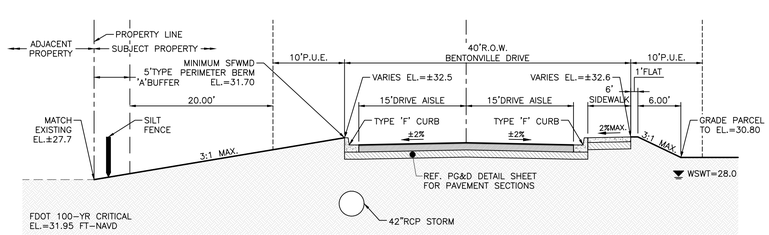
SECTION G–G
N.T.S.



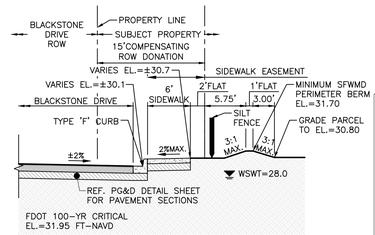
SECTION H–H
N.T.S.



SECTION I–I
N.T.S.



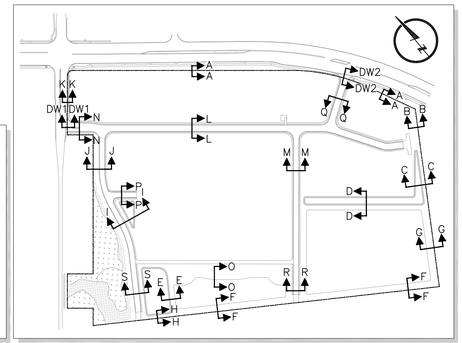
SECTION J–J
N.T.S.



SECTION K–K
N.T.S.

NOTES:

1. AFTER THE LOTS ARE FILLED AND GRADED, THE SITE IS TO BE STABILIZED WITH SEED AND MULCH.
2. ALL ACCESSIBLE PATHWAYS, ACCESS AISLES, ACCESSIBLE MANEUVERING SPACES AND PARKING SPACES TO CONFORM TO THE FLORIDA ACCESSIBILITY CODE. CONTRACTOR TO VERIFY ACCESSIBILITY AND COORDINATE WITH OWNER AND ENGINEER OF ANY DISCREPANCIES.
3. MINIMUM 3' LANDING REQUIRED AT TOP OF CURB RAMP WITH LESS THAN 2% SLOPE IN ALL DIRECTIONS PER FLORIDA ACCESSIBILITY CODE 406 (TYPICAL ALL CURB RAMPS).
4. RAMP SLOPE TO BE LESS THAN 5% RUNNING SLOPE AND LESS THAN 2% CROSS SLOPE PER FLORIDA ACCESSIBILITY CODE 406.
5. PARCELS 2, 16, AND 17 FILL TO 30.30 FT-NAVD. REMAINDER PARCELS FILL TO 30.80 FT-NAVD.
6. ALL DISTURBED AREAS IN FDOT ROW WILL BE SODDED.
7. NO MULCH ON SLOPES TOWARDS FDOT ROW (SR 82).



KEY MAP
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

TYPICAL CROSS SECTIONS 1

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

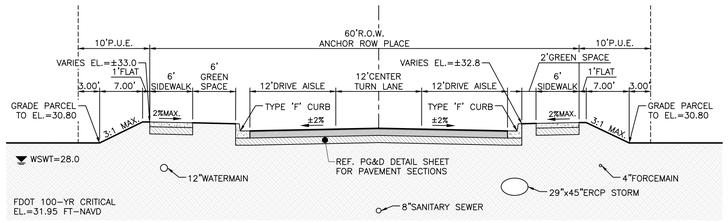
ALL UTILITIES ARE SHOWN FROM INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND CONFIGURATION ARE CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

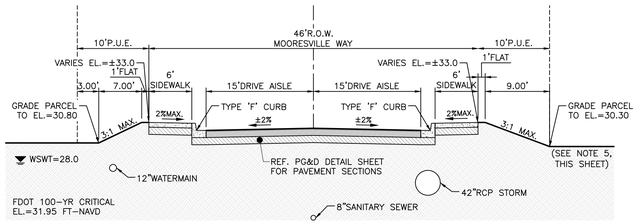
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 11

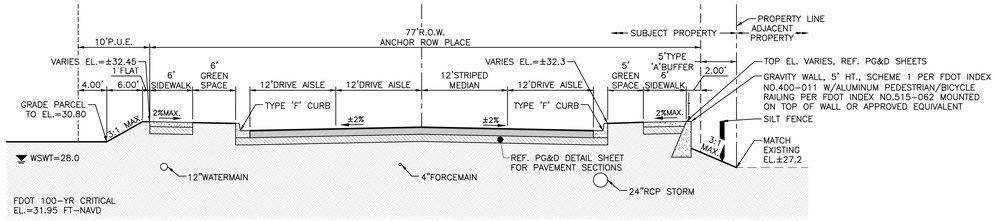
x:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\11 TYPICAL CROSS SECTIONS.dwg, 7/9/2025 11:47:06 AM, VIVIAN ZALTZMAN, ATWELL, LLC



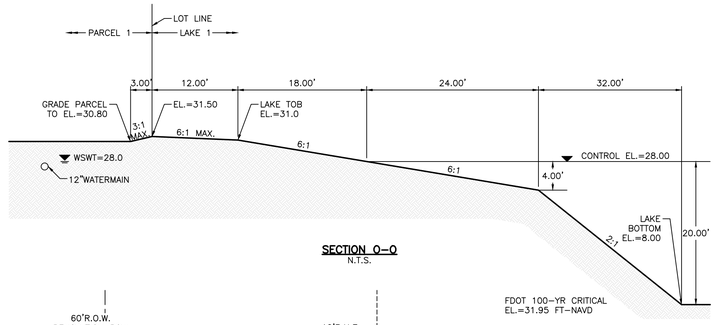
SECTION I–I
N.T.S.



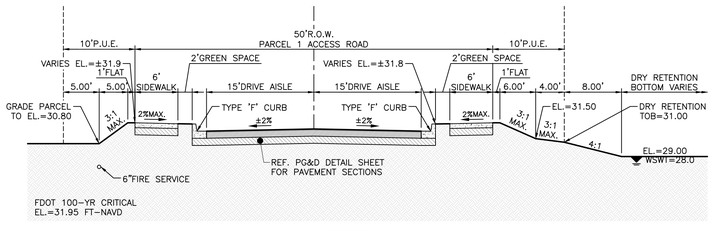
SECTION M–M
N.T.S.



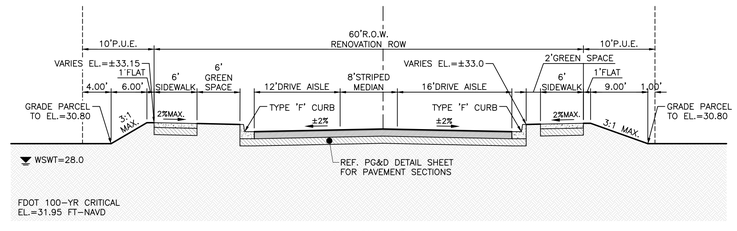
SECTION N–N
N.T.S.



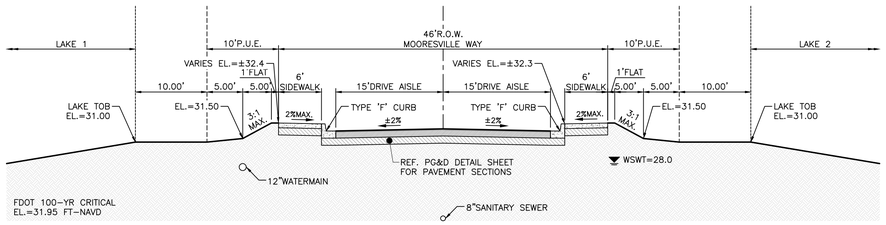
SECTION O–O
N.T.S.



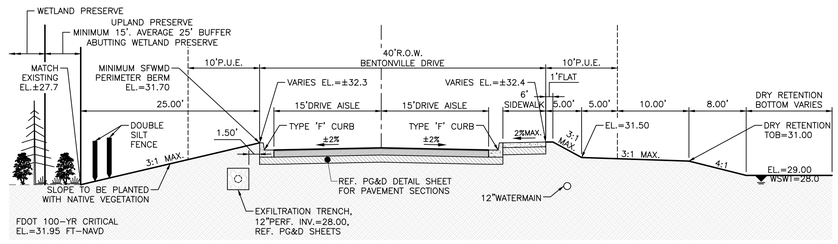
SECTION P–P
N.T.S.



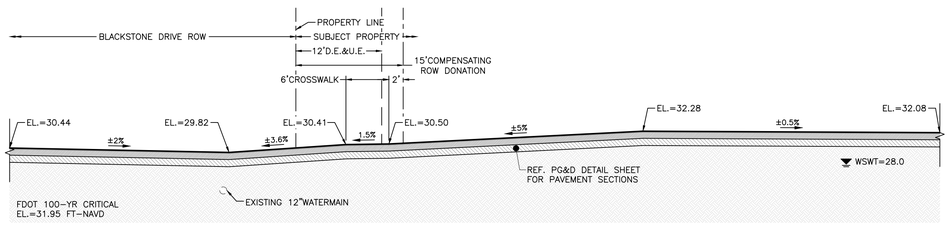
SECTION Q–Q
N.T.S.



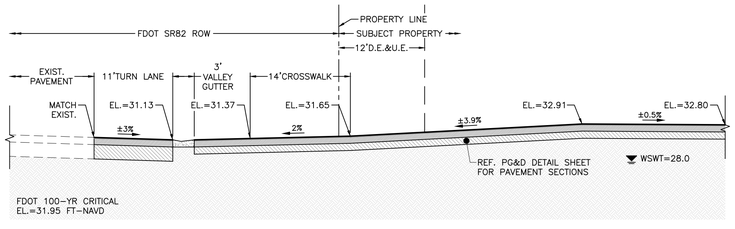
SECTION R–R
N.T.S.



SECTION S–S
N.T.S.



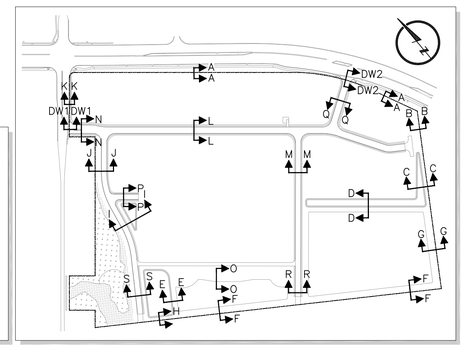
SECTION DW1–DW1
N.T.S.



SECTION DW2–DW2
N.T.S.

NOTES:

1. AFTER THE LOTS ARE FILLED AND GRADED, THE SITE IS TO BE STABILIZED WITH SEED AND MULCH.
2. ALL ACCESSIBLE PATHWAYS, ACCESS AISLES, ACCESSIBLE MANEUVERING SPACES AND PARKING SPACES TO CONFORM TO THE FLORIDA ACCESSIBILITY CODE. CONTRACTOR TO VERIFY ACCESSIBILITY AND COORDINATE WITH OWNER AND ENGINEER OF ANY DISCREPANCIES.
3. MINIMUM 3' LANDING REQUIRED AT TOP OF CURB RAMP WITH LESS THAN 2% SLOPE IN ALL DIRECTIONS PER FLORIDA ACCESSIBILITY CODE 406 (TYPICAL ALL CURB RAMPS).
   RAMP SLOPE TO BE LESS THAN 5% RUNNING SLOPE AND LESS THAN 2% CROSS SLOPE PER FLORIDA ACCESSIBILITY CODE 406.
4. PARCELS 2, 16, AND 17 FILL TO 30.30 FT-NAVD. REMAINDER PARCELS FILL TO 30.80 FT-NAVD.
5. ALL DISTURBED AREAS IN FDOT ROW WILL BE SODDED.
6. NO MULCH ON SLOPES TOWARDS FDOT ROW (SR 82).



KEY MAP
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

TYPICAL CROSS SECTIONS 2

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:

CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ
2	10/17/2025	PER COUNTY COMMENTS	VZ

811 Know what's below. Call before you dig.

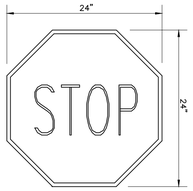
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THE ENGINEER ASSUMES NO LIABILITY FOR FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZES FOR EACH FACILITY ARE CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE ONES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

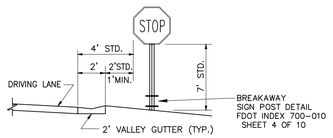
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 12

s:\fmwell\jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\11 TYPICAL CROSS SECTIONS.dwg, 2/5/2026 11:47:11 AM, VIVIAN ZALESNIKOW, ATWELL, LLC



24" R1-1 STOP SIGN
N.T.S.

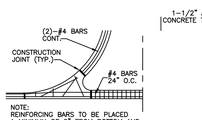


TYPICAL SIGN PLACEMENT
N.T.S.

PRESERVE
PROTECTED AREA
NO MOWING
NO VEGETATION REMOVAL
NO FILLING
NO DUMPING

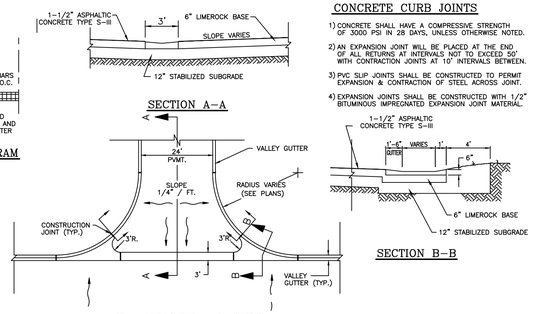
PRESERVE SIGN

PRESERVE SIGNS ARE TO BE A MAXIMUM SIZE OF 2 SQUARE FEET AND A MAXIMUM HEIGHT OF 4 FEET. SPACING OF SIGNS SHOWN ON PLAN IS EVERY 300 FEET MAXIMUM.



NOTE:
REINFORCING BARS TO BE PLACED A MINIMUM OF 2" FROM BOTTOM AND 3" FROM SIDES OF CURB & GUTTER

REINFORCING DIAGRAM



VALLEY CROSSING DETAIL
N.T.S.

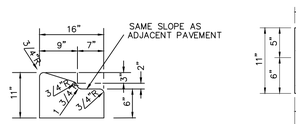
CONCRETE CURB JOINTS

1) CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI IN 28 DAYS, UNLESS OTHERWISE NOTED.

2) AN EXPANSION JOINT WILL BE PLACED AT THE END OF ALL RETURNS AT INTERVALS NOT TO EXCEED 50' WITH CONTRACTION JOINTS AT 10' INTERVALS BETWEEN.

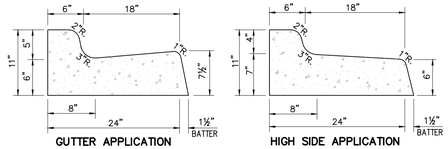
3) PVC SLIP JOINTS SHALL BE CONSTRUCTED TO PERMIT EXPANSION & CONTRACTION OF STEEL ACROSS JOINT.

4) EXPANSION JOINTS SHALL BE CONSTRUCTED WITH 1/2" BITUMINOUS IMPREGNATED EXPANSION JOINT MATERIAL.



NOTES:
1. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 PSI MINIMUM STRENGTH)
2. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 520.

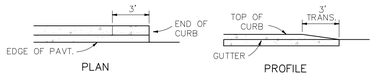
TYPE 'A' CURB
N.T.S.



GUTTER APPLICATION

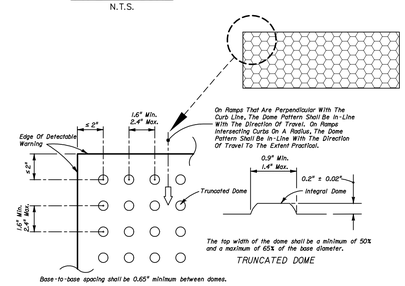
NOTES:
1. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL. (2,500 P.S.I. MINIMUM STRENGTH)
2. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 520.
3. WHEN USED ON HIGH SIDE OF PAVEMENTS, THE CROSS SLOPE OF THE GUTTER SHALL SPILL OUT WITH 1" FALL AS SHOWN.

TYPE "F" CURB
N.T.S.



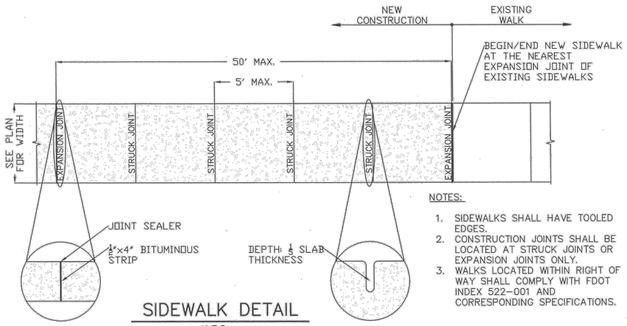
STRAIGHT END

CURB & GUTTER ENDING
N.T.S.



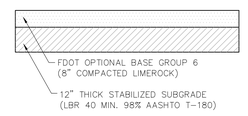
All Sidewalk Curb Ramps Shall Have Detectable Warning Surfaces That Extend The Full Width Of The Ramp And In The Direction Of Travel 24 inches (600 mm) From The Back Of Curb.

DETECTABLE WARNING PAD DETAIL
N.T.S.

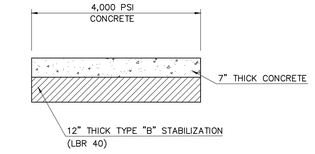


NOTES:
1. SIDEWALKS SHALL HAVE TOOLED EDGES.
2. CONSTRUCTION JOINTS SHALL BE LOCATED AT STRUCK JOINTS OR EXPANSION JOINTS ONLY.
3. WALKS LOCATED WITHIN RIGHT OF WAY SHALL COMPLY WITH FDOT INDEX 522-001 AND CORRESPONDING SPECIFICATIONS.

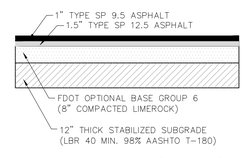
SIDEWALK DETAIL
N.T.S.



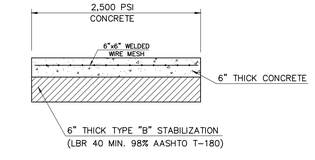
STABILIZED SUBGRADE SECTION (ON-SITE)
N.T.S.



FGUA DRIVEWAY CONCRETE SECTION (ON-SITE)
N.T.S.

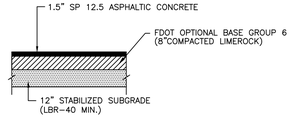


ASPHALT SECTION (ON-SITE)
N.T.S.



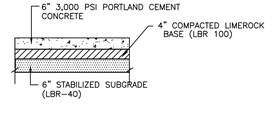
NOTES:
1. SIDEWALKS TO BE 4" THICK WITHOUT WELDED WIRE MESH.
2. SOLID WASTE PAD TO BE 6" THICK WITH WELDED WIRE MESH AS SHOWN.

SIDEWALK CONCRETE SECTION (ON-SITE)
N.T.S.



TYPICAL PAVEMENT SECTION (OFF-SITE)
LEEDOT R/W – BLACKSTONE DRIVE
N.T.S.

NOTE: PER LEEDOT SEC 10-296, TABLE 3



TYPICAL SIDEWALK SECTION (OFF-SITE)
LEEDOT R/W
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PAVING, GRADING & DRAINAGE DETAILS 1

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

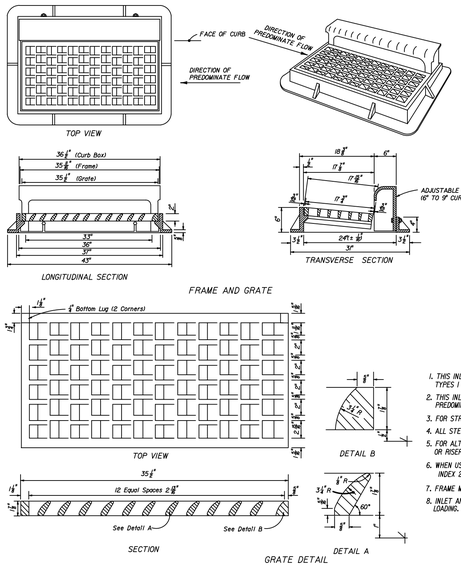
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE ARE ACCURATE OR CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

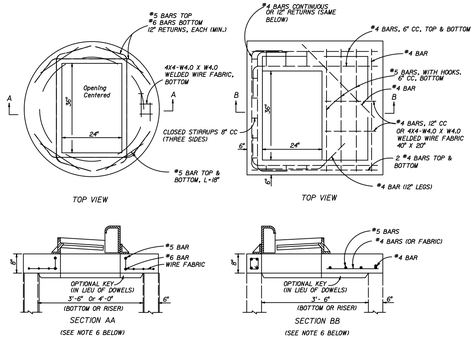
RACHEL J. TRACEY, P.E.
P.E. LIC. #85332

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET
13

c:\Users\vzach\24xx\24003608 - fort myers blackstone\engineering\dev plans\13 PAVING, GRADING & DRAINAGE DETAILS.dwg, 2/9/2026 11:47:19 AM, VIKAN ZALEWSKI, ATWELL, LLC





TOP SLABS

GENERAL NOTES

1. THIS INLET IS PRIMARILY INTENDED FOR LOCATIONS WITH LIGHT TO MODERATE FLOWS WHERE RIGHT OF WAY DOES NOT PERMIT THE USE OF THROATED CURB INLETS TYPES I THROUGH 6. THE TYPICAL APPLICATION IS ON CURB RETURNS TO CITY STREETS. THE INLET GRATE IS SUITABLE FOR PEDESTRIAN AND BICYCLE TRAFFIC.
2. THIS INLET TO BE LOCATED IN VERTICAL FACED CURBS SUCH AS CURB AND GUTTER TYPE F. GRATE SHALL BE ORIENTED WITH VANES DIRECTED TOWARD PREDOMINATE FLOW. INLET TO BE LOCATED OUTSIDE PEDESTRIAN CROSSWALK WHERE PRACTICAL.
3. FOR STRUCTURE BOTTOMS SEE INDEX NO. 200. FOR SUPPLEMENTAL DETAILS SEE INDEX NO. 201.
4. ALL STEEL IN SLAB TOPS SHALL HAVE ¾" MINIMUM COVER UNLESS OTHERWISE SHOWN. TOPS SHALL BE EITHER CAST-IN-PLACE OR PRECAST CONCRETE.
5. FOR ALTERNATE B APPLICATIONS, TOP SLAB OPENINGS SHALL BE PLACED SUCH THAT 2 EDGES OF INLET FRAME WILL BE LOCATED DIRECTLY ABOVE BOTTOM WALL OR RISER WALL.
6. WHEN USED ON A STRUCTURE WITH DIMENSIONS LARGER THAN THOSE DETAILED ABOVE AND RISERS ARE NOT APPLIED, THE TOP SLAB SHALL BE CONSTRUCTED USING INDEX 200 WITH THE SLAB OPENING ADJUST TO 24"x 36". THE "SPECIAL TOP SLAB" ON INDEX 200 IS NOT PERMITTED.
7. FRAME MAY BE ADJUSTED WITH ONE TO SIX COURSES OF BRICK.
8. INLET AND GRATE DETAIL SHOWN IS NEENAH R-3067-L. VANED GRATES WITH APPROXIMATELY EQUAL OPENINGS WILL BE PERMITTED THAT SATISFY AASHTO H-20 LOADING. INLET AND GRATE SHALL BE CLASS 30 CASTINGS IN ACCORDANCE WITH ASTM A 48. GRATES SHALL BE REVERSIBLE, RIGHT OR LEFT.

INLET TOPS TYPE 9
N.T.S.

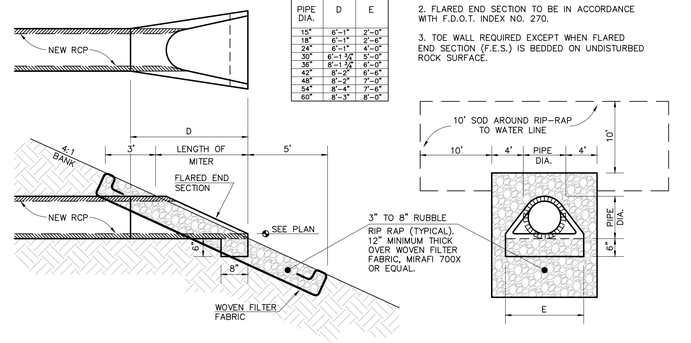
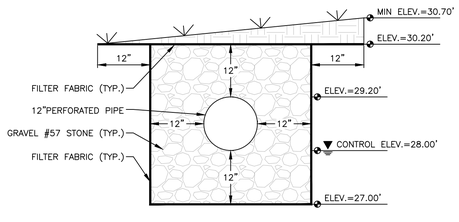


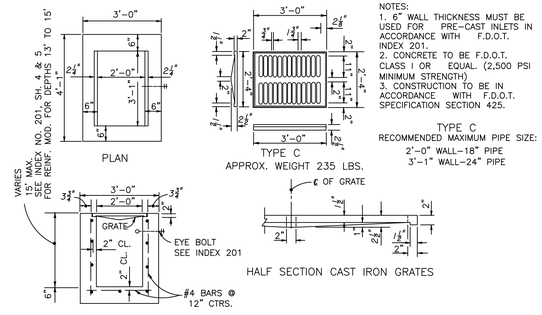
TABLE		
PIPE DIA.	D	E
15"	6'-1"	2'-0"
18"	6'-1"	2'-6"
24"	6'-1"	4'-0"
30"	6'-1 ½"	5'-0"
36"	8'-1 ½"	6'-0"
42"	8'-2"	7'-0"
48"	8'-2"	7'-6"
54"	8'-4"	7'-6"
60"	8'-3"	8'-0"

NOTES:
1. TYPICAL ALL PIPES INTO LAKES.
2. FLARED END SECTION TO BE IN ACCORDANCE WITH F.D.O.T. INDEX NO. 270.
3. TOE WALL REQUIRED EXCEPT WHEN FLARED END SECTION (F.E.S.) IS BEDDED ON UNDISTURBED ROCK SURFACE.

FLARED END SECTION PIPE TREATMENT
N.T.S.



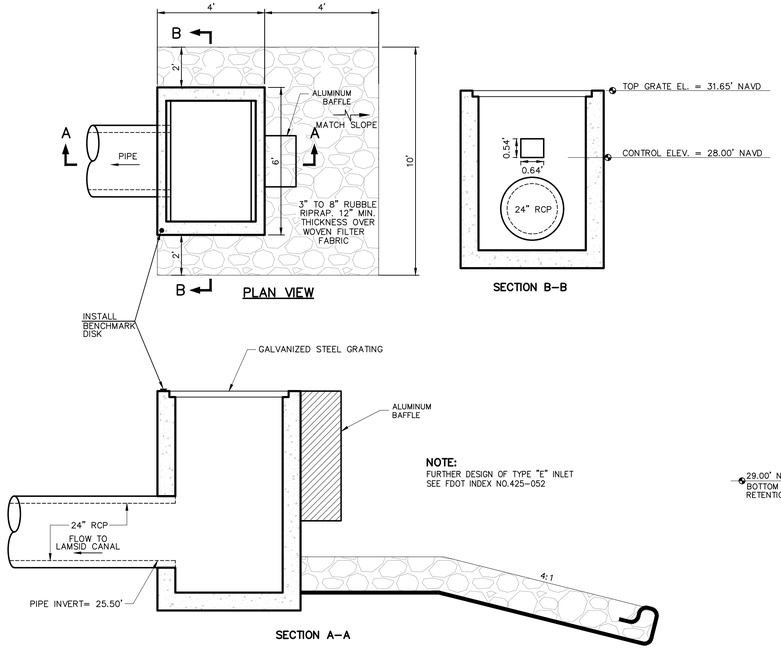
EXFILTRATION TRENCH DETAIL
N.T.S.



NOTES:
1. 6" WALL THICKNESS MUST BE USED FOR PRE-CAST INLETS IN ACCORDANCE WITH F.D.O.T. INDEX 201.
2. CONCRETE TO BE F.D.O.T. CLASS I OR EQUAL (2,500 PSI MINIMUM STRENGTH)
3. CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. SPECIFICATION SECTION 425.

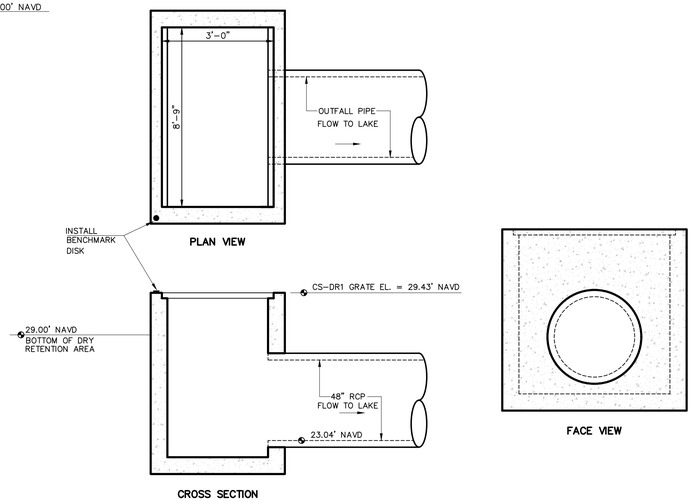
TYPE C
RECOMMENDED MAXIMUM PIPE SIZE:
2'-0" WALL-18" PIPE
3'-1" WALL-24" PIPE

INLET "C" DETAIL
N.T.S.

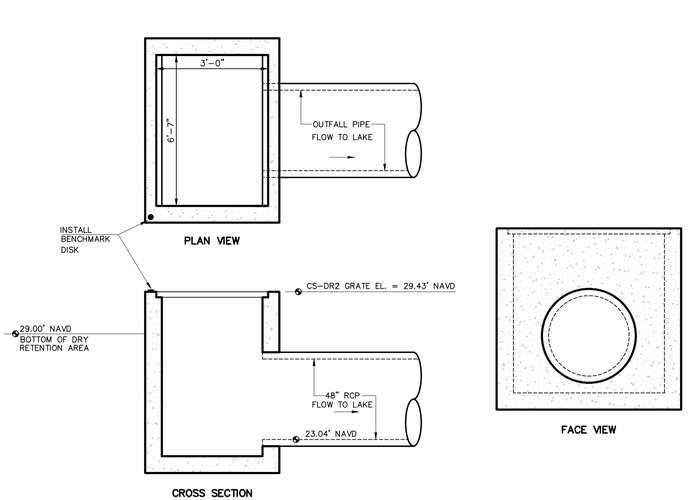


NOTE:
FURTHER DESIGN OF TYPE "E" INLET SEE FDOT INDEX NO.425-052

WATER CONTROL STRUCTURE DETAIL
CS-1
N.T.S.



WATER CONTROL STRUCTURE DETAIL
CS-DR1 (MODIFIED TYPE H, 4 GRATE)
N.T.S.



WATER CONTROL STRUCTURE DETAIL
CS-DR2 (MODIFIED TYPE H, 3 GRATE)
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PAVING, GRADING & DRAINAGE DETAILS 2

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE AT THE TIME. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE EACH IS SHOWN IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 14

I:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\13 PAVING, GRADING & DRAINAGE DETAILS.dwg, 2/3/2026 11:47:23 AM, VIVIAN ZALESKARSKI, ATWELL, LLC



LEGEND:

- = PROTECTED CONSERVATION AREA SIGNAGE
- = PROPOSED ON-SITE PRESERVE
- = PROPOSED OFF-SITE PRESERVE
- = FUTURE INDUSTRIAL PRESERVE

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PRESERVE PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:

CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's **below.** **Call** before you dig.

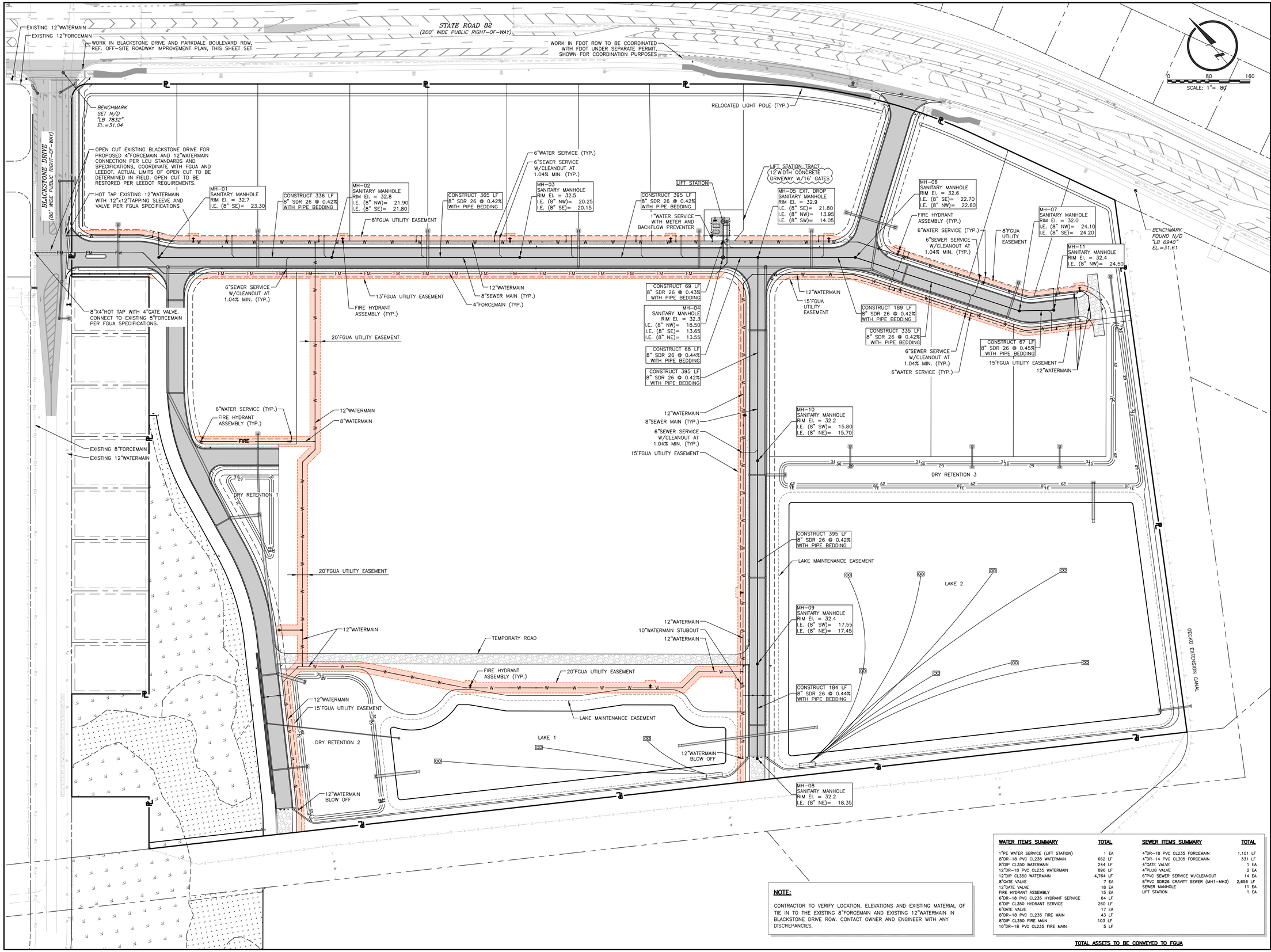
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THEY DO NOT GUARANTEE ALL UTILITIES ARE SHOWN, OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 15

s:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\15 PRESERVE PLAN.dwg, 2/6/2025 11:48:06 AM, VIVIAN ZALEWSKI, ATWELL, LLC



NOTE:

CONTRACTOR TO VERIFY LOCATION, ELEVATIONS AND EXISTING MATERIAL OF TIE IN TO THE EXISTING 8"FORCEMAIN AND EXISTING 12"WATERMAIN IN BLACKSTONE DRIVE ROW. CONTACT OWNER AND ENGINEER WITH ANY DISCREPANCIES.

WATER ITEMS SUMMARY	TOTAL
1"PE WATER SERVICE (LIFT STATION)	1 EA
8"DR-18 PVC CL235 WATERMAIN	662 LF
8"DIP CL350 WATERMAIN	244 LF
12"DR-18 PVC CL235 WATERMAIN	896 LF
12"DIP CL350 WATERMAIN	4,764 LF
8"GATE VALVE	7 EA
12"GATE VALVE	18 EA
FIRE HYDRANT ASSEMBLY	15 EA
6"DR-18 PVC CL235 HYDRANT SERVICE	64 LF
6"DIP CL350 HYDRANT SERVICE	260 LF
6"GATE VALVE	17 EA
8"DR-18 PVC CL235 FIRE MAIN	43 LF
8"DIP CL350 FIRE MAIN	103 LF
10"DR-18 PVC CL235 FIRE MAIN	5 LF

SEWER ITEMS SUMMARY	TOTAL
4"DR-18 PVC CL235 FORCEMAIN	1,101 LF
4"DR-14 PVC CL305 FORCEMAIN	331 LF
4"GATE VALVE	1 EA
4"PLUG VALVE	2 EA
6"PVC SEWER SERVICE W/CLEANOUT	14 EA
8"PVC SDR26 GRAVITY SEWER (MH1-MH3)	2,656 LF
SEWER MANHOLE	11 EA
LIFT STATION	1 EA

TOTAL ASSETS TO BE CONVEYED TO FGUA

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

MASTER UTILITY PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ
3	02/06/2026	PER COUNTY COMMENTS	VZ

811 Know what's below. Call before you dig.

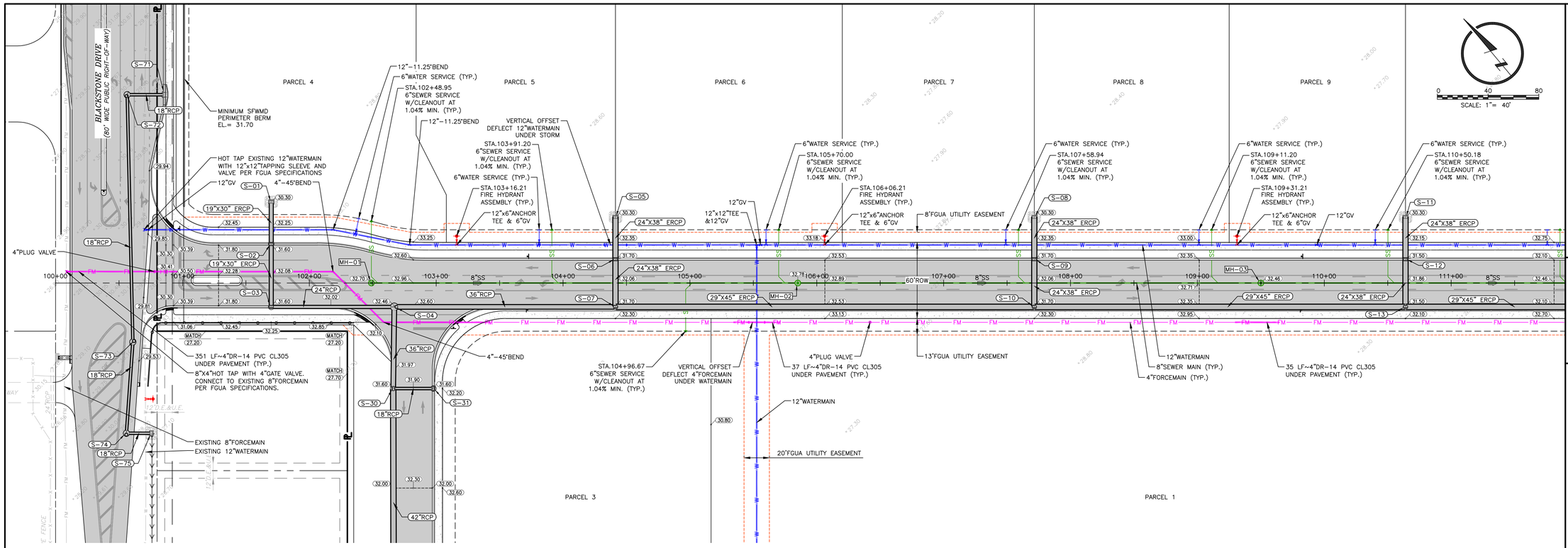
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THE ACCURACY OF UNDERGROUND FACILITIES ARE SHOWN AS TO THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY. CONTRACTOR OR SUB-CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

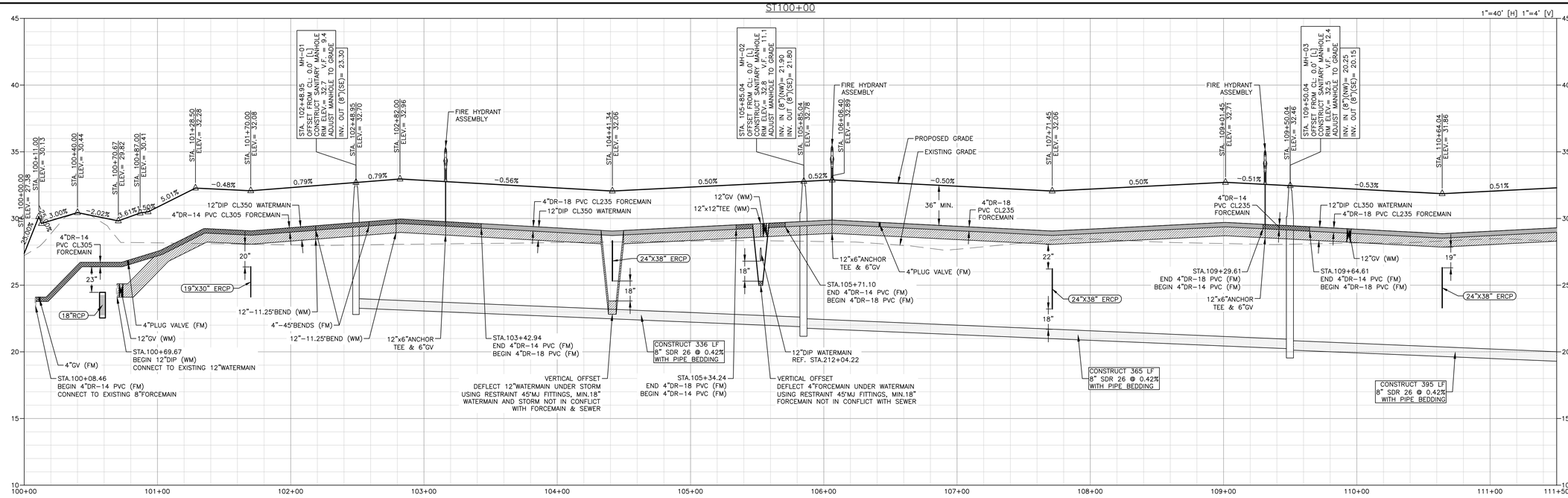
RACHEL J. TRACEY, P.E.
P.E. LIC. #65322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 16

X:\Atwell\Jobs\24\24003608 - fort myers blackstone\engineering\dev plans\16 MASTER UTILITY PLAN.dwg, 2/9/2026 11:48:25 AM, IVAN ZALESKOV, ATWELL, LLC

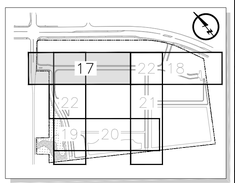




WATER ITEMS	TOTAL
1"PE WATER SERVICE (LIFT STATION)	0 EA
8"DR-18 PVC CL235 WATERMAIN	0 LF
8"DIP CL350 WATERMAIN	0 LF
12"DR-18 PVC CL235 WATERMAIN	0 LF
12"DIP CL350 WATERMAIN	1,082 LF
8"GATE VALVE	0 EA
12"GATE VALVE	3 EA
FIRE HYDRANT ASSEMBLY	3 EA
6"DR-18 PVC CL235 HYDRANT SERVICE	21 LF
6"DIP CL350 HYDRANT SERVICE	0 LF
6"GATE VALVE	3 EA
8"DR-18 PVC CL235 FIRE MAIN	0 LF
8"DIP CL350 FIRE MAIN	0 LF
10"DR-18 PVC CL235 FIRE MAIN	0 LF

SEWER ITEMS	TOTAL
4"DR-18 PVC CL235 FORCEMAIN	908 LF
4"DR-14 PVC CL305 FORCEMAIN	232 LF
4"GATE VALVE	1 EA
4"PLUG VALVE	2 EA
6"PVC SEWER SERVICE W/CLEANOUT	6 EA
8"PVC SDR26 GRAVITY SEWER (MH1-MH3)	559 LF
SEWER MANHOLE	3 EA
LIFT STATION	0 EA

ASSETS TO BE CONVEYED TO FGUA
STA.100+00 TO STA.111+50



KEY MAP
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PLAN & PROFILE 1

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. ALL EXISTING UTILITY FACILITIES ARE SHOWN TO REFLECT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY AS DETERMINED. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #65322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 17

x:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\17 PLAN & PROFILE 1.dwg, 7/9/2025 11:48:47 AM, IRVAN ZALTZMAN, ATWELL, LLC

WATER ITEMS	TOTAL	SEWER ITEMS	TOTAL
1"PE WATER SERVICE (LIFT STATION)	0 EA	4"DR-18 PVC CL235 FORCEMAIN	193 LF
8"DR-18 PVC CL235 WATERMAIN	0 LF	4"DR-14 PVC CL305 FORCEMAIN	18 LF
8"DIP CL350 WATERMAIN	0 LF	4"GATE VALVE	0 EA
12"DR-18 PVC CL235 WATERMAIN	0 LF	4"PLUG VALVE	0 EA
12"DIP CL350 WATERMAIN	935 LF	6"PVC SEWER SERVICE W/CLEANOUT	6 EA
8"GATE VALVE	0 EA	8"PVC SDR26 GRAVITY SEWER (MH4-MH11)	1,054 LF
12"GATE VALVE	2 EA	SEWER MANHOLE	5 EA
FIRE HYDRANT ASSEMBLY	4 EA	LIFT STATION	0 EA
6"DR-18 PVC CL235 HYDRANT SERVICE	28 LF		
6"DIP CL350 HYDRANT SERVICE	0 LF		
6"GATE VALVE	4 EA		
8"DR-18 PVC CL235 FIRE MAIN	0 LF		
8"DIP CL350 FIRE MAIN	0 LF		
10"DR-18 PVC CL235 FIRE MAIN	0 LF		

ASSETS TO BE CONVEYED TO FGUA
STA.111+50 TO STA.121+50

KEY MAP
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PLAN & PROFILE 2

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

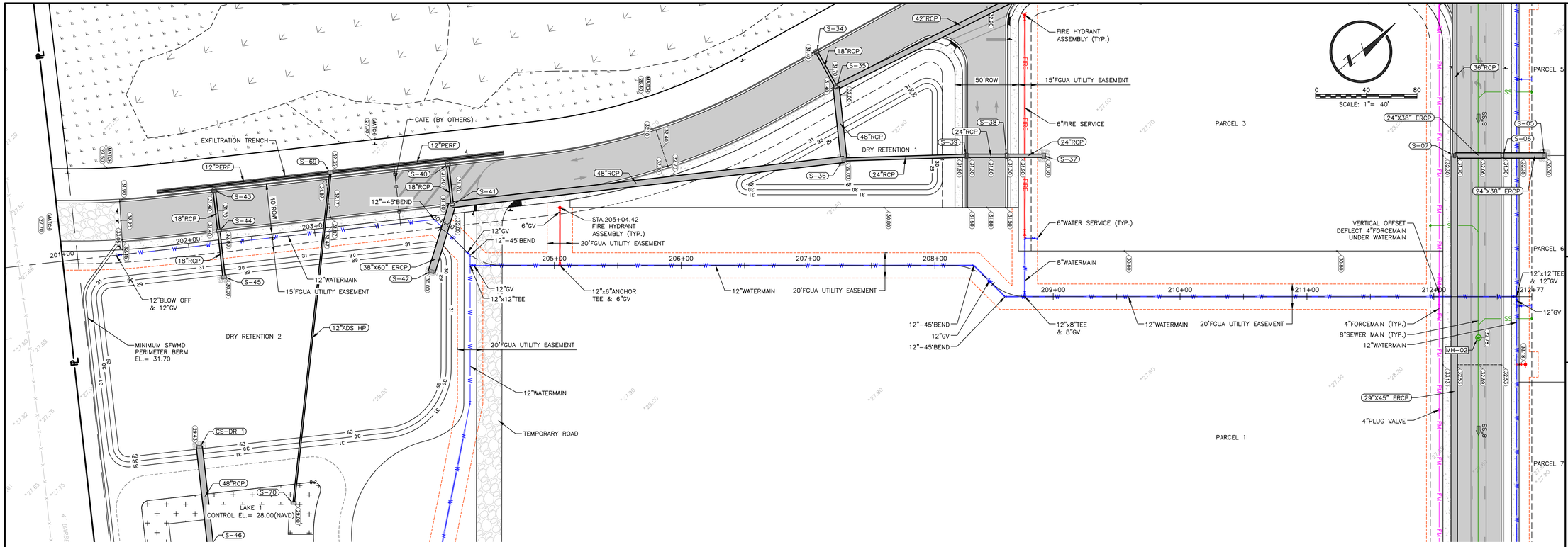
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THE CONTRACTOR SHALL VERIFY THE LOCATION, DEPTH, AND CONFLICT EACH FACILITY. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

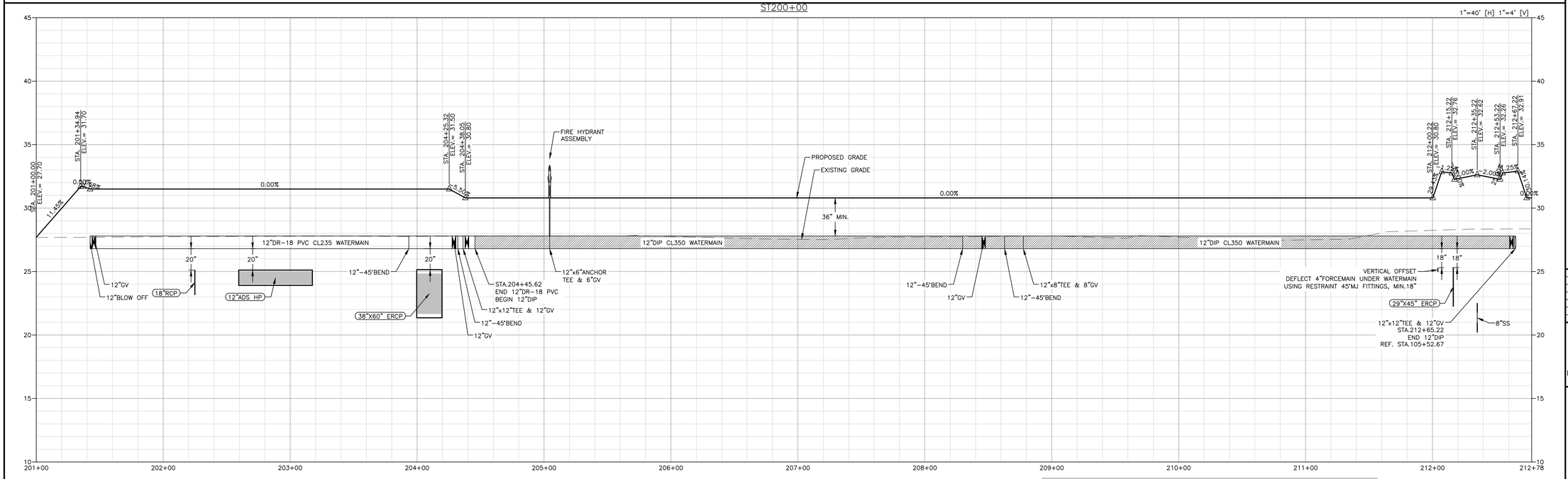
RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 18

s:\fmwell\jobs\24xx\24003608 - fort myers blackstone\engineering\site plans\18 PLAN & PROFILE 2.dwg, 2/3/2026 11:49:06 AM, VIVIAN ZALEANOH, ATWELL, LLC

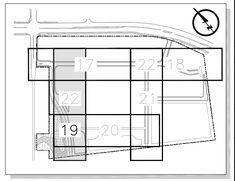




WATER ITEMS	TOTAL
1"PE WATER SERVICE (LIFT STATION)	0 EA
8"DR-18 PVC CL235 WATERMAIN	0 LF
8"DIP CL350 WATERMAIN	0 LF
12"DR-18 PVC CL235 WATERMAIN	301 LF
12"DIP CL350 WATERMAIN	835 LF
8"GATE VALVE	0 EA
12"GATE VALVE	5 EA
FIRE HYDRANT ASSEMBLY	1 EA
6"DR-18 PVC CL235 HYDRANT SERVICE	0 LF
6"DIP CL350 HYDRANT SERVICE	42 LF
6"GATE VALVE	2 EA
8"DR-18 PVC CL235 FIRE MAIN	0 LF
8"DIP CL350 FIRE MAIN	0 LF
10"DR-18 PVC CL235 FIRE MAIN	0 LF

SEWER ITEMS	TOTAL
4"DR-18 PVC CL235 FORCEMAIN	0 LF
4"DR-14 PVC CL305 FORCEMAIN	0 LF
4"GATE VALVE	0 EA
4"PLUG VALVE	0 EA
6"PVC SEWER SERVICE W/CLEANOUT	0 EA
8"PVC SDR26 GRAVITY SEWER	0 LF
SEWER MANHOLE	0 EA
LIFT STATION	0 EA

ASSETS TO BE CONVEYED TO FGUA
STA.201+00 TO STA.212+78



KEY MAP
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PLAN & PROFILE 3

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

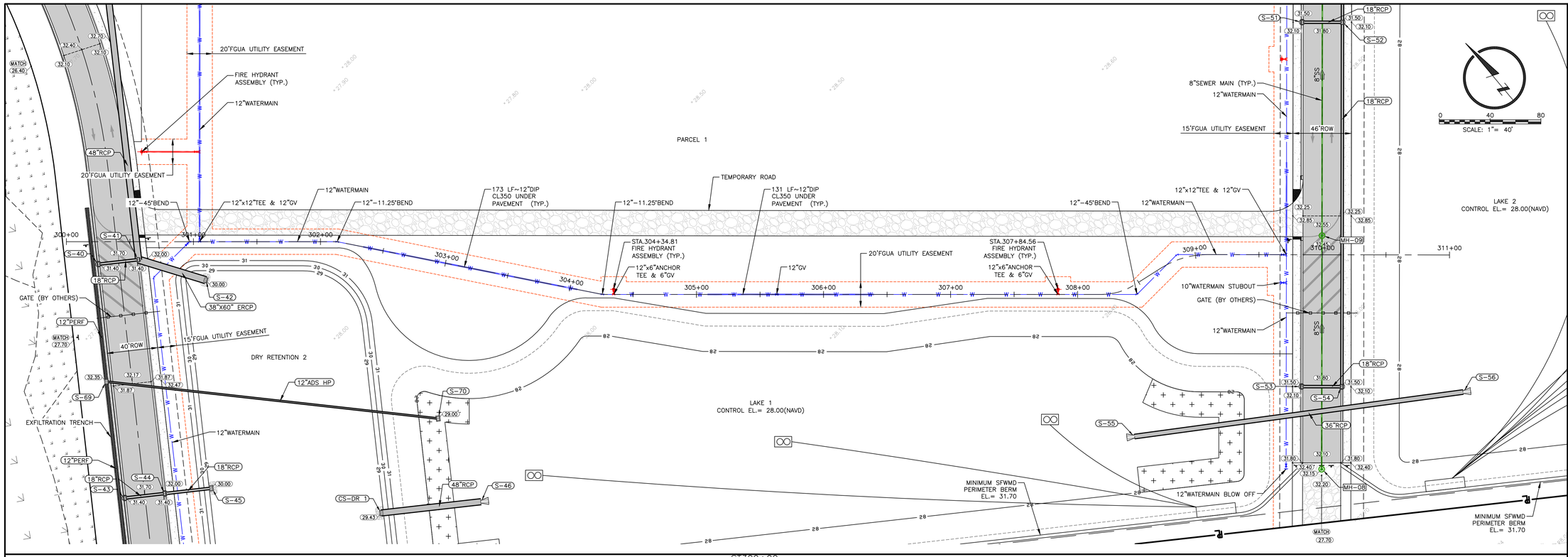
811 Know what's below. Call before you dig.
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION AND/OR THE SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

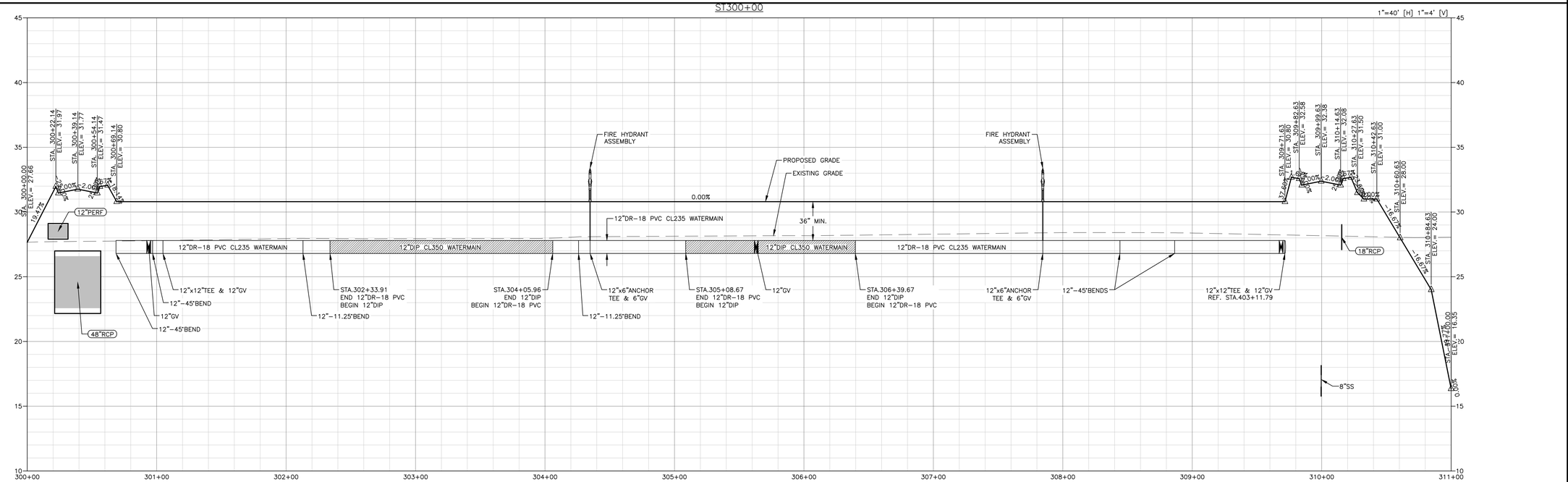
RACHEL J. TRACEY, P.E.
P.E. LIC. #65322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 19

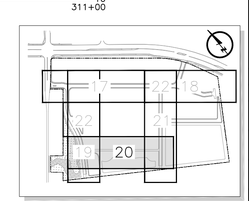
x:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\civil plans\19 PLAN & PROFILE 3.dwg, 2/9/2026 11:49:27 AM, KWIN ZALESKOSKI, ATWELL, LLC





WATER ITEMS	TOTAL	SEWER ITEMS	TOTAL
1"PE WATER SERVICE (LIFT STATION)	0 EA	4"DR-18 PVC CL235 FORCEMAIN	0 LF
8"DR-18 PVC CL235 WATERMAIN	0 LF	4"DR-14 PVC CL305 FORCEMAIN	0 LF
8"DIP CL350 WATERMAIN	0 LF	4"GATE VALVE	0 EA
12"DR-18 PVC CL235 WATERMAIN	472 LF	4"PLUG VALVE	0 EA
12"DIP CL350 WATERMAIN	655 LF	6"PVC SEWER SERVICE W/CLEANOUT	0 EA
8"GATE VALVE	0 EA	8"PVC SDR26 GRAVITY SEWER	0 LF
12"GATE VALVE	4 EA	SEWER MANHOLE	0 EA
FIRE HYDRANT ASSEMBLY	2 EA	LIFT STATION	0 EA
6"DR-18 PVC CL235 HYDRANT SERVICE	8 LF		
6"DIP CL350 HYDRANT SERVICE	0 LF		
6"GATE VALVE	2 EA		
8"DR-18 PVC CL235 FIRE MAIN	43 LF		
8"DIP CL350 FIRE MAIN	103 LF		
10"DR-18 PVC CL235 FIRE MAIN	5 LF		

ASSETS TO BE CONVEYED TO FGUA
STA.300+00 TO STA.311+50



KEY MAP
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.938.5490

PLAN & PROFILE 4

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

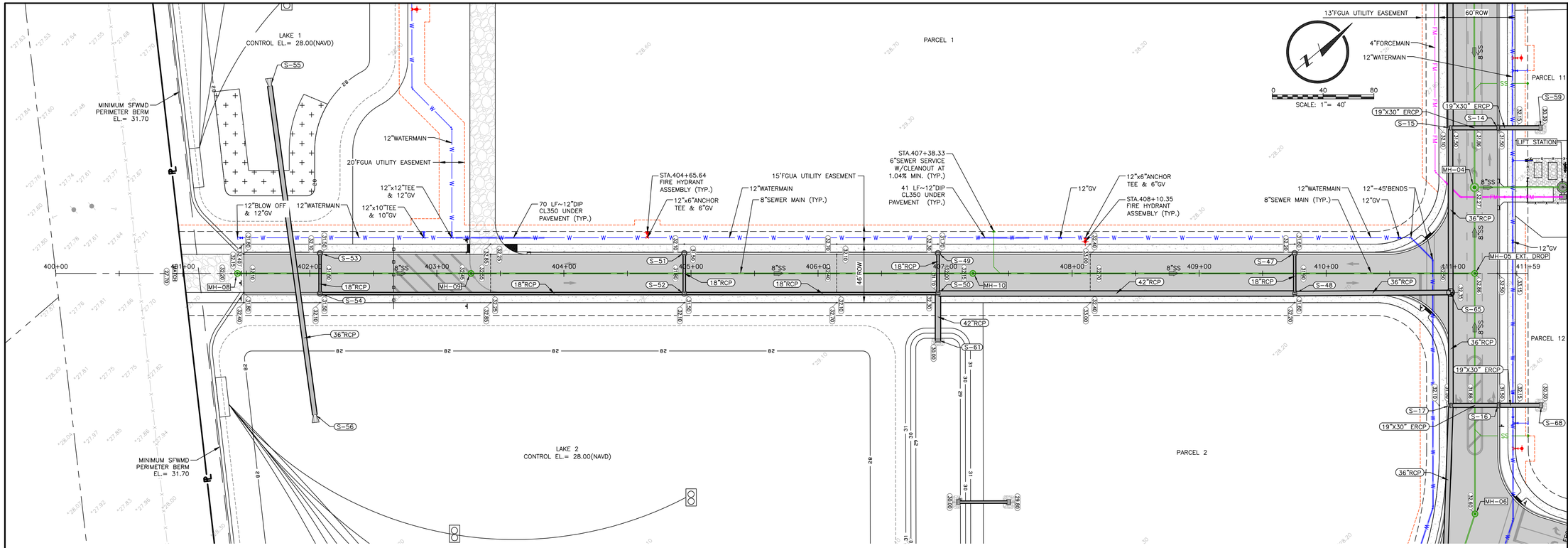
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE AT THE TIME. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE EACH IS SHOWN IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

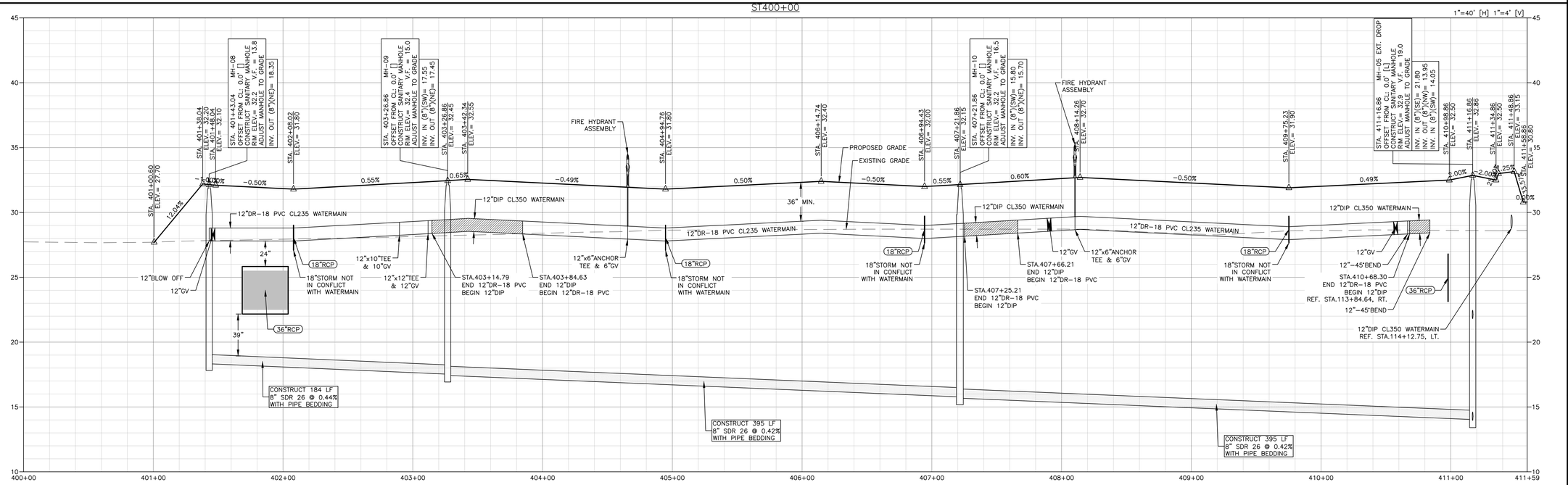
RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 20

I:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\05 PLAN & PROFILE 4.dwg, 2/5/2026 11:49:48 AM, VIVIAN ZALEWSKI, ATWELL, LLC

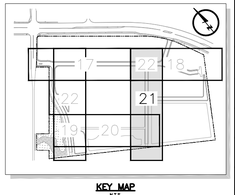




WATER ITEMS	TOTAL
1"PE WATER SERVICE (LIFT STATION)	0 EA
8"DR-18 PVC CL235 WATERMAIN	662 LF
8"DIP CL350 WATERMAIN	193 LF
12"DR-18 PVC CL235 WATERMAIN	123 LF
12"DIP CL350 WATERMAIN	80 LF
8"GATE VALVE	4 EA
12"GATE VALVE	2 EA
FIRE HYDRANT ASSEMBLY	2 EA
6"DR-18 PVC CL235 HYDRANT SERVICE	7 LF
6"DIP CL350 HYDRANT SERVICE	0 LF
6"GATE VALVE	2 EA
8"DR-18 PVC CL235 FIRE MAIN	0 LF
8"DIP CL350 FIRE MAIN	0 LF
10"DR-18 PVC CL235 FIRE MAIN	0 LF

SEWER ITEMS	TOTAL
4"DR-18 PVC CL235 FORCEMAIN	0 LF
4"DR-14 PVC CL305 FORCEMAIN	0 LF
4"GATE VALVE	0 EA
4"PLUG VALVE	0 EA
6"PVC SEWER SERVICE W/CLEANOUT	2 EA
8"PVC SDR26 GRAVITY SEWER (MH8-MH5)	974 LF
SEWER MANHOLE	3 EA
LIFT STATION	0 EA

ASSETS TO BE CONVEYED TO FGUA
STA.400+00 TO STA.411+59



KEY MAP
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PLAN & PROFILE 5

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

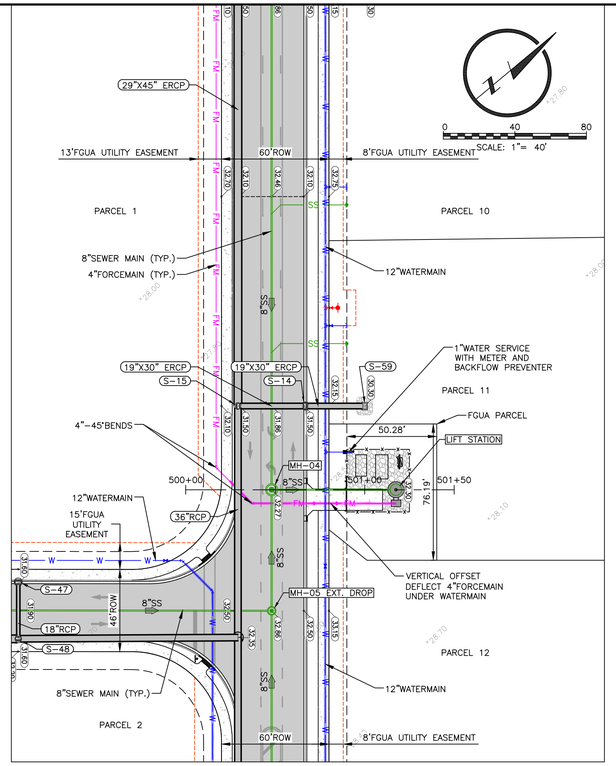
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATIONS SHOWN AS TO SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

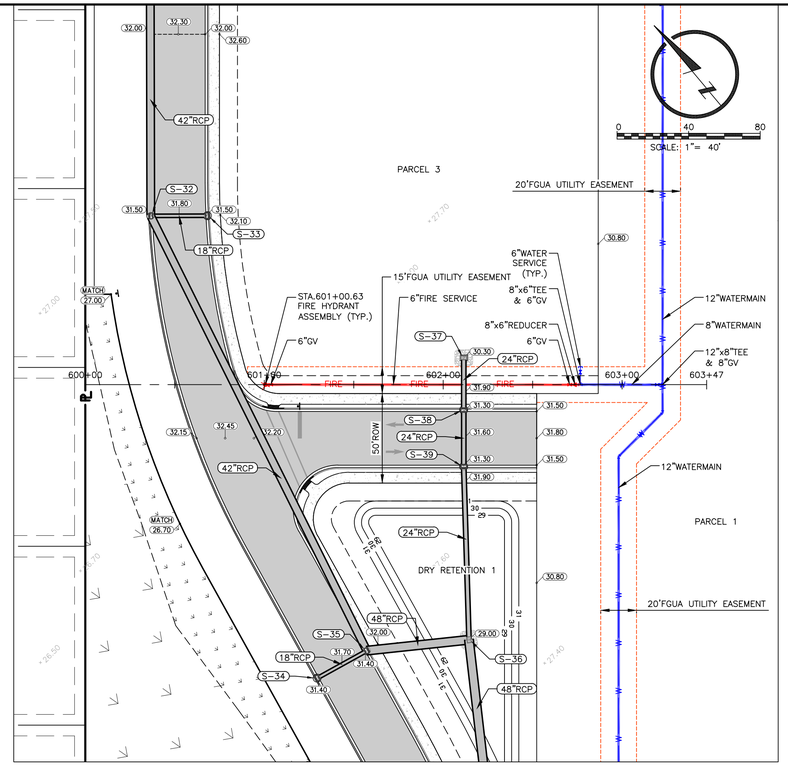
RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

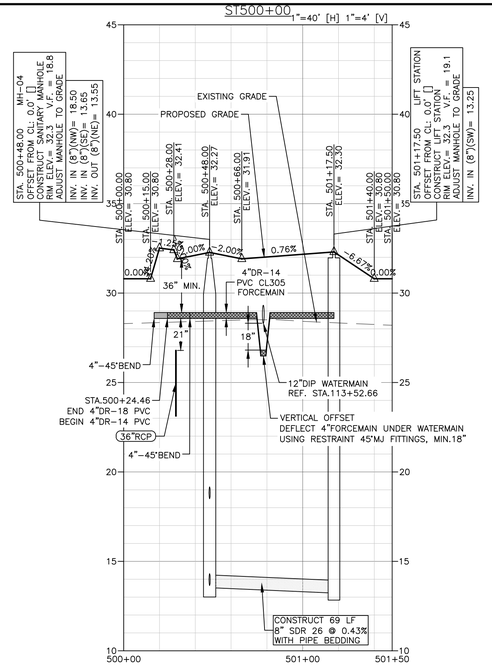
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88
BOOK AND PAGE:		

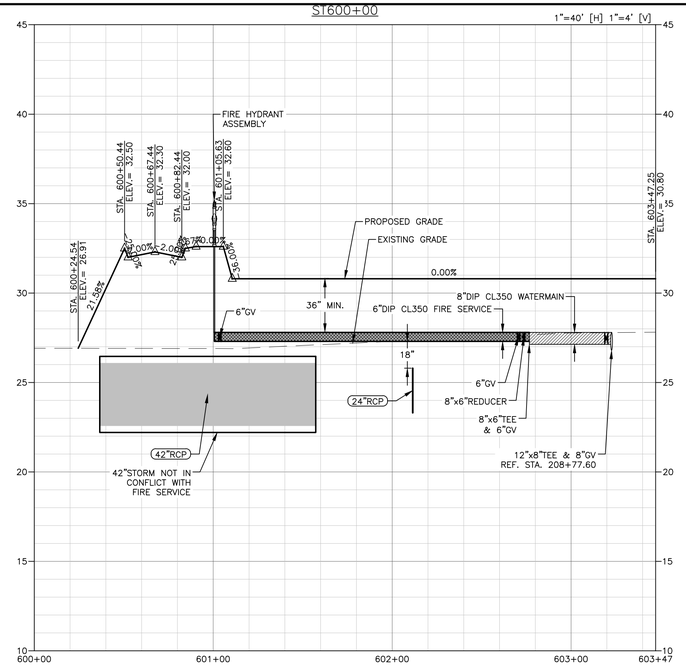
SHEET
21

x:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dwg plans\21 PLAN & PROFILE 5.dwg, 2/9/2026 11:50:09 AM, VANN ZALEWSKI, ATWELL, LLC







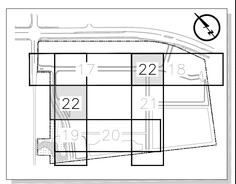


WATER ITEMS	TOTAL	SEWER ITEMS	TOTAL
1"PE WATER SERVICE (LIFT STATION)	1 EA	4"DR-18 PVC CL235 FORCEMAIN	0 LF
8"DR-18 PVC CL235 WATERMAIN	0 LF	4"DR-14 PVC CL305 FORCEMAIN	81 LF
8"DIP CL350 WATERMAIN	0 LF	4"GATE VALVE	0 EA
12"DR-18 PVC CL235 WATERMAIN	0 LF	4"PLUG VALVE	0 EA
12"DIP CL350 WATERMAIN	0 LF	6"PVC SEWER SERVICE W/CLEANOUT	0 EA
8"GATE VALVE	0 EA	8"PVC SDR26 GRAVITY SEWER (MH4—LS)	69 LF
12"GATE VALVE	0 EA	SEWER MANHOLE	0 EA
FIRE HYDRANT ASSEMBLY	0 EA	LIFT STATION	1 EA
6"DR-18 PVC CL235 HYDRANT SERVICE	0 LF		
6"DIP CL350 HYDRANT SERVICE	0 LF		
6"GATE VALVE	0 EA		
8"DR-18 PVC CL235 FIRE MAIN	0 LF		
8"DIP CL350 FIRE MAIN	0 LF		
10"DR-18 PVC CL235 FIRE MAIN	0 LF		

ASSETS TO BE CONVEYED TO FGUA
STA.500+00 TO STA.501+50

WATER ITEMS	TOTAL	SEWER ITEMS	TOTAL
1"PE WATER SERVICE (LIFT STATION)	0 EA	4"DR-18 PVC CL235 FORCEMAIN	0 LF
8"DR-18 PVC CL235 WATERMAIN	0 LF	4"DR-14 PVC CL305 FORCEMAIN	0 LF
8"DIP CL350 WATERMAIN	46 LF	4"GATE VALVE	0 EA
12"DR-18 PVC CL235 WATERMAIN	0 LF	4"PLUG VALVE	0 EA
12"DIP CL350 WATERMAIN	0 LF	6"PVC SEWER SERVICE W/CLEANOUT	0 EA
8"GATE VALVE	2 EA	8"PVC SDR26 GRAVITY SEWER	0 LF
12"GATE VALVE	0 EA	SEWER MANHOLE	0 EA
FIRE HYDRANT ASSEMBLY	1 EA	LIFT STATION	0 EA
6"DR-18 PVC CL235 HYDRANT SERVICE	0 LF		
6"DIP CL350 HYDRANT SERVICE	177 LF		
6"GATE VALVE	2 EA		
8"DR-18 PVC CL235 FIRE MAIN	0 LF		
8"DIP CL350 FIRE MAIN	0 LF		
10"DR-18 PVC CL235 FIRE MAIN	0 LF		

ASSETS TO BE CONVEYED TO FGUA
STA.600+00 TO STA.603+47



KEY MAP
N.T.S.

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

PLAN & PROFILE 6

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's **below.** **Call** before you dig.

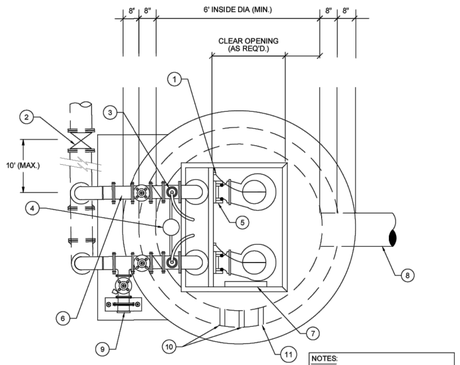
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. ALL UTILITIES SHOWN OR NOT SHOWN ON THESE PLANS ARE SHOWN AS TO THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY AND THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #65322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 22

x:\Atwell Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plan\22 PLAN & PROFILE 6.dwg, 2/9/2026 11:50:40 AM, IRVAN ZALTZMAN, ATWELL, LLC

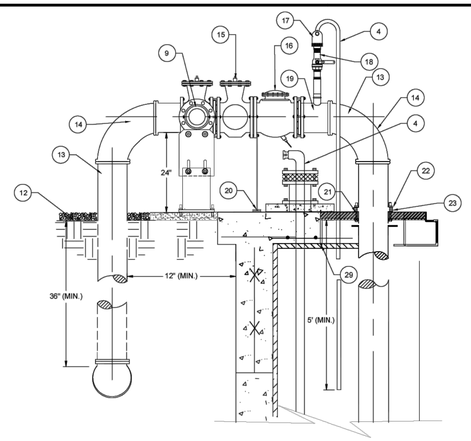


WETWELL
PLAN

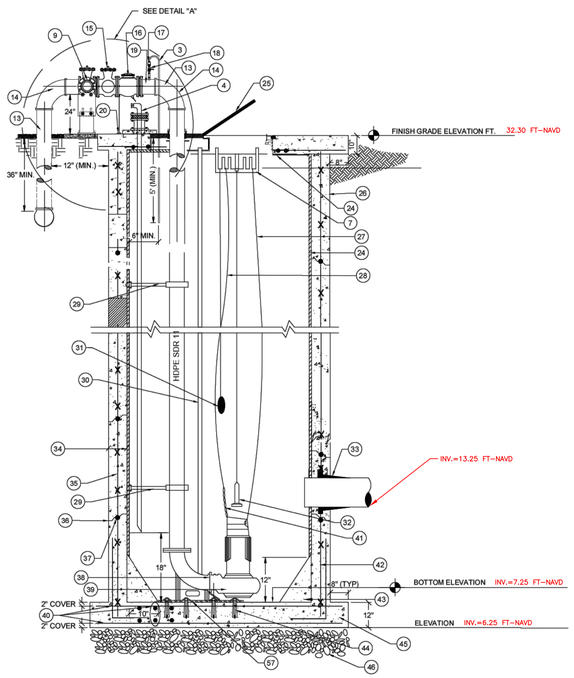
NOTES:

CONTRACTOR TO SET PUMPS AND ACCESS COVER TO ENSURE ACCURATE ALIGNMENT OF GUIDE RAILS.

CONDUIT FOR PUMP POWER CABLES & FLOAT SWITCH CABLES, PLACEMENT IS TO BE DETERMINED BY LOCATION OF CONTROL PANEL.

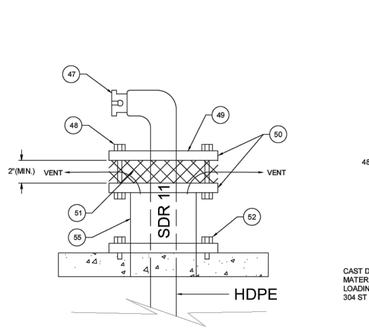


DETAIL "A"

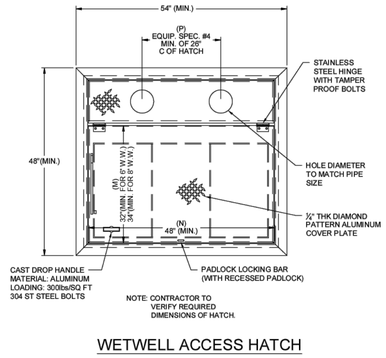


SECTION

DATUM: NAVD'88

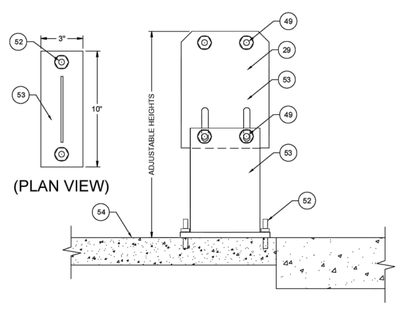


EMERGENCY PUMP OUT
AND AIRVENT DETAIL



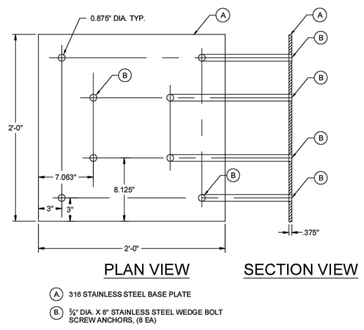
NOTE: CONTRACTOR TO
VERIFY REQUIRED
DIMENSIONS OF HATCH.

WETWELL ACCESS HATCH



(PLAN VIEW)

PIPE SUPPORT DETAIL
(INSTALLED AT PLUG VALVE
& PUMP OUT)



PLAN VIEW SECTION VIEW

(A) 316 STAINLESS STEEL BASE PLATE

(B) ¾" DIA. X 8" STAINLESS STEEL WEDGE BOLT SCREW ANCHORS, (8 EA)

PUMP DISCHARGE
BASEPLATE

GENERAL NOTES AND REQUIREMENTS

1. ½" THK. SERIES 300 STAINLESS STEEL HOOKS FOR HOIST CABLES
2. PLUG VALVE
3. PVC DRAIN LINE FROM AIR RELEASE VALVE TO WETWELL
4. EMERGENCY PUMP OUT AND AIRVENT (SAME DIAMETER AS DISCHARGE PIPING) (SEE DETAIL)
5. GUIDE RAILS
6. SPOOL PIECE
7. STAINLESS STEEL FLOAT CABLE SUPPORT
8. INFLUENT PIPE INSTALLED RADIALLY FROM WETWELL CENTER
9. FEMALE QUICK CONNECT COUPLER WITH CAP & PLUG VALVE
10. POWER CABLE
11. FLOAT CONTROLS
12. FILTER CLOTH WITH 4" OF COMPACTED #57 STONE
13. HDPE PIPE
14. LONG RADIUS 90° BEND
15. PLUG VALCE (PAINT BLACK)
16. SWING FLEX CHECK VALVE (PAINT BLACK)
17. 2" AUTOMATIC AIR RELEASE VALVE; (SEE LCU APPROVED MATERIAL LIST) ON EACH DISCHARGE PIPE
18. 2" STAINLESS STEEL BALL VALVE (SEE APPROVED MATERIAL LIST)
19. HDPE BRANCH SADDLE PE 3408 ASTM D3350 (BLACK) WITH FEMALE THREADED INSERT BRASS ALLOY STAINLESS COMPRESSION RING GRADE TYPE 304 OR 316
20. STAINLESS STEEL PIPE SUPPORT (SEE DETAIL)
21. NEOPRENE GASKET
22. STAINLESS STEEL FLANGE RING
23. HDPE RIM (FUSED TO HDPE PIPE)
24. LINER OR COATING (SEE LCU APPROVED MATERIAL LIST)
25. ALUMINUM WETWELL HATCH (SEE DETAIL)
26. COAL TAR EPOXY (BELOW FINISHED GRADE)
27. SENSOR CABLE
28. SERIES 316 STAINLESS STEEL ½" CABLE
29. SERIES 316 STAINLESS STEEL PIPE SUPPORT
30. 2" SERIES 316 STAINLESS STEEL GUIDE RAILS
31. HIGH WATER ALARM SIGNAL ON
32. SUBMERSIBLE LEVEL TRANSMITTER
33. CAST IN PLACE FLEXIBLE PIPE CONNECTION WITH SERIES 316 STAINLESS STEEL CLAMP C-623. CHARDON RUBBER, O/E (TYP.)
34. WALL THICKNESS 8" MIN.
35. STEEL REINFORCEMENT
36. RAM-NEK BLANKET OUTER SIDE OF WETWELL
37. RAM-NEK OR EQUAL PREFORMED PLASTIC GASKET MATERIAL BETWEEN PRECAST UNITS (TYP.)
38. AUTOMATIC DISCHARGE CONNECTION TO SUIT PUMP
39. SUBMERSIBLE PUMPS
40. #5 AT 8" CENTER
41. SERIES 316 STAINLESS STEEL ⅜" CHAIN (1'-6" LONG)
42. #4 AT 8" CENTER
43. CONC. OR PORTLAND TYPE II & SAND MIX
44. ⅜" DIA. X 8" STAINLESS STEEL WEDGE BOLT SCREW ANCHORS, 8 EA REFER TOLCU STANDARD DETAIL
45. BASE SECTION TO BE A MONOLITHIC CASTING
46. 12" MIN COMPACTED #57 STONE
47. CAM LOCK COUPLING ON EMERGENCY PUMP OUT
48. STAINLESS STEEL BOLTS
49. BLIND FLANGE
50. STAINLESS STEEL CLAMP
51. STAINLESS STEEL SCREEN CLOTH
52. STAINLESS STEEL CONCRETE BOLTS
53. ⅜" STAINLESS STEEL
54. 4" THICK 3,000 P.S.I. CONCRETE SLAB
55. AIR VENT TO BE 2PIPE SIZES LARGER THAN EMERGENCY PUMP OUT. THE ACTUAL LOCATION IS TO BE DETERMINED IN THE FIELD.
56. LCU PREFERS THE LIFT STATION FENCE TO HAVE DARK GREEN VINYL COATING. HOWEVER, BLACK VINYL IS ACCEPTABLE.
57. PUMP DISCHARGE BASE PLATE (SEE DETAIL)

NUMBER OF PUMPS	# 1	# 2	# 3	
PUMP MANUFACTURER	FLYGT	FLYGT		
PUMP MODEL NUMBER	3127 SH3 -248	3127 SH3 -248		
IMPELLER NUMBER	155 MM	155 MM		
TYPE OF PUMP	SUBMERSIBLE			
DESIGN CAPACITY PER PUMP	150 GPM	150 GPM		(GPM)
TOTAL DYNAMIC HEAD	118 FT	118 FT		(FEET)
SHUT-OFF HEAD	157 FT	157 FT		(FEET)
DESIGN SPEED	3500 RPM	3500 RPM		(PRM)
MIN. HORSEPOWER PER PUMP	11 HP	11 HP		(HP)
VOLTS	480 V	480 V		(VOLTS)
PHASE POWER	3 PHASE	3 PHASE		(PH)
PUMP DISCHARGE SIZE	3 IN	3 IN		(INCHES)
DIAMETER RISER PIPE	4 IN	4 IN		(INCHES)
PUMP ON ELEVATION	10.45 FT	11.95 FT		(FEET)
HIGH WATER ALARM	12.45 FT			(FEET)
PUMP OFF ELEVATION	9.45 FT			(FEET)
DIAMETER PIPE AFTER RISER	4 IN			(INCHES)
DIAMETER WETWELL (I.D.)	6 FT			(FEET)
TOP OF WETWELL ELEVATION	32.30 FT-NAVD			(FEET)
BOTTOM ELEVATION	7.25 FT-NAVD			(FEET)
INFLUENT GRAVITY PIPE ELEVATION	13.25 FT-NAVD			(FEET)

BASIS OF DESIGN			
	ADF (gpm)	PHF (gpm)	EDU (1 EDU=250 gpd)
EXISTING	0.0	0.0	0.0
INTERIM	0.0	0.0	0.0
FUTURE	43.90 GPM	131.70 GPM	63,250 GPD (1EDU/250 GPD) =253 EDU

NOTE: FUTURE INCLUDES ESTIMATED FLOWS FOR THE BUILDOUT OF PROPOSED COMMERCIAL USE AREA A PER MCP IN LEE COUNTY ZONING ORD Z-23-040 AND ESTIMATED FLOWS FOR INDUSTRIAL USE AREA B.

Proposed LS Pump

NP 3127 SH 3~ Adaptive 248

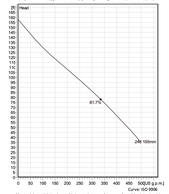
Patented self-cleaning semi-open channel impeller, ideal for pumping in waste water applications. Modular based design with high adaptation grade.

FLYGT a xylem brand

Technical specification

Curves according to: Water, pure Water, pure (100%),39.2 °F,62.42 lb/ft³,1.68916 ft²/s





Configuration

Motor number: N3127.060 21-11-24A/ W 17hp
Installation type: P - Semi permanent, Wet
Impeller diameter: 155 mm
Discharge diameter: 3 inch

Pump information

Impeller diameter: 155 mm
Discharge diameter: 3 inch
Inlet diameter: 100 mm
Maximum operating speed: 3500 rpm
Number of blades: 2
Max. fluid temperature: 40 °C

Material

Impeller: Hard-iron ™
Stator housing material: Grey cast iron

Project: Xylect-20620271
Block
Created by
Created on: 9/16/2024 Last update: 9/16/2024

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

LIFT STATION DETAILS 1

BLACKSTONE CORNE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE AT THE TIME. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF THE FACILITIES ARE CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

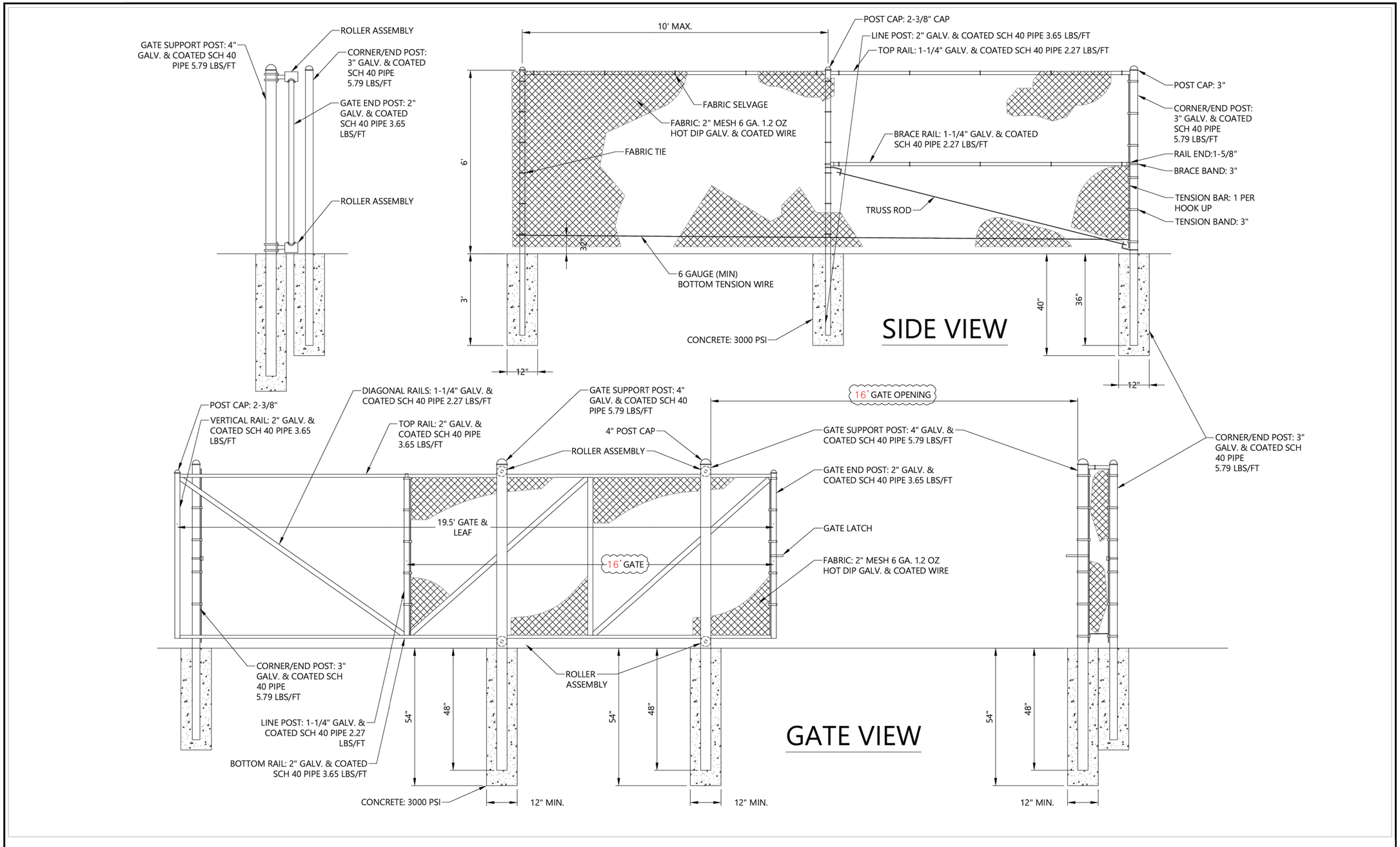
SHEET 23

NOT TO SCALE

March 20, 2024 7:50 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\LS-01.dwg

TITLE: LIFT STATION DETAIL
REVISION: 3/20/24 DRAWING NO: LS-01.dwg

I:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\23 LIFT STATION DETAILS.dwg, 2/9/2025 11:50:49 AM, VIVIAN ZAATOUNOU, ATWELL, LLC



NOT TO SCALE

March 20, 2024 7:51 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\LS-02.dwg

TITLE:	LIFT STATION FENCING DETAIL
REVISION: 3/20/24	DETAIL NO: LS-02.dwg

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

LIFT STATION DETAILS 2

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
3	02/06/2026	PER COUNTY COMMENTS	VZ

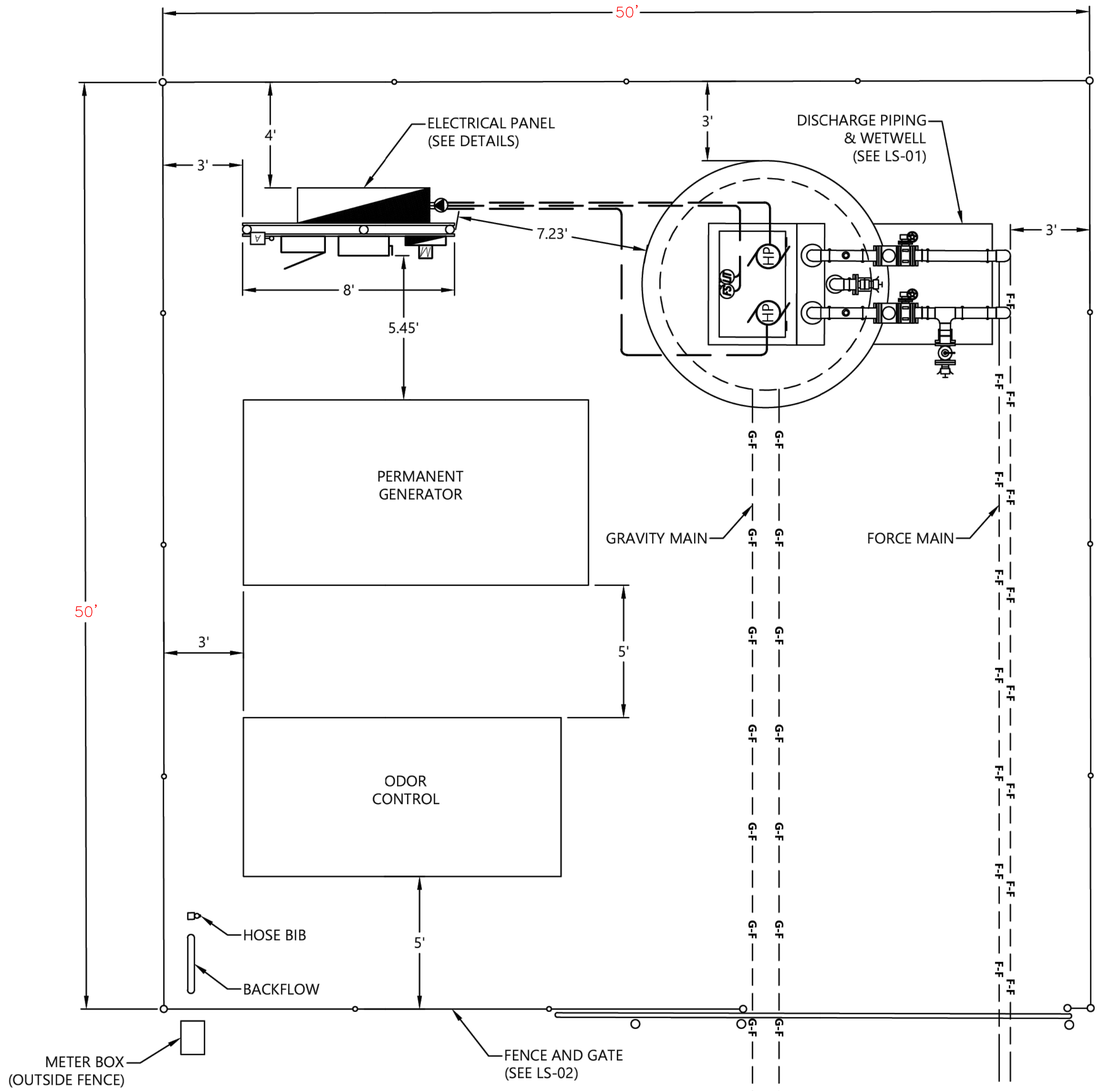
811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 24



NOT TO SCALE

March 20, 2024 7:50 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\LS-04.dwg

TITLE: STANDARD 50'X50' LIFT STATION LAYOUT

REVISION: 3/20/24 LS-04 DETAIL NO: LS-04.dwg

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

LIFT STATION DETAILS 3

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
1	07/11/2025	PER COUNTY & FGUA COMMENTS	VZ

811 Know what's below. Call before you dig.

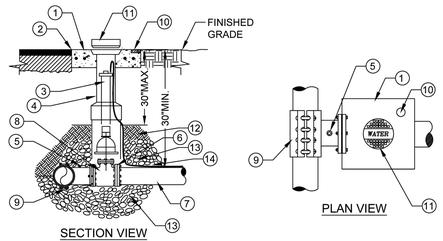
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.

P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 25



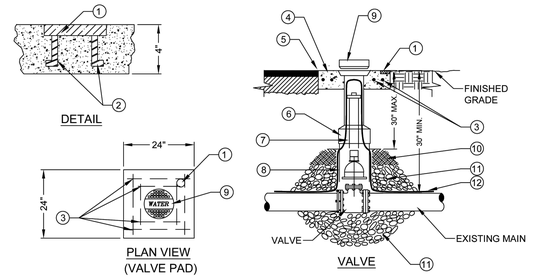
SECTION VIEW

1. CONCRETE PAD - 24"X24"X4" THK. (SEE VALVE INSTALLATION DETAIL G-02)
2. SET TOP OF BOX FLUSH WITH FINISHED GRADE
3. EXTENSION STEM IF TOP OF OPERATING NUT IS MORE THAN 30" BELOW FINISH GRADE
4. VALVE BOX (SEE VALVE INSTALLATION DETAIL G-02)
5. OUTLET FOR TESTING
6. MJ RESTRAINED
7. MAIN SIZE VARIES
8. TAPPING VALVE (SEE VALVE INSTALLATION DETAIL G-02)
9. LCU APPROVED ALL STAINLESS STEEL PRESSURE RATED TAPPING SLEEVE. FOR SIZE ON SIZE TAPS A FLANGED TAPPING SLEEVE IS REQUIRED. SPRING LOADED TAPPING SLEEVE IS REQUIRED WHEN CONNECTING TO HDPE (SEE APPROVED MATERIAL LIST)
10. BRASS PLATE (SEE VALVE INSTALLATION DETAIL G-02)
11. CAST IRON DROP COVER MARKED "WATER", "SEWER" OR "REUSE"
12. COMPACTED SUITABLE EARTH BACKFILL
13. ¾" GRANULAR MATERIAL #57 STONE
14. 10 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

TITLE: TAPPING SLEEVE & VALVE
REVISION: 1/26/24 DETAIL NO: G-01.dwg

NOT TO SCALE

January 26, 2024 6:29 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\G-01.dwg

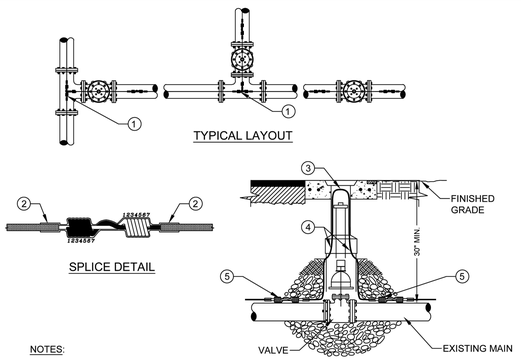


1. BRASS PLATE: SIZE OF VALVE, VALVE TYPE NO. OF TURNS & DIRECTION TO OPEN VALVE MFG & YEAR INSTALL SYSTEM "WATER" OR "SEWER" OR "REUSE" OR "FIRE"
2. ANCHOR
3. EIGHT (8) #4 BARS OVERLAP EACH CORNER BY 2"
4. 24" SQ X 4" THK. CONC. PAD SURROUNDING BOX, MIN. 3000 P.S.I. POURED IN PLACE
5. SET TOP OF BOX FLUSH WITH FINISHED GRADE
6. HEAVY DUTY TRAFFIC BEARING CAST IRON VALVE BOX, ADJUSTABLE SCREW TYPE, 5 ¼" DIAMETER SHAFT THAT IS LCU APPROVED
7. EXTENSION STEM WITH 2" OPERATING NUT AS REQUIRED, IF NUT IS MORE THAN 30" BELOW FINISH GRADE.
8. RISER NOT TO BEAR ON VALVE OR PIPE (RISER PIPE MATERIAL MUST BE PVC)
9. CAST IRON DROP COVER MARKED "WATER" OR "SEWER" OR "REUSE" OR "FIRE"
10. COMPACTED SUITABLE EARTH BACKFILL
11. ¾" GRANULAR MATERIAL #57 STONE
12. 10 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU DETAIL G-03)

TITLE: GATE VALVE
REVISION: 1/26/24 DETAIL NO: G-02.dwg

NOT TO SCALE

January 26, 2024 10:28 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\G-02.dwg



NOTES:

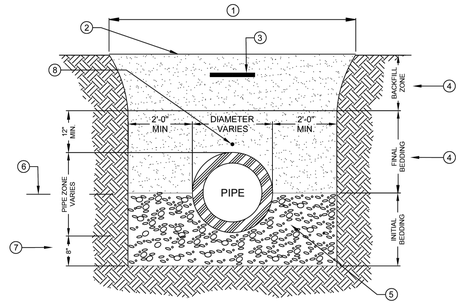
1. WIRE SHALL BE CONTINUOUS BETWEEN VALVE BOXES. THE PVC RISER PIPE SHALL BE NOTCHED AND THE WIRE ATTACHED TO THE RISER PIPE TO ALLOW FOR THE WIRE TO FIT INSIDE THE CAST IRON VALVE BOX WITHOUT SHEARING THE WIRE.
2. LOCATING WIRE SHALL BE LAID ON TOP OF THE MAIN, AND SHALL BE TAPED TO IT AT 10' INTERVALS AND TAPED AT ALL FITTINGS. TAPE SHALL BE 10 MIL POLYETHYLENE.
3. CONTRACTOR SHALL CONDUCT A CONTINUITY TEST ON ALL LOCATING WIRE SPLICES. PRIOR TO SCHEDULING THE OFFICIAL FINAL INSPECTION WITH LCU.
4. ALL SPLICES SHALL UTILIZE A DIRECT BURY WATERPROOF SPLICE KIT. (SEE APPROVED MATERIAL LIST)

1. SPLICE RUN TOGETHER, KEEP CLEAR OF FITTING, TYPICAL
2. COVER SPLICE WITH 2 WRAPS OF 10 MIL. POLYETHYLENE TAPE
3. 6" LOOP OF BARE WIRE, 9" MAX DEPTH TO TOP OF BARE WIRE
4. 10 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE
5. SPLICE (SEE DETAIL THIS SHEET)

TITLE: LOCATING WIRE FOR PRESSURE MAINS
REVISION: 1/26/24 DETAIL NO: G-03.dwg

NOT TO SCALE

January 26, 2024 11:18 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\G-03.dwg

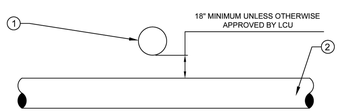


1. EXCAVATED TRENCH
2. FINISHED GRADE
3. MARKING TAPE 12" DEPTH MAX.
4. MATERIALS CLEAN DRY SAND, FINE LIME ROCK OR PIT SHELL MECHANICALLY COMPACTED IN 6" LIFTS TO AT LEAST 95% OF MAX. DENSITY, OR 98% IF UNDER PAVED AREA OF ROADWAY.
5. #57 STONE LOCATED UNDER PIPE OR EXISTING UNDISTURBED SUITABLE MATERIAL
6. SPRING LINE
7. STANDARD 8" MIN TRENCH UNDERCUT AND BACKFILL WITH #57 STONE COMPACTED IN 6" LIFTS ADDITIONAL UNDERCUT AND CRUSHED ROCK BEDDING FOUNDATION WHEN DIRECTED BY L.C.U. THE CONTRACTOR IS TO REMOVE UNSTABLE MATERIAL FROM THE TRENCH FOUNDATION. THEN INSTALL STABILIZED CRUSHED ROCK BACKFILL WITH #57 STONE COMPACTED IN 6" LIFTS
8. LOCATING WIRE IS REQUIRED FOR ALL PRESSURIZED PIPELINES.

TITLE: TRENCH CROSSING
REVISION: 1/29/24 DETAIL NO: G-05.dwg

NOT TO SCALE

January 29, 2024 7:12 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\G-05.dwg

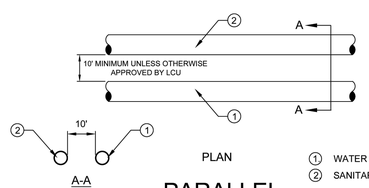


SECTION

CROSSING

1. WATER MAIN
2. SANITARY SEWER MAIN

NOTE: IF LESS THAN 18" CLEARANCE ENCASE WATER MAIN IN STEEL CASING



PLAN

PARALLEL

1. WATER MAIN
2. SANITARY SEWER MAIN

NOTE: IF LESS THAN 10' CLEARANCE ENCASE WATER MAIN IN STEEL CASING

TITLE: WATER AND SEWER CROSSING
REVISION: 6/27/24 DETAIL NO: G-08.dwg

NOT TO SCALE

June 27, 2024 6:30 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\G-08.dwg

PIPE SIZE	HORIZONTAL DEFLECTIONS 90° BENDS Lr (FT.)	45° BENDS Lr (FT.)	22.5° BENDS Lr (FT.)	11.25° BENDS Lr (FT.)	VERTICAL DEFLECTIONS 45° BENDS (SEE NOTE 2) Lhs (FT.)	Lls (FT.)	DEAD ENDS/ VALVES Lr (FT.)
4"	27	11	5	3	31	8	60
6"	37	15	7	4	43	11	84
8"	48	20	10	5	57	15	110
10"	57	24	11	6	68	17	131
12"	57	24	11	6	68	19	131
16"	58	24	12	6	55	19	106
20"	70	29	14	7	67	23	129
24"	81	33	16	8	78	27	150

REDUCERS (SEE NOTES) SIZE	L(A) (FT.)	L(B) (FT.)
6 x 4	64	43
8 x 6	60	46
8 x 4	153	79
10 x 8	55	44
10 x 6	133	81
12 x 10	46	38
12 x 8	104	69
16 x 12	61	45
16 x 10	103	63
20 x 16	58	45
20 X 12	139	81
24 x 20	55	45
24 x 16	128	82

TEES RUN SIZE	BRANCH SIZE	Lm (FT.)	Lbr (FT.)
4"	4"	10	11
6"	6" 4" OR LESS	10 10	34 FITTING ONLY
8"	8" 6" 4" OR LESS	10 10 10	59 17 FITTING ONLY
10"	10" 8" 6" OR LESS	10 10 10	80 45 FITTING ONLY
12"	12" 10" 8" 6" OR LESS	10 10 10 10	79 49 15 FITTING ONLY
16"	16" 12" 10" 8" OR LESS	10 10 10 10	73 38 16 FITTING ONLY
20"	20" 16" 12" 10" OR LESS	10 10 10 10	95 63 24 FITTING ONLY
24"	24" 20" 16" 12" 10" OR LESS	10 10 10 10 10	116 87 53 10 FITTING ONLY

TABLE.

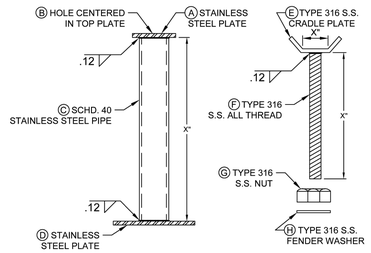
NOTES:
1. LENGTHS SHOWN IN THIS TABLE ARE TO BE INTERPRETED AS FOLLOWS:
   Lhs = RESTRAINED LENGTH FROM HIGH SIDE FITTING OF A VERTICAL DEFLECTION
   Lls = RESTRAINED LENGTH FROM LOW SIDE FITTING OF A VERTICAL DEFLECTION
   Lr = RESTRAINED LENGTH ON BOTH SIDES FROM FITTING
   Lm = MINIMUM LENGTH TO FIRST JOINT OF RUN ON BOTH SIDES
   Lbr = RESTRAINED LENGTH REQUIRED ON BRANCH FROM FITTING
   L(A) = LENGTH OF PIPE ON SMALL SIDE FREE OF FITTING
   L(B) = RESTRAINED LENGTH ON LARGE SIDE OF REDUCER
2. A SAFETY FACTOR = 2.5 IS USED FOR VERTICAL DEFLECTIONS TO COMPENSATE FOR SATURATED SOIL CONDITIONS. A SAFETY FACTOR = 2.0 IS USED FOR ALL OTHER CASES.
3. L(A) AND L(B) ABOVE ARE TWO OPTIONS FOR PROVIDING THRUST RESTRAINT FOR THE REDUCER. EITHER L(A) LENGTH OF PIPE FREE OF FITTINGS (BENDS, TEES, REDUCERS, VALVES) ON THE SMALL SIDE OF THE REDUCER OR L(B) RESTRAINED LENGTH ON THE LARGE SIDE OF THE REDUCER WILL PROVIDE THE REQUIRED RESTRAINT. NO RESTRAINT IS REQUIRED ON THE SMALL SIDE OF A REDUCER.
4. ALL DEAD END LINES OR VALVES, PERMANENT OR TEMPORARY, SHALL BE RESTRAINED PER THE ABOVE RESTRAINT

RESTRAINT TABLE

TITLE: RESTRAINED LENGTH SCHEDULE
REVISION: 6/17/24 DETAIL NO: G-10.dwg

NOT TO SCALE

June 17, 2024 1:26 PM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\G-10.dwg



X" LENGTH VARIES DEPENDING ON OBJECT BEING SUPPORTED
SEE TABLE THIS SHEET FOR DIMENSIONS A THRU H

PIPING SUPPORT

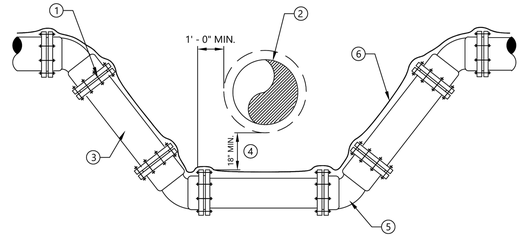
PIPING SUPPORT DIMENSIONS	FOR 3/4" & 1"	FOR 1-1/2" TO 3"	FOR 4" & LARGER
A	1/4" X 3" X 3"	1/4" X 3-1/4" X 3-1/4"	1/4" X 5" X 5"
B	1.0"	1.0"	1-1/2"
C	2.0"	2-1/2"	4"
D	1/4" X 6" X 6"	1/4" X 8" X 8"	1/4" X 10" X 10"
E	1/4" X 2" X 4"	1/2" X 2" X 4"	1/2" X 4" X 6"
F	3/4"	7/8"	1-1/4"
G	3/4"	7/8"	1-1/4"
H	1.0"	1.0"	1-1/2"

NOTE:
1. HOT DIPPED GALVANIZED STEEL MAY BE USED IN PLACE OF STAINLESS STEEL.

TITLE: PIPING SUPPORT
REVISION: 4/4/24 DETAIL NO: G-11.dwg

NOT TO SCALE

April 4, 2024 6:43 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\G-11.dwg



1. LCU APPROVED JOINT RESTRAINT FOR D.I., M.J. PIPE AND FITTINGS PLACED IN ACCORDANCE WITH JOINT RESTRAINT SCHEDULE (SEE DETAIL G-10).
2. CONFLICT PIPE
3. LCU APPROVED DUCTILE IRON PIPING PRESSURE CLASS 350
4. 18" MIN. UNLESS OTHERWISW APPROVED BY LCU
5. 45° MECHANICAL JOINT FITTING, TYPICAL
6. 10 GAUGE DOUBLE INSULATED COPPER WIRE (SEE LCU STANDARD DETAIL)

TITLE: WATER PRESSURE LINE VERTICAL OFFSET W/DIP & MJ FITTINGS
REVISION: 2/27/24 DETAIL NO: W-01.dwg

NOT TO SCALE

February 27, 2024 8:54 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\W-01.dwg

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

UTILITY DETAILS 1

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE AT THE TIME. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND NUMBER OF FACILITIES IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

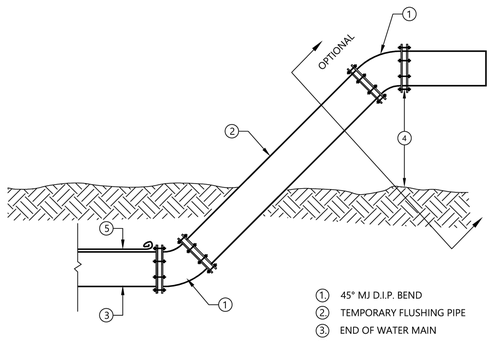
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA

PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88

BOOK AND PAGE:

SHEET 26

e:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\26 UTILITY DETAILS.dwg, 2/5/2026 11:51:10 AM, VIVIAN ZALEWSKA, ATWELL, LLC



1. 45° MJ D.I.P. BEND
2. TEMPORARY FLUSHING PIPE
3. END OF WATER MAIN
4. 24" MIN.

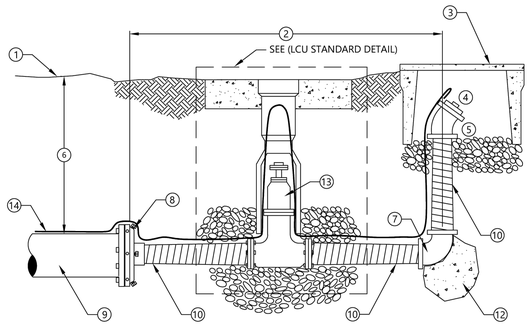
NOTES:

1. FLUSHING DETAIL TO BE USED ON WATER LINES 8" DIAMETER OR GREATER.
2. FLUSHING PIPE SHALL BE THE SAME DIAMETER AS WATER MAIN.
3. CONTRACTOR SHALL PROVIDE ADEQUATE DRAINAGE FOR FLUSHED WATER.
4. PROVIDE M.J. CAP IF ASSEMBLY IS LEFT IN PLACE UNATTENDED.
5. 10 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

NOT TO SCALE

TITLE:	WATER MAIN FLUSHING DETAIL	
REVISION:	2/27/24	DETAIL NO: W-02.dwg

February 27, 2024 9:30 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\W-02.dwg



1. FINISHED GRADE
2. 5'-0" MAX.
3. WATER METER BOX W/GRAVEL BOTTOM (SEE APPROVED MATERIAL LIST)
4. 2" THREADED PLUG (BRASS)
5. 2" - 45° BEND (BRASS)
6. 30" MIN."
7. 2" - 90° BEND (BRASS)
8. M.J. CAP WITH 2" TAP
9. END OF WATER MAIN, R.J. PER SCHEDULE
10. 2" THREADED BRASS NIPPLE
11. NOT USED
12. POURED IN PLACE THRUST BLOCKING (SEE DETAIL X-XX)
13. 2" IRON BODY GATE VALVE, THREADED
14. 10 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

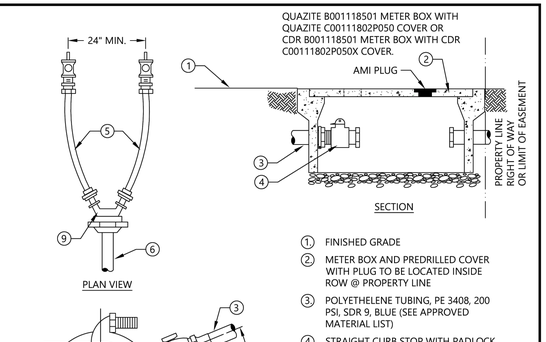
NOTE:

ALL PIPING TO BE 2" THREADED FITTINGS OR SOLVENT

NOT TO SCALE

TITLE:	BLOW OFF DETAIL	
REVISION:	2/27/24	DETAIL NO: W-03.dwg

February 27, 2024 11:11 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\W-03.dwg



NOTES:

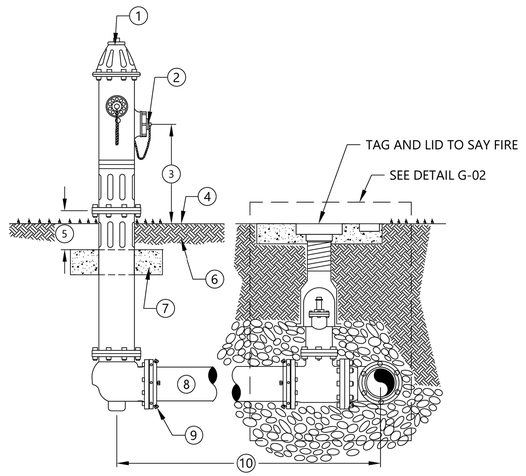
1. TUBING UNDER ROADWAY SHALL BE ENCASED IN 2" P.V.C. PIPE SCHEDULE 40 MINIMUM AND EXTEND 4 FEET BEYOND THE EDGES OF PAVEMENT OR BACK OF CURB.
2. ANY SERVICE TAPS THAT ARE APPROVED WITHIN PAVEMENT SHALL INCLUDE THE USE OF A 2" CAST IRON GATE VALE IN LIEU OF THE CORPORATION STOP.

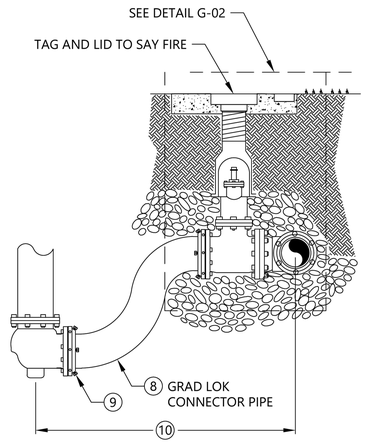
1. FINISHED GRADE
2. METER BOX AND PREDRILLED COVER WITH PLUG TO BE LOCATED INSIDE ROW @ PROPERTY LINE
3. POLYETHELENE TUBING, PE 3408, 200 PSI, SDR 9, BLUE (SEE APPROVED MATERIAL LIST)
4. STRAIGHT CURB STOP WITH PADLOCK WINGS CONNECTED TO LEE COUNTY WATER METER (SEE APPROVED MATERIAL LIST)
5. ¾" POLYETHELENE, PE 3408, 200 PSI, SDR 9, BLUE (SEE APPROVED MATERIAL LIST) WITH LOCATING WIRE
6. 1" POLYETHELENE TUBING, PE 3408, SDR 9, BLUE (SEE APPROVED MATERIALS LIST) WITH LOCATING WIRE
7. MIN. 22 ½° NOT TO EXCEED 45°
8. BALL CORPORATION STOP (SEE APPROVED MATERIAL LIST)
9. BRASS "Y" OR "T" CONNECT (SEE APPROVED MATERIAL LIST)
10. DOUBLE STRAP SERVICE SADDLE PROVIDE SPRING LOADED TAPPING SADDLE WHEN TAPPING HDPE (SEE APPROVED MATERIAL LIST)
11. WATER MAIN

NOT TO SCALE

TITLE:	WATER SERVICE INSTALL 5/8" THROUGH 1"	
REVISION:	2/28/24	DETAIL NO: W-05.dwg

February 28, 2024 6:32 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\W-05.dwg



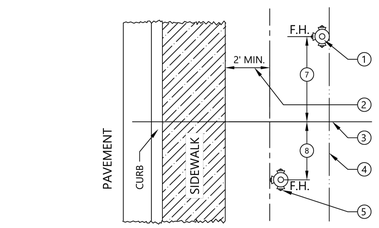


1. FIRE HYDRANT: STANDARD A.W.W.A. SAFETY YELLOW (SEE APPROVED MATERIAL LIST)
2. PUMPER CONNECTION FACING AS PER FIRE DISTRICT RECOMMENDATION
3. 18" MIN. - 24" MAX.
4. FINISHED GRADE
5. 6" MIN. CLEARANCE
6. COMPACTED FILL
7. 24" X 2" X 4" CONCRETE BREAK AWAY CONCRETE PAD, 3000 P.S.I.
8. PIPE TO BE D.I.P., PRESSURE CLASS 350 OR GRADE LOK CONNECTOR PIPE WITH SPLIT GLAND MECHANICAL RESTRAINED JOINT & FITTINGS
9. RESTRAINED JOINTS, FROM HYDRANT TO WM
10. VARIABLE DISTANCE, IF GREATER THAN 100' A VALVE SHALL BE INSTALLED AT HYDRANT BASE

NOT TO SCALE

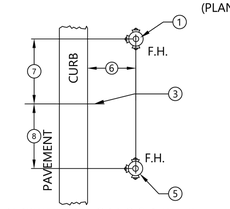
TITLE:	FIRE HYDRANT ASSEMBLY	
REVISION:	3/6/24	DETAIL NO: W-12.dwg

March 6, 2024 7:05 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\W-12.dwg



PARKWAY AREA WITH SIDEWALK

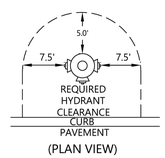
(PLAN VIEW)



PARKWAY AREA OR NO SIDEWALK

(PLAN VIEW)

1. PREFERRED LOCATION
2. F.H. TO BE LOCATED 2' MINIMUM FROM EDGE OF SIDEWALK
3. P.T. OR P.C. OF CURB RETURN
4. PROPERTY LINE RIGHT-OF-WAY OR LIMIT OF EASEMENT
5. ACCEPTABLE LOCATION WITH CURB RADIUS OF 20' OR MORE
6. BETWEEN 5'-0" MIN. AND 10'-0" MAX.
7. 6'-0"
8. 4'-0"



(PLAN VIEW)

NOTES:

1. OBSTRUCTIONS SUCH AS UTILITY POLES, STREET SIGNS, IRRIGATION BOXES, FENCES, ETC. SHALL NOT BE PLACED BETWEEN CURB AND HYDRANT.
2. SOME LOCATIONS APPLY AT EITHER END OF CURB RETURNS.
3. DIMENSIONS SHOWN ON CONSTRUCTION DRAWINGS SUPERCEDE LOCATIONS SHOWN HERE.

NOT TO SCALE

TITLE:	WATER PRESSURE LINE VERTICAL OFFSET W/DIP & MJ FITTINGS	
REVISION:	3/7/24	DETAIL NO: W-13.dwg

March 7, 2024 6:51 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\W-13.dwg

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

UTILITY DETAILS 2

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's **below.** **Call** before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE AT THE TIME. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND NUMBER OF LINES ARE CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

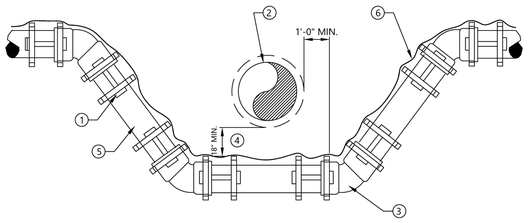
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA

PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88

BOOK AND PAGE:

SHEET 27

c:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\26 UTILITY DETAILS.dwg, 2/5/2026 11:51:19 AM, VIVIAN ZALEWSKA, ATWELL, LLC



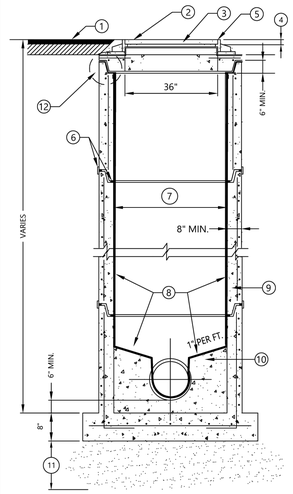
WASTE WATER FORCE MAIN

1. LCU APPROVED JOINT RESTRAINT FOR P.V.C. M.J. PIPE AND FITTINGS PLACED IN ACCORDANCE WITH JOINT RESTRAINT SCHEDULE. (SEE DETAIL G-10)
2. CONFLICT PIPE
3. LCU APPROVED C-900 PVC DR 14
4. 18" MIN. UNLESS OTHERWISE APPROVED BY LCU
5. 45° MECHANICAL JOINT FITTING, TYPICAL
6. 10 GAUGE DOUBLE INSULATED COPPER WIRE (SEE LCU STANDARD DETAIL)

NOT TO SCALE

TITLE:	WASTE WATER FM VERTICAL OFFSET WITH FITTINGS	
REVISION: 3/7/24	DETAIL NO:	S-01.dwg

March 7, 2024 10:42 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-01.dwg



1. GRADE
2. GROUT TYP.
3. SEE MASTER MANHOLE COVER DETAIL, S-03
4. SET TOP OF COVER 2 ½" ABOVE FINISHED GRADE IN UNPAVED AREAS
5. SET TOP OF COVER FLUSH WITH PAVEMENT IN PAVED AREA
6. ALL JOINTS SHALL BE SEALED ON INTERIOR & EXTERIOR WITH A FLEXIBLE PREFORMED BITUMASTIC JOINT MATERIAL. WRAP TAPE AROUND OUTSIDE OF MANHOLE JOINTS. (SEE APPROVED MATERIAL LIST)
7. 6' DIA. PER DISCRETION OF LEE COUNTY UTILITIES
8. INTERIOR COATING/LINER (SEE APPROVED MATERIAL LIST) SYSTEM TO COVER ENTIRE BENCH AND INVERT CHANNEL
9. REINFORCED PRECAST CONCRETE CLASS "A" TYPE II 4,000 PSI
10. POURED CONCRETE. NO BRICK OR GRAVEL
11. 12" CRUSHED ROCK BASE
12. KEYED JOINT (TYP)

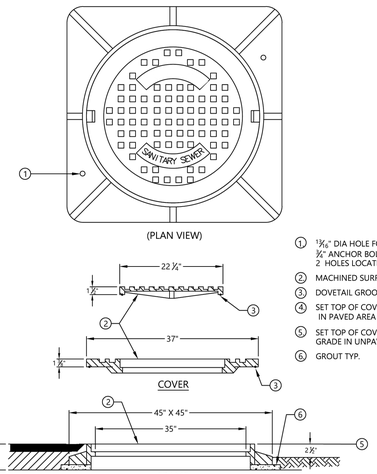
NOTES:

1. MASTER MANHOLES FOR WASTEWATER PUMPING STATIONS SHALL HAVE ONLY ONE INFLUENT AND ONE EFFLUENT PIPE, UNLESS OTHERWISE APPROVED BY LEE COUNTY UTILITIES.
2. FLEXIBLE RESILIENT CONNECTIONS SHALL BE INSTALLED FOR ALL CONNECTIONS BETWEEN MANHOLE AND PIPES.

NOT TO SCALE

TITLE:	MASTER PRE-CAST MANHOLE	
REVISION: 3/7/24	DETAIL NO:	S-02.dwg

March 7, 2024 10:45 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-02.dwg



(PLAN VIEW)

(SECTION VIEW)

COVER

FRAME

FOR TRAFFIC AREAS OR IN ROAD RIGHTS OF WAY

1. 1⅜₁₆" DIA HOLE FOR ½" ANCHOR BOLT 2 HOLES LOCATED 180° APART
2. MACHINED SURFACE
3. DOVETAIL GROOVE & "O" RING GASKET
4. SET TOP OF COVER FLUSH WITH PAVEMENT IN PAVED AREA
5. SET TOP OF COVER 2 ½" ABOVE FINISHED GRADE IN UNPAVED AREAS
6. GROUT TYP.

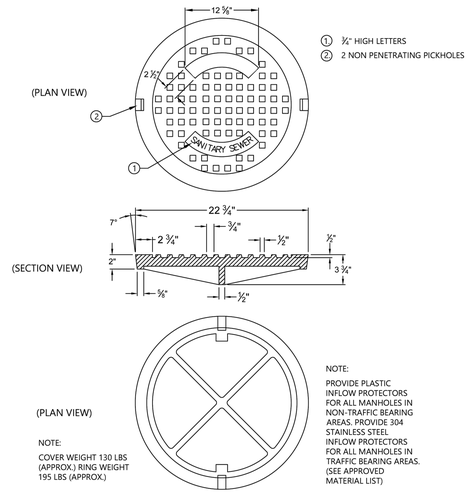
NOTE:

MANHOLE COVERS ARE NOT TO BE CAST WITH THE WORDS LEE COUNTY ON THEM.

NOT TO SCALE

TITLE:	MASTER MANHOLE COVER	
REVISION: 3/7/24	DETAIL NO:	S-03.dwg

March 7, 2024 2:15 PM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-03.dwg



(PLAN VIEW)

(SECTION VIEW)

(PLAN VIEW)

1. ¾" HIGH LETTERS
2. 2 NON PENETRATING PICKHOLES

NOTE:

COVER WEIGHT 130 LBS (APPROX.) RING WEIGHT 195 LBS (APPROX.)

NOTE:

PROVIDE PLASTIC INFLOW PROTECTORS FOR ALL MANHOLES IN NON-TRAFFIC BEARING AREAS. PROVIDE 304 STAINLESS STEEL INFLOW PROTECTORS FOR ALL MANHOLES IN TRAFFIC BEARING AREAS. (SEE APPROVED MATERIAL LIST)

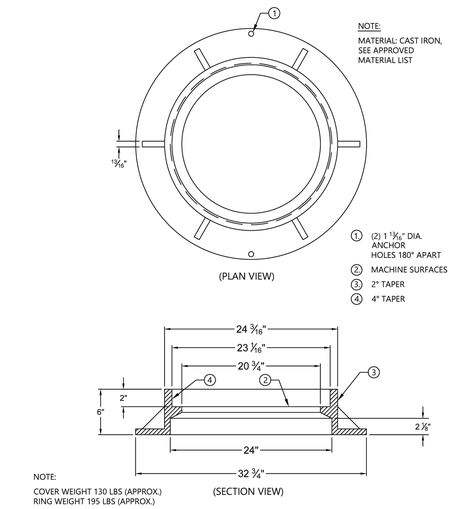
NOTE:

MANHOLE COVERS ARE NOT TO BE CAST WITH THE WORDS LEE COUNTY ON THEM.

NOT TO SCALE

TITLE:	MANHOLE FRAME COVER	
REVISION: 3/8/24	DETAIL NO:	S-04.dwg

March 8, 2024 6:19 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-04.dwg



NOTE:

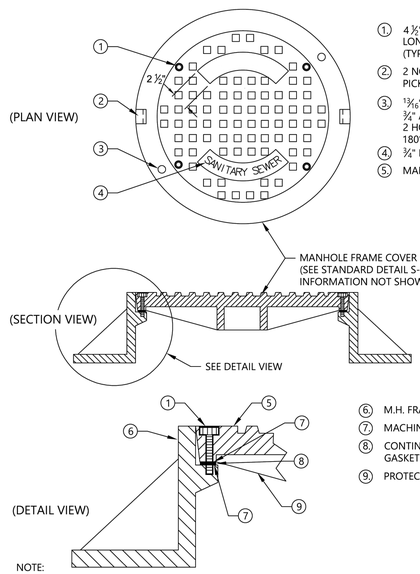
COVER WEIGHT 130 LBS (APPROX.) RING WEIGHT 195 LBS (APPROX.)

(SECTION VIEW)

NOT TO SCALE

TITLE:	MANHOLE FRAME RING	
REVISION: 3/8/24	DETAIL NO:	S-05.dwg

March 8, 2024 1:40 PM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-05.dwg



(PLAN VIEW)

(SECTION VIEW)

(DETAIL VIEW)

1. 4 ½" DIA. X 2 ½" LONG S.S. BOLTS (TYP.)
2. 2 NON PENETRATING PICKHOLES
3. 1⅜₁₆" DIA HOLE FOR ¾" ANCHOR BOLT 2 HOLES LOCATED 180° APPART
4. ¾" HIGH LETTERS
5. MANHOLE COVER

MANHOLE FRAME COVER (SEE STANDARD DETAIL S-04 FOR INFORMATION NOT SHOWN HERE)

6. M.H. FRAME RING
7. MACHINED BEARING SEAT
8. CONTINUOUS FLAT GASKET
9. PROTECTIVE SKIRT

NOTE:

COVER WEIGHT 130 LBS (APPROX.) RING WEIGHT 195 LBS (APPROX.)

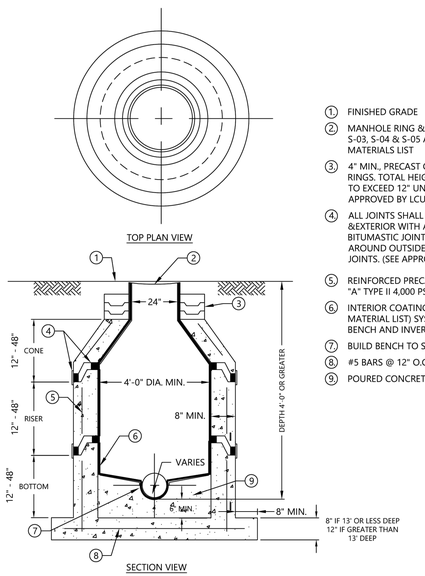
NOTE:

MANHOLE COVERS ARE NOT TO BE CAST WITH THE WORDS LEE COUNTY ON THEM.

NOT TO SCALE

TITLE:	SEALED MANHOLE FRAME RING	
REVISION: 5/30/24	DETAIL NO:	S-06.dwg

May 30, 2024 3:05 PM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-06.dwg



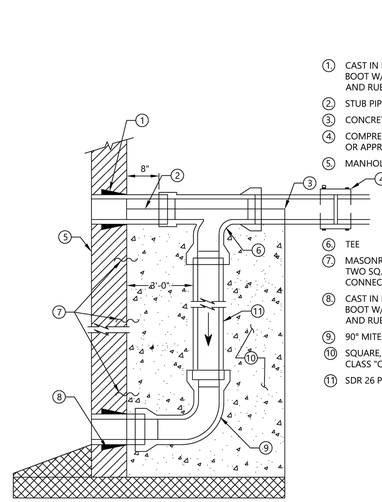
SECTION VIEW

1. FINISHED GRADE
2. MANHOLE RING & COVER (SEE DETAILS S-03, S-04 & S-05 AND APPROVED MATERIALS LIST
3. 4" MIN., PRECAST CONCRETE ADJUSTING RINGS. TOTAL HEIGHT OF CHIMNEY NOT TO EXCEED 12" UNLESS OTHERWISE APPROVED BY LCU
4. ALL JOINTS SHALL BE SEALED ON INTERIOR &EXTERIOR WITH A FLEXIBLE PREFORMED BITUMASTIC JOINT MATERIAL. WRAP TAPE AROUND OUTSIDE OF MANHOLE AT ALL JOINTS. (SEE APPROVED MATERIAL LIST)
5. REINFORCED PRECAST CONCRETE CLASS "A" TYPE II 4,000 PSI
6. INTERIOR COATING/LINER (SEE APPROVED MATERIAL LIST) SYSTEM TO COVER ENTIRE BENCH AND INVERT CHANNEL
7. BUILD BENCH TO SPRING LINE OR ABOVE
8. #5 BARS @ 12" O.C. E.W. TYPICAL
9. POURED CONCRETE. NO BRICK OR GRAVEL

NOT TO SCALE

TITLE:	MASTER PRE-CAST MANHOLE	
REVISION: 3/21/24	DETAIL NO:	S-07.dwg

March 21, 2024 7:10 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-07.dwg



1. CAST IN PLACE FLEX PIPE CONNECTION BOOT W/SERIES 300 S.S. CLAMP C-923 AND RUBBER BOOT
2. STUB PIPE
3. CONCRETE TO SPRING LINE OF PIPE
4. COMPRESSION COUPLING CONNECTION OR APPROVED EQUAL
5. MANHOLE WALL
6. TEE
7. MASONRY ANCHORS MIN. ONE TIE PER TWO SQ. FT. OF CONTACT AREA FOR DROP CONNECTIONS TO EXISTING M.H.S. ONLY
8. CAST IN PLACE FLEX PIPE CONNECTION BOOT W/SERIES 300 S.S. CLAMP C-923 AND RUBBER BOOT
9. 90° MITERED BEND
10. SQUARE, CONCRETE ENCASEMENT CLASS "C"
11. SDR 26 PVC PIPE

NOT TO SCALE

TITLE:	MANHOLE 4' DROP OR GREATER	
REVISION: 3/21/24	DETAIL NO:	S-09.dwg

March 21, 2024 9:53 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-09.dwg

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE 101
FORT MYERS, FL 33966
239.938.5490

UTILITY DETAILS 3

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:

CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

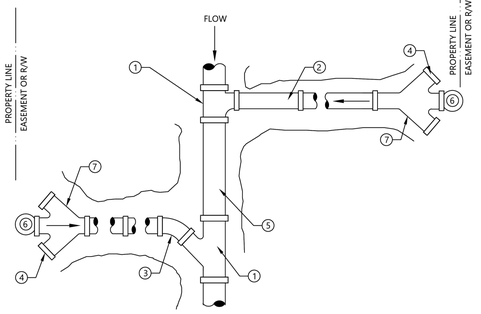
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE AT THE TIME. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND CONFIGURATION SHOWN IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 28

c:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\26 UTILITY DETAILS.dwg, 2/9/2026 11:51:24 AM, VIVIAN ZALEWSKI, ATWELL, LLC



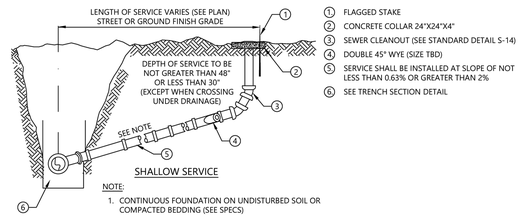
NOTE: 1. SEE SPECS FOR DETAILS OF PIPE AND FITTINGS.

1. STANDARD WYE OR TEE WYE
2. PVC SEWER SERVICE LINE
3. SHORT RADIUS 45° BEND WITH WYE FITTINGS
4. PLUG ENDS WITH HEAVY WALL PVC CAPS (TYP)
5. 5' MIN. PIPE BETWEEN FITTINGS
6. 6" OR 8" CLEANOUT
7. DOUBLE 45° WYE (SIZE TBD)

NOT TO SCALE

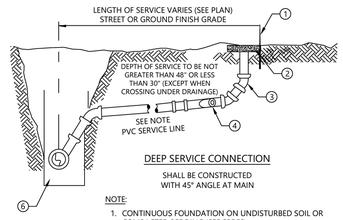
TITLE:	STANDARD SEWER SERVICE LATERAL	
REVISION: 3/27/24		DETAIL NO: S-12.dwg

March 27, 2024 6:24 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-12.dwg



SHALLOW SERVICE

NOTE:
1. CONTINUOUS FOUNDATION ON UNDISTURBED SOIL OR COMPACTED BEDDING (SEE SPECS)



DEEP SERVICE CONNECTION

SHALL BE CONSTRUCTED WITH 45° ANGLE AT MAIN

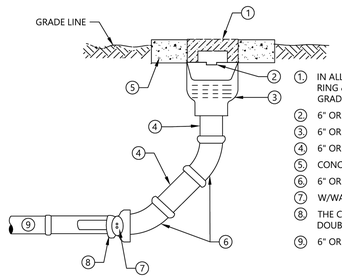
NOTE:
1. CONTINUOUS FOUNDATION ON UNDISTURBED SOIL OR COMPACTED BEDDING (SEE SPECS)

1. FLAGGED STAKE
2. CONCRETE COLLAR 24"X24"X4"
3. SEWER CLEANOUT (SEE STANDARD DETAIL S-14)
4. DOUBLE 45° WYE (SIZE TBD)
5. SERVICE SHALL BE INSTALLED AT SLOPE OF NOT LESS THAN 0.63% OR GREATER THAN 2%
6. SEE TRENCH SECTION DETAIL

NOT TO SCALE

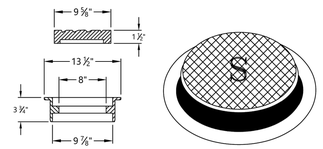
TITLE:	STANDARD SEWER SERVICE LATERAL	
REVISION: 3/27/24		DETAIL NO: S-13.dwg

March 27, 2024 7:33 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-13.dwg



1. IN ALL CASES USE TRAFFIC BEARING HANDHOLE RING & COVER OR EQUAL ADJUSTED TO FINAL GRADE WITH CLEANOUT RECESSER
2. 6" OR 8" THREADED CAP
3. 6" OR 8" PVC FEMALE ADAPTER
4. 6" OR 8" PVC RISER
5. CONCRETE COLLAR 24"X 24"X4"
6. 6" OR 8" 45° ELBOW
7. W/WATERTIGHT PLUG
8. THE CONNECTION MUST BE MADE TO THE DOUBLE 45° WYE (SIZE TBD)
9. 6" OR 8" PVC SEWER PIPE

REVERSIBLE HANDHOLE RING AND COVER



COVERS ARE MARKED WITH "S"

COVER TYPE	LOAD RATING	COVER WEIGHT	TOTAL WEIGHT
FE	HEAVY DUTY	20	45

NOT TO SCALE

TITLE:	6" TO 8" SEWER CLEANOUT	
REVISION: 4/2/24		DETAIL NO: S-14.dwg

April 2, 2024 10:46 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\S-14.dwg

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

UTILITY DETAILS 4

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's **below.** **Call** before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE AT THE TIME. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE EACH IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 29

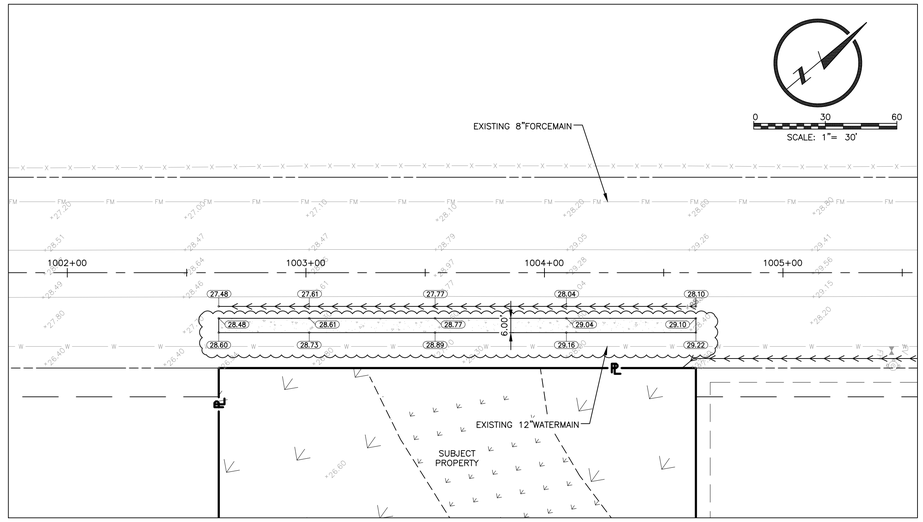
c:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\08 UTILITY DETAILS.dwg, 2/9/2026 11:51:29 AM, IVAN ZALESNOV, ATWELL, LLC

CONSTRUCTION NOTES:

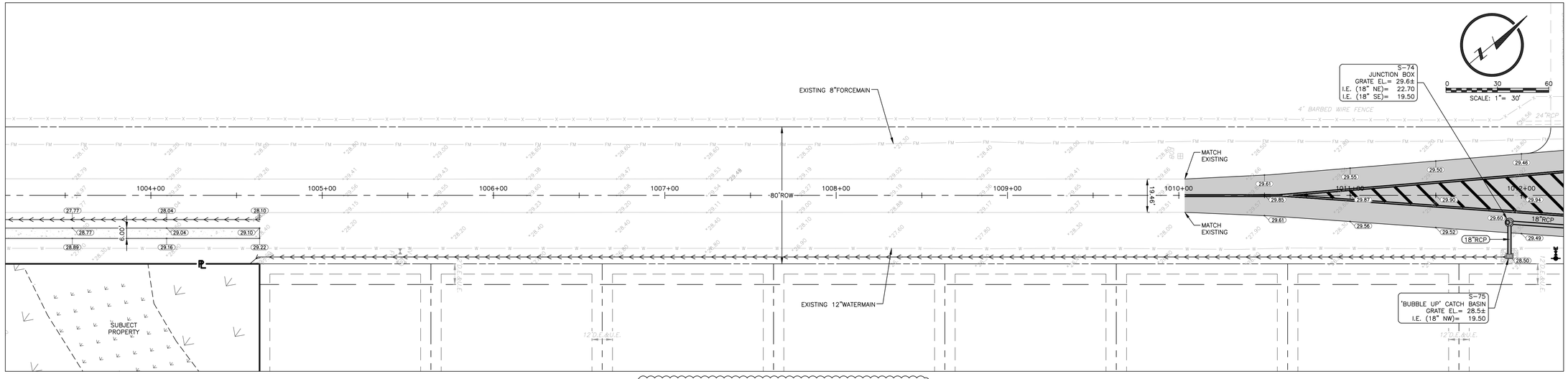
1. ALL CONSTRUCTION TO BE IN ACCORDANCE WITH FLORIDA DEPT. OF TRANSPORTATION (F.D.O.T.) STANDARD SPECIFICATIONS FOR ROAD & BRIDGE CONSTRUCTION, LATEST EDITION.
2. CONTRACTOR TO DETERMINE & VERIFY LOCATION & ELEVATION OF ANY BURIED UTILITY LINES OR CABLES. CONTRACTOR IS RESPONSIBLE FOR THE REPAIR &/OR REPLACEMENT OF ANY UTILITY LINES OR CABLES DISTRIBUTED BY CONSTRUCTION OPERATIONS.
3. THE PERMITTEE SHALL BE RESPONSIBLE FOR DAMAGE TO ANY EXISTING UTILITIES, DRAINAGE STRUCTURES, SIDEWALK/SHARED USE PATH, REGULATORY AND TRAFFIC SIGNS, TRAFFIC CONTROL DEVICES, TRAFFIC COUNT STATIONS, PULL BOXES, FIBER OPTIC CABLES, AND PAVEMENT WITHIN THE COUNTY ROW DUE TO THE PROPOSED CONSTRUCTION ACTIVITIES. THE EXISTING LEEDOT'S TRAFFIC DETECTORS AND COMMUNICATIONS INFRASTRUCTURE INCLUDING TRAFFIC COUNT STATIONS, PULL BOXES, AND FIBER OPTIC CABLE SHALL BE PROTECTED DURING THE CONSTRUCTION OF THE PROJECT. THE CONTRACTOR SHALL COORDINATE WITH THE LEEDOT TRAFFIC SECTION TO REPAIR ALL DAMAGES TO TRAFFIC SIGNAL EQUIPMENT, TRAFFIC DETECTORS, AND COMMUNICATIONS INFRASTRUCTURE WITHIN 24 HOURS OF BEING DAMAGED.
4. NO UTILITY STRUCTURES, WATER VALVES, METER BOXES, POWER POLES, OR PULL BOXES SHALL BE ALLOWED IN THE SIDEWALK/SHARED USE PATH. THE PERMITTEE SHALL COORDINATE WITH THE UTILITY AND POWER COMPANIES TO RELOCATE THE EXISTING STRUCTURES WITHIN THE SIDEWALK/SHARED USE PATH. WHEN RELOCATION IS NOT FEASIBLE, THE SURFACE FEATURE OF THE STRUCTURES IN THE LOCATION OF THE SIDEWALK/SHARED USE PATH SHALL BE ADJUSTED TO MEET THE ADA REQUIREMENTS FOR SURFACES INCLUDING NON-SLIP TOP SURFACES AND ADJUSTMENT TO BE FLUSH WITH AND AT THE SAME SURFACE SLOPE AS THE SIDEWALK/SHARED USE PATH.
5. SUBGRADE & LIMEROCK BASE COURSES TO BE COMPACTED TO 98% MIN. OF THE MAX. DRY DENSITY AS DETERMINED BY AASHTO T-180 TESTING.
6. THE CONTRACTOR SHALL SUPPLY SATISFACTORY DENSITY TESTS OF SUBGRADE & LIMEROCK BASE MATERIALS TO THE ENGINEER PRIOR TO PAYMENT &/OR FINAL ACCEPTANCE. (ONE TEST PER COURSE PER 400 SQUARE YARDS).
7. TEMPORARY TRAFFIC CONTROL (TTC) OR MAINTENANCE OF TRAFFIC (MOT) PLANS SHALL BE REVIEWED AND APPROVED IN THE ROW PERMIT REVIEW PROCESS. THE PERMITTEE/CONTRACTOR SHALL COORDINATE WITH LEEDOT STAFF TO CONDUCT CONSTRUCTION OPERATIONS TO MINIMIZE INTERFERENCE WITH, OR INTERRUPTION OF VEHICULAR, BICYCLE, OR PEDESTRIAN TRAFFIC UPON AND ALONG THE ROADWAYS. NO WORK CAN PROCEED UNTIL THE MOT PLANS ARE APPROVED BY LEEDOT. PEDESTRIAN/BICYCLE TRAFFIC ACCOMMODATION MUST BE INCLUDED AS PART OF THE MOT PLANS; ANY TEMPORARY FACILITY MUST MEET CURRENT ADA STANDARDS.
8. CONTRACTORS SHALL SOD ALL DISTURBED AREAS & OTHERWISE RESTORE ALL AREAS TO EXISTING CONDITION OR BETTER.
9. UPON COMPLETION OF ALL IMPROVEMENTS TO BLACKSTONE DRIVE AND PARKDALE BOULEVARD, THE INITIAL STRIPING SHALL BE ACCOMPLISHED BY PAINT. AFTER A 30 DAY 'CURING' PERIOD THE ROADWAY WILL THEN BE RE-STRIPED IN THERMOPLASTIC.
10. SIDEWALK AND CURB REPLACEMENT LIMITS SHALL EXTEND TO THE NEAREST CONSTRUCTION JOINT TO PREVENT SMALL SECTIONS OF EXISTING PANELS/CURB FROM BEING LEFT IN PLACE.
11. THE CONCRETE THICKNESS OF THE SIDEWALK WITHIN THE COUNTY ROW AND EASEMENT SHALL BE A MINIMUM THICKNESS OF SIX (6) INCHES. SIDEWALK SHALL BE RECONSTRUCTED WITH A CROSS SLOPE OF 2% OR LESS, AND UNLESS AT A CURB RAMP, SHALL HAVE A RUNNING SLOPE OF 5% OR LESS. ALL SIDEWALK RECONSTRUCTION MUST COMPLY WITH LATEST EDITION OF THE FLORIDA GREENBOOK AND FDOT'S DESIGN STANDARDS, INCLUDING CURB RAMPS AND DETECTABLE WARNING SURFACES.
12. AN AVERAGE DEPTH OF CUT FOR MILLING AND RESURFACING THE COUNTY ROW IS ONE AND A HALF INCHES (1.5"). HOWEVER, DEPENDING UPON THE CONDITIONS OF THE ROADWAY, A GREATER CUT DEPTH MAY BE REQUIRED AT THE PERMITTEE'S SOLE EXPENSE.
13. ASPHALT CORE SAMPLES MUST BE TAKEN AND THE MILLING DEPTH MUST BE AGREED UPON WITH LEEDOT PRIOR TO MILLING.

SIGNING & PAVEMENT MARKING NOTES:

1. ALL SIGNING & STRIPING WITHIN BLACKSTONE DRIVE AND PARKDALE BOULEVARD RIGHT-OF-WAY IS TO BE THERMOPLASTIC & IN ACCORDANCE WITH FLORIDA DEPT. OF TRANSPORTATION (F.D.O.T.) STANDARD SPECIFICATIONS FOR ROAD & BRIDGE CONSTRUCTION, LATEST EDITION.
2. ALL EXISTING PAVEMENT MARKINGS CONFLICTING WITH NEW STRIPING SHALL BE IMMEDIATELY REMOVED BY HYDROBLASTING TO ELIMINATE THE PREVIOUS MARKINGS REGARDLESS OF WEATHER AND LIGHT CONDITIONS.
3. PREFORMED THERMOPLASTIC MATERIAL PAVEMENT MARKINGS SHALL BE APPLIED FOR ALL PAVEMENT MARKINGS WITHIN THE COUNTY RIGHT-OF-WAY.
4. ANY EXISTING SIGN TO REMAIN THAT IS DISTURBED DURING CONSTRUCTION OR RELOCATED SHALL BE RESET TO CURRENT STANDARDS FOR HEIGHT, OFFSET, & METHOD OF INSTALLATION. INSTALLATION OF ALL TRAFFIC SIGNS WITHIN BLACKSTONE DRIVE AND PARKDALE BOULEVARD RIGHT-OF-WAY SHALL COMPLY WITH THE CURRENT LEE COUNTY DESIGN STANDARDS FOR SIGN INSTALLATION.
5. CAUTION SHALL BE EXERCISED WHILE RELOCATING EXISTING SIGNS SO AS TO PREVENT DAMAGE TO THE SIGNS. IF THE SIGNS ARE DAMAGED BEYOND USE, AS DETERMINED BY LEEDOT, THEY SHALL BE REPLACED BY THE CONTRACTOR AT HIS/HER EXPENSE.
6. POSTED SPEED IS 35 MPH. DESIGN SPEED IS 40 MPH.
7. CONTRACTOR TO SECURE AND COMPLY WITH LEE COUNTY RIGHT-OF-WAY USE PERMIT.
8. CROSSWALKS AND CURB RAMPS (I.E., WHEELCHAIR RAMPS) SHALL BE PROVIDED AT ALL INTERSECTIONS WHERE PEDESTRIAN WAYS INTERSECT ROADWAYS/DRIVEWAYS. CROSSWALKS AND CURB RAMPS MUST BE CONSISTENT WITH F.D.O.T. DESIGN STANDARDS. ALL DETECTABLE WARNING SURFACE SURFACES SHALL BE CAST-IN-PLACE (WET-SET AND ANCHORED IN CONCRETE).
9. INSTALLATION OF ALL TRAFFIC SIGNS WITHIN THE COUNTY ROW SHALL COMPLY WITH THE CURRENT LEE COUNTY DESIGN STANDARD FOR SIGN INSTALLATION.



BLACKSTONE DRIVE SOUTHERN PROPERTY FRONTAGE SIDEWALK IMPROVEMENT



BLACKSTONE DRIVE STORMWATER CONVEYANCE TO WETLAND

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

OFF-SITE ROADWAY IMPROVEMENT PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
2	10/17/2025	PER COUNTY COMMENTS	VZ
3	12/19/2025	PER COUNTY COMMENTS	VZ

811 Know what's below. Call before you dig.

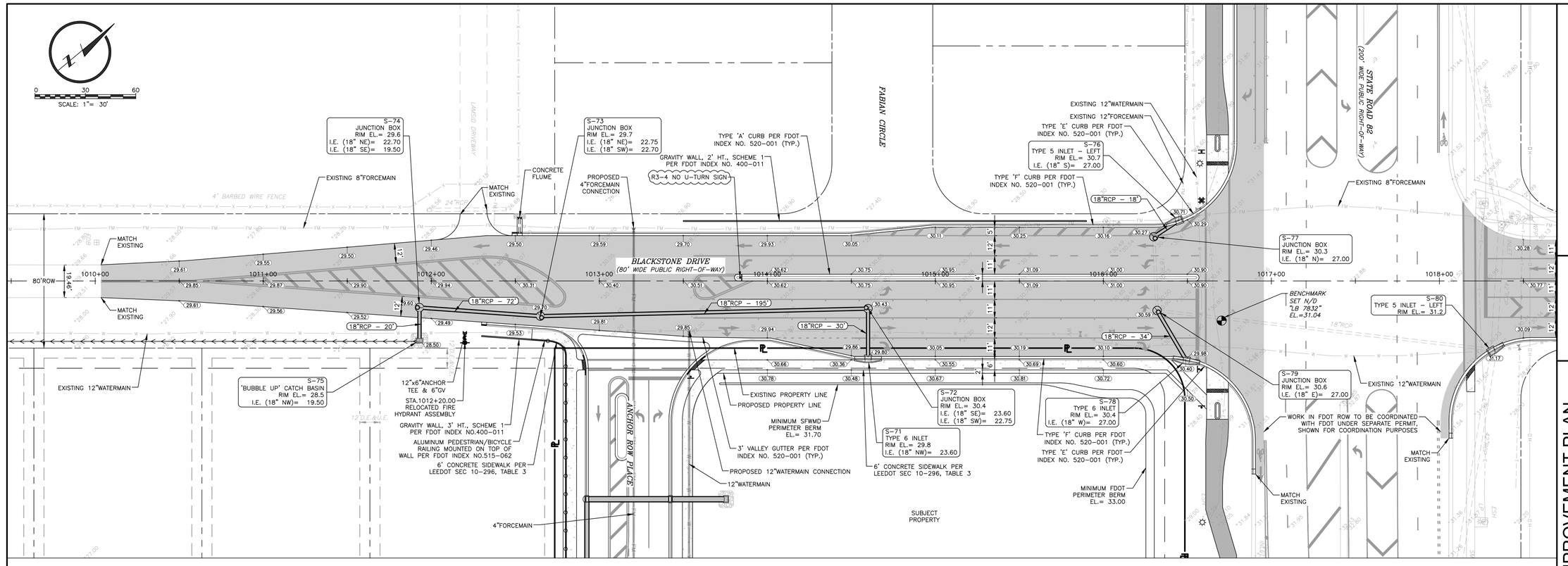
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN AND THAT THE LOCATION, DEPTH AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING UTILITIES AND PROTECTING UTILITY LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

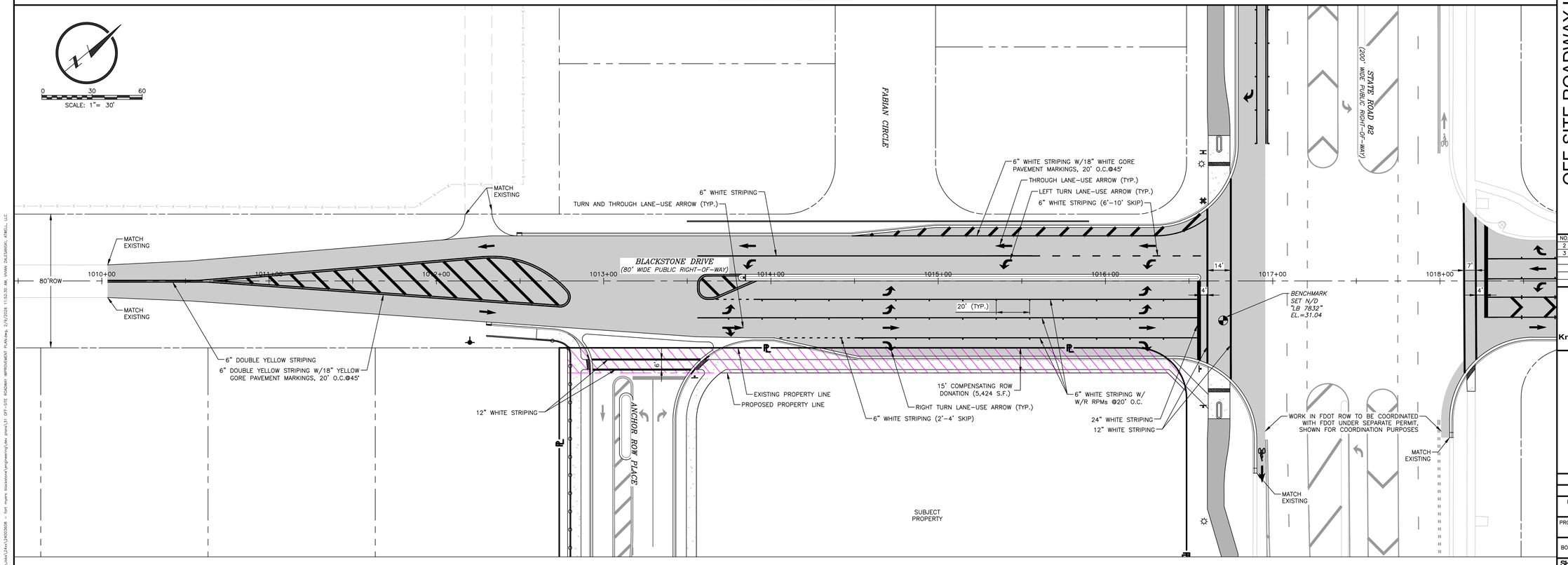
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88
BOOK AND PAGE:		

SHEET
30

n:\Projects\2024\24003608 - fort myers blackstone\Engineering\Dev plans\30 OFF-SITE ROADWAY IMPROVEMENT PLAN.dwg, 2/9/2026 11:52:03 AM, IVAN ZALESKOVSKI, ATWELL, LLC



ROADWAY IMPROVEMENT PLAN



SIGNING AND MARKING PLAN

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

OFF-SITE ROADWAY IMPROVEMENT PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
2	10/17/2025	PER COUNTY COMMENTS	VZ
3	12/19/2025	PER COUNTY COMMENTS	VZ

811 Know what's below. Call before you dig.

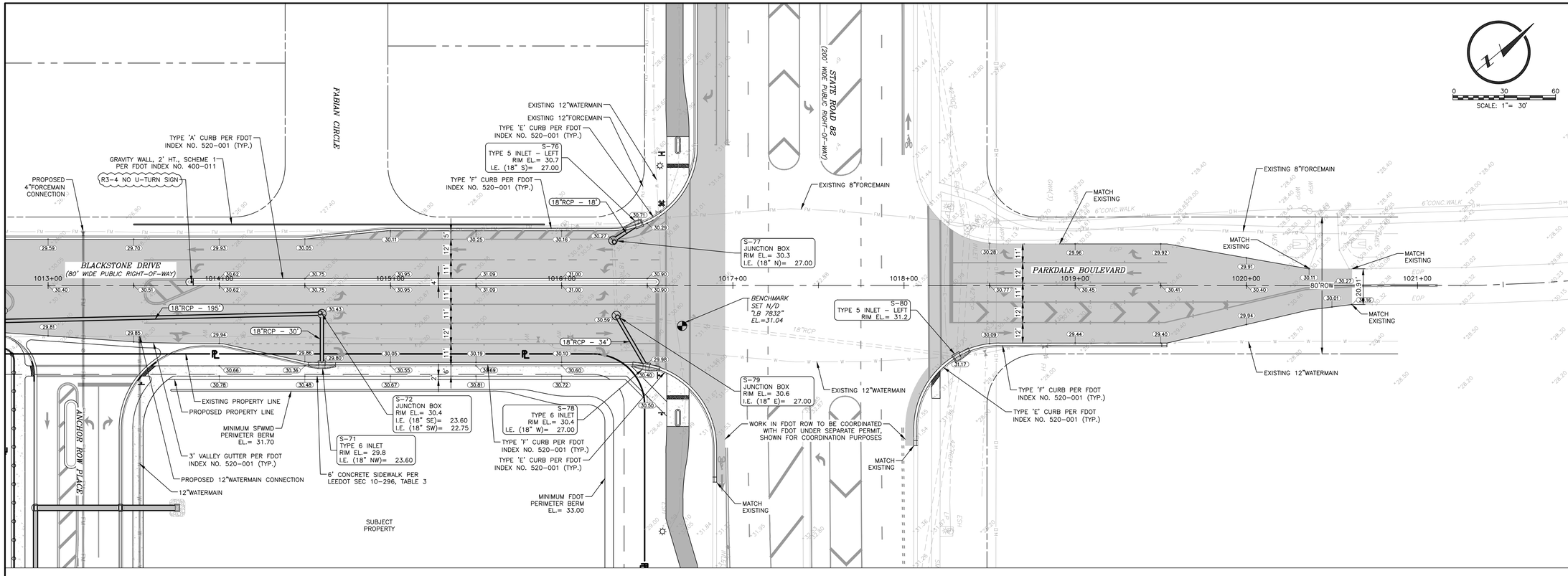
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THEREFORE, THE CONTRACTOR ALL UTILITIES AND SHALL VERIFY THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

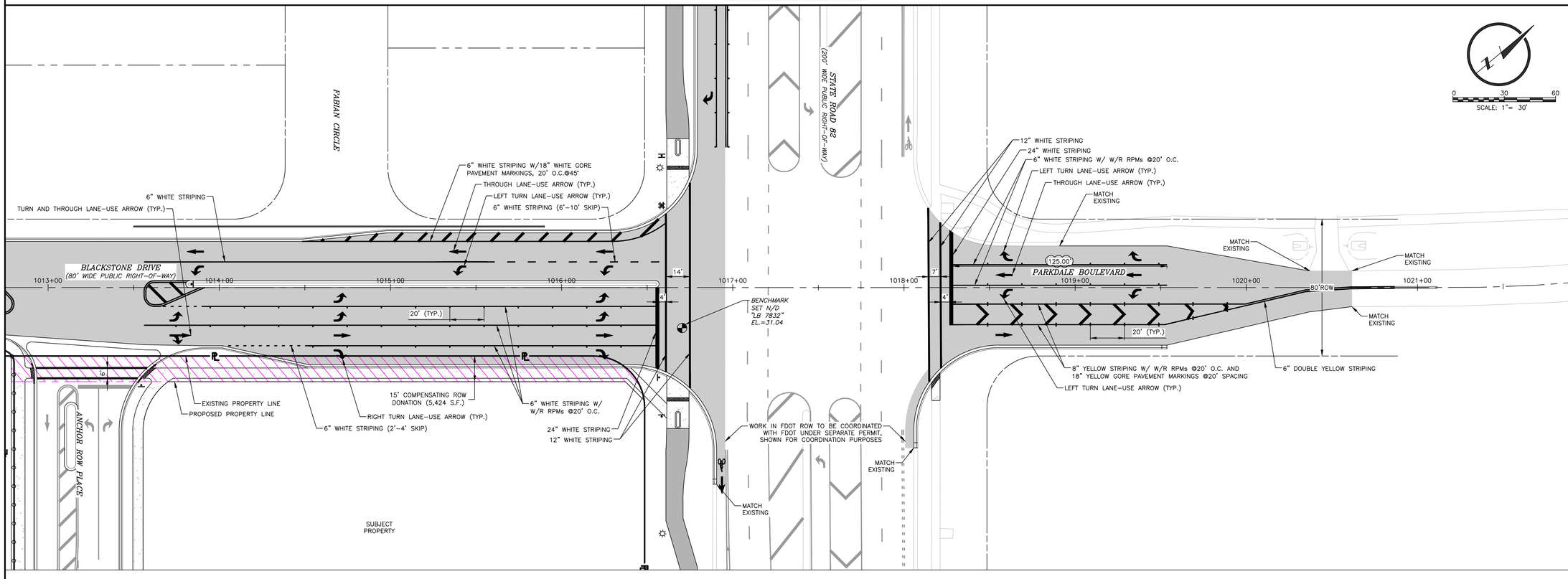
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88
BOOK AND PAGE:		

SHEET 31

s:\Fmen\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\31 OFF-SITE ROADWAY IMPROVEMENT PLAN.dwg, 2/9/2026 11:53:30 AM, VIVIAN ZALEWSKI, ATWELL, LLC



ROADWAY IMPROVEMENT PLAN



SIGNING AND MARKING PLAN

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

OFF-SITE ROADWAY IMPROVEMENT PLAN

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
2	10/17/2025	PER COUNTY COMMENTS	VZ
3	12/19/2025	PER COUNTY COMMENTS	VZ

811 Know what's below. Call before you dig.

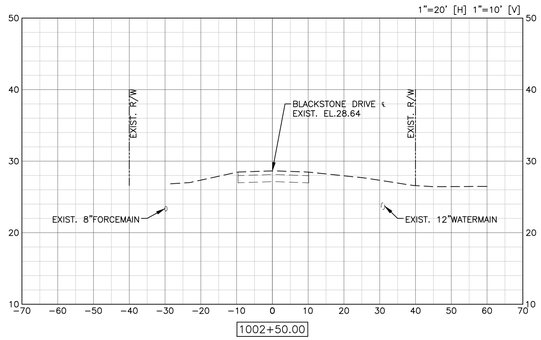
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THEREFORE, ALL UTILITIES ARE SHOWN AS TO THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

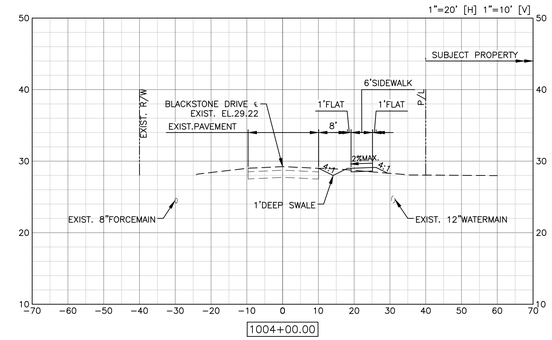
RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

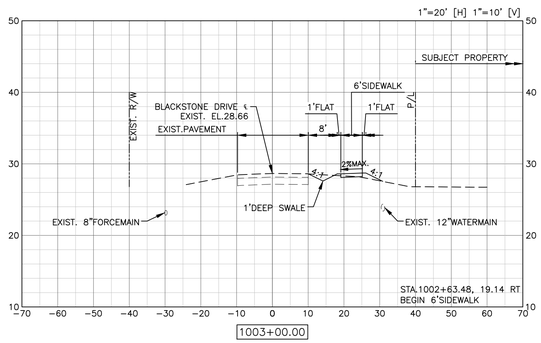
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

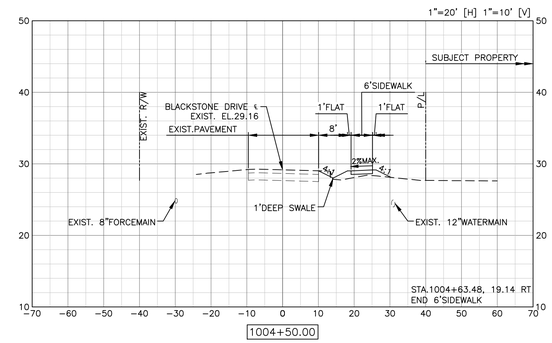
SHEET 32

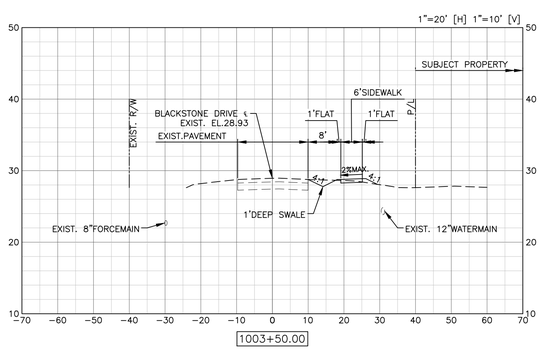
s:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\32 OFF-SITE ROADWAY IMPROVEMENT PLAN.dwg, 2/3/2026 11:53:58 AM, VANN ZALEWSKI, ATWELL, LLC

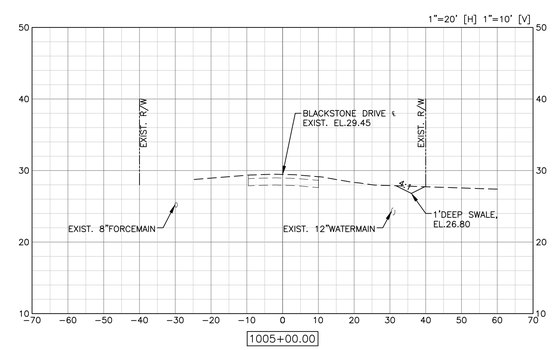












ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

OFF-SITE ROADWAY SECTIONS 1

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
2	10/17/2025	PER COUNTY COMMENTS	VZ

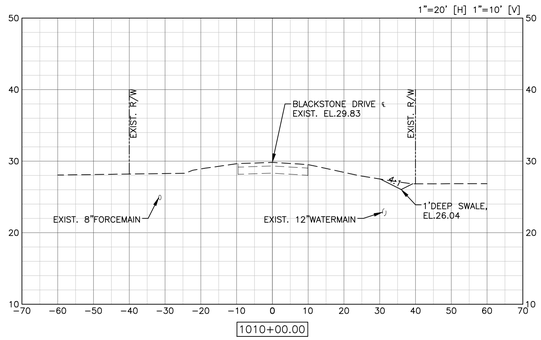
811 Know what's below. Call before you dig.

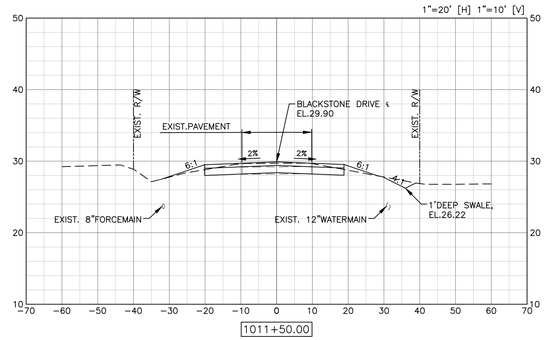
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

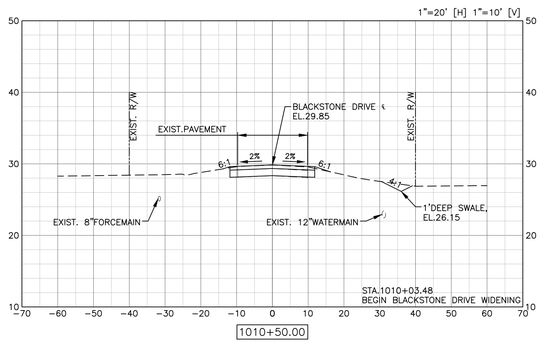
RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

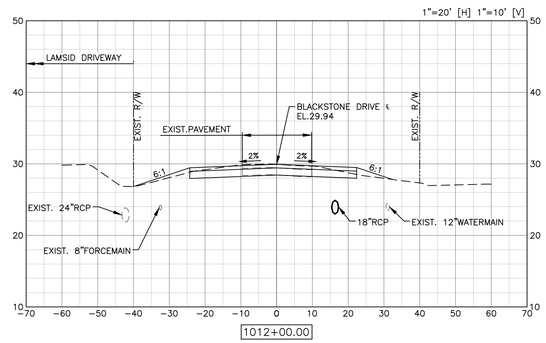
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

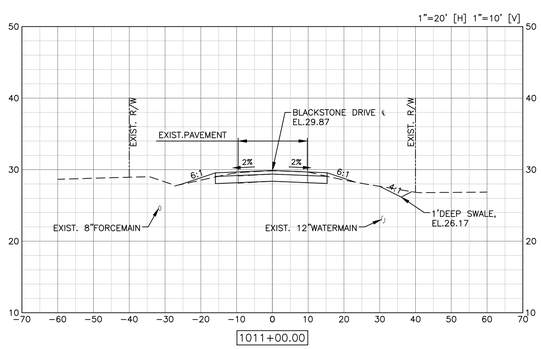
SHEET 33

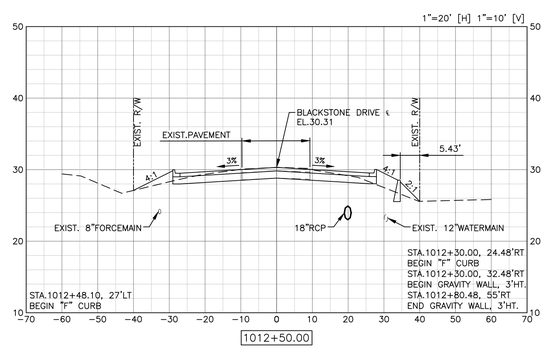












ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

OFF-SITE ROADWAY SECTIONS 2

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
2	10/17/2025	PER COUNTY COMMENTS	VZ

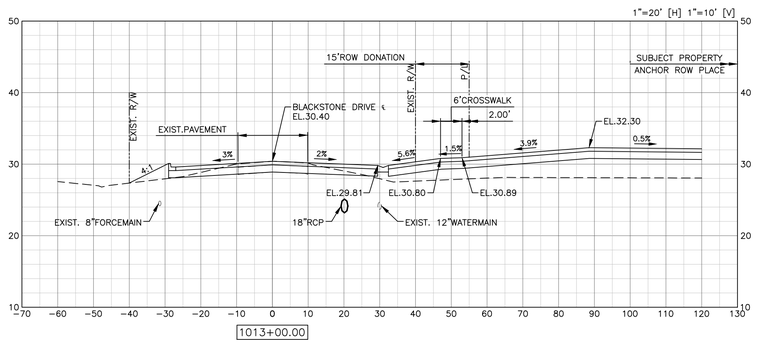
811 Know what's below. Call before you dig.

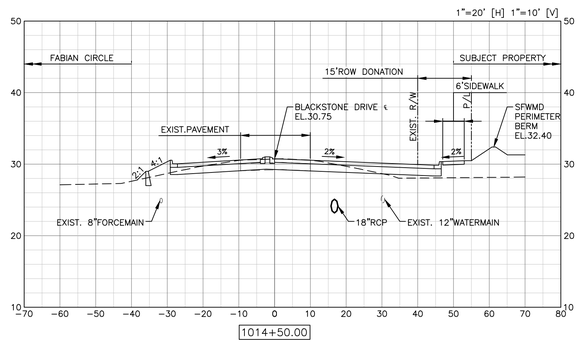
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

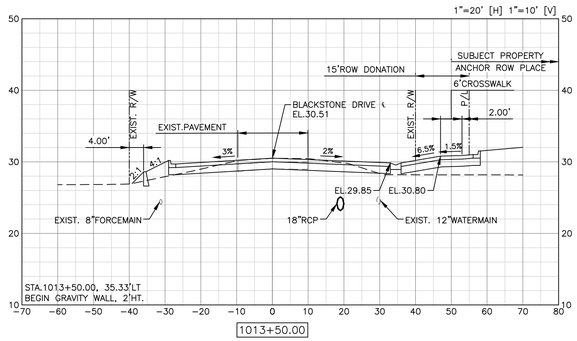
RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

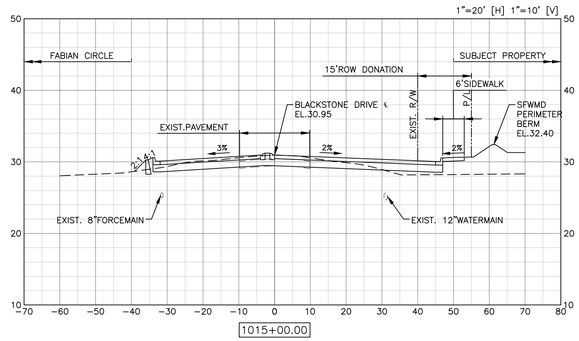
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

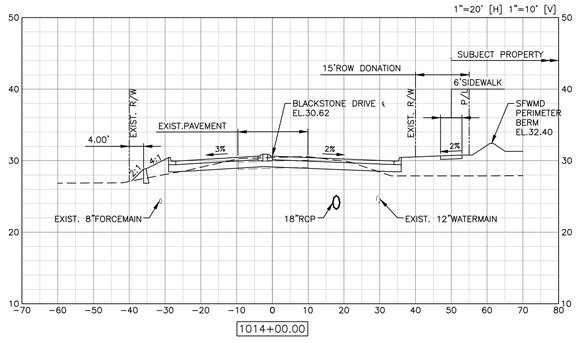
SHEET 34

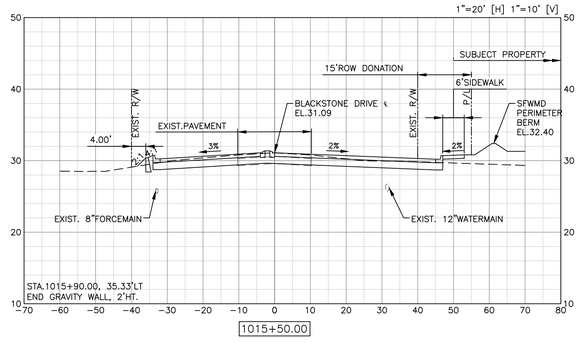












ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

OFF-SITE ROADWAY SECTIONS 3

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
2	10/17/2025	PER COUNTY COMMENTS	VZ

811 Know what's below. Call before you dig.

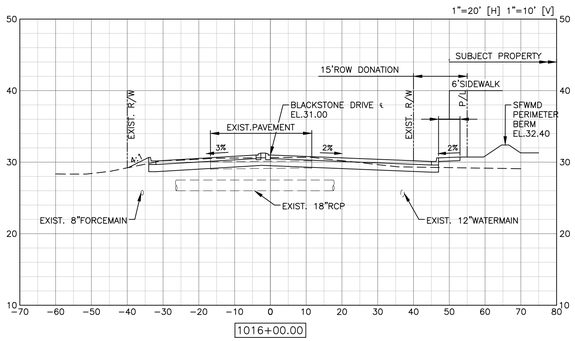
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATIONS SHOWN ARE THE SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

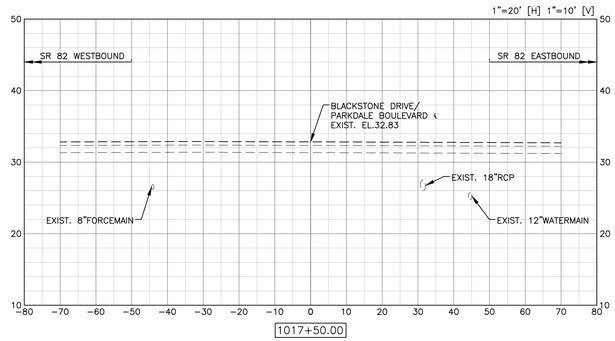
RACHEL J. TRACEY, P.E.
P.E. LIC. #65322

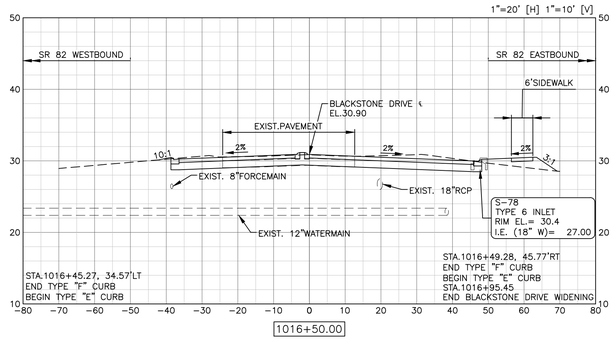
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88
BOOK AND PAGE:		

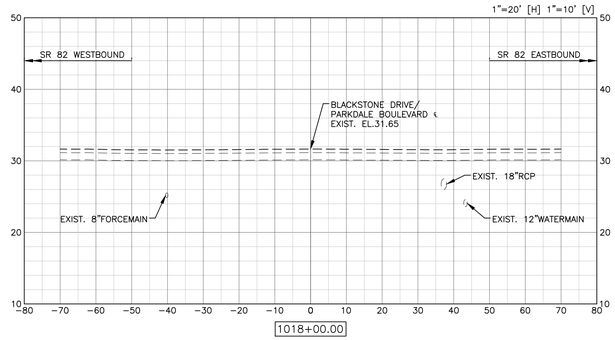
SHEET
35

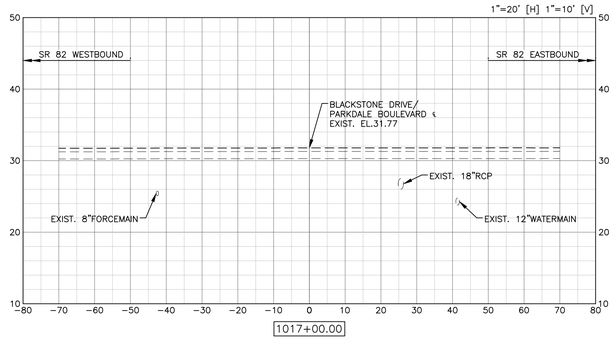
x:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\35 OFF-SITE ROADWAY SECTIONS 3.dwg, 2/6/2026 11:54:00 AM, VIVIAN ZALEWSKA, ATWELL, LLC

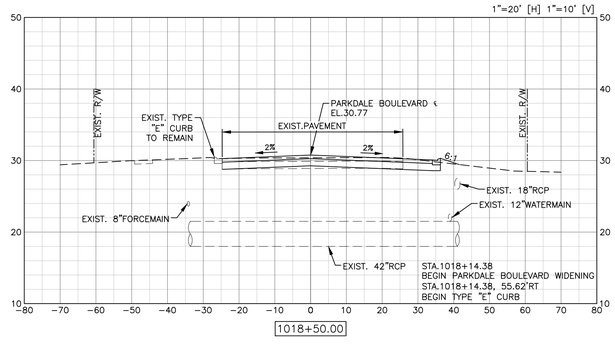












ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

OFF-SITE ROADWAY SECTIONS 4

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR:
CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
2	10/17/2025	PER COUNTY COMMENTS	VZ

811 Know what's below. Call before you dig.

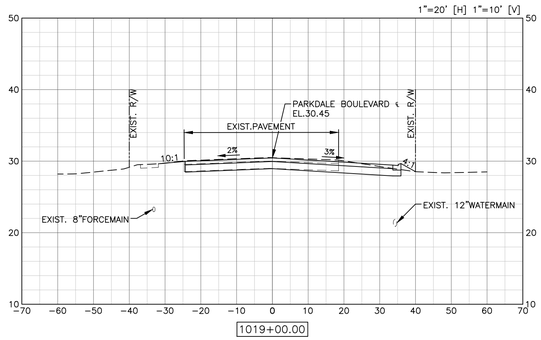
ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATIONS SHOWN AS TO SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

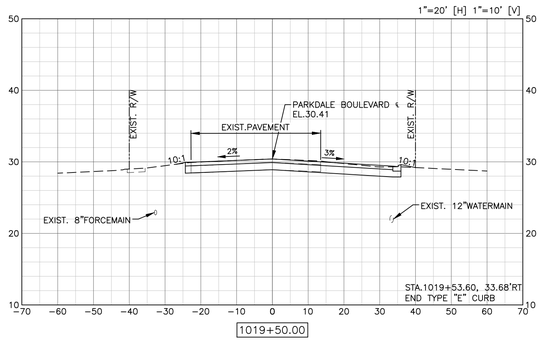
RACHEL J. TRACEY, P.E.
P.E. LIC. #65322

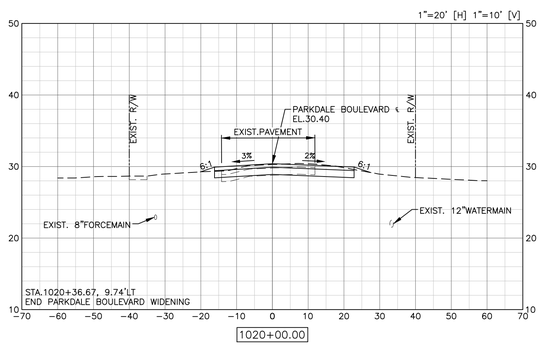
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88
BOOK AND PAGE:		

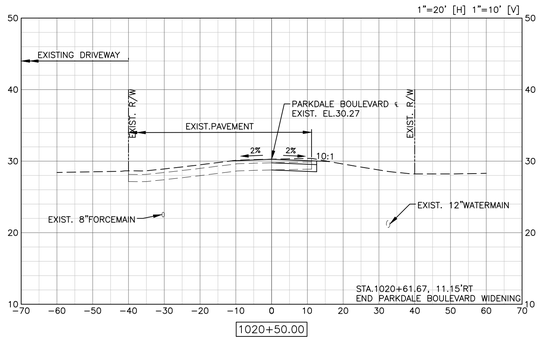
SHEET
36

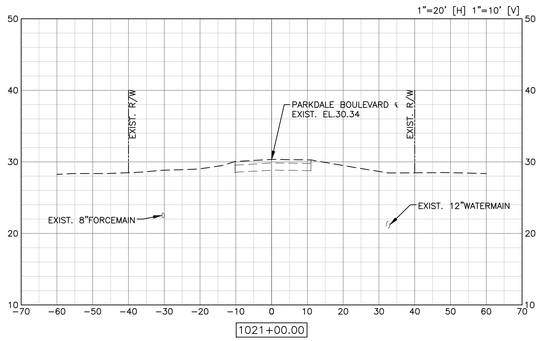
x:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\35 OFF-SITE ROADWAY SECTIONS 4.dwg, 2/5/2026 11:54:22 AM, VIVIAN ZALEWSKA, ATWELL, LLC











ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

OFF-SITE ROADWAY SECTIONS 5

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

NO.	DATE	REVISION DESCRIPTION	BY
2	10/17/2025	PER COUNTY COMMENTS	VZ

811 Know what's **below.** **Call** before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

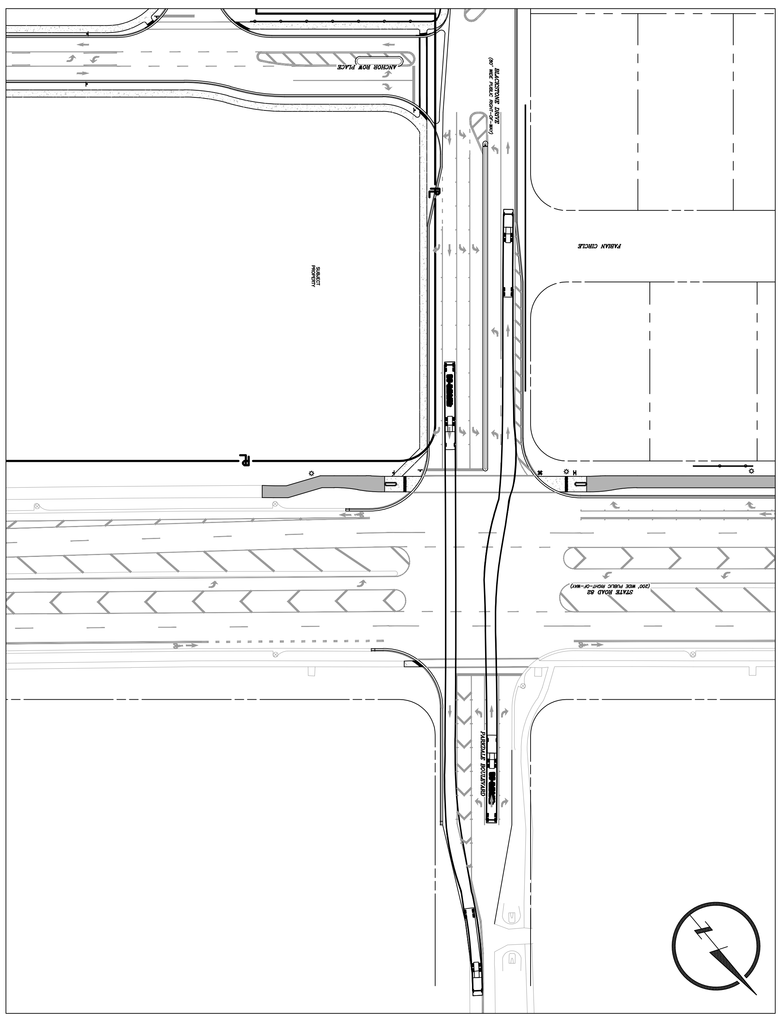
RACHEL J. TRACEY, P.E.

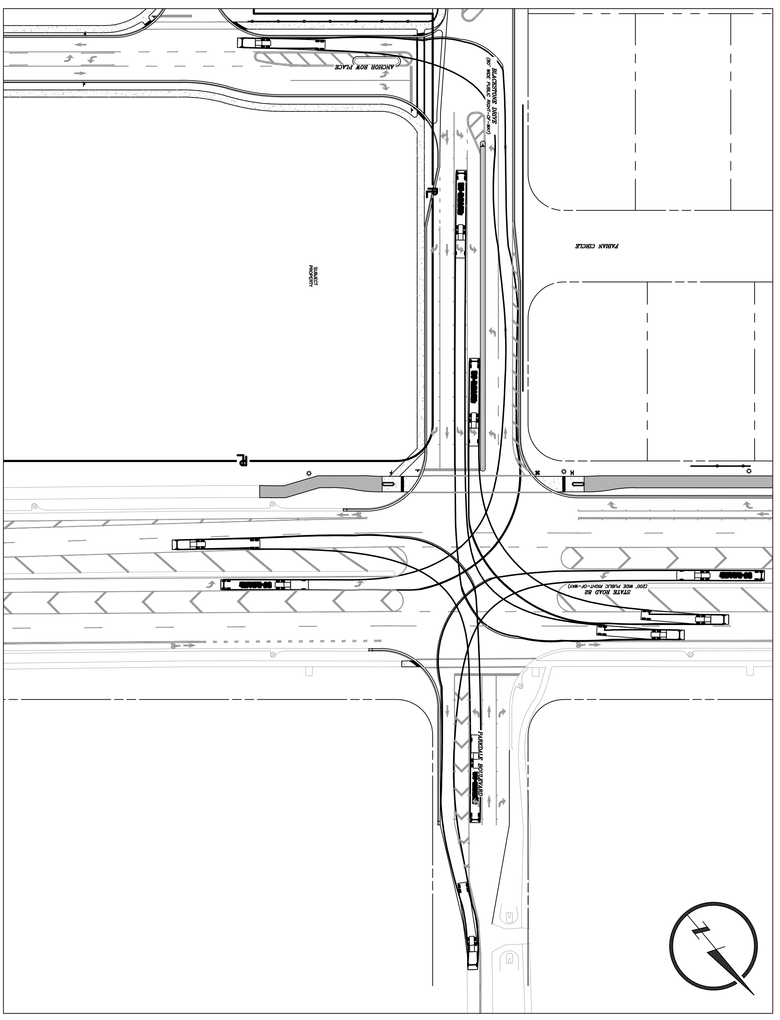
P.E. LIC. #85322

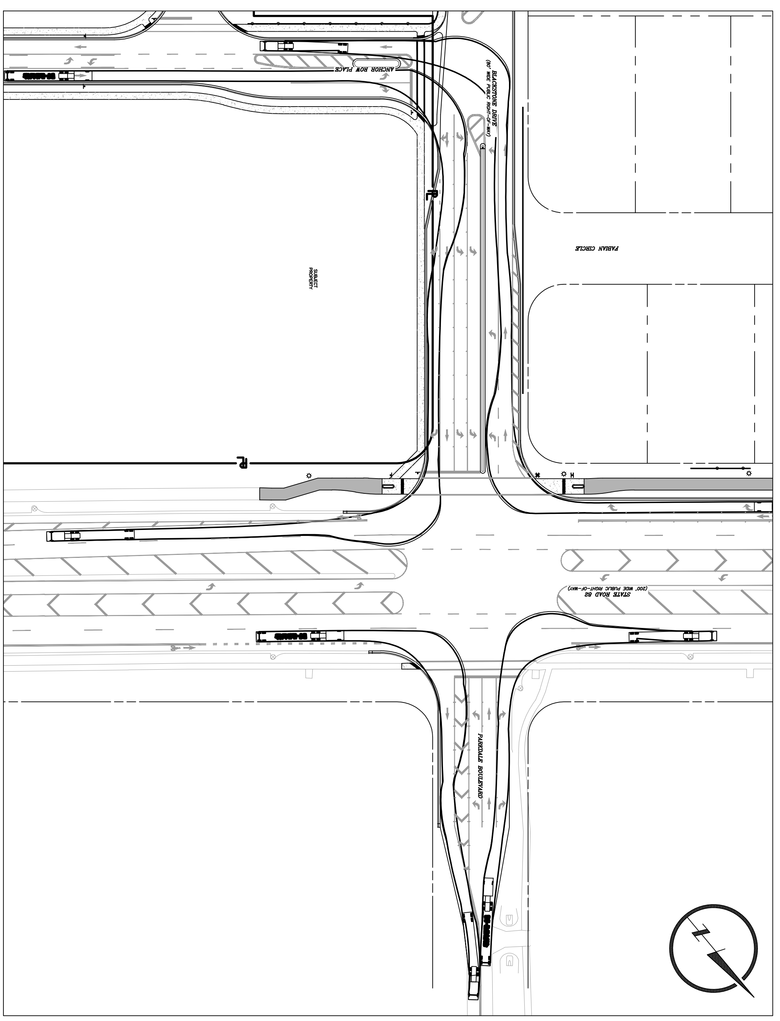
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
VZ	RJT	DBA
PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
24003608	01/31/2025	NAVD'88
BOOK AND PAGE:		

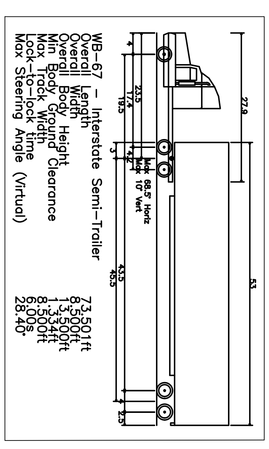
SHEET 37

x:\Atwell\Jobs\24xx\24003608 - fort myers blackstone\engineering\dev plans\38 OFF-SITE ROADWAY AUTOTURN ANALYSES.dwg, 2/9/2026 11:55:07 AM, VIVIAN ZALESANSKI, ATWELL, LLC









WB-67 TRUCK PROFILE

NO.	DATE	REVISION DESCRIPTION	BY
3	12/19/2025	PER COUNTY COMMENTS	V2

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THE ENGINEER AND OWNER ARE PROVIDING THIS INFORMATION AS THAT PROVIDED BY THE FACILITY OR SUBJECT TO THE INFORMATION IS RESPONSIBLE FOR LOCATING UNLESS THEY PRIOR TO CONSTRUCTION.

OFF-SITE ROADWAY AUTOTURN ANALYSES

BLACKSTONE CORPORATE PARK

LEE COUNTY, FLORIDA

PREPARED FOR: CNL FORT MYERS BLACKSTONE, LLC

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

RACHEL J. TRACEY, P.E.
P.E. LIC. #85322

DRAFTED BY: VZ	DESIGNED BY: RJT	REVIEWED BY: DBA
PROJECT NUMBER: 24003608	COMPLETION DATE: 01/31/2025	SURVEY DATUM: NAVD'88
BOOK AND PAGE:		

SHEET 38

BLACKSTONE CORPORATE PARK

Stormwater Management Report

Revised February 6, 2026

PREPARED FOR:

CNL Fort Myers Blackstone, LLC
5391 Lakewood Ranch Blvd N Ste 100
Lakewood Ranch, FL 34240

PREPARED BY:

Atwell, LLC
10511 Six Mile Cypress Parkway, Suite 101
Fort Myers, FL 33966

Rachel J. Tracey, P.E.
Florida License No. 85322

TABLE OF CONTENTS

1.0 CONSULTANT'S STORMWATER STAFF REPORT

1.1 Project Overview
1.2 Site Description
1.3 Background
1.4 Ownership, Operation, and Maintenance
1.5 Control Elevation and Tailwater
1.6 Allowable Discharge and Time of Concentration
1.7 FEMA Information
1.8 Storm Events
  1.8.1 SFWMD Storm Events
  1.8.2 FDOT Strom Events
1.9 Water Quantity
1.10 Results
  1.10.1 SFWMD Results
  1.10.2 Proposed Discharge Rate
  1.10.3 Recovery
  1.10.4 One Hundred Year, Zero Discharge
  1.10.5 FDOT Drainage Information
1.11 Nutrient Loading
1.12 Recovery Analysis
1.13 Water Use and Dewatering
1.14 Water and Wastewater Provider

2.0 APPENDIX

A: LAMSID Canal Map
B: Basin Map and Stages
C: Parcel Buildout Assumptions and Site Data
D: Location Map
E: LAMSID Allowable Discharge Rate
F: Time of Concentration and Allowable Discharge
G: FEMA FIRMette
H: NOAA Rainfall Data
I: Water Quantity
J: StormWise Input Summary and Nodal Diagram
K: StormWise Output Reports
L: Recovery Analysis
M: Nutrient Loading Report
N: HUC Map

1.0 CONSULTANT'S STORMWATER STAFF REPORT

Project Name:	Blackstone Corporate Park
Permittee:	CNL Fort Myers Blackstone, LLC 5391 Lakewood Ranch Blvd N Ste 100, Lakewood Ranch, FL 34240
Operating Entity:	Blackstone Corporate Park Property Owners Association, Inc.
Location:	Lee County, FL
Project Land Use:	Commercial
Drainage Basin:	Orange River via Gecko Extension Canal
Special Drainage District:	Lehigh Acres Municipal Services Improvement District (LAMSID) – Gecko Extension Canal
Water Body Classification:	CLASS III
FDEP Water Body ID:	3240K, 3258D3
Conservation Easement to District:	No
Sovereign Submerged Lands:	No
Permit No.:	Modification to Ex ERP 36-110668-P
Application No.:	TBD
Application Type:	Environmental Resource Permit (Major Mod.)

1.1 PROJECT OVERVIEW

This application is a request for a modification to existing ERP 36-110668-P for the development of Blackstone Corporate Park. Per a pre-application meeting with SFWMD Staff on January 13, 2026, this project proposes a modification to the recently approved ERP 36-110668-P. Please note there is a companion minor modification to the same permit, Permit Number 36-110668-P, concurrent with this application for a break away permit for two separate basins with two separate owners. This was discussed at the pre-application meeting. The enclosed permit included two separate drainage basins under one permit for commercial development on the north side and an industrial portion on the south side. This was reviewed with Christophe McAvoy during the pre-application and with associated follow up. This permit also requests update to ownership entity due to recent purchase of the property and slight modifications to the master surface water management system for the north side (Commercial Development).

Requests – This Modification

- Breakaway permits for the northern commercial basin area from the southern industrial area. See enclosed survey and SFWMD plans. North area is 61.78 acres as noted on sheet 2 of SFWMD plans.
- Ownership Update for the northern portion. The property is now owned by CNL Fort Myers Blackstone, LLC. Enclosed are associated documents to support the ownership updates. Please note the southern industrial is separate ownership due to recent purchase of northern parcel by CNL Fort Myers Blackstone, LLC.
- Modification to the master surface water management system for the commercial northern breakaway permit. A slight revision to the layout of the southeastern side has been adjusted. The previous large parcel 2 with a large box store buildout has been removed. The eastern lake has been expanded and parcel 2 is proposed for a much smaller parcel than previous with added lots 16 and 17. Due to lake expansion the site provides enhanced attenuation and a decrease in impervious area due to reduction in previous parcel 2 area. Please see enclosed stormwater management report.

This project includes roadways and stormwater management with discharge southeast of the site to the existing Lehigh Acres Municipal Services Improvement District (LAMSID) Gecko Canal with ultimate discharge to the Orange River. Appendix A includes the LAMSID Canal Map with the project location noted. The proposed project conveys stormwater to one of three dry retention areas prior to discharge to two interconnected wet detention areas. The southeast side of Lake 2 includes the control structure which discharges to the east to the LAMSID canal.

The proposed modification area is split into four sub-basins for modeling in StormWise. Please see Appendix B which provides a Basin Map and the associated stages for each basin. This is also included on the plans notes sheet. The site is split into Site Basin West, Site Basin East, Lake Basin West, and Lake Basin East. Site Basin West includes Parcel 1, Parcels 3 through 8, dry retention areas 1 and 2, and associated roadway. Site Basin East

includes Parcel 2, Parcels 9 through 17 for future buildout, dry retention area 3, and associated roadways. Parcel 1 is proposed to support a large box store development with a maximum percent impervious of 90%. Parcels 2 through 17 are smaller parcels and are allotted 80% impervious area at buildout. Please see Appendix C for additional details of the buildout assumptions for the parcels and site data for Site Basin West and Site Basin East. Please note the preserve area is outside the stormwater basin.

Site Basin West is conveyed via interconnected inlets to dry retention areas 1 and 2. Lakes 1 and 2 are interconnected with a control structure at the east side of lake 2 with discharge to the existing LAMSID Canal.

At time of buildout Parcel 1 will be required to convey stormwater to dry retention 1 and/or 2. This information is noted on the Master Drainage Plan in the bottom right corner of the sheet.

Offsite improvements in the SR 82 Right of Way will be coordinated with FDOT under an FDOT drainage and access permit via an offsite plan set and are not requested under this cover. Associated offsite improvements in the Blackstone Drive Right of Way are being coordinated with LeeDOT under separate off-site plans and cover as well. The proposed improvements in those two right of ways are shown for coordination purposes only. The site does provide compensating water treatment for offsite improvements. See Appendix M for nutrient loading report.

1.2 SITE DESCRIPTION

The property is located at the southeast intersection of State Road 82 (FDOT ROW) and Blackstone Drive (Lee County ROW) in Lee County. A Location Map is enclosed in Appendix D.

1.3 BACKGROUND

This application is a request for a modification to the existing Permit #36-110668-P; Application Number 250221-50419 with a corrected copy issued February 2, 2026. Please note this ERP requests approval for modification and construction of 61.78 acres on site and 5.83 acres of off-site proposed preserve to the south. The existing ERP permitted 61.78 acres. For the commercial development on north side. A separate breakout permit for the industrial southern portion is proposed via minor modification concurrent with this application as was discussed with SFWMD staff.

1.4 OWNERSHIP, OPERATION, AND MAINTENANCE

Perpetual operation and maintenance of the stormwater management system will be the responsibility of Blackstone Corporate Pak Property Owner’s Association, Inc.

1.5 CONTROL ELEVATION AND TAILWATER

The existing ERP established the control elevation of 28.0 FT-NAVD and it is proposed to remain at 28.0 FT-NAVD. The site discharges to the LAMSID Gecko Extension Canal. The Canal wraps around to the northwest where a controlling weir is located called S-ML-1B. See Exhibit A for additional details. The existing ERP established a peak elevation for the tailwater at 28.90 FT-NAVD which is 0.90 feet above control. This modification holds assumption and used the information in below table for the model.

Time (Hours)	Stage (FT-NAVD)
0.0	28.00
72.0	28.90
360.0	28.00

1.6 ALLOWABLE DISCHARGE RATE AND TIME OF CONCENTRATION

The site discharges to the LAMSID Canal and LAMSID has an allowable discharge rate of 30.1 cubic feet per second per square mile. Please see Appendix E for the LAMSID criteria. Based on a basin area of 58.18 acres the allowable discharge is 2.73 cfs. Please see Appendix F for detailed calculations.

Time of Concentration Calculation for each sub-basin are also listed on Appendix F with the minimum 10 minutes used as required by FDOT.

1.7 FEMA INFORMATION

The project site is located in FEMA Zone X per Panel 12071C0475F effective 8.28.2008. A copy of the FIRMette is in Appendix G.

1.8 STORM EVENTS

1.8.1 SFWMD STORM EVENTS

The original ERP application number 240321-42964 ERP used the SFWMD 5 year/1 day, 10 year/1 day, 25 year/3 day, and 100 year/3 day events based on NOAA site specific data of 5.83", 6.96", 10.80", and 14.30" respectively. The NOAA site specific data from Today's conditions were reviewed and are in substantial compliance with the amounts in the existing ERP. Therefore, for the above rainfall events those values were held. Below is summary chart and enclosed is the current NOAA data in Appendix I.

SFWMD Storm Events	
Storm Event	Rainfall Amount (inches)
5 year, 1 day	5.83
10 year, 1 day	6.96
25 year, 3 day	10.80
100 year, 3 day	14.30

1.8.2 FDOT STORM EVENTS

The site abuts a Florida Department of Transportation (FDOT) right of way on the north side which requires an FDOT drainage permit. FDOT requires the 100 year FDOT storm events be modeled and the FDOT 100 year critical elevation be noted for the project. The required six FDOT 100 year storm events were run and the critical elevation determined to be 31.95 FT-NAVD. Below is a summary of the FDOT 100 year storm events with the associated rainfall amounts taken from NOAA site specific data. Appendix I provides the NOAA information for reference.

FDOT 100 year Rainfall Events (Drainage Exception Permitting)	
Duration	Rainfall (inches)
1 Hour	4.29
2 Hour	5.33
4 Hour	6.46
8 Hour	8.41
24 Hour	11.5
72 Hour	14.3

1.9 WATER QUANITITY

StormWise was used to model the stormwater management system. As noted in the project overview the site is split into four sub-basins. Please see additional information in the project overview section on Site Basin West, Site Basin East, Lake Basin West, and Lake Basin East. Appendix I provides the water quantity sheets used to support the modeling for each.

1.10 RESULTS

1.10.1 SFWMD Results

The results of the model and associated basin map are enclosed in Appendix B and noted on the Notes Sheet of the plans. Please also see Appendix J for the StormWise Input Summary Report with Nodal Diagram and Appendix K for the StormWise output reports.

1.10.2 Proposed Discharge Rate

The site meets the required discharge rate as it discharges 2.72 cfs for the 25 year, 3 day event which is less than allowable 2.73 cfs.

1.10.3 Recovery

The node time series in the output reports notes the site recovered only to 28.33 FT-NAVD. This elevation was used as the initial stage in back to back 25 year, 3 day storm events. The perimeter berm for SFWMD is set above the node maximum of the back to back storm events. The inputs summary, node time series, and node maximums are included in the output reports in the Appendix.

1.10.4 One Hundred Year, Zero Discharge Storm Event

The 100 year link discharge is enclosed noting the storm was run with zero discharge. The minimum finished floor elevation is this evaluation as the site is in FEMA Zone X.

1.10.5 FDOT Drainage Exception Results

The FDOT 100 year critical storm event was determined to be 31.95 FT-NAVD. Abutting SR 82 a five foot flat top berm is required on green space at a minimum of one foot above the critical storm event. Six inches is required above this storm event on a hardened structure and 3 inches on a case by case basis as determined by FDOT. Please see Appendix K for output report of the 100 year events.

1.11 NUTRIENT LOADING

BMP Trains was used to analyze the nutrient loading discharge during pre-development and compare it during post-development to show the proposed project provides no adverse impact to the downstream water body, pursuant to Section 8.3 of the ERP AH Vol. I. Performance criteria was also analyzed and is detailed in this section with a summary table.

I. Project Description

Blackstone Corporate Park development consists of 17 commercial parcels with associated infrastructure and parking. The site will provide the required nutrient reduction via an exfiltration trench, dry retention ponds, and two lakes. Stormwater runoff will be routed to dry retention ponds first, before being conveyed to the lakes. An exfiltration trench is included and treats stormwater from Lake 1, it was also was placed on the west side of the site via coordination with SFWMD staff under ERP application number 250221-50419 to provide hydration to existing wetland to the west. The dry retention areas will contain a control structure with a grate elevation at the required retention elevation to provide required volume per BMP Trains model. The site does provide compensating nutrient loading for offsite improvements which is shown in the nutrient loading report as “Catchment 2: Offsite Compensation”. Please refer to Appendix M for the Nutrient Loading Report.

Although the site is identified within the Estero Basin based on GIS data, the downstream outfall associated with the Gecko Canal watershed is to the Caloosahatchee River, as depicted on the LAMSID Canal Map. A copy of the LAMSID canal map is in Appendix A. The stormwater from the site ultimately goes to the Caloosahatchee not Estero Bay. Also, see appendix N for existing HUC map.

II. Project Characteristics

a. Type of Project: High-Intensity Commercial
b. Location: Lee County
c. HUC 12: 030902050604
d. Development within an Impaired Basin: Yes
    i. WBID: 3240K
    ii. Impairment: Dissolved Oxygen
    iii. Impairment Watershed Downstream: Caloosahatchee River
    iv. OFW Downstream: No

III. Water Quality Treatment Criteria Pursuant to Section 8.3.4, ERP AH Vol. I

a. TP Reduction = 80%
b. TN Reduction = 80%
c. Post-Development < Pre-Development

IV. Pre-Development Analysis

The BMP Trains analysis is consistent with the existing ERP, the land use for pre-development is general agriculture. The HSG is D Pre and Post. Pursuant to section 9.9.2, Table 9.2, of the ERP AH Vol. I, the BMP Trains nutrient loading option for a User Defined Value was used to represent the loading rates. The land use and CNs for the pre-development was input into the BMP Trains program. They are provided in the tables below.

Site Basin Catchment

Land Use	BMP Trains Land Use	CN	Area	TN (mg/L)	TP (mg/L)
General Agricultural	User Defined Values	86.00	58.18	2.290	0.381

The surface water management provides compensating nutrient loading for offsite improvements on SR82 Blackstone Drive and Parkdale Blvd. 1.96 acres of impervious road is taken for the compensation area. A curve number of 86 is used for HSG D and land use of roadway.

Compensating Nutrient Loading Catchment
(Offsite Roadway Improvements)

Land Use	BMP Trains Land Use	CN	Area	TN (mg/L)	TP (mg/L)
Highway	User Defined Values	86.00	1.96	1.250	0.173

Please refer to section Appendix M for the nutrient loading analysis pre-development inputs and results.

V. Post Development Analysis

The land use of the developed project site is high intensity commercial. These areas were determined by the land use table and were input into the Treatment Trains program. Pursuant to section 9.9.2, Table 9.2, of the ERP AH Vol. I, the BMP Trains nutrient loading option for a User Defined Value was used to represent the loading rates. The BMP Trains land use with single nutrient loading rate, and CNs for the post development land uses input into the BMP Trains program are provided in the tables below, while the software calculated the composite CN, runoff coefficients, and yearly loading rates for both nutrients.

Offsite road compensation is taken as 100% impervious.

Site Basin Catchment

Land Use	BMP Trains Land Use	Non DCIA CN	DCIA (%)	Area	TN (mg/L)	TP (mg/L)
High Intensity Commercial	User Defined Values	88.60	61.20	58.18	2.400	0.345

Compensating Nutrient Loading Catchment
(Offsite Roadway Improvements)

Land Use	BMP Trains Land Use	Non DCIA CN (%)	DCIA CN (%)	Area	TN (mg/L)	TP (mg/L)
Highway	User Defined Values	86.00	100	1.96	1.250	0.173

Please refer to Appendix L for the nutrient loading analysis post development inputs and results.

VI. Nutrient Analysis Summary

The site will adhere to both the required post development less than predevelopment and 80 % / 80% reduction. In this case 80% / 80% governs.

The table below summarizes the result of the findings which is the total for the site basin and offsite catchments. Please refer to Appendix M for the full BMP Trains report.

Nitrogen		Phosphorus	
Pre-Development Annual Mass Loading	147.252 kg/yr	Pre-Development Annual Mass Loading	24.425 kg/yr
Post-Development Annual Mass Loading (W/O BMPs)	394.385 kg/yr	Post-Development Annual Mass Loading (W/O BMPs)	56.634 kg/yr
Required Pre/Post Reduction	63%	Required Pre/Post Reduction	57%
Required Load Reduction	80% and Pre/Post (80% Governs)	Required Load Reduction	80% and Pre/Post (80% Governs)
Total Provided Load Reduction	80%	Total Provided Load Reduction	96%
Post-Development Load (W/ BMPs)	79.850 kg/yr	Post-Development Load (W/ BMPs)	2.190 kg/yr
Total Load Removed	314.54 kg/yr	Total Load Removed	54.44 kg/yr

1.12 RECOVERY ANALYSIS

As dry retention is proposed a slug load analysis was conducted and Appendix L includes details. The analysis was run for each of the 3 retention areas and shows recovery in less than 72 hours.

1.13 WATER USE AND DEWATERING PERMIT STATUS

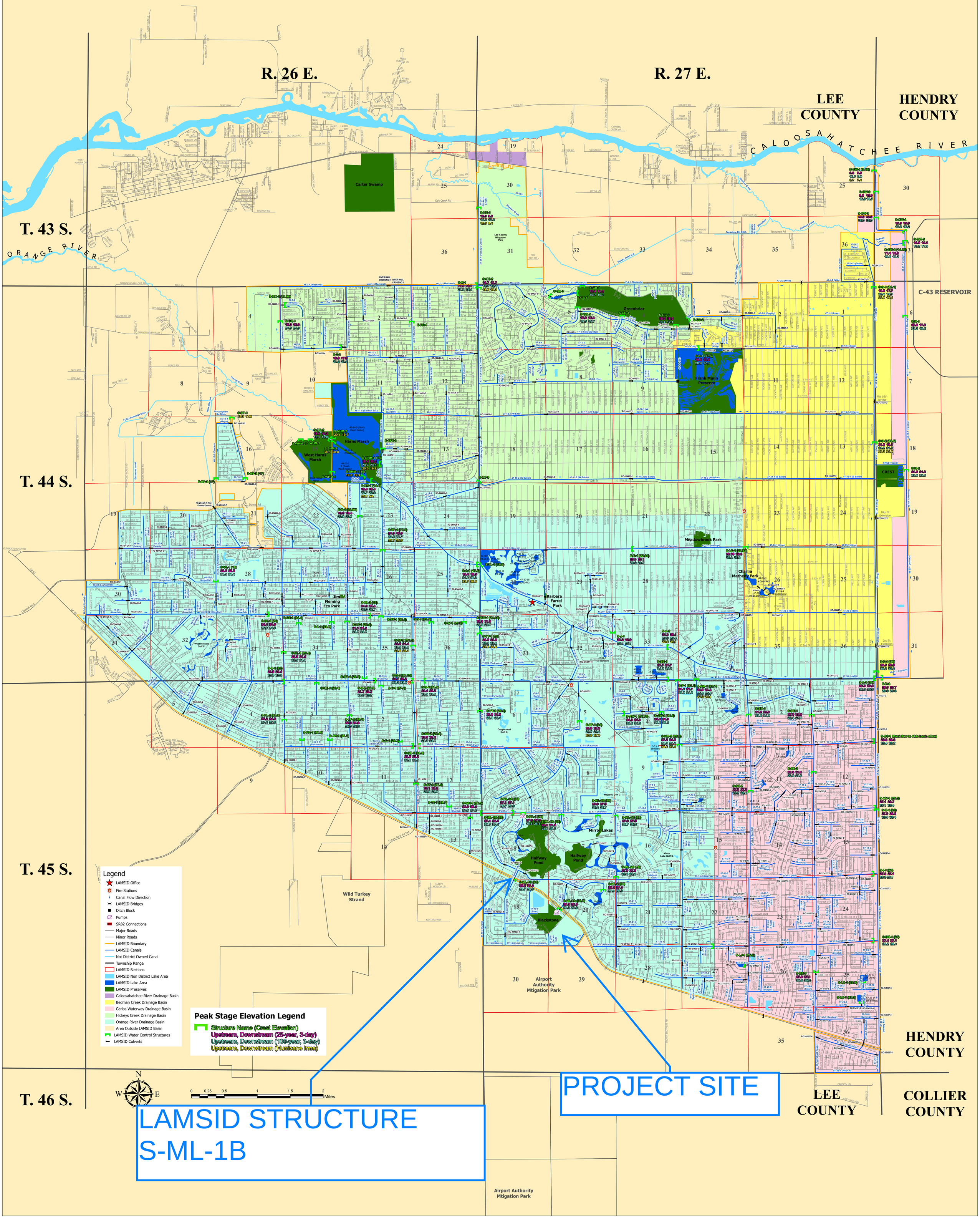
A water use and dewatering permit modification will be applied for concurrent with this application. Irrigation is proposed via well and on-site lakes. Dewatering will be required for the lakes and utility installation.

1.14 WATER AND WASTEWATER SERVICE

Florida Governmental Utility Authority (FGUA) provides water and sewer to the project. A copy of the letter of availability is enclosed in the submittal package.

Appendix A: LAMSID Canal Map

LEHIGH ACRES - MUNICIPAL SERVICES IMPROVEMENT DISTRICT

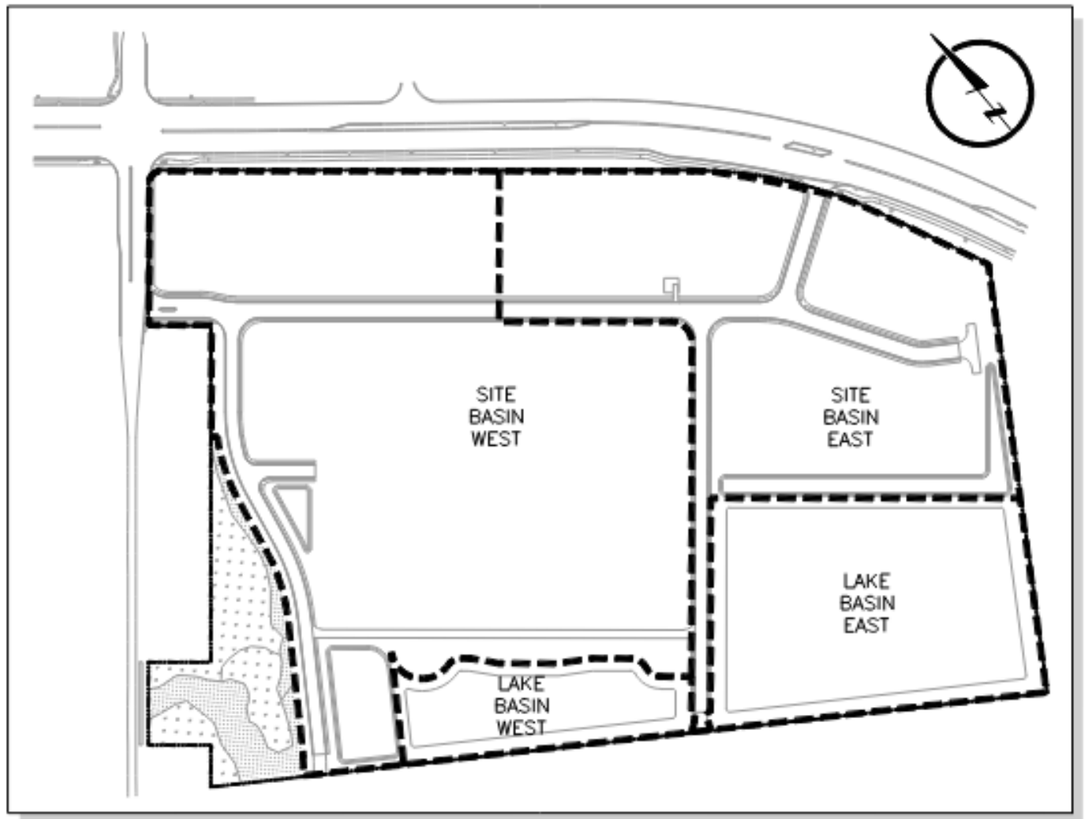


Appendix B: Basin Map and Stages

SURFACE WATER MANAGEMENT SUMMARY:

	SITE BASIN WEST (FT-NAVD)	SITE BASIN EAST (FT-NAVD)	LAKE BASIN WEST (FT-NAVD)	LAKE BASIN EAST (FT-NAVD)
SFWMD 5 YEAR, 1 DAY	30.36	30.23	30.23	30.22
SFWMD 10 YEAR, 1 DAY	30.67	30.64	30.64	30.64
SFWMD 25 YEAR, 3 DAY	31.60	31.59	31.59	31.59
SFWMD 25 YEAR, 3 DAY BACK TO BACK	31.65	31.64	31.64	31.64
SFWMD 100 YEAR, ZERO DISCHARGE	32.30	32.30	32.30	32.30
FDOT 100 YEAR CRITICAL	31.95	31.95	31.95	31.95
MINIMUM ROAD	30.67	30.67	30.67	30.67
SFWMD MINIMUM PERIMETER BERM	31.7	31.7	31.7	31.7
FDOT MINIMUM PERIMETER BERM – GREEN SPACE BERM (1 FT ABOVE FDOT 100 YEAR CRITICAL)	33.0	33.0	33.0	33.0
FDOT MINIMUM PERIMITER BERM – HARDENED STRUCTURE (6" ABOVE 100 YEAR FDOT 100 YEAR CRITICAL)	32.5	32.5	32.5	32.5
MINIMUM FINISHED FLOOR ELEVATION	32.30	32.30	32.30	32.30

FEMA: ZONE X PER PANEL 12071C0475F



BASIN MAP

N.T.S.

Appendix C: Parcel Buildout Assumptions and Site Data

Site Basin West Buildout Assumptions

Site Basin West- Parcels 1, Parcels 3 through 8 assumed land use

Parcel 1 (Box Store)

	Parcel Area (acres)	Building Area (acres)	Pavement/Concrete (assume total 90% total impervious)	Open Space (acres)	Percent Impervious	Percent Pervious
Parcel 1	16.74	4.35	10.71	1.68	90.0%	10.0%

Parcels 3 through 7

Number	Parcel Area (acres)	Building Area (acres)	Pavement/Concrete (assume total 80% total impervious)	Total Impervious Area	Open Space (acres)	Percent Impervious
Parcel 3	1.60	0.19	1.09	1.28	0.32	80.0%
Parcel 4	1.31	0.17	0.88	1.05	0.26	80.0%
Parcel 5	1.07	0.12	0.74	0.86	0.21	80.0%
Parcel 6	1.25	0.15	0.85	1.00	0.25	80.0%
Parcel 7	1.06	0.13	0.72	0.85	0.21	80.0%
Parcel 8	1.08	0.13	0.73	0.86	0.22	80.0%

Parcel Totals 1 through 8	Building Area (acres)	Pavement/Concrete (assume total 80% total impervious)	Open Space (acres)
	5.24	15.72	3.15

Pavement ROW	1.87	ac
Sidewalk ROW	0.44	ac

Building Area (Future Parcels)	5.24	ac
Pavement/Concrete (Pavement ROW, Sidewalk ROW, Pavement/Concrete Parcels)	18.03	ac
Dry Retention Bottom	0.93	ac
Dry Retention Bank	0.22	ac
Open Space	4.59	ac
Total Site Basin West	29.01	ac

Site Basin East Buildout Assumptions

Site Basin East -Parcels A, 2B, 2C, Parcels 9 through 15, & FGUA Parcel

	Parcel Area (acres)	Building Area (acres)	Pavement/Concrete (acres) (assume total 80% total impervious)	Open Space (acres)	Percent Impervious	Percent Pervious
Parcel 2	2.48	0.35	1.63	0.50	80.0%	20.0%
Parcel 17	1.04	0.15	0.68	0.21	80.0%	20.0%
Parcel 16	1.07	0.15	0.71	0.21	80.0%	20.0%

Parcels 9 through 15 and FGUA Parcel

Number	Parcel Area (acres)	Building Area (acres)	Pavement/Concrete (acres) (assume total 80% total impervious)	Total Impervious Area	Open Space (acres)	Percent Impervious
Parcel 9	0.97	0.12	0.66	0.78	0.19	80.0%
Parcel 10	0.98	0.12	0.66	0.78	0.20	80.0%
Parcel 11	1.15	0.14	0.78	0.92	0.23	80.0%
Parcel 12	1.59	0.20	1.07	1.27	0.32	80.0%
Parcel 13	1.12	0.14	0.76	0.90	0.22	80.0%
Parcel 14	0.75	0.09	0.51	0.60	0.15	80.0%
Parcel 15	0.68	0.08	0.46	0.54	0.14	80.0%
FGUA Parcel	0.10	-	0.03	0.03	0.07	30.0%

Parcel Totals 2A-2C, 9-15, and FGUA Parcel	Building Area (acres)	Pavement/Concrete (assume total 80% total impervious)	Open Space (acres)
	1.54	7.95	2.44

Pavement ROW	2.21	ac
Sidewalk ROW	0.63	ac

Building Area (Future Parcels)	1.540	ac
Pavement/Concrete (Pavement ROW, Sidewalk ROW, Pavement/Concrete Parcels)	10.794	ac
Dry Retention Bottom	0.693	ac
Dry Retention Bank	0.360	ac
Open Space	3.253	ac
Total Site Basin East	16.640	ac

Appendix D: Location Map



ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

LOCATION MAP

BLACKSTONE CORPORATE PARK

LEE COUNTY

COMPLETION DATE:	PROJECT:	DRAFTED BY:	DESIGNED BY:	REVIEWED BY:	SHEET:
1/16/2025	24003608				1

S:\Atwell\Jobs\24xx\24003608 - Fort Myers Blackstone\Engineering\Exhibits\Location Map.dwg, 1/16/2025 2:53:22 PM, HALIM CAMEJO, ATWELL, LLC

Appendix E: LAMSID Allowable Discharge Rate

3. **Maximum Discharge Criteria:** Allowable discharge limits apply to all developments and/or road improvements as defined in the current LA-MSID Water Control Plan as summarized below:

    - All developments within the Lehigh Acres Municipal Services Improvement District shall be limited to a maximum discharge rate of **30.1 CSM (cubic feet per second per square mile)** for the 25-year, 3- day SFWMD design storm event, unless otherwise approved by the LA-MSID Board of Commissioners.

      Note: As the minimum bleeder diameter is 3 inches, exceedance of the maximum 30.1 CSM discharge rate is acceptable for systems where a single 3-inch diameter bleeder is proposed as the sole outfall capable of containing the 25-year, 3-day design storm within the proposed development.

    - **Culvert Design Criteria:** Discharge pipes shall be reinforced concrete pipe (RCP) unless approved otherwise by the District Engineer and District Manager.

      Discharge culvert sizes, locations, and elevations are subject to the District Engineer's review and LA-MSID's approval. The discharge orifice invert elevation shall be above the control elevation in the receiving LA-MSID water management facility.

      Pipe culvert ends shall be blended/mitered to match the existing canal slope (or other receiving LA-MSID water management facility) so as not to extend into the effective flow cross-sectional area. Erosion control (such as rip-rap) must be placed per FDOT Specifications at the culvert outlet to prevent erosion to the receiving LA-MSID water management facility's bank. Discharge culverts within the District's rights-of-way must be clearly marked with a sign or marker approved by District Staff, shall be maintained by the project owner or association, and must have legal rights for construction, use, maintenance, and replacement.

      All abandoned Asbestos Cement (AC) pipe is required to be removed from LA-MSID Canal rights-of-way. Abandoned in place will not be allowed on District owned property.

4. **Minimum Design Elevation Criteria:** The Drainage Calculations shall include stormwater modeling of design storm events to demonstrate compliance with the minimum design elevation criteria below, which shall include assurance of reasonable peak-stage recovery times and consideration of tailwater conditions:

    - **Minimum Road & Parking Track Elevations:**
        - Above the 10-year, 1-day design storm peak stage elevation
        - Assure reasonable stage recovery times

    - **Minimum Perimeter Berm Elevation:**
        - Above the 25-year, 3-day design storm peak stage elevation

    - **Minimum Finished Floor Elevations:**
        - Above the 100-year, 3-day design storm peak stage elevation
        - Assure consideration of computed and/or previously experienced high-water levels external to the project

    Note: The "Peak Stage Elevation" map can be referenced as a guidance-only source for reviewing previously documented experienced high-water levels external to the project; however, data is generalized and not available at all locations. Localized drainage issues can impact anticipated system recovery times. The design engineer shall be responsible for the design elevation criteria determination.

Appendix F: Time of Concentration and Allowable Discharge Calculations

Blackstone Corporate Park

Time of Concentration Calculation
tc=2*(Total Basin Area/640)^.5

Item	Site Basin West	Site Basin East	Lake Basin West	Lake Basin East
Area	29.01	16.64	3.28	9.25
Tc	25.5	19.3	8.6	14.4
			*Use min 10 min	

Allowable Discharge

Site Discharges to the East to the Lehigh Acres Municipal Services Improvement District (LAMSID) Gecko Canal.
Per Section 4.28 of LAMSID Permitting Policies, Design Criteria, the maximum allowable discharge is 30.1 CSM (cubic feet per second per square mile)

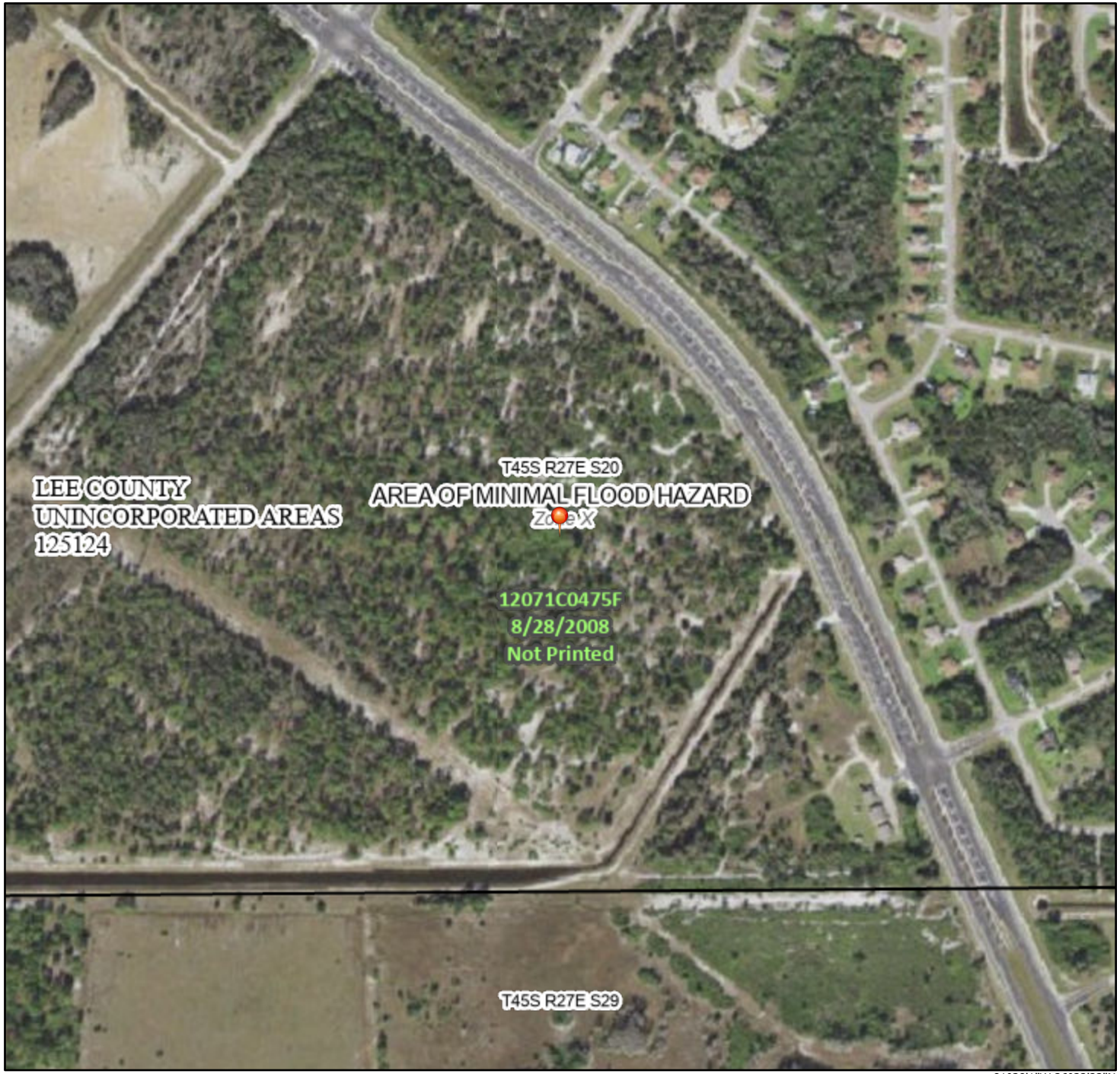
30.1 CSM =	0.047	cfs/acre
SWM Basin Area =	58.18	acres
Allowable Discharge =	2.73	cfs

Appendix G: FEMA FIRMette

National Flood Hazard Layer FIRMette

FEMA

81°38'38"W 26°32'54"N



81°38'1"W 26°32'22"N

0 250 500 1,000 1,500 2,000 Feet

1:6,000

Basemap Imagery Source: USGS National Map 2023

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
- Area with Reduced Flood Risk due to Levee. See Notes. *Zone X*
- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard *Zone X*
- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- 20.2 / 17.5 Cross Sections with 1% Annual Chance / Water Surface Elevation
- Coastal Transect
- Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 12/26/2024 at 7:01 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

Appendix H: NOAA Rainfall Data

www.nws.noaa.gov

NOAA's National Weather Service
Hydrometeorological Design Studies Center
Precipitation Frequency Data Server (PFDS)

General Information
Homepage
Progress Reports
FAQ
Glossary

Precipitation Frequency
Data Server
GIS Grids
Maps
Time Series
Temporals
Documents

Probable Maximum Precipitation
Documents

Miscellaneous
Publications
Storm Analysis
Record Precipitation

Contact Us
Inquiries

NOAA ATLAS 14 POINT PRECIPITATION FREQUENCY ESTIMATES: FL

Data description

Data type: Precipitation depth **Units:** English **Time series type:** Partial duration

Select location

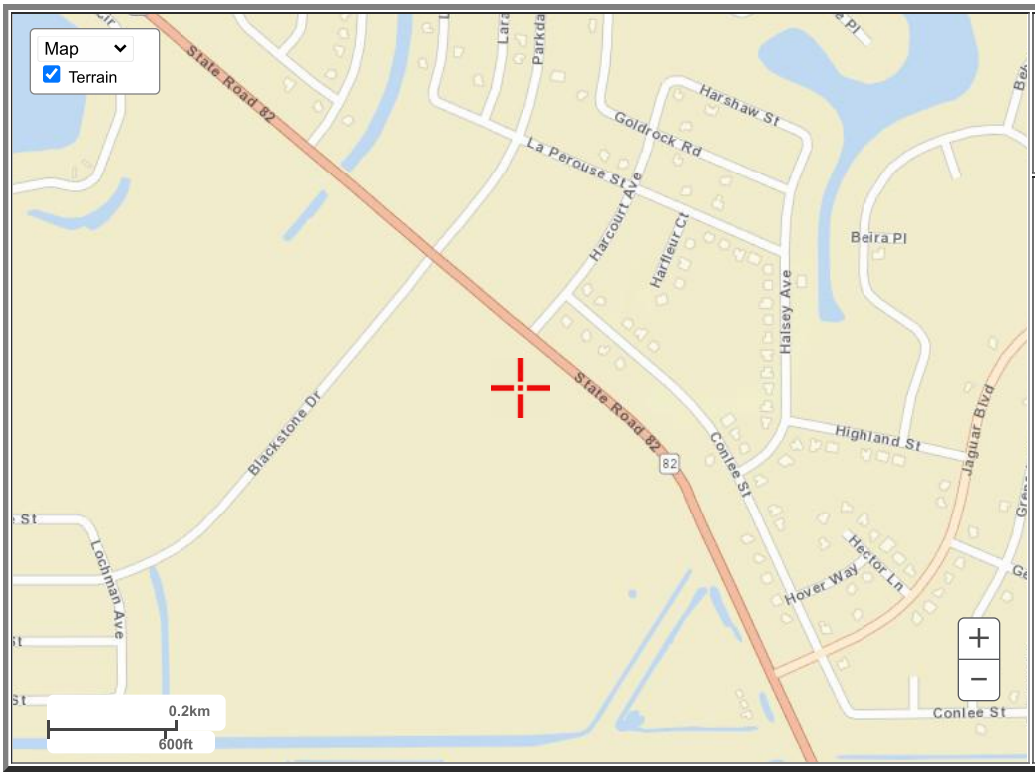
1) Manually:

a) By location (decimal degrees, use "-" for S and W): Latitude: Longitude: Submit

b) By station (list of FL stations): Select station

c) By address BLACKSTONE DRIVE, FORT MYERS

2) Use map:



a) Select location
Move crosshair or double click

b) Click on station icon
Show stations on map

Location information:
Name: Fort Myers, Florida, USA*
Latitude: 26.5461°
Longitude: -81.6394°
Elevation: 28 ft **

* Source: ESRI Maps
** Source: USGS

POINT PRECIPITATION FREQUENCY (PF) ESTIMATES

WITH 90% CONFIDENCE INTERVALS AND SUPPLEMENTARY INFORMATION
NOAA Atlas 14, Volume 9, Version 2

PF tabular | PF graphical | Supplementary information | Print page

PDS-based precipitation frequency estimates with 90% confidence intervals (in inches)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	**0.551** (0.445-0.693)	**0.621** (0.501-0.782)	**0.734** (0.591-0.928)	**0.829** (0.663-1.05)	**0.958** (0.740-1.26)	**1.06** (0.799-1.42)	**1.16** (0.844-1.60)	**1.26** (0.880-1.80)	**1.39** (0.936-2.05)	**1.49** (0.978-2.25)
10-min	**0.807** (0.652-1.02)	**0.909** (0.734-1.14)	**1.08** (0.865-1.36)	**1.21** (0.970-1.54)	**1.40** (1.08-1.84)	**1.55** (1.17-2.07)	**1.69** (1.24-2.34)	**1.84** (1.29-2.63)	**2.04** (1.37-3.01)	**2.18** (1.43-3.29)
15-min	**0.984** (0.795-1.24)	**1.11** (0.895-1.40)	**1.31** (1.06-1.66)	**1.48** (1.18-1.88)	**1.71** (1.32-2.25)	**1.89** (1.43-2.53)	**2.07** (1.51-2.85)	**2.25** (1.57-3.21)	**2.48** (1.67-3.67)	**2.66** (1.75-4.02)
30-min	**1.55** (1.25-1.95)	**1.74** (1.41-2.20)	**2.06** (1.66-2.61)	**2.33** (1.86-2.96)	**2.69** (2.08-3.54)	**2.97** (2.24-3.97)	**3.24** (2.37-4.48)	**3.52** (2.47-5.03)	**3.89** (2.62-5.75)	**4.17** (2.74-6.29)
60-min	**2.04** (1.65-2.57)	**2.30** (1.86-2.89)	**2.71** (2.18-3.43)	**3.06** (2.45-3.89)	**3.54** (2.74-4.66)	**3.91** (2.96-5.25)	**4.29** (3.13-5.92)	**4.67** (3.27-6.67)	**5.18** (3.49-7.66)	**5.57** (3.65-8.40)
2-hr	**2.54** (2.06-3.18)	**2.85** (2.31-3.57)	**3.36** (2.72-4.22)	**3.79** (3.05-4.78)	**4.39** (3.42-5.75)	**4.86** (3.69-6.48)	**5.33** (3.92-7.32)	**5.82** (4.10-8.26)	**6.47** (4.39-9.51)	**6.97** (4.60-10.4)
3-hr	**2.76** (2.25-3.44)	**3.10** (2.52-3.87)	**3.68** (2.98-4.60)	**4.17** (3.36-5.24)	**4.86** (3.80-6.35)	**5.41** (4.13-7.19)	**5.96** (4.40-8.18)	**6.54** (4.64-9.28)	**7.33** (5.00-10.7)	**7.94** (5.27-11.9)

6-hr	**3.09** (2.53-3.82)	**3.54** (2.89-4.38)	**4.30** (3.50-5.34)	**4.95** (4.01-6.19)	**5.90** (4.64-7.69)	**6.66** (5.12-8.83)	**7.45** (5.54-10.2)	**8.28** (5.91-11.7)	**9.42** (6.47-13.8)	**10.3** (6.90-15.3)
12-hr	**3.41** (2.80-4.19)	**4.02** (3.30-4.94)	**5.06** (4.14-6.24)	**5.97** (4.86-7.41)	**7.30** (5.78-9.50)	**8.39** (6.48-11.1)	**9.52** (7.12-13.0)	**10.7** (7.71-15.1)	**12.4** (8.58-18.0)	**13.7** (9.24-20.2)
24-hr	**3.90** (3.22-4.76)	**4.60** (3.79-5.62)	**5.83** (4.80-7.15)	**6.94** (5.68-8.56)	**8.61** (6.88-11.2)	**10.0** (7.79-13.2)	**11.5** (8.65-15.6)	**13.1** (9.47-18.3)	**15.3** (10.7-22.2)	**17.1** (11.6-25.1)
2-day	**4.67** (3.88-5.66)	**5.37** (4.45-6.52)	**6.66** (5.50-8.11)	**7.86** (6.46-9.62)	**9.70** (7.82-12.6)	**11.3** (8.85-14.8)	**13.0** (9.86-17.5)	**14.8** (10.8-20.7)	**17.5** (12.3-25.2)	**19.7** (13.5-28.7)
3-day	**5.18** (4.31-6.25)	**5.99** (4.98-7.24)	**7.44** (6.17-9.03)	**8.77** (7.23-10.7)	**10.8** (8.69-13.9)	**12.5** (9.80-16.3)	**14.3** (10.9-19.2)	**16.2** (11.9-22.5)	**19.0** (13.4-27.2)	**21.2** (14.6-30.8)
4-day	**5.63** (4.69-6.78)	**6.53** (5.44-7.87)	**8.11** (6.74-9.82)	**9.53** (7.87-11.6)	**11.6** (9.39-14.9)	**13.4** (10.5-17.4)	**15.2** (11.6-20.3)	**17.2** (12.6-23.8)	**20.0** (14.1-28.5)	**22.2** (15.3-32.1)
7-day	**6.96** (5.82-8.34)	**7.94** (6.64-9.53)	**9.64** (8.04-11.6)	**11.1** (9.22-13.5)	**13.3** (10.7-16.8)	**15.0** (11.9-19.3)	**16.9** (12.9-22.4)	**18.8** (13.9-25.8)	**21.5** (15.3-30.4)	**23.6** (16.4-34.0)
10-day	**8.21** (6.89-9.81)	**9.23** (7.74-11.0)	**11.0** (9.16-13.1)	**12.5** (10.4-15.0)	**14.6** (11.8-18.3)	**16.3** (12.9-20.9)	**18.1** (13.9-23.8)	**20.0** (14.8-27.2)	**22.5** (16.1-31.8)	**24.6** (17.1-35.2)
20-day	**11.8** (9.98-14.0)	**13.0** (11.0-15.5)	**15.0** (12.6-17.8)	**16.6** (13.8-19.8)	**18.8** (15.2-23.2)	**20.5** (16.2-25.8)	**22.1** (17.0-28.8)	**23.8** (17.7-32.0)	**26.1** (18.7-36.3)	**27.7** (19.4-39.4)
30-day	**14.8** (12.6-17.5)	**16.3** (13.8-19.2)	**18.5** (15.6-22.0)	**20.4** (17.1-24.3)	**22.7** (18.4-27.9)	**24.5** (19.5-30.7)	**26.2** (20.2-33.8)	**27.8** (20.7-37.1)	**29.9** (21.5-41.3)	**31.3** (22.0-44.4)
45-day	**18.6** (15.8-21.9)	**20.5** (17.3-24.1)	**23.3** (19.7-27.5)	**25.5** (21.4-30.3)	**28.3** (23.0-34.4)	**30.2** (24.1-37.6)	**32.1** (24.8-41.1)	**33.7** (25.1-44.7)	**35.7** (25.7-48.9)	**37.0** (26.1-52.2)
60-day	**21.8** (18.6-25.6)	**24.1** (20.4-28.3)	**27.5** (23.3-32.4)	**30.1** (25.4-35.7)	**33.3** (27.1-40.4)	**35.6** (28.4-44.0)	**37.5** (29.1-47.9)	**39.3** (29.3-51.8)	**41.3** (29.8-56.4)	**42.5** (30.1-59.8)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

Estimates from the table in CSV format: Precipitation frequency estimates Submit

Main Link Categories:
Home | OWP

US Department of Commerce
National Oceanic and Atmospheric Administration
National Weather Service
Office of Water Prediction (OWP)
1325 East West Highway
Silver Spring, MD 20910
Page Author: HDSC webmaster
Page last modified: April 21, 2017

Map Disclaimer
Disclaimer
Credits
Glossary

Privacy P
Abou
Career Opportur

Appendix I: Water Quantity

Blackstone Corporate Park - Site Basin West

Water Quantity Spreadsheet	
Lake Control (FT-NAVD)	28.00
Dry Retention Bottom (FT-NAVD)	29.00
Lake-TOB (FT-NAVD)	31.00
Parking (FT-NAVD)	32.00
Roads and 2nd Break (FT-NAVD)	32.75
Finished Floor (FT-NAVD)	33.50
Lake	
Area (AC)	0.00
Dry Retention Bottom	
Area (AC)	0.93
Dry Retention Bank	
Area (AC)	0.22
Open	
Area (AC)	4.59
Parking and Roadway	
Area (AC)	18.03
FF	
Area (AC)	5.24
Total Acres	29.01

Stage (FT-NAVD)	Area (AC)	Storage (AC-FT)
28.00	0.00	0.00
29.00	0.93	0.00
31.00	1.15	2.08
32.00	12.46	8.89
32.75	22.62	22.04
33.50	23.77	39.44

Impervious Area FF + Parking and Roadway

23.27 ac

80.2%

DCIA 23.27 ac

80.2%

Composite CN Used (As Required by FDOT)

FDOT Requires Composite Curve Number:

Location	CN	Area (ac)
Dry Retention Bottom & Bank (FDOT requires 100)	100	1.15
Pavement/Concrete/Impervious	98	23.27
Open Space (TR55, HSG D, Open Space, Good Condition)	80	4.59

Composite CN	95.2	29.01

Blackstone Corporate Park - Site Basin East

Water Quantity Spreadsheet	
Lake Control (FT-NAVD)	28.00
Dry Retention Bottom (FT-NAVD)	29.00
Lake-TOB (FT-NAVD)	31.00
Parking (FT-NAVD)	32.00
Roads and 2nd Break (FT-NAVD)	32.75
Finished Floor (FT-NAVD)	33.50
Lake	
Area (AC)	0.00
Dry Retention Bottom	
Area (AC)	0.693
Dry Retention Bank	
Area (AC)	0.360
Open	
Area (AC)	3.253
Parking and Roadway	
Area (AC)	10.794
FF	
Area (AC)	1.540
Total Acres	16.640

Stage (FT-NAVD)	Area (AC)	Storage (AC-FT)
28.00	0.000	0.00
29.00	0.693	0.00
31.00	1.053	1.75
32.00	8.077	6.31
32.75	14.287	14.70
33.50	15.100	25.72

Impervious Area FF + Parking and Roadway
12.33 ac
74.1%

DCIA 12.33 ac
74.1%

Composite CN Used (As Required by FDOT)

FDOT Requires Composite Curve Number:

Location	CN	Area (ac)
Dry Retention Bottom & Bank (FDOT requires 100)	100	1.053
Pavement/Concrete/Impervious	98	12.334
Open Space (TR55, HSG D, Open Space, Good Condition)	80	3.253

16.640

Composite CN	94.6

Blackstone Corporate Park - Lake Basin West

Water Quantity Spreadsheet	
Lake Control (FT-NAVD)	28.00
Dry Retention Bottom (FT-NAVD)	29.00
Lake-TOB (FT-NAVD)	31.00
Parking (FT-NAVD)	31.50
Roads and 2nd Break (FT-NAVD)	32.50
Finished Floor (FT-NAVD)	33.50
Lake	
Area (AC)	1.96
Dry Retention Bottom	
Area (AC)	0.00
Bank	
Area (AC)	0.66
Open	
Area (AC)	0.66
Parking and Roadway	
Area (AC)	0.00
FF	
Area (AC)	0.00
Total Acres	3.28

Stage (FT-NAVD)	Area (AC)	Storage (AC-FT)
28.00	1.96	0.00
29.00	1.96	1.96
31.00	2.62	6.54
31.50	2.95	7.93
32.50	3.12	10.97
33.50	3.28	14.16

Impervious Area

Lake Area
1.96 ac
59.8%

DCIA 59.8%

Composite CN Used (As Required by FDOT)

FDOT Requires Composite Curve Number:

Location	CN	Area (ac)
Lake	100	2.62
Open Space (TR55, HSG D, Open Space, Good Condition)	80	0.66

Composite CN	96.0

3.28

Blackstone Corporate Park - Lake Basin East

Water Quantity Spreadsheet	
Lake Control (FT-NAVD)	28.00
Dry Retention Bottom (FT-NAVD)	29.00
Lake-TOB (FT-NAVD)	31.00
Parking (FT-NAVD)	31.50
Roads and 2nd Break (FT-NAVD)	32.50
Finished Floor (FT-NAVD)	33.50
Lake	
Area (AC)	7.25
Dry Retention Bottom	
Area (AC)	0.00
Bank	
Area (AC)	0.97
Open	
Area (AC)	1.03
Parking and Roadway	
Area (AC)	0.00
FF	
Area (AC)	0.00
Total Acres	9.25

Stage (FT-NAVD)	Area (AC)	Storage (AC-FT)
28.00	7.25	0.00
29.00	7.25	7.25
31.00	8.22	22.72
31.50	8.74	26.96
32.50	8.99	35.82
33.50	9.25	44.94

Impervious Area

Lake Area
7.25 ac
78.4%

DCIA 78.4%

Composite CN Used (As Required by FDOT)

FDOT Requires Composite Curve Number:

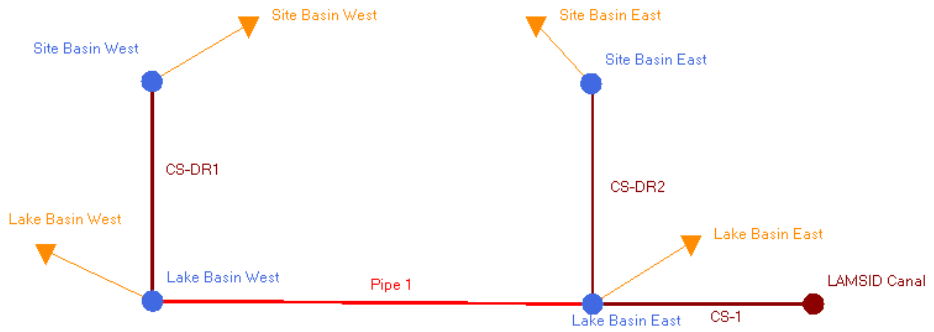
Location	CN	Area (ac)
Lake	100	8.22
Open Space (TR55, HSG D, Open Space, Good Condition)	80	1.03

Composite CN	97.8

9.25

Appendix J: StormWise Input Summary and Nodal Diagram

Background Image: Nodal Diagram



Simple Basin: Lake Basin East

Scenario:	Scenario1
Node:	Lake Basin East
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	14.4000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH256
Peaking Factor:	256.0
Area:	9.2500 ac
Curve Number:	97.8
Ia/S:	0.00
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment: Composite CN used as required for FDOT Drainage Exception, Min Tc 10 min

Simple Basin: Lake Basin West

Scenario:	Scenario1
Node:	Lake Basin West
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number

Time of Concentration:	10.0000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH256
Peaking Factor:	256.0
Area:	3.2800 ac
Curve Number:	96.0
Ia/S:	0.00
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment: Composite CN used as required for FDOT Drainage Exception, Min Tc 10 min

Simple Basin: Site Basin East

Scenario:	Scenario1
Node:	Site Basin East
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	19.3000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH256
Peaking Factor:	256.0
Area:	16.6400 ac
Curve Number:	94.6
Ia/S:	0.00
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Simple Basin: Site Basin West

Scenario:	Scenario1
Node:	Site Basin West
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	25.5000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH256
Peaking Factor:	256.0

Area:	29.0100 ac
Curve Number:	95.2
Ia/S:	0.00
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment: Composite CN used as required for FDOT Drainage Exception

Node: Lake Basin East

Scenario:	Scenario1
Type:	Stage/Area
Base Flow:	0.00 cfs
Initial Stage:	28.00 ft
Warning Stage:	0.00 ft
Alert Stage:	0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
28.00	7.2500	315810
29.00	7.2500	315810
31.00	8.2200	358063
31.50	8.7400	380714
32.50	8.9900	391604
33.50	9.2500	402930

Comment:

Node: Lake Basin West

Scenario:	Scenario1
Type:	Stage/Area
Base Flow:	0.00 cfs
Initial Stage:	28.00 ft
Warning Stage:	0.00 ft
Alert Stage:	0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
28.00	1.9600	85378
29.00	1.9600	85378
31.00	2.6200	114127
31.50	2.9500	128502
32.50	3.1200	135907
33.50	3.2800	142877

Comment:

Node: LAMSID Canal

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 28.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	28.00
0	0	0	72.0000	28.90
0	0	0	360.0000	28.00

Comment:

Node: Site Basin East

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 29.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
28.00	0.0000	0
28.99	0.0100	436
29.00	0.6930	30187
31.00	1.0530	45869
32.00	8.0770	351834
32.75	14.2870	622342
33.50	15.1000	657756

Comment: FDOT requires initial stage at retention bottom

Node: Site Basin West

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 29.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
28.00	0.0000	0

Stage [ft]	Area [ac]	Area [ft2]
28.99	0.0100	436
29.00	0.9300	40511
31.00	1.1500	50094
32.00	12.4600	542758
32.75	22.6200	985327
33.50	23.7700	1035421

Comment: FDOT requires initial stage at dry retention bottom

Drop Structure Link: CS-1		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	25.50 ft	Invert:	25.50 ft
From Node:	Lake Basin East	Manning's N:	0.0120	Manning's N:	0.0120
To Node:	LAMSID Canal	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	60.00 ft	Top Clip			
FHWA Code:	1	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	0.50	Op Table:		Op Table:	
Exit Loss Coef:	0.50	Ref Node:		Ref Node:	
Bend Loss Coef:	0.00	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Sharp Crested Vertical	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	28.00 ft	Ref Node:	
Control Elevation:	28.00 ft	Discharge Coefficients	
Max Depth:	0.54 ft	Weir Default:	3.200
Max Width:	0.64 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component		Bottom Clip
Weir:	2	

Weir Count:	1
Weir Flow Direction:	Both
Damping:	0.0000 ft
Weir Type:	Sharp Crested Vertical
Geometry Type:	Rectangular
Invert:	31.65 ft
Control Elevation:	28.00 ft
Max Depth:	3.00 ft
Max Width:	4.50 ft
Fillet:	0.00 ft

Default:	0.00 ft
Op Table:	
Ref Node:	

Top Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	3.200
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Drop Structure Comment:

Drop Structure Link: CS-DR1

Scenario:	Scenario1
From Node:	Site Basin West
To Node:	Lake Basin West
Link Count:	1
Pipe Flow Direction:	Both
Solution:	Combine
Increments:	0
Pipe Count:	1
Damping:	0.0000 ft
Length:	69.00 ft
FHWA Code:	1
Entr Loss Coef:	0.50
Exit Loss Coef:	0.50
Bend Loss Coef:	0.00
Bend Location:	0.00 dec
Energy Switch:	Energy

Upstream Pipe

Invert:	23.04 ft
Manning's N:	0.0120

Geometry: Circular

Max Depth:	4.00 ft

Bottom Clip

Default:	0.00 ft
Op Table:	
Ref Node:	
Manning's N:	0.0000

Top Clip

Default:	0.00 ft
Op Table:	
Ref Node:	
Manning's N:	0.0000

Downstream Pipe

Invert:	23.04 ft
Manning's N:	0.0120

Geometry: Circular

Max Depth:	4.00 ft

Bottom Clip

Default:	0.00 ft
Op Table:	
Ref Node:	
Manning's N:	0.0000

Top Clip

Default:	0.00 ft
Op Table:	
Ref Node:	
Manning's N:	0.0000

Pipe Comment:

Weir Component

Weir:	1
Weir Count:	1
Weir Flow Direction:	Both
Damping:	0.0000 ft
Weir Type:	Horizontal
Geometry Type:	Rectangular
Invert:	29.43 ft
Control Elevation:	28.00 ft
Max Depth:	8.75 ft
Max Width:	3.00 ft

Bottom Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Top Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	3.200

Fillet:	0.00 ft		
		Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Drop Structure Comment:

Drop Structure Link: CS-DR2		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	23.04 ft	Invert:	23.04 ft
From Node:	Site Basin East	Manning's N:	0.0120	Manning's N:	0.0120
To Node:	Lake Basin East	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	4.00 ft	Max Depth:	4.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	69.00 ft	Top Clip			
FHWA Code:	1	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	0.50	Op Table:		Op Table:	
Exit Loss Coef:	0.50	Ref Node:		Ref Node:	
Bend Loss Coef:	0.00	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Horizontal	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	29.43 ft	Op Table:	
Control Elevation:	28.00 ft	Ref Node:	
Max Depth:	6.58 ft	Discharge Coefficients	
Max Width:	3.00 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Drop Structure Comment:

Pipe Link: Pipe 1		Upstream		Downstream	
Scenario:	Scenario1	Invert:	22.50 ft	Invert:	22.50 ft
From Node:	Lake Basin West	Manning's N:	0.0120	Manning's N:	0.0120
To Node:	Lake Basin East	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	3.00 ft	Max Depth:	3.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	259.00 ft	Op Table:		Op Table:	
FHWA Code:	1	Ref Node:		Ref Node:	
Entr Loss Coef:	0.50	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0.50	Top Clip			
Bend Loss Coef:	0.00	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Simulation: 100YR-1HR

Scenario: Scenario1
Run Date/Time: 1/26/2026 8:32:52 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FDOT-1
		Rainfall Amount:	4.29 in
		Storm Duration:	1.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 100YR-24HR

Scenario: Scenario1
Run Date/Time: 1/26/2026 8:33:26 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

Hydrology [sec]	Surface Hydraulics

		[sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FDOT-24
Rainfall Amount: 11.50 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 100YR-2HR

Scenario: Scenario1
Run Date/Time: 1/26/2026 8:34:03 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec

dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FDOT-2
		Rainfall Amount:	5.33 in
		Storm Duration:	2.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 100YR-4HR

Scenario: Scenario1
Run Date/Time: 1/26/2026 8:34:37 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources		Lookup Tables	
Rainfall Folder:		Boundary Stage Set:	
		Extern Hydrograph Set:	
Unit Hydrograph Folder:		Curve Number Set:	
		Green-Ampt Set:	
		Vertical Layers Set:	
		Impervious Set:	

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FDOT-4
		Rainfall Amount:	6.46 in
		Storm Duration:	4.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 100YR-72HR

Scenario:	Scenario1
Run Date/Time:	1/26/2026 8:35:13 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FDOT-72
Rainfall Amount: 14.30 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 100YR-8HR

Scenario: Scenario1
Run Date/Time: 1/26/2026 8:35:50 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec

dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FDOT-8
		Rainfall Amount:	8.41 in
		Storm Duration:	8.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 100YR-3 DAY

Scenario: Scenario1
Run Date/Time: 1/28/2026 2:51:50 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources		Lookup Tables	
Rainfall Folder:		Boundary Stage Set:	
		Extern Hydrograph Set:	
Unit Hydrograph Folder:		Curve Number Set:	
		Green-Ampt Set:	
		Vertical Layers Set:	
		Impervious Set:	

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~SFWMD-72
		Rainfall Amount:	14.30 in
		Storm Duration:	72.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 10YR-1 DAY

Scenario:	Scenario1
Run Date/Time:	1/28/2026 2:44:03 PM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FLMOD
		Rainfall Amount:	6.96 in
		Storm Duration:	24.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 25YR-3 DAY

Scenario: Scenario1
Run Date/Time: 1/28/2026 2:45:05 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec

dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~SFWMD-72
		Rainfall Amount:	10.80 in
		Storm Duration:	72.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 5 YR-1 DAY

Scenario: Scenario1
Run Date/Time: 1/28/2026 2:46:14 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:	
Unit Hydrograph Folder:	

Lookup Tables

Boundary Stage Set:	
Extern Hydrograph Set:	
Curve Number Set:	
Green-Ampt Set:	
Vertical Layers Set:	
Impervious Set:	

Tolerances & Options

Time Marching:	SAOR
Max Iterations:	6
Over-Relax Weight Fact:	0.5 dec
dZ Tolerance:	0.0010 ft
Max dZ:	1.0000 ft
Link Optimizer Tol:	0.0001 ft

IA Recovery Time:	24.0000 hr
Ia/S:	0.20 dec
Smp/Man Basin Rain Opt:	Global
Rainfall Name:	~FLMOD
Rainfall Amount:	5.83 in
Storm Duration:	24.0000 hr
Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (1D):	100 ft2
Energy Switch (1D):	Energy

Comment:

Appendix K: StormWise Output Reports

5 Year, 1 Day Node Max

Scenario	Sim	Node Name	Relative Time [hrs]	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	SFWMD 5 YR-1 DAY	Lake Basin East	356.0026	30.22	24.3609
Scenario1	SFWMD 5 YR-1 DAY	Lake Basin West	356.0026	30.23	24.2859
Scenario1	SFWMD 5 YR-1 DAY	Site Basin East	356.0026	30.23	24.2193
Scenario1	SFWMD 5 YR-1 DAY	Site Basin West	356.0026	30.36	12.5164

10 Year, 1 Day Node Max

Scenario	Sim	Node Name	Relative Time [hrs]	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	SFWMD 10YR-1 DAY	Lake Basin East	356.0014	30.64	24.3848
Scenario1	SFWMD 10YR-1 DAY	Lake Basin West	356.0014	30.64	24.3098
Scenario1	SFWMD 10YR-1 DAY	Site Basin East	356.0014	30.64	24.2848
Scenario1	SFWMD 10YR-1 DAY	Site Basin West	356.0014	30.67	12.7454

25 Year, 3 Day Node Max

Scenario	Sim	Node Name	Relative Time [hrs]	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	356.0016	31.59	72.5349
Scenario1	SFWMD 25YR-3 DAY	Lake Basin West	356.0016	31.59	72.4516
Scenario1	SFWMD 25YR-3 DAY	Site Basin East	356.0016	31.59	72.4016
Scenario1	SFWMD 25YR-3 DAY	Site Basin West	356.0016	31.60	72.3933

100 Year, 3 Day Node Max (Zero Discharge)

Scenario	Sim	Node Name	Relative Time [hrs]	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	SFWMD 100YR-3 DAY	Lake Basin East	356.0073	32.30	167.1990
Scenario1	SFWMD 100YR-3 DAY	Lake Basin West	356.0073	32.30	75.6156
Scenario1	SFWMD 100YR-3 DAY	Site Basin East	356.0073	32.30	182.9240
Scenario1	SFWMD 100YR-3 DAY	Site Basin West	356.0073	32.30	73.6740

100 Year, 3 Day Link Discharge (Zero Discharge)

Scenario	Sim	Link Name	From Node Name	To Node Name	Maximum Flow Rate [cfs]	Relative Time [hrs]	Time to Maximum Flow Rate [hrs
Scenario1	SFWMD 100YR-3 DAY	CS-1	~~D~CS-1~N	LAMSID Canal	0.00	356.0073	0.0000

25 Year, 3 Day Link Discharge

Scenario	Sim	Link Name	From Node Name	To Node Name	Relative Time [hrs]	Maximum Flow Rate [cfs]	Time to Maximum Flow Rate [hrs
Scenario1	SFWMD 25YR-3 DAY	CS-1	~~D~CS-1~N	LAMSID Canal	356.0016	2.72	68.7683

25 Year, 3 Day Node Time Series

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	0.0000	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	0.2511	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	0.5050	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	0.7527	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	1.0027	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	1.2527	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	1.5027	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	1.7527	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	2.0027	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	2.2527	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	2.5027	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	2.7527	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	3.0027	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	3.2527	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	3.5027	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	3.7527	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	4.0027	28.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	4.2527	28.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	4.5027	28.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	4.7527	28.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	5.0027	28.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	5.2527	28.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	5.5027	28.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	5.7527	28.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	6.0027	28.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	6.2527	28.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	6.5027	28.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	6.7527	28.02
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	7.0027	28.02
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	7.2527	28.02
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	7.5027	28.02
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	7.7527	28.02
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	8.0027	28.02
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	8.2527	28.02
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	8.5027	28.02
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	8.7527	28.03
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	9.0027	28.03
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	9.2527	28.03
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	9.5027	28.03
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	9.7527	28.03
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	10.0027	28.03
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	10.2527	28.03
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	10.5027	28.03
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	10.7527	28.04
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	11.0027	28.04
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	11.2527	28.04
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	11.5027	28.04
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	11.7527	28.04

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	12.0027	28.05
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	12.2527	28.05
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	12.5027	28.05
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	12.7527	28.06
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	13.0027	28.06
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	13.2527	28.07
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	13.5027	28.07
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	13.7527	28.07
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	14.0027	28.08
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	14.2527	28.08
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	14.5027	28.09
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	14.7527	28.09
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	15.0027	28.10
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	15.2527	28.10
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	15.5027	28.11
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	15.7527	28.12
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	16.0027	28.12
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	16.2527	28.13
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	16.5027	28.13
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	16.7527	28.14
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	17.0027	28.14
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	17.2527	28.15
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	17.5027	28.15
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	17.7527	28.16
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	18.0027	28.17
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	18.2527	28.17
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	18.5027	28.18
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	18.7527	28.18
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	19.0027	28.19
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	19.2527	28.19
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	19.5027	28.20
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	19.7527	28.21
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	20.0027	28.21
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	20.2527	28.22
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	20.5027	28.22
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	20.7527	28.23
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	21.0027	28.24
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	21.2527	28.24
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	21.5027	28.25
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	21.7527	28.25
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	22.0027	28.26
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	22.2527	28.26
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	22.5027	28.27
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	22.7527	28.28
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	23.0027	28.28
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	23.2527	28.29
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	23.5027	28.29
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	23.7527	28.30

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	24.0027	28.30
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	24.2527	28.31
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	24.5027	28.32
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	24.7527	28.32
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	25.0027	28.33
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	25.2527	28.34
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	25.5027	28.34
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	25.7527	28.35
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	26.0027	28.36
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	26.2527	28.37
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	26.5027	28.38
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	26.7527	28.38
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	27.0027	28.39
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	27.2527	28.40
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	27.5027	28.41
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	27.7527	28.42
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	28.0027	28.42
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	28.2527	28.43
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	28.5027	28.44
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	28.7527	28.45
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	29.0027	28.45
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	29.2527	28.46
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	29.5027	28.47
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	29.7527	28.48
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	30.0027	28.49
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	30.2527	28.49
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	30.5027	28.50
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	30.7527	28.51
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	31.0027	28.52
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	31.2527	28.52
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	31.5027	28.53
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	31.7527	28.54
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	32.0027	28.55
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	32.2527	28.55
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	32.5027	28.56
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	32.7527	28.57
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	33.0027	28.58
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	33.2527	28.58
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	33.5027	28.59
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	33.7527	28.60
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	34.0027	28.61
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	34.2527	28.61
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	34.5027	28.62
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	34.7527	28.63
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	35.0027	28.63
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	35.2527	28.64
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	35.5027	28.65
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	35.7527	28.66

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	36.0027	28.66
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	36.2527	28.67
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	36.5027	28.68
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	36.7527	28.68
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	37.0027	28.69
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	37.2527	28.70
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	37.5027	28.71
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	37.7527	28.71
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	38.0027	28.72
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	38.2527	28.73
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	38.5027	28.73
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	38.7527	28.74
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	39.0027	28.75
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	39.2527	28.76
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	39.5027	28.76
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	39.7527	28.77
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	40.0027	28.78
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	40.2527	28.78
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	40.5027	28.79
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	40.7527	28.80
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	41.0027	28.80
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	41.2527	28.81
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	41.5027	28.82
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	41.7527	28.83
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	42.0027	28.83
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	42.2527	28.84
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	42.5027	28.85
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	42.7527	28.85
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	43.0027	28.86
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	43.2527	28.87
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	43.5027	28.87
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	43.7527	28.88
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	44.0027	28.89
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	44.2527	28.90
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	44.5027	28.90
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	44.7527	28.91
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	45.0027	28.92
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	45.2527	28.92
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	45.5027	28.93
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	45.7527	28.94
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	46.0027	28.94
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	46.2527	28.95
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	46.5027	28.96
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	46.7527	28.96
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	47.0027	28.97
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	47.2527	28.98
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	47.5027	28.98
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	47.7527	28.99

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	48.0027	29.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	48.2527	29.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	48.5027	29.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	48.7527	29.02
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	49.0027	29.03
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	49.2527	29.03
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	49.5027	29.04
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	49.7527	29.05
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	50.0027	29.06
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	50.2527	29.06
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	50.5027	29.07
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	50.7527	29.08
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	51.0027	29.09
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	51.2527	29.10
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	51.5027	29.11
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	51.7527	29.12
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	52.0027	29.13
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	52.2527	29.14
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	52.5027	29.15
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	52.7527	29.16
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	53.0027	29.18
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	53.2527	29.19
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	53.5027	29.20
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	53.7527	29.22
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	54.0027	29.24
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	54.2527	29.25
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	54.5027	29.27
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	54.7527	29.29
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	55.0027	29.31
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	55.2527	29.33
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	55.5027	29.36
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	55.7527	29.38
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	56.0027	29.40
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	56.2527	29.43
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	56.5027	29.45
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	56.7527	29.48
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	57.0027	29.51
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	57.2527	29.54
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	57.5061	29.56
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	57.7544	29.59
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	58.0043	29.63
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	58.2521	29.66
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	58.5007	29.69
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	58.7510	29.73
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	59.0012	29.77
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	59.2518	29.81
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	59.5005	29.87
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	59.7504	29.95

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	60.0003	30.14
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	60.2504	30.39
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	60.5007	30.61
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	60.7502	30.78
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	61.0003	30.91
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	61.2525	31.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	61.5028	31.09
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	61.7509	31.16
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	62.0003	31.21
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	62.2525	31.25
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	62.5060	31.29
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	62.7502	31.33
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	63.0016	31.36
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	63.2516	31.38
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	63.5016	31.40
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	63.7516	31.42
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	64.0016	31.44
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	64.2516	31.45
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	64.5016	31.46
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	64.7516	31.47
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	65.0016	31.48
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	65.2516	31.49
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	65.5016	31.49
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	65.7516	31.50
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	66.0016	31.50
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	66.2516	31.51
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	66.5016	31.51
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	66.7516	31.52
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	67.0016	31.52
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	67.2516	31.53
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	67.5016	31.53
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	67.7516	31.54
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	68.0016	31.54
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	68.2516	31.55
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	68.5016	31.55
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	68.7516	31.56
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	69.0016	31.56
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	69.2516	31.56
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	69.5016	31.56
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	69.7516	31.57
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	70.0016	31.57
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	70.2516	31.57
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	70.5016	31.57
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	70.7516	31.58
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	71.0016	31.58
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	71.2516	31.58
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	71.5016	31.58
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	71.7516	31.59

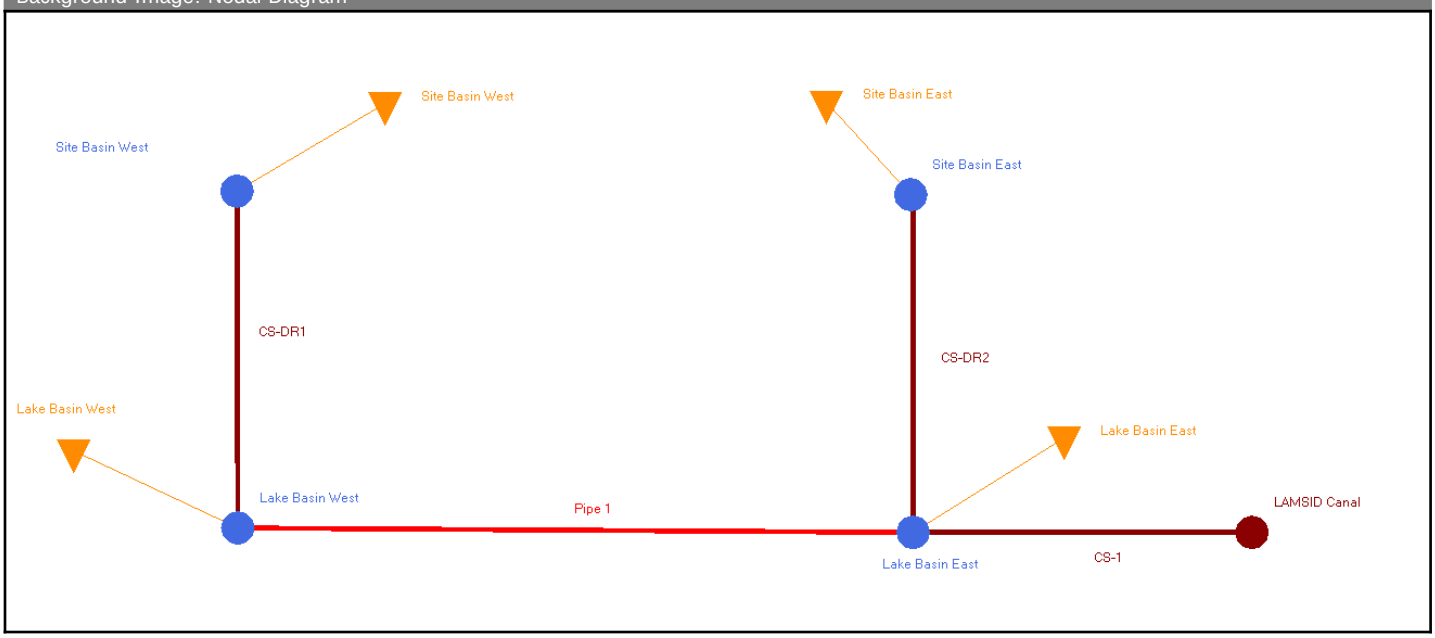
Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	72.0016	31.59
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	76.0016	31.56
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	80.0016	31.53
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	84.0016	31.49
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	88.0016	31.45
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	92.0016	31.41
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	96.0016	31.37
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	100.0016	31.32
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	104.0016	31.28
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	108.0016	31.23
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	112.0016	31.18
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	116.0016	31.13
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	120.0016	31.07
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	124.0016	31.01
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	128.0016	30.95
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	132.0016	30.89
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	136.0016	30.83
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	140.0016	30.76
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	144.0016	30.70
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	148.0016	30.64
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	152.0016	30.58
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	156.0016	30.52
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	160.0016	30.46
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	164.0016	30.40
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	168.0016	30.34
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	172.0016	30.28
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	176.0016	30.23
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	180.0016	30.17
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	184.0016	30.11
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	188.0016	30.06
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	192.0016	30.00
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	196.0016	29.94
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	200.0016	29.89
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	204.0016	29.83
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	208.0016	29.78
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	212.0016	29.73
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	216.0016	29.68
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	220.0016	29.62
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	224.0016	29.57
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	228.0016	29.52
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	232.0016	29.47
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	236.0016	29.42
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	240.0016	29.36
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	244.0016	29.31
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	248.0016	29.25
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	252.0016	29.20
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	256.0016	29.14
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	260.0016	29.09

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	264.0016	29.04
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	268.0016	28.99
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	272.0016	28.94
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	276.0016	28.89
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	280.0016	28.84
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	284.0016	28.80
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	288.0016	28.76
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	292.0016	28.72
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	296.0016	28.68
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	300.0016	28.64
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	304.0016	28.61
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	308.0016	28.57
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	312.0016	28.55
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	316.0016	28.52
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	320.0016	28.49
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	324.0016	28.47
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	328.0016	28.45
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	332.0016	28.43
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	336.0016	28.41
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	340.0016	28.39
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	344.0016	28.37
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	348.0016	28.36
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	352.0016	28.34
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	356.0016	28.33

RUN 25 YEAR BACK TO BACK WITH INITIAL STAGE AT 28.33

25 Year, 3 Day Back to Back Input Summary

Background Image: Nodal Diagram



Node: Lake Basin East

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 28.33 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

INITIAL STAGE SET AT 25 YEAR RECOVERY STAGE OF 28.33

Stage [ft]	Area [ac]	Area [ft2]
28.00	7.2500	315810
29.00	7.2500	315810
31.00	8.2200	358063
31.50	8.7400	380714
32.50	8.9900	391604
33.50	9.2500	402930

Comment:

Node: Lake Basin West

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 28.33 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

INITIAL STAGE SET AT 25 YEAR RECOVERY STAGE OF 28.33

Stage [ft]	Area [ac]	Area [ft2]
28.00	1.9600	85378
29.00	1.9600	85378

Stage [ft]	Area [ac]	Area [ft2]
31.00	2.6200	114127
31.50	2.9500	128502
32.50	3.1200	135907
33.50	3.2800	142877

Comment:

Node: LAMSID Canal

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 28.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	28.00
0	0	0	72.0000	28.90
0	0	0	360.0000	28.00

Comment:

Node: Site Basin East

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 29.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

FDOT REQUIRES INITIAL STAGE AT RETENTION BOTTOM OR 36 HR RECOVERY WHICHEVER GREATER, 29 IS GREATER

Stage [ft]	Area [ac]	Area [ft2]
28.00	0.0000	0
28.99	0.0100	436
29.00	0.6930	30187
31.00	1.0530	45869
32.00	8.0770	351834
32.75	14.2870	622342
33.50	15.1000	657756

Comment: FDOT requires initial stage at retention bottom

Node: Site Basin West

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 29.00 ft

FDOT REQUIRES INITIAL STAGE AT RETENTION BOTTOM OR 36 HR RECOVERY WHICHEVER GREATER, 29 IS GREATER

Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
28.00	0.0000	0
28.99	0.0100	436
29.00	0.9300	40511
31.00	1.1500	50094
32.00	12.4600	542758
32.75	22.6200	985327
33.50	23.7700	1035421

Comment: FDOT requires initial stage at dry retention bottom

Simple Basin: Lake Basin East

Scenario: Scenario1
Node: Lake Basin East
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 14.4000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 9.2500 ac
Curve Number: 97.8
Ia/S: 0.00
% Impervious: 0.00
% DCIA: 0.00
% Direct: 0.00
Rainfall Name:

Comment: Composite CN used as required for FDOT Drainage Exception, Min Tc 10 min

Simple Basin: Lake Basin West

Scenario: Scenario1
Node: Lake Basin West
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 3.2800 ac
Curve Number: 96.0
Ia/S: 0.00
% Impervious: 0.00
% DCIA: 0.00
% Direct: 0.00
Rainfall Name:

Comment: Composite CN used as required for FDOT Drainage Exception, Min Tc 10 min

Simple Basin: Site Basin East

Scenario:	Scenario1
Node:	Site Basin East
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	19.3000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH256
Peaking Factor:	256.0
Area:	16.6400 ac
Curve Number:	94.6
Ia/S:	0.00
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Simple Basin: Site Basin West

Scenario:	Scenario1
Node:	Site Basin West
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	25.5000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH256
Peaking Factor:	256.0
Area:	29.0100 ac
Curve Number:	95.2
Ia/S:	0.00
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment: Composite CN used as required for FDOT Drainage Exception

Drop Structure Link: CS-1

Scenario:	Scenario1
From Node:	Lake Basin East
To Node:	LAMSID Canal
Link Count:	1

Upstream Pipe	
Invert:	25.50 ft
Manning's N:	0.0120
Geometry: Circular	
Max Depth:	2.00 ft

Downstream Pipe	
Invert:	25.50 ft
Manning's N:	0.0120
Geometry: Circular	
Max Depth:	2.00 ft

Pipe Flow Direction:	Both
Solution:	Combine
Increments:	0
Pipe Count:	1
Damping:	0.0000 ft
Length:	60.00 ft
FHWA Code:	1
Entr Loss Coef:	0.50
Exit Loss Coef:	0.50
Bend Loss Coef:	0.00
Bend Location:	0.00 dec
Energy Switch:	Energy

Bottom Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000
Top Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000

Pipe Comment:

Weir Component

Weir:	1
Weir Count:	1
Weir Flow Direction:	Both
Damping:	0.0000 ft
Weir Type:	Sharp Crested Vertical
Geometry Type:	Rectangular
Invert:	28.00 ft
Control Elevation:	28.00 ft
Max Depth:	0.54 ft
Max Width:	0.64 ft
Fillet:	0.00 ft

Bottom Clip	
Default:	0.00 ft
Op Table:	
Ref Node:	
Top Clip	
Default:	0.00 ft
Op Table:	
Ref Node:	
Discharge Coefficients	
Weir Default:	3.200
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Weir Component

Weir:	2
Weir Count:	1
Weir Flow Direction:	Both
Damping:	0.0000 ft
Weir Type:	Sharp Crested Vertical
Geometry Type:	Rectangular
Invert:	31.65 ft
Control Elevation:	28.00 ft
Max Depth:	3.00 ft
Max Width:	4.50 ft
Fillet:	0.00 ft

Bottom Clip	
Default:	0.00 ft
Op Table:	
Ref Node:	
Top Clip	
Default:	0.00 ft
Op Table:	
Ref Node:	
Discharge Coefficients	
Weir Default:	3.200
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Drop Structure Comment:

Drop Structure Link: CS-DR1

Scenario:	Scenario1
From Node:	Site Basin West
To Node:	Lake Basin West
Link Count:	1
Pipe Flow Direction:	Both

Upstream Pipe		Downstream Pipe	
Invert:	23.04 ft	Invert:	23.04 ft
Manning's N:	0.0120	Manning's N:	0.0120
Geometry: Circular		Geometry: Circular	
Max Depth:	4.00 ft	Max Depth:	4.00 ft
Bottom Clip			

Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	69.00 ft	Top Clip			
FHWA Code:	1	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	0.50	Op Table:		Op Table:	
Exit Loss Coef:	0.50	Ref Node:		Ref Node:	
Bend Loss Coef:	0.00	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Horizontal	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	29.43 ft	Ref Node:	
Control Elevation:	28.00 ft	Discharge Coefficients	
Max Depth:	8.75 ft	Weir Default:	3.200
Max Width:	3.00 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Drop Structure Comment:

Drop Structure Link: CS-DR2		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	23.04 ft	Invert:	23.04 ft
From Node:	Site Basin East	Manning's N:	0.0120	Manning's N:	0.0120
To Node:	Lake Basin East	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	4.00 ft	Max Depth:	4.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	69.00 ft	Top Clip			
FHWA Code:	1	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	0.50	Op Table:		Op Table:	
Exit Loss Coef:	0.50	Ref Node:		Ref Node:	
Bend Loss Coef:	0.00	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both		

Damping:	0.0000 ft
Weir Type:	Horizontal
Geometry Type:	Rectangular
Invert:	29.43 ft
Control Elevation:	28.00 ft
Max Depth:	6.58 ft
Max Width:	3.00 ft
Fillet:	0.00 ft

Ref Node:	
Top Clip	
Default:	0.00 ft
Op Table:	
Ref Node:	
Discharge Coefficients	
Weir Default:	3.200
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Drop Structure Comment:

Pipe Link: Pipe 1		Upstream		Downstream	
Scenario:	Scenario1	Invert:	22.50 ft	Invert:	22.50 ft
From Node:	Lake Basin West	Manning's N:	0.0120	Manning's N:	0.0120
To Node:	Lake Basin East	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	3.00 ft	Max Depth:	3.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	259.00 ft	Op Table:		Op Table:	
FHWA Code:	1	Ref Node:		Ref Node:	
Entr Loss Coef:	0.50	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0.50	Top Clip			
Bend Loss Coef:	0.00	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Simulation: 100YR-1HR

Scenario:	Scenario1
Run Date/Time:	1/26/2026 8:32:52 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FDOT-1
Rainfall Amount: 4.29 in
Storm Duration: 1.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 100YR-24HR

Scenario: Scenario1
Run Date/Time: 1/26/2026 8:33:26 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000

Max Calculation Time: 30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FDOT-24
Rainfall Amount: 11.50 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 100YR-2HR

Scenario: Scenario1
Run Date/Time: 1/26/2026 8:34:03 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

Year	Month	Day	Hour [hr]

Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FDOT-2
Rainfall Amount: 5.33 in
Storm Duration: 2.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 100YR-4HR

Scenario: Scenario1
Run Date/Time: 1/26/2026 8:34:37 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FDOT-4
Rainfall Amount: 6.46 in
Storm Duration: 4.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 100YR-72HR

Scenario: Scenario1
Run Date/Time: 1/26/2026 8:35:13 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FDOT-72
Rainfall Amount: 14.30 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 100YR-8HR

Scenario: Scenario1
Run Date/Time: 1/26/2026 8:35:50 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FDOT-8

Rainfall Amount:	8.41 in
Storm Duration:	8.0000 hr
Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (1D):	100 ft2
Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 100YR-3 DAY

Scenario: Scenario1
Run Date/Time: 1/22/2026 3:18:25 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
IA Recovery Time: 24.0000 hr

Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~SFWMD-72
		Rainfall Amount:	14.30 in
		Storm Duration:	72.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 10YR-1 DAY

Scenario: Scenario1
Run Date/Time: 1/26/2026 3:38:13 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FLMOD
		Rainfall Amount:	6.96 in
		Storm Duration:	24.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 25YR-3 DAY

Scenario: Scenario1
Run Date/Time: 1/28/2026 2:35:36 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Lookup Tables

Boundary Stage Set:

Unit Hydrograph Folder:

Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~SFWMD-72
		Rainfall Amount:	10.80 in
		Storm Duration:	72.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 5 YR-1 DAY

Scenario: Scenario1
Run Date/Time: 1/26/2026 3:39:14 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:	
Unit Hydrograph Folder:	

Lookup Tables

Boundary Stage Set:	
Extern Hydrograph Set:	
Curve Number Set:	
Green-Ampt Set:	
Vertical Layers Set:	
Impervious Set:	

Tolerances & Options

Time Marching:	SAOR
Max Iterations:	6
Over-Relax Weight Fact:	0.5 dec
dZ Tolerance:	0.0010 ft
Max dZ:	1.0000 ft
Link Optimizer Tol:	0.0001 ft

IA Recovery Time:	24.0000 hr
Ia/S:	0.20 dec
Smp/Man Basin Rain Opt:	Global
Rainfall Name:	~FLMOD
Rainfall Amount:	5.83 in
Storm Duration:	24.0000 hr
Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (1D):	100 ft2
Energy Switch (1D):	Energy

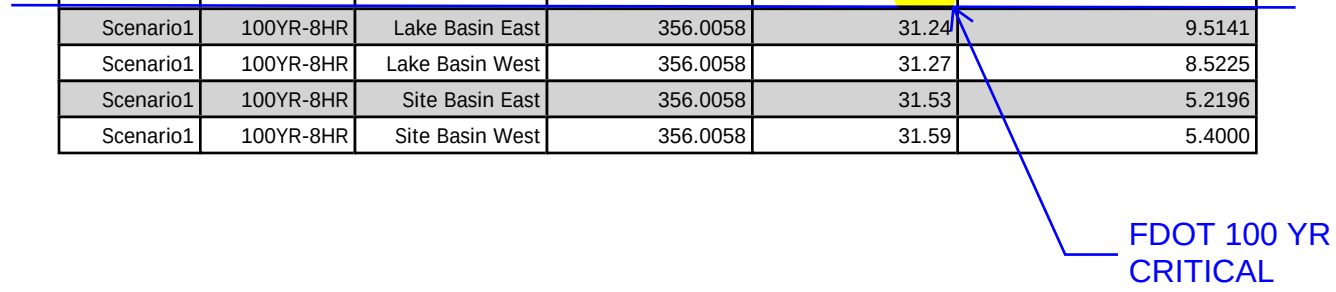
Comment:

25 Year, 3 Day Back to Back Node Max

Scenario	Sim	Node Name	Relative Time [hrs]	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	SFWMD 25YR-3 DAY	Lake Basin East	356.0008	31.64	72.5258
Scenario1	SFWMD 25YR-3 DAY	Lake Basin West	356.0008	31.64	72.4841
Scenario1	SFWMD 25YR-3 DAY	Site Basin East	356.0008	31.64	72.4008
Scenario1	SFWMD 25YR-3 DAY	Site Basin West	356.0008	31.65	72.3924

FDOT 100 Year Node Max

Scenario	Sim	Node Name	Relative Time [hrs]	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	100YR-1HR	Lake Basin East	356.0012	29.77	3.7745
Scenario1	100YR-1HR	Lake Basin West	356.0012	30.09	2.3420
Scenario1	100YR-1HR	Site Basin East	356.0012	31.29	1.3006
Scenario1	100YR-1HR	Site Basin West	356.0012	31.30	1.3183
Scenario1	100YR-24HR	Lake Basin East	356.0006	31.77	24.5089
Scenario1	100YR-24HR	Lake Basin West	356.0006	31.77	24.4256
Scenario1	100YR-24HR	Site Basin East	356.0006	31.77	24.3506
Scenario1	100YR-24HR	Site Basin West	356.0006	31.78	24.3423
Scenario1	100YR-2HR	Lake Basin East	356.0008	30.18	4.4433
Scenario1	100YR-2HR	Lake Basin West	356.0008	30.49	3.0431
Scenario1	100YR-2HR	Site Basin East	356.0008	31.38	1.8641
Scenario1	100YR-2HR	Site Basin West	356.0008	31.38	1.9473
Scenario1	100YR-4HR	Lake Basin East	356.0040	30.60	6.1210
Scenario1	100YR-4HR	Lake Basin West	356.0040	30.83	4.8873
Scenario1	100YR-4HR	Site Basin East	356.0040	31.42	3.3645
Scenario1	100YR-4HR	Site Basin West	356.0040	31.46	3.5331
Scenario1	100YR-72HR	Lake Basin East	356.0057	31.93	68.9557
Scenario1	100YR-72HR	Lake Basin West	356.0057	31.94	68.7140
Scenario1	100YR-72HR	Site Basin East	356.0057	31.94	68.6057
Scenario1	100YR-72HR	Site Basin West	356.0057	31.95	68.6140
Scenario1	100YR-8HR	Lake Basin East	356.0058	31.24	9.5141
Scenario1	100YR-8HR	Lake Basin West	356.0058	31.27	8.5225
Scenario1	100YR-8HR	Site Basin East	356.0058	31.53	5.2196
Scenario1	100YR-8HR	Site Basin West	356.0058	31.59	5.4000



Appendix L: Recovery Analysis

StormWise Recovery Analysis Design Assumptions and Calculations

Time/Stage Node-Dry Retention (Abatement Volume)
Dry Retention node set at initial stage required by BMP Trains. The volume required by BMP Trains is 0.81 ac-ft. Please see enclosed calculation noting retention elevation if 29.43 FT-NAVD provides 0.82 ac-ft for the project.

Saturated Horizontal Hydraulic Conductivity
Please see enclosed Geotechnical report and associated excerpts in this report. There 5 exfiltration tests conducted for the site with average of 57.53 ft/day which was used.

Unsaturated Vertical Hydraulic Conductivity
The MODRET Design manual provides recommendations for determining the vertical permeability, saturated and unsaturated, based on the known horizontal permeability and per the following guidelines and equations:

Saturated Horizontal Conductivity = K_H = 57.53 ft/day

Saturated Vertical Permeability (K_V) = Saturated Horizontal Permeability (K_H)/1.5
K_V = 57.53/1.5 = 38.35 ft/day

Unsaturated Vertical Permeability (K_{VU}) = Saturated Vertical Permeability (K_V)*(2/3)
K_{VU} = 38.35 ft*(2/3) = 25.57 ft/day

Aquifer Base Elevation

Elevation of effective aquifer base is the depth to the semi-conforming soil layer or poorly permeable soil layer that occurs below the stormwater retention pond area. A geotechnical report was completed by UES and the borings were reviewed. The depth to the effective base is approximately 12.3 feet below existing grade. The average existing grade is 28.1 FT-NAVD. See list of the borings with existing grade, depth to confining layer, and associated elevation enclosed. The average 15.8 FT-NAVD was used.

Percolation Link Inputs- Computation Parameters, Rectangular Pond Per StormWise Technical Reference Section 3.4.9.2.9.2. Dry retention 1 and 2 connected by large pipe and modeled as average. Dry retention 3 modeled as such.

Dry Retention 1
Perimeter 1: P1=2(L+W)=2(50+80)=260 ft
Perimeter 2: P1+2πr=426 + 2*π*50=574 ft
Perimeter 3: P1+2πr=426 + 2*π*450=3,086 ft
Distance P1 to P2: 50 ft
Distance P2 to P3: 450 ft
# of Cells P1 to P2: 5
# of Cells P2 to P3: 10

Dry Retention 2

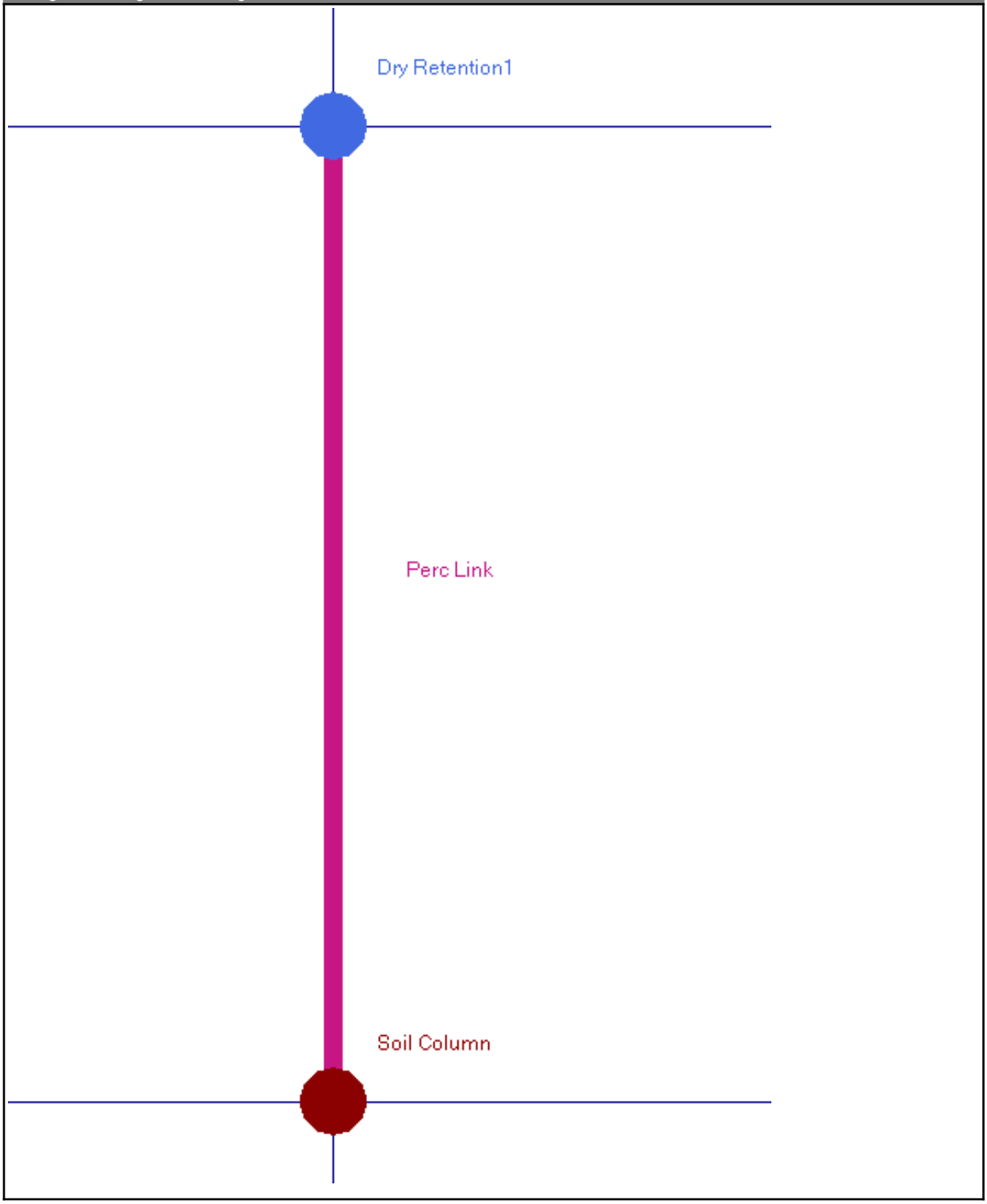
Perimeter 1: P1=2(L+W)=2(265+134)=788 ft
Perimeter 2: P1+2πr=818 + 2*π*50=1,102 ft
Perimeter 3: P1+2πr=818 + 2*π*450=3,614 ft
Distance P1 to P2: 50 ft
Distance P2 to P3: 450 ft
# of Cells P1 to P2: 5
# of Cells P2 to P3: 10

Dry Retention 3

Perimeter 1: P1=2(L+W)=2(485+35)=1,040 ft
Perimeter 2: P1+2πr=1,410 + 2*π*50=1,354 ft
Perimeter 3: P1+2πr=1,410 + 2*π*450=3,866 ft
Distance P1 to P2: 50 ft
Distance P2 to P3: 450 ft
# of Cells P1 to P2: 5
# of Cells P2 to P3: 10

StormWise Input Summary – Dry Retention 1

Background Image: Nodal Diagram



Node: Dry Retention1

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 29.43 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

INITIAL STAGE SET AT WEIR INVERT ELEVATION

Stage [ft]	Area [ac]	Area [ft2]
28.00	0.0100	436
29.00	0.1300	5663
31.00	0.2000	8712

Comment: Initial Stage set at retention elevation of 29.43 FT-NAVD

Node: Soil Column

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 28.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	28.00
0	0	0	72.0000	28.00
0	0	0	90.0000	28.00

Comment:

Percolation Link: Perc Link

Scenario: Scenario1
From Node: Dry Retention1
To Node: Soil Column
Link Count: 1
Flow Direction: Both
Aquifer Base Elevation: 15.80 ft
Water Table Elevation: 28.00 ft
Annual Recharge Rate: 0 ipy
Horizontal Conductivity: 57.530 fpd
Vertical Conductivity: 25.570 fpd
Fillable Porosity: 0.200
Layer Thickness: 0.00 ft

Surface Area Option: Use 1st Point in Stage/Area Table
Vertical Flow Termination: Horizontal Flow Algorithm
Perimeter 1: 260.00 ft
Perimeter 2: 574.00 ft
Perimeter 3: 3086.00 ft
Distance P1 to P2: 50.00 ft
Distance P2 to P3: 450.00 ft
# of Cells P1 to P2: 5
# of Cells P2 to P3: 10

Comment:

Simulation: Slug Load

Scenario: Scenario1
Run Date/Time: 1/14/2026 10:03:10 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	80.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

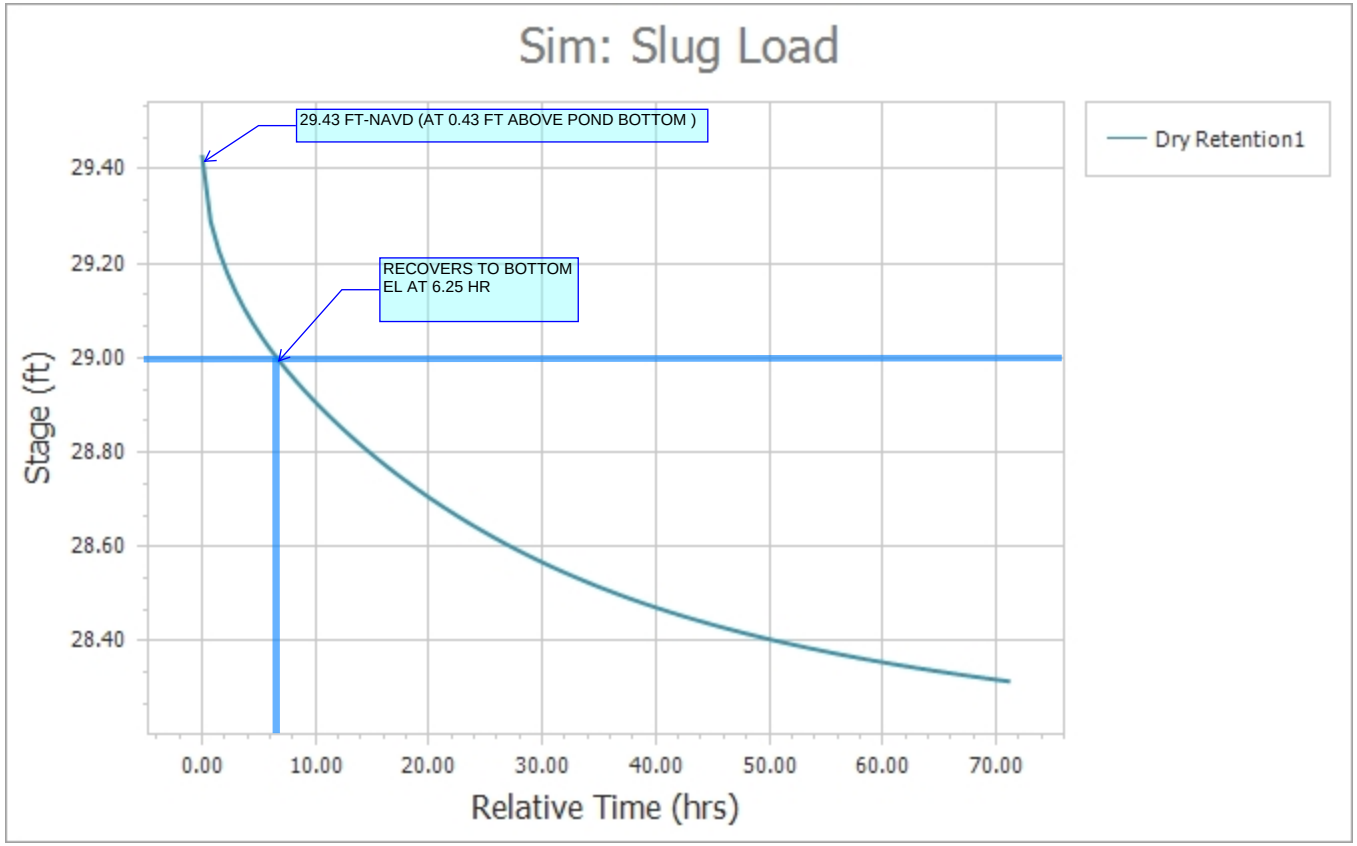
Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec

Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	No Rainfall
Link Optimizer Tol:	0.0001 ft		
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

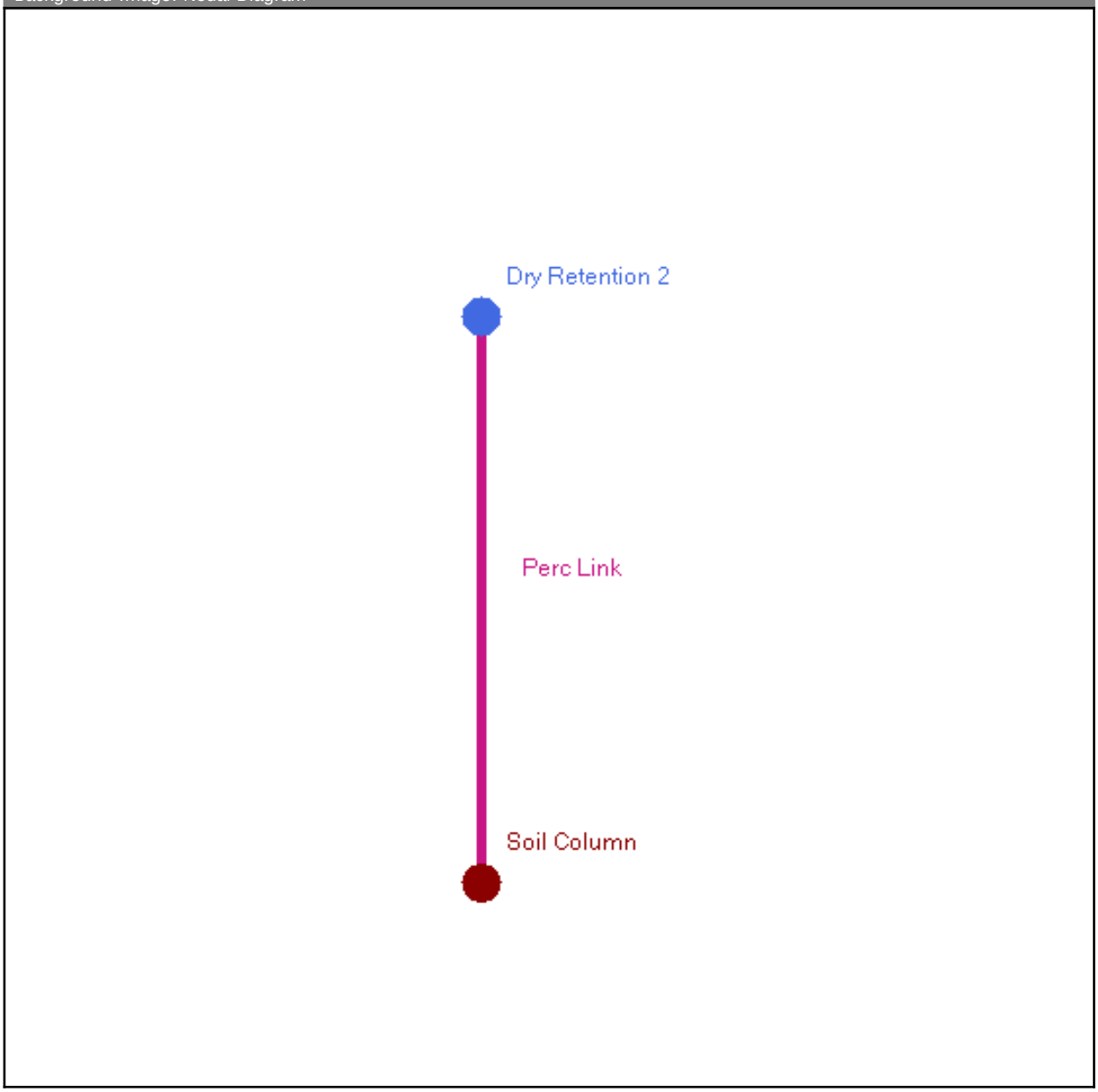
Comment:

Recovery Chart – StormWise - Dry Retention 1


Sim: Slug Load
29.43 FT-NAVD (AT 0.43 FT ABOVE POND BOTTOM)
RECOVERS TO BOTTOM
EL AT 6.25 HR
Dry Retention1
Stage (ft)
29.40
29.20
29.00
28.80
28.60
28.40
0.00
10.00
20.00
30.00
40.00
50.00
60.00
70.00
Relative Time (hrs)

StormWise Input Summary – Dry Retention 2

Background Image: Nodal Diagram



Node: Dry Retention 2

Scenario:	Scenario1
Type:	Stage/Area
Base Flow:	0.00 cfs
Initial Stage:	29.43 ft
Warning Stage:	0.00 ft
Alert Stage:	0.00 ft

INITIAL STAGE SET AT WEIR INVERT ELEVATION

Stage [ft]	Area [ac]	Area [ft2]
28.00	0.0100	436
29.00	0.8000	34848
31.00	0.9500	41382

Comment:

Node: Soil Column

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 28.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	28.00
0	0	0	72.0000	28.00
0	0	0	90.0000	28.00

Comment:

Percolation Link: Perc Link

Scenario: Scenario1
From Node: Dry Retention 2
To Node: Soil Column
Link Count: 1
Flow Direction: Both
Aquifer Base Elevation: 15.80 ft
Water Table Elevation: 28.00 ft
Annual Recharge Rate: 0 ipy
Horizontal Conductivity: 57.530 fpd
Vertical Conductivity: 25.570 fpd
Fillable Porosity: 0.200
Layer Thickness: 0.00 ft

Surface Area Option: Use 1st Point in Stage/Area Table
Vertical Flow Termination: Horizontal Flow Algorithm
Perimeter 1: 788.00 ft
Perimeter 2: 1102.00 ft
Perimeter 3: 3614.00 ft
Distance P1 to P2: 50.00 ft
Distance P2 to P3: 450.00 ft
# of Cells P1 to P2: 5
# of Cells P2 to P3: 10

Comment:

Simulation: Slug Load

Scenario: Scenario1
Run Date/Time: 2/6/2026 9:04:45 AM

Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	80.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

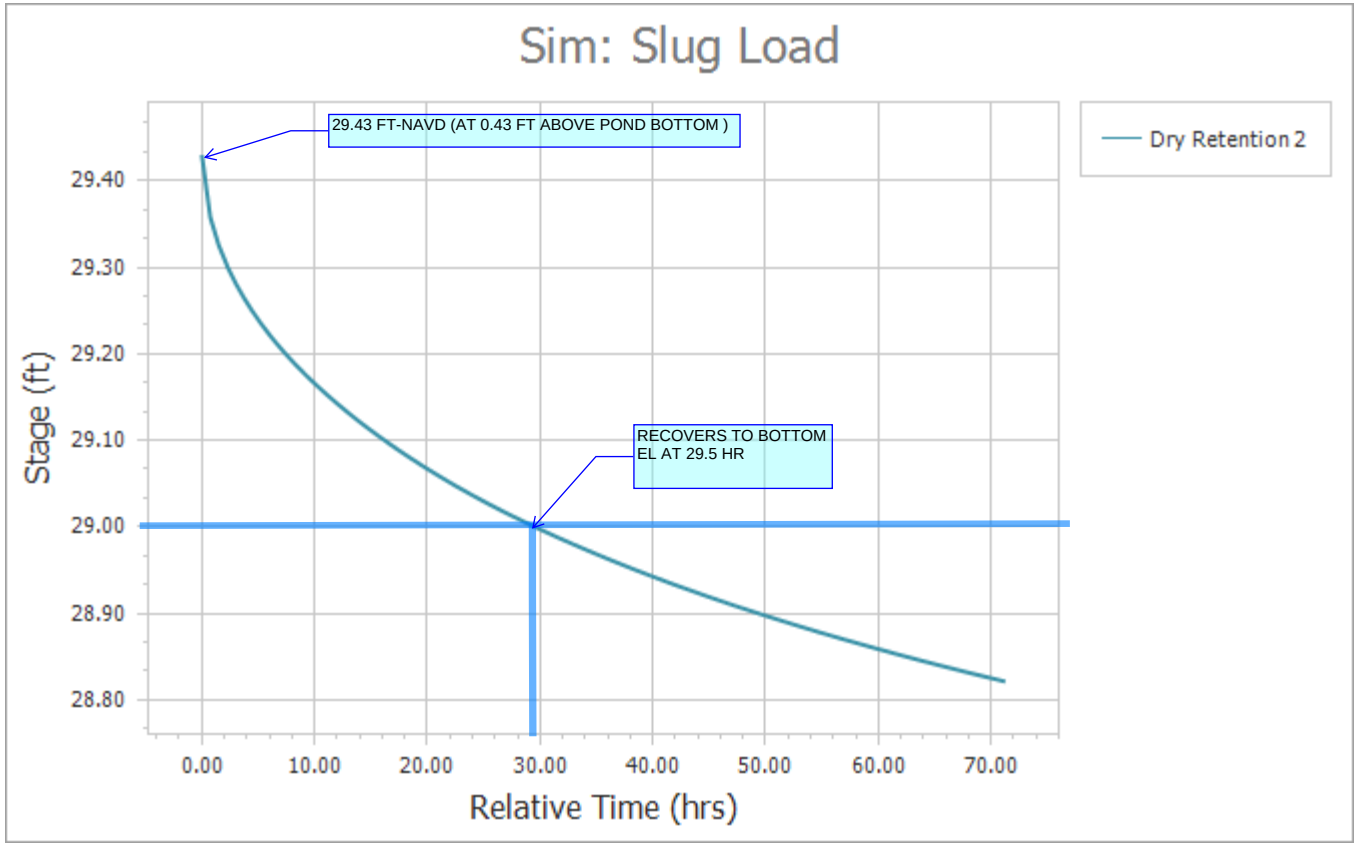
Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: No Rainfall

Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (1D):	100 ft2
Energy Switch (1D):	Energy

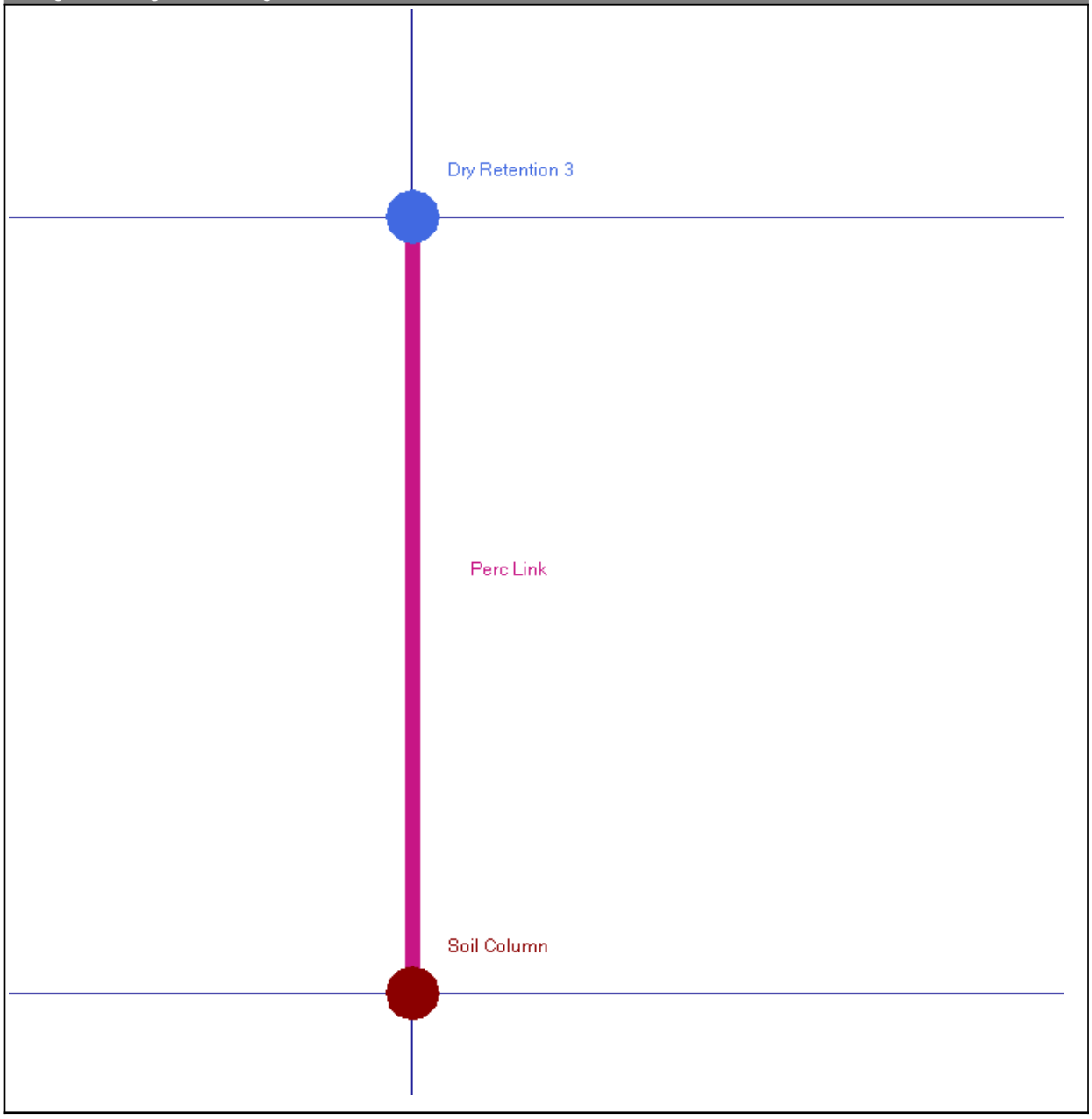
Comment:

Recovery Chart – StormWise - Dry Retention 2


Sim: Slug Load
29.43 FT-NAVD (AT 0.43 FT ABOVE POND BOTTOM)
RECOVERS TO BOTTOM EL AT 29.5 HR
Dry Retention 2
Stage (ft)
29.40
29.30
29.20
29.10
29.00
28.90
28.80
0.00
10.00
20.00
30.00
40.00
50.00
60.00
70.00
Relative Time (hrs)

StormWise Input Summary – Dry Retention 3

Background Image: Nodal Diagram



Node: Dry Retention 3

Scenario:	Scenario1
Type:	Stage/Area
Base Flow:	0.00 cfs
Initial Stage:	29.43 ft

INITIAL STAGE SET AT WEIR INVERT ELEVATION

Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
28.00	0.0100	436
29.00	0.6930	30187
31.00	1.0500	45738

Comment:

Node: Soil Column

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 28.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	28.00
0	0	0	72.0000	28.00
0	0	0	90.0000	28.00

Comment:

Percolation Link: Perc Link

Scenario: Scenario1
From Node: Dry Retention 3
To Node: Soil Column
Link Count: 1
Flow Direction: Both
Aquifer Base Elevation: 15.80 ft
Water Table Elevation: 28.00 ft
Annual Recharge Rate: 0 ipy
Horizontal Conductivity: 57.530 fpd
Vertical Conductivity: 25.570 fpd
Fillable Porosity: 0.200
Layer Thickness: 0.00 ft

Surface Area Option: Use 1st Point in Stage/Area Table
Vertical Flow Termination: Horizontal Flow Algorithm
Perimeter 1: 1040.00 ft
Perimeter 2: 1354.00 ft
Perimeter 3: 3866.00 ft
Distance P1 to P2: 50.00 ft
Distance P2 to P3: 450.00 ft
# of Cells P1 to P2: 5
# of Cells P2 to P3: 10

Comment:

Simulation: Slug Load

Scenario:	Scenario1
Run Date/Time:	2/6/2026 9:39:00 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	80.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	45.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:	
Unit Hydrograph Folder:	

Lookup Tables

Boundary Stage Set:	
Extern Hydrograph Set:	
Curve Number Set:	
Green-Ampt Set:	
Vertical Layers Set:	
Impervious Set:	

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain	No Rainfall

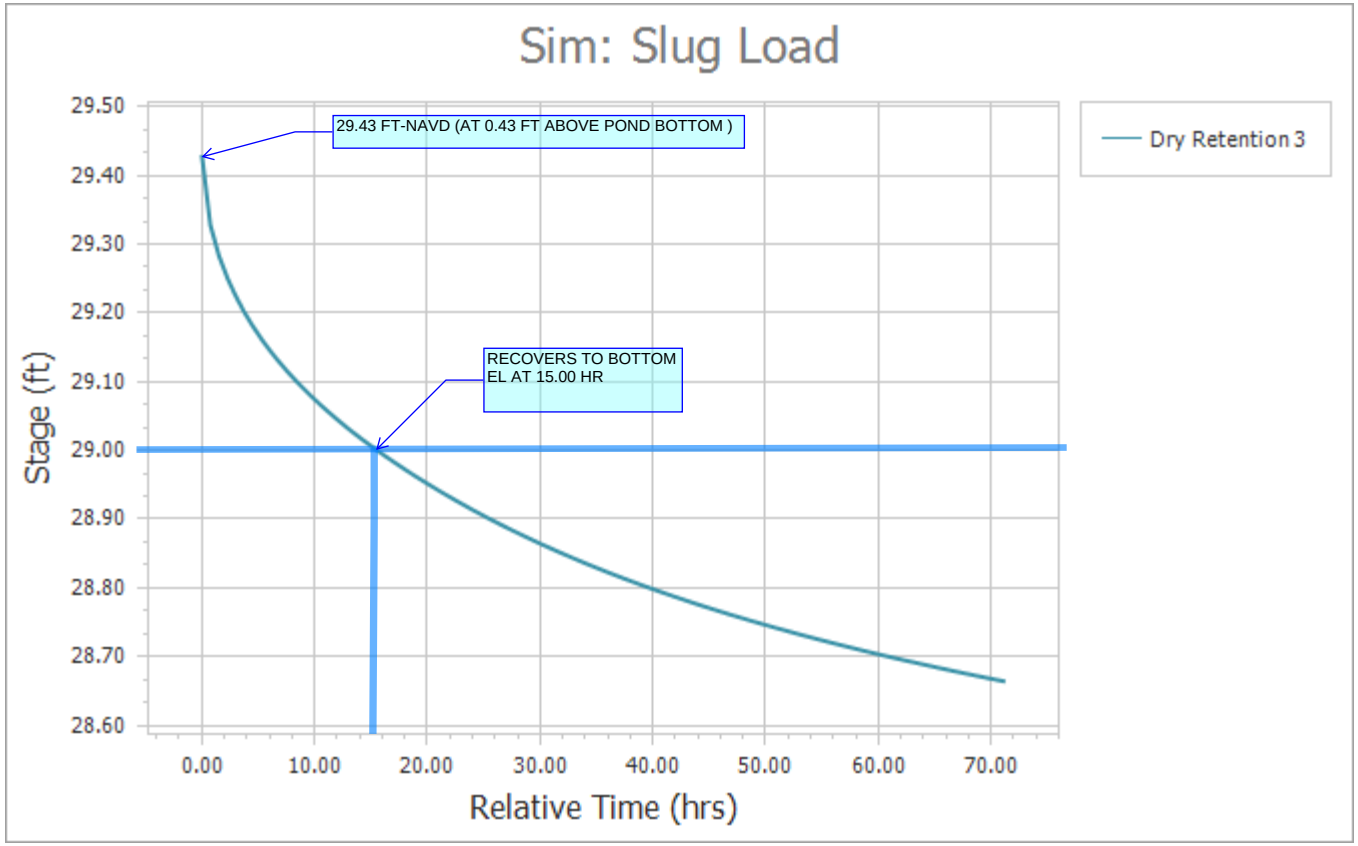
Opt:

Link Optimizer Tol: 0.0001 ft

Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (1D):	100 ft2
Energy Switch (1D):	Energy

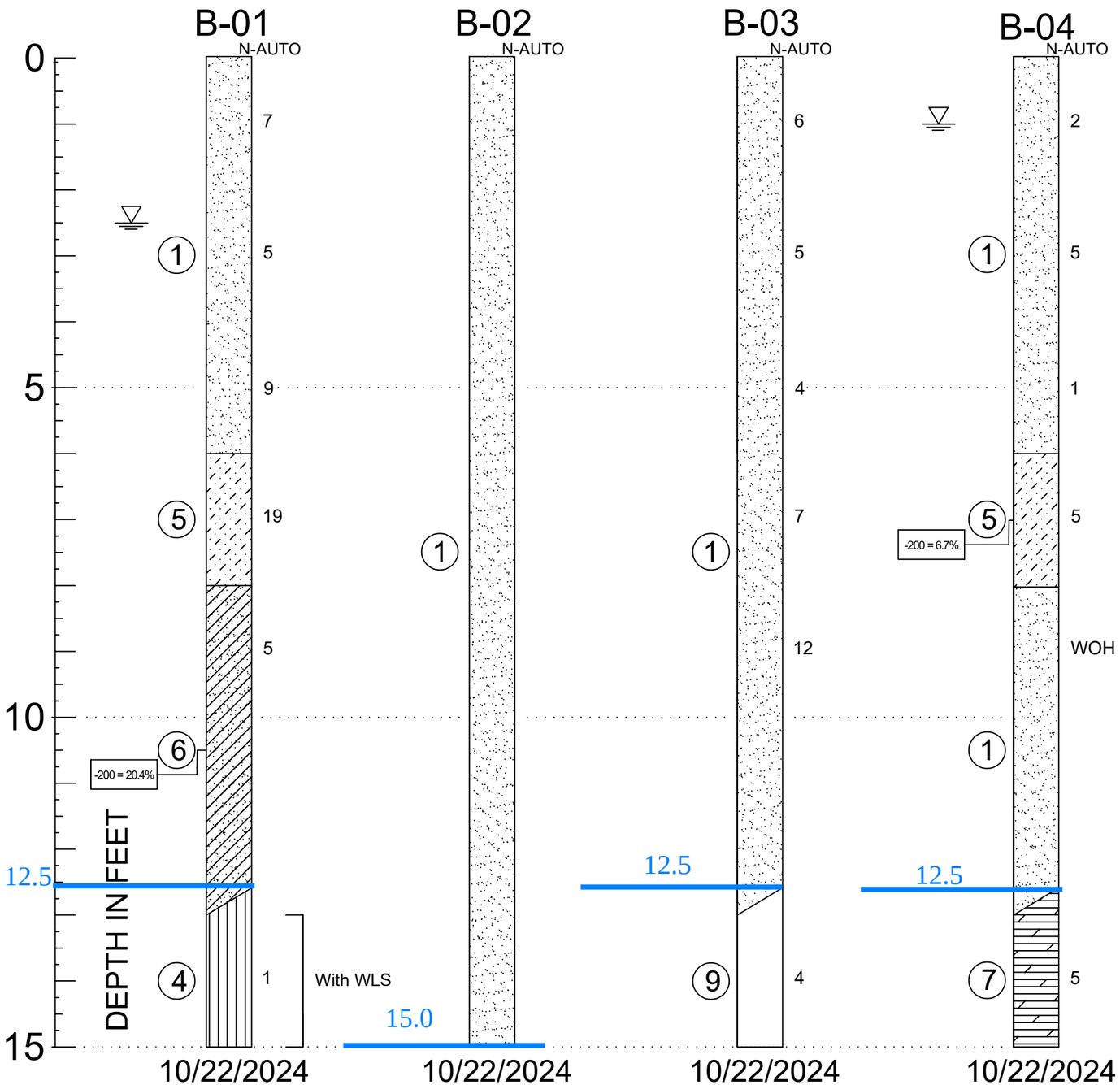
Comment:

Recovery Chart – StormWise - Dry Retention 3

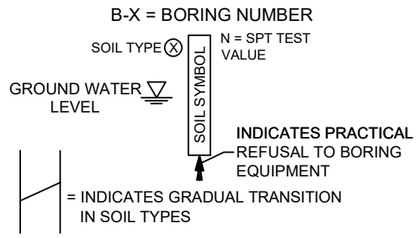

Sim: Slug Load
29.43 FT-NAVD (AT 0.43 FT ABOVE POND BOTTOM)
RECOVERS TO BOTTOM EL AT 15.00 HR
Dry Retention 3
Stage (ft)
29.50
29.40
29.30
29.20
29.10
29.00
28.90
28.80
28.70
28.60
0.00
10.00
20.00
30.00
40.00
50.00
60.00
70.00
Relative Time (hrs)

Soil Borings for Effective Aquifer Base

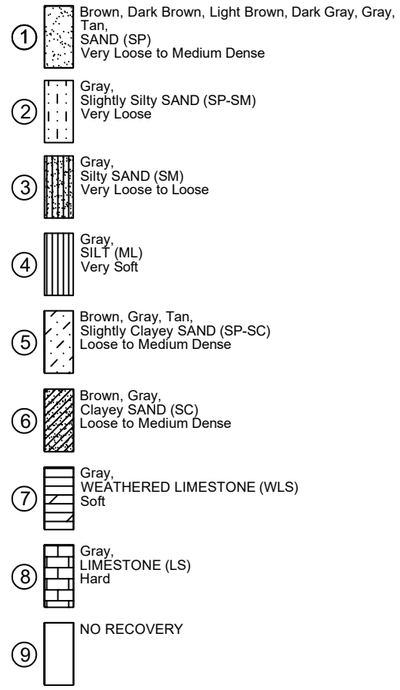
SOIL PROFILES



SOIL PROFILE LEGEND



SOIL LEGEND



NOTES:

N - STANDARD PENETRATION RESISTANCE TEST (SPT) VALUE. NUMBERS TO THE RIGHT OF BORINGS INDICATE SPT VALUE FOR 12-INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).

WOH - BORING INTERVAL ADVANCED UNDER WEIGHT OF HAMMER.

LFC - LOSS OF DRILLING FLUID CIRCULATION.

WLS - WEATHERED LIMESTONE

HD - HARD DRILLING

-200 - AMOUNT OF SOIL FINES PASSING NO. 200 SIEVE

MC - MOISTURE CONTENT

HA - NO RECORDED N VALUE DUE TO HAND AUGERING PROCEDURE

SOIL CLASSIFICATION

CORRELATION OF N - VALUES WITH RELATIVE DENSITY AND CONSISTENCY

COHESIONLESS SOIL

N - VALUE (SAFETY)	N - VALUE (AUTO)	RELATIVE DENSITY
0 - 3	0 - 3	VERY LOOSE
4 - 10	4 - 8	LOOSE
11 - 30	9 - 24	MEDIUM DENSE
31 - 50	25 - 40	DENSE
OVER 50	OVER 40	VERY DENSE

SILTS AND CLAYS

N - VALUE (SAFETY)	N - VALUE (AUTO)	CONSISTENCY
0 - 2	0 - 1	VERY SOFT
3 - 5	2 - 4	SOFT
6 - 7	5 - 6	FIRM
8 - 15	7 - 12	STIFF
16 - 30	13 - 24	VERY STIFF
OVER 30	OVER 24	HARD

CORRELATION OF N - VALUES WITH HARDNESS DESCRIPTION

LIMEROCK

N - VALUE	RELATIVE DENSITY
0 - 50	SOFT
51 - 50/0"	HARD

APPROXIMATE FINES CONTENT / MODIFIERS

5% TO 12%	SLIGHTLY SILTY OR SLIGHTLY CLAYEY
12% TO 25%	SILTY OR CLAYEY
26% TO 49%	VERY SILTY OR VERY CLAYEY

APPROXIMATE SHELL CONTENT / MODIFIERS

0% TO 5%	NO MENTION
5% TO 12%	SLIGHTLY SHELLY
13% TO 30%	SHELLY
31% TO 50%	VERY SHELLY

APPROXIMATE ORGANIC CONTENT / MODIFIERS

0% TO 2.5%	NO MENTION
2.5% TO 5%	WITH A TRACE
5% TO 20%	WITH ORGANICS
20% TO 75%	HIGHLY ORGANIC
75% TO 100%	PEAT

DEFINITION OF DESCRIPTIVE TERMS OF MODIFIERS FOR SILTS/CLAYS/SHELLS/GRAVELS ARE DESCRIBED AS FOLLOWS:

PERCENTAGE OF MODIFIER MATERIAL	FIRST QUALIFIER	SECOND QUALIFIER
5 - 12	SLIGHTLY + MODIFIER + Y	WITH A LITTLE
12 - 30	MODIFIER + Y	WITH SOME
30 - 50	VERY + MODIFIER + Y	AND

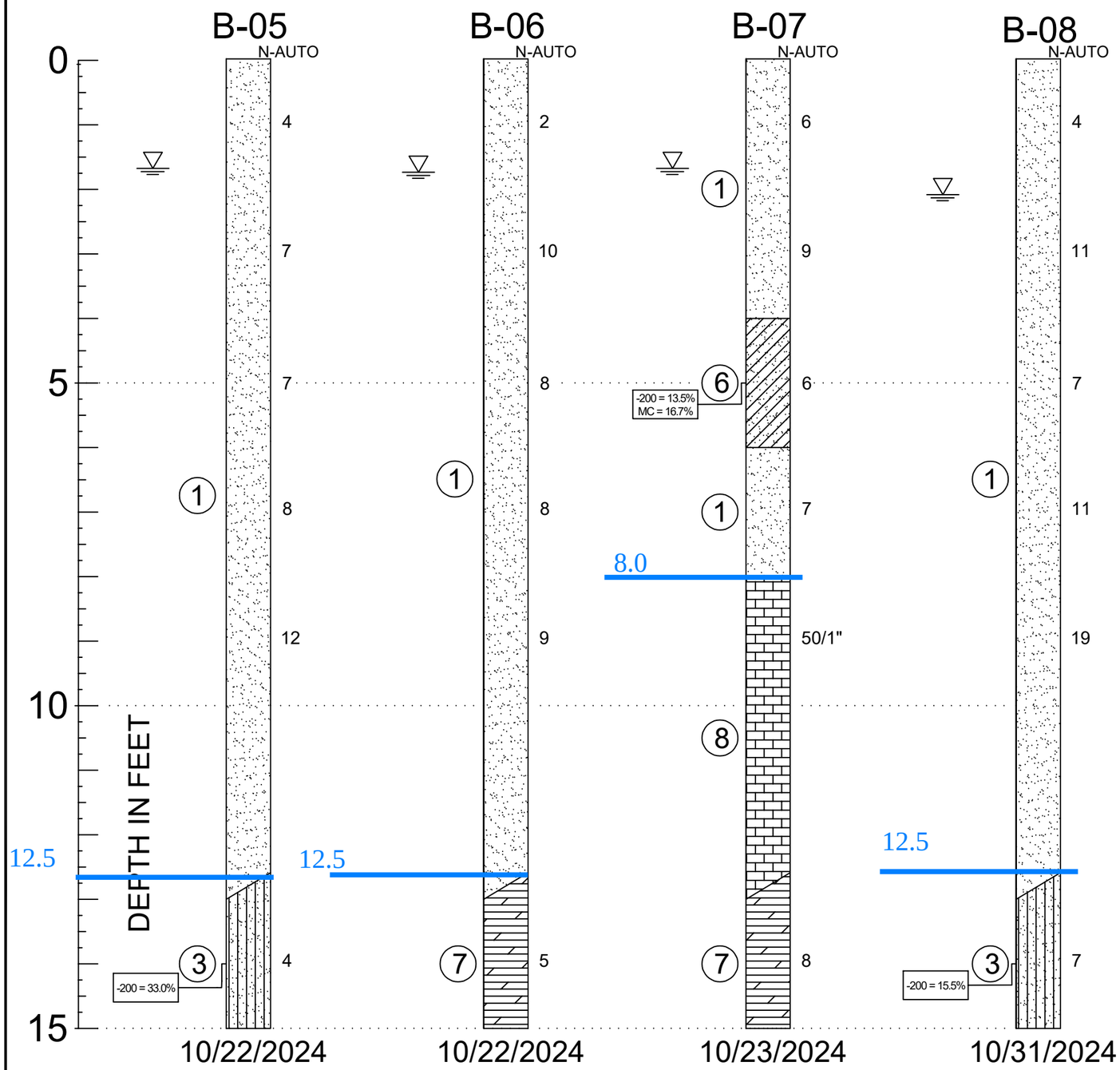
RECORD OF TEST BORINGS

UES
201 Waldo Ave. N.
Lehigh Acres, Florida 33971
239-489-2443
www.teamues.com

Client: Casto Southeast Realty Services
Project No: 0530.2400296.0000
Project:
Blackstone Retail
SR-82 & Blackstone Drive
Lehigh Acres, Lee County, Florida

Date: 12/03/2024
RIG: GP-7822/CME-45/55
Drilled By: LSJ/LS/DB
Drawn By: AM
Approved By: AJD

SOIL PROFILES



SOIL PROFILE LEGEND

B-X = BORING NUMBER

SOIL TYPE (X) — SOIL SYMBOL — N = SPT TEST VALUE

GROUND WATER LEVEL

INDICATES PRACTICAL REFUSAL TO BORING EQUIPMENT

= INDICATES GRADUAL TRANSITION IN SOIL TYPES

SOIL LEGEND

1. Brown, Dark Brown, Light Brown, Dark Gray, Gray, Tan, SAND (SP) Very Loose to Medium Dense
2. Gray, Slightly Silty SAND (SP-SM) Very Loose
3. Gray, Silty SAND (SM) Very Loose to Loose
4. Gray, SILT (ML) Very Soft
5. Brown, Gray, Tan, Slightly Clayey SAND (SP-SC) Loose to Medium Dense
6. Brown, Gray, Clayey SAND (SC) Loose to Medium Dense
7. Gray, WEATHERED LIMESTONE (WLS) Soft
8. Gray, LIMESTONE (LS) Hard
9. NO RECOVERY

NOTES:

N - STANDARD PENETRATION RESISTANCE TEST (SPT) VALUE. NUMBERS TO THE RIGHT OF BORINGS INDICATE SPT VALUE FOR 12-INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).

WOH - BORING INTERVAL ADVANCED UNDER WEIGHT OF HAMMER.

LFC - LOSS OF DRILLING FLUID CIRCULATION.

WLS - WEATHERED LIMESTONE

HD - HARD DRILLING

-200 - AMOUNT OF SOIL FINES PASSING NO. 200 SIEVE

MC - MOISTURE CONTENT

HA - NO RECORDED N VALUE DUE TO HAND AUGERING PROCEDURE

SOIL CLASSIFICATION

CORRELATION OF N - VALUES WITH RELATIVE DENSITY AND CONSISTENCY

COHESIONLESS SOIL

N - VALUE (SAFETY)	N - VALUE (AUTO)	RELATIVE DENSITY
0 - 3	0 - 3	VERY LOOSE
4 - 10	4 - 8	LOOSE
11 - 30	9 - 24	MEDIUM DENSE
31 - 50	25 - 40	DENSE
OVER 50	OVER 40	VERY DENSE

SILTS AND CLAYS

N - VALUE (SAFETY)	N - VALUE (AUTO)	CONSISTENCY
0 - 2	0 - 1	VERY SOFT
3 - 5	2 - 4	SOFT
6 - 7	5 - 6	FIRM
8 - 15	7 - 12	STIFF
16 - 30	13 - 24	VERY STIFF
OVER 30	OVER 24	HARD

CORRELATION OF N - VALUES WITH HARDNESS DESCRIPTION

LIMEROCK

N - VALUE	RELATIVE DENSITY
0 - 50	SOFT
51 - 50/0"	HARD

APPROXIMATE FINES CONTENT / MODIFIERS

FINES CONTENT	MODIFIERS
5% TO 12%	SLIGHTLY SILTY OR SLIGHTLY CLAYEY
12% TO 25%	SILTY OR CLAYEY
26% TO 49%	VERY SILTY OR VERY CLAYEY

APPROXIMATE SHELL CONTENT / MODIFIERS

SHELL CONTENT	MODIFIERS
0% TO 5%	NO MENTION
5% TO 12%	SLIGHTLY SHELLY
13% TO 30%	SHELLY
31% TO 50%	VERY SHELLY

APPROXIMATE ORGANIC CONTENT / MODIFIERS

ORGANIC CONTENT	MODIFIERS
0% TO 2.5%	NO MENTION
2.5% TO 5%	WITH A TRACE
5% TO 20%	WITH ORGANICS
20% TO 75%	HIGHLY ORGANIC
75% TO 100%	PEAT

DEFINITION OF DESCRIPTIVE TERMS OF MODIFIERS FOR SILTS/CLAYS/SHELLS/GRAVELS ARE DESCRIBED AS FOLLOWS:

PERCENTAGE OF MODIFIER MATERIAL	FIRST QUALIFIER	SECOND QUALIFIER
5 - 12	SLIGHTLY + MODIFIER + Y	WITH A LITTLE
12 - 30	MODIFIER + Y	WITH SOME
30 - 50	VERY + MODIFIER + Y	AND

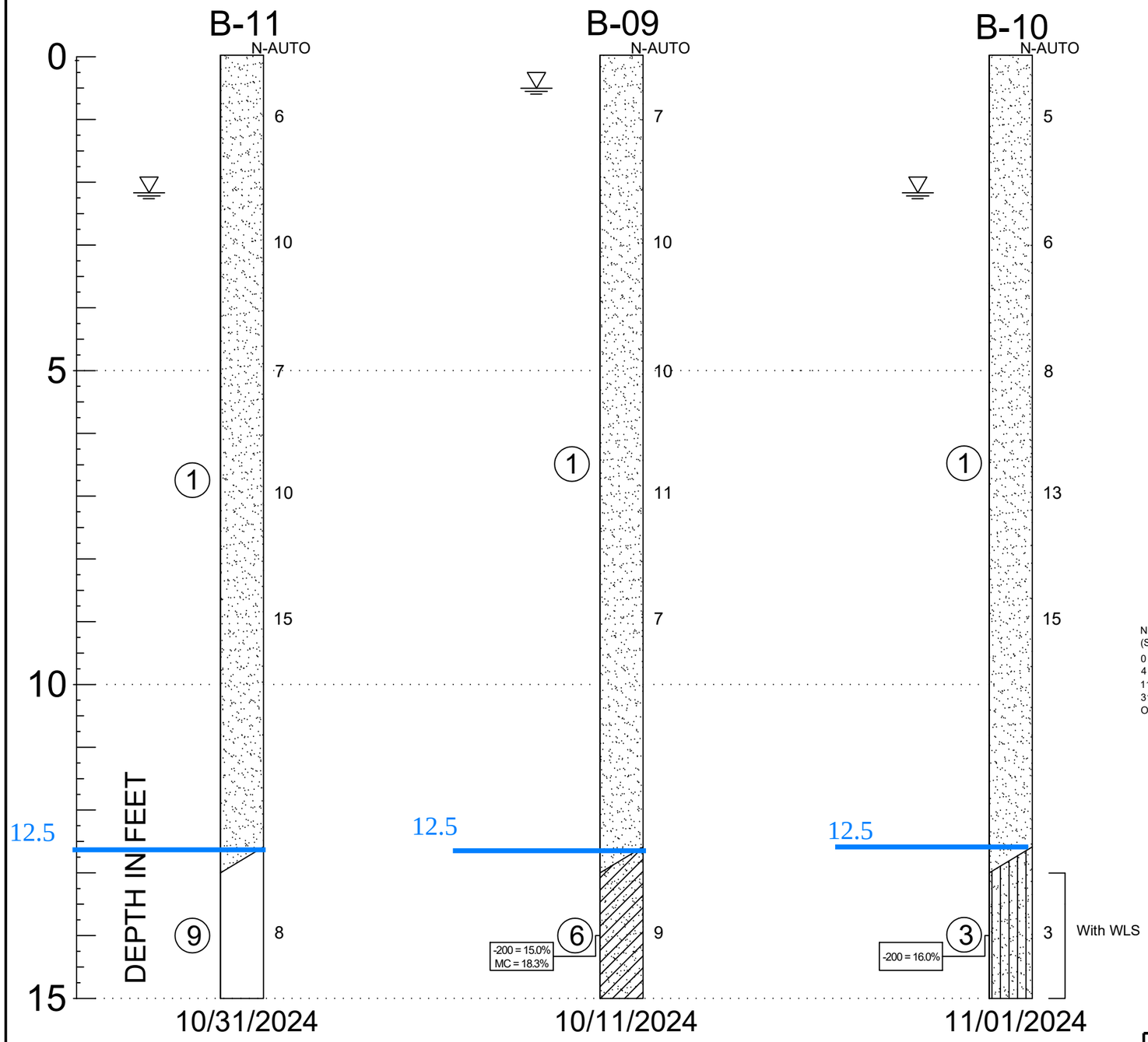
RECORD OF TEST BORINGS

UES
201 Waldo Ave. N.
Lehigh Acres, Florida 33971
239-489-2443
www.teamues.com

Client: Casto Southeast Realty Services
Project No: 0530.2400296.0000
Project:
Blackstone Retail
SR-82 & Blackstone Drive
Lehigh Acres, Lee County, Florida

Date: 12/03/2024
RIG: GP-7822/CME-45/55
Drilled By: LSJ/LS/DB
Drawn By: AM
Approved By: AJD

SOIL PROFILES



SOIL PROFILE LEGEND

B-X = BORING NUMBER

SOIL TYPE (X) — SOIL SYMBOL — N = SPT TEST VALUE

GROUND WATER LEVEL

INDICATES PRACTICAL REFUSAL TO BORING EQUIPMENT

= INDICATES GRADUAL TRANSITION IN SOIL TYPES

NOTES:

N - STANDARD PENETRATION RESISTANCE TEST (SPT) VALUE. NUMBERS TO THE RIGHT OF BORINGS INDICATE SPT VALUE FOR 12-INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).

WOH - BORING INTERVAL ADVANCED UNDER WEIGHT OF HAMMER.

LFC - LOSS OF DRILLING FLUID CIRCULATION.

WLS - WEATHERED LIMESTONE

HD - HARD DRILLING

-200 - AMOUNT OF SOIL FINES PASSING NO. 200 SIEVE

MC - MOISTURE CONTENT

HA - NO RECORDED N VALUE DUE TO HAND AUGERING PROCEDURE

SOIL LEGEND

1. Brown, Dark Brown, Light Brown, Dark Gray, Gray, Tan, SAND (SP) Very Loose to Medium Dense
2. Gray, Slightly Silty SAND (SP-SM) Very Loose
3. Gray, Silty SAND (SM) Very Loose to Loose
4. Gray, SILT (ML) Very Soft
5. Brown, Gray, Tan, Slightly Clayey SAND (SP-SC) Loose to Medium Dense
6. Brown, Gray, Clayey SAND (SC) Loose to Medium Dense
7. Gray, WEATHERED LIMESTONE (WLS) Soft
8. Gray, LIMESTONE (LS) Hard
9. NO RECOVERY

SOIL CLASSIFICATION

CORRELATION OF N - VALUES WITH RELATIVE DENSITY AND CONSISTENCY

COHESIONLESS SOIL			SILTS AND CLAYS		
N - VALUE (SAFETY)	N - VALUE (AUTO)	RELATIVE DENSITY	N - VALUE (SAFETY)	N - VALUE (AUTO)	CONSISTENCY
0 - 3	0 - 3	VERY LOOSE	0 - 2	0 - 1	VERY SOFT
4 - 10	4 - 8	LOOSE	3 - 5	2 - 4	SOFT
11 - 30	9 - 24	MEDIUM DENSE	6 - 7	5 - 6	FIRM
31 - 50	25 - 40	DENSE	8 - 15	7 - 12	STIFF
OVER 50	OVER 40	VERY DENSE	16 - 30	13 - 24	VERY STIFF
			OVER 30	OVER 24	HARD

CORRELATION OF N - VALUES WITH HARDNESS DESCRIPTION

LIMEROCK	
N - VALUE	RELATIVE DENSITY
0 - 50	SOFT
51 - 50/0"	HARD

APPROXIMATE FINES CONTENT	MODIFIERS
5% TO 12%	SLIGHTLY SILTY OR SLIGHTLY CLAYEY
12% TO 25%	SILTY OR CLAYEY
26% TO 49%	VERY SILTY OR VERY CLAYEY

APPROXIMATE SHELL CONTENT	MODIFIERS
0% TO 5%	NO MENTION
5% TO 12%	SLIGHTLY SHELLY
13% TO 30%	SHELLY
31% TO 50%	VERY SHELLY

APPROXIMATE ORGANIC CONTENT	MODIFIERS
0% TO 2.5%	NO MENTION
2.5% TO 5%	WITH A TRACE
5% TO 20%	WITH ORGANICS
20% TO 75%	HIGHLY ORGANIC
75% TO 100%	PEAT

DEFINITION OF DESCRIPTIVE TERMS OF MODIFIERS FOR SILTS/CLAYS/SHELLS/GRAVELS ARE DESCRIBED AS FOLLOWS:

PERCENTAGE OF MODIFIER MATERIAL	FIRST QUALIFIER	SECOND QUALIFIER
5 - 12	SLIGHTLY + MODIFIER + Y	WITH A LITTLE
12 - 30	MODIFIER + Y	WITH SOME
30 - 50	VERY + MODIFIER + Y	AND

RECORD OF TEST BORINGS

UES
201 Waldo Ave. N.
Lehigh Acres, Florida 33971
239-489-2443
www.teamues.com

Client: Casto Southeast Realty Services
Project No: 0530.2400296.0000
Project:
Blackstone Retail
SR-82 & Blackstone Drive
Lehigh Acres, Lee County, Florida

Date: 12/03/2024
RIG: GP-7822/CME-45/55
Drilled By: LSJ/LS/DB
Drawn By: AM
Approved By: AJD

SURVEY PLAT

OF

A TRACT OR PARCEL OF LAND LYING IN
SECTION 20, TOWNSHIP 45 SOUTH, RANGE 27 EAST,
LEE COUNTY, FLORIDA

LEGEND:

PG.	INDICATES PAGE
PGS.	INDICATES PAGES
MB	INDICATES MAIL BOX
FV	INDICATES FIRE VALVE
LP	INDICATES LIGHT POLE
P.B.	INDICATES PLAT BOOK
S.R.	INDICATES STATE ROAD
CB	INDICATES CATCH BASIN
WV	INDICATES WATER VALVE
WM	INDICATES WATER METER
FH	INDICATES FIRE HYDRANT
R/W	INDICATES RIGHT-OF-WAY
28	INDICATES NOT NUMBER 28
-OH-	INDICATES OVERHEAD LINES
U.E.	INDICATES UTILITY EASEMENT
FOB	INDICATED FIBER OPTICS BOX
O.R.	INDICATES OFFICIAL RECORDS
GWA	INDICATES GUY WIRE ANCHOR
WPP	INDICATES WOOD POWER POLE
FOR	INDICATES FIBER OPTICS RISER
ES	INDICATES ELECTRICAL SERVICE
P.O.B.	INDICATES POINT OF BEGINNING
FOV	INDICATES FIBER OPTICS VAULT
D.E.	INDICATES DRAINAGE EASEMENT
I.N.	INDICATES INSTRUMENT NUMBER
MES	INDICATES MITERED END SECTION
SMH	INDICATES STORMWATER MANHOLE
ID.#	INDICATES IDENTIFICATION NUMBER
P.I.	INDICATES POINT OF INTERSECTION
OPP	INDICATES CONCRETE POWER POLE
P.O.C.	INDICATES POINT OF COMMENCEMENT
TSR	INDICATES TELEPHONE SERVICE RISER
CATV	INDICATES CABLE TELEVISION SERVICE
SSMH	INDICATES SANITARY SEWER MANHOLE
L1	INDICATES LIEN DATA: SEE LINE TABLE
RCP	INDICATES REINFORCED CONCRETE PIPE
ESH	INDICATES ELECTRIC SERVICE HANDHOLE
C1	INDICATES CURVE DATA: SEE CURVE TABLE
P.S.M.	INDICATES PROFESSIONAL SURVEYOR AND MAPPER
F.D.O.T.	INDICATES FLORIDA DEPARTMENT OF TRANSPORTATION
NAVD 88	INDICATES NORTH AMERICAN VERTICAL DATUM OF 1988
CORS	INDICATES CONTINUOUSLY OPERATING REFERENCE STATION
I.R.	INDICATES 5/8" IRON ROD "ATWELL LB 7832" UNLESS OTHERWISE NOTED
N:802603.30 E:774404.02	INDICATES THE "STATE PLANE COORDINATE SYSTEM" FLORIDA ZONE WEST NAD 83/2011 (CORS)

CURVE TABLE

CURVE	RADIUS	DELTA	ARC	CHORD	CHORD BEARING
C1	30.00'	90°00'00"	47.12'	42.43'	N 84°52'15" E
C2	1045.92'	25°00'22"	456.48'	452.86'	S 37°37'34" E

NOTE: ALL LOT, BLOCK, PARCELS, TRACTS, AND EASEMENTS, UNLESS OTHERWISE NOTED, ARE PART OF UNITS 1 THRU 73, MIRROR LAKES, S SUBDIVISION OF LEHIGH ACRES, RECORDED IN PLAT BOOK 27, PAGES 83 THRU 160, PUBLIC RECORDS OF LEE COUNTY, FLORIDA.(ITEM 6&7)

SEE SHEET 1 OF 3 FOR SURVEYOR'S NOTES AND CERTIFICATION

SEE SHEET 3 OF 3 FOR DETAILS

NO.	DATE	REVISION DESCRIPTION	BY

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PARKWAY – SUITE 101
FORT MYERS, FLORIDA 33966
(239) 939-5490 – LB# 7832

ALTA/NSPS LAND TITLE SURVEY

FORT MYERS BLACKSTONE

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DRAWN	CHECKED	SCALE	SHEET	OF	FILE NO. (S-T-R)
07-09-2024	24003608	ALTA-NSPS SR	JPB	AMV	1" = 150'	2	3	20-45-27

Ksat from Testing

2.6 Exfiltration Test Results

As requested, UES performed five (5) constant head usual open hole tests at the desired locations. The detailed test results are attached in "Appendix F: Laboratory & Exfiltration Testing Results". A summary of the test results of the SFWMD's Usual Open Hole Test, the hydraulic conductivity rates are shown in Table 2 below.

TABLE 2: EXFILTRATION TEST RESULTS		
Test Location	**Hydraulic Conductivity Result**	AVERAGE=57.53 FT/DAY
EX-1	6.23E-04 cfs/ft^2-ft.head	53.83 ft/day
EX-2	7.29E-04 cfs/ft^2-ft.head	62.99 ft/day
EX-3	4.98E-04 cfs/ft^2-ft.head	43.03 ft/day
EX-4	6.21E-04 cfs/ft^2-ft.head	53.65 ft/day
EX-5	8.58E-04 cfs/ft^2-ft.head	74.13 ft/day

UES performs hydraulic conductivity tests using generally accepted practices of the local stormwater management system design. However, the user of this information is cautioned that the potential variability of results and reproducibility associated with these influencing the permeability of a soil. These factors include, but are not limited to, soil grain size, soil particle arrangement and structure, dispersion of soil fines, density, and degree of saturation, soil heterogeneity, and soil anisotropy. Also, the permeability measured by such tests may not be representative of that of the total effective aquifer thickness. Factors of safety can compensate for part of the inherent test limitations but the designer must exercise judgment regarding final selection and applicability of provided soil design input parameters. Should the modeling analysis indicate marginally acceptable compliance with Water Management District design criteria, it may be advisable to perform more extensive and representative in-situ permeability testing by installing grouted piezometers or wells for slug testing. UES can perform these field tests if desired.

3.0 ENGINEERING EVALUATION AND RECOMMENDATIONS

3.1 General

A foundation system for any structure must be designed to resist bearing capacity failures, have settlements that are tolerable, and resist the environmental forces that the foundation may be subjected to over the life of the structure.

The soil bearing capacity is the soil's ability to support loads without plunging into the soil profile. Bearing capacity failures are analogous to shear failures in structural design and are usually sudden and catastrophic.

Appendix M: Nutrient Loading Report

BMP Trains Report

Complete Report (not including cost) Ver 5.3.2

Project: Blackstone Corporate Park
Date: 2/6/2026 7:13:08 AM

Site and Catchment Information

Analysis: Specified Removal Efficiency

Catchment Name	Site Basin	Offsite Compensation
Rainfall Zone	Florida Zone 4	Florida Zone 4
Annual Mean Rainfall	53.13	53.13

Pre-Condition Landuse Information

Landuse	User Defined Values	User Defined Values
Area (acres)	58.18	1.96
Rational Coefficient (0-1)	0.20	0.20
Non DCIA Curve Number	86.00	86.00
DCIA Percent (0-100)	0.00	0.00
Nitrogen EMC (mg/l)	2.290	1.250
Phosphorus EMC (mg/l)	0.381	0.173
Runoff Volume (ac-ft/yr)	51.209	1.725
Groundwater N (kg/yr)	0.000	0.000
Groundwater P (kg/yr)	0.000	0.000
Nitrogen Loading (kg/yr)	144.593	2.659
Phosphorus Loading (kg/yr)	24.057	0.368

Post-Condition Landuse Information

Landuse	High-Intensity Commercial: TN=2.40 TP=0.345	User Defined Values
Area (acres)	58.18	1.96
Rational Coefficient (0-1)	0.60	0.82
Non DCIA Curve Number	88.60	86.00
DCIA Percent (0-100)	61.20	100.00
Wet Pond Area (ac)	9.21	0.00
Nitrogen EMC (mg/l)	2.400	1.250
Phosphorus EMC (mg/l)	0.345	0.173
Runoff Volume (ac-ft/yr)	129.555	7.142
Groundwater N (kg/yr)	0.000	0.000
Groundwater P (kg/yr)	0.000	0.000
Nitrogen Loading (kg/yr)	383.378	11.007
Phosphorus Loading (kg/yr)	55.111	1.523

Catchment Number: 1 Name: Site Basin

Project: Blackstone Corporate Park
Date: 2/6/2026

0.81 AC-FT REQUIRED OF RETENTION, SEE CALCULATION NOTED RETENTION EL 29.43 FT-NAVD MEETS THE REQUIREMENT, DRAWDOWN ANALYSIS ALSO RUN

Multiple BMP in Series Design Parameters

BMP in Series Number: 1
BMP Type: Retention

Retention Depth (in)	0.199
Retention Volume (ac-ft)	0.812

PPV=12 FT*9.21AC=110.52 AC-FT, REDUCED IN MODEL TO 70.8 AC-FT TO ENSURE LESS THAN 200 RESIDENCE DAYS AS REQUIRED BY SFWMD

BMP in Series Number: 2
BMP Type: Wet Detention

Permanent Pool Volume (ac-ft)	70.800
Permanent Pool Volume (ac-ft) for 31 days residence	11.003
Annual Residence Time (days)	199
Littoral Zone Efficiency Credit	10
Wetland Efficiency Credit	

LESS THAN 200 DAYS

BMP in Series Number: 3
BMP Type: Exfiltration

Pipe Span (in)	12.0
Pipe Rise (in)	12.0
Pipe Length (ft)	275.0
Trench Width (ft)	3.0
Trench Depth (ft)	3.0
Trench Length (ft)	275.0
Aggregate Void (fraction)	40.00
Storage Volume (Ac-ft)	2.08
Retention Depth (in over CA)	0.510

BMP in Series Number: 4
BMP Type: None

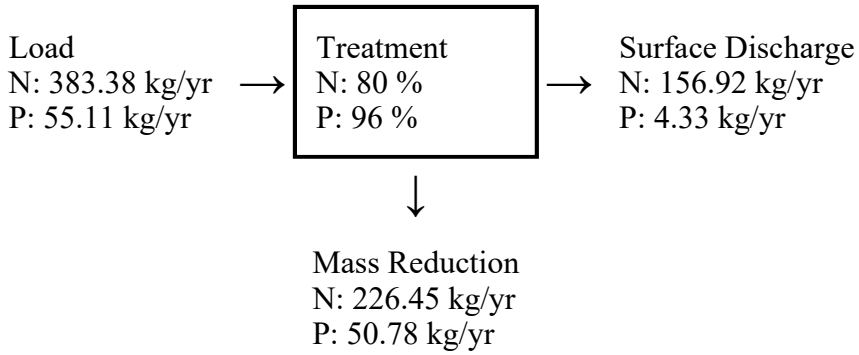
Watershed Characteristics

Catchment Area (acres)	58.18
Contributing Area (acres)	48.970
Non-DCIA Curve Number	88.60
DCIA Percent	61.20
Rainfall Zone	Florida Zone 4
Rainfall (in)	53.13

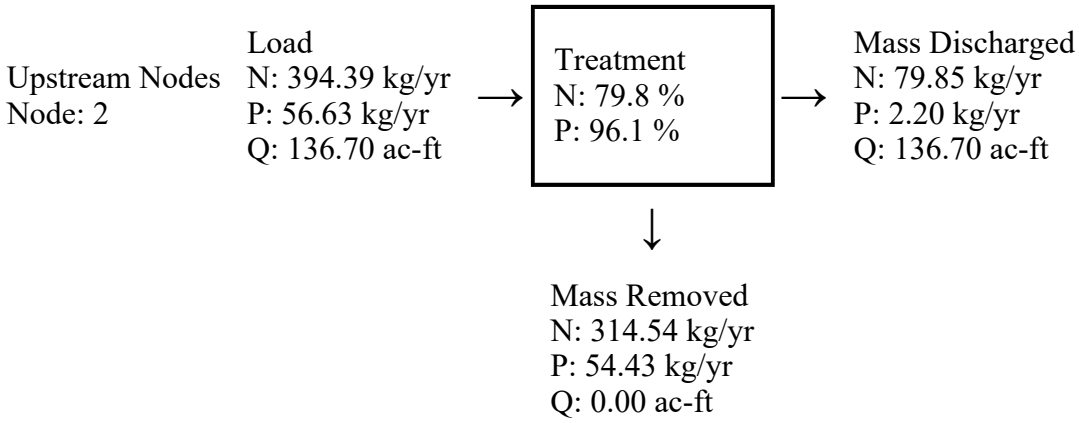
Surface Water Discharge

Required TN Treatment Efficiency (%)	80
Provided TN Treatment Efficiency (%)	80
Required TP Treatment Efficiency (%)	80
Provided TP Treatment Efficiency (%)	96

Load for Multiple BMP in Series



Load Diagram for Multiple BMP (As Used In Routing)



Catchment Number: 2 Name: Offsite Compensation

Project: Blackstone Corporate Park
Date: 2/6/2026

None Design

Watershed Characteristics

Catchment Area (acres)	1.96
Contributing Area (acres)	1.960

Non-DCIA Curve Number	86.00
DCIA Percent	100.00
Rainfall Zone	Florida Zone 4
Rainfall (in)	53.13

Surface Water Discharge

Required TN Treatment Efficiency (%)	80
Provided TN Treatment Efficiency (%)	
Required TP Treatment Efficiency (%)	80
Provided TP Treatment Efficiency (%)	

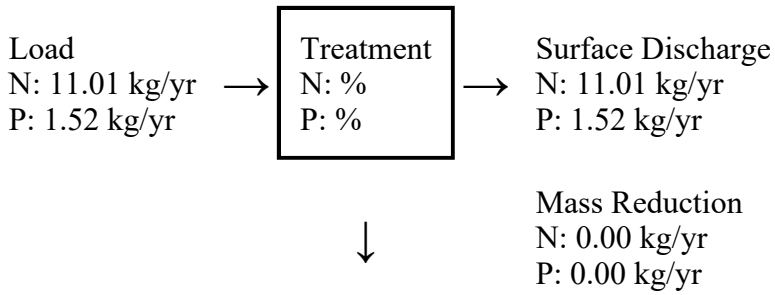
Media Mix Information

Type of Media Mix	Not Specified
Media N Reduction (%)	0.000
Media P Reduction (%)	0.000

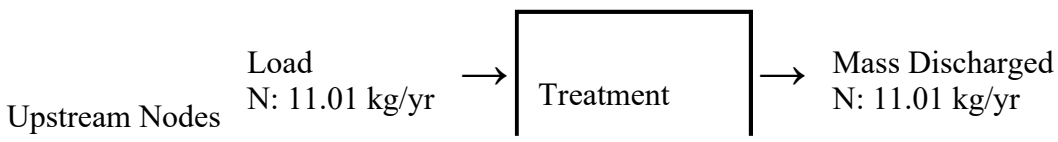
Groundwater Discharge (Stand-Alone)

Treatment Rate (MG/yr)	0.000
TN Mass Load (kg/yr)	0.000
TN Concentration (mg/L)	0.000
TP Mass Load (kg/yr)	0.000
TP Concentration (mg/L)	0.000

Load Diagram for None (stand-alone)



Load Diagram for None (As Used In Routing)



None

P: 1.52 kg/yr
Q: 7.14 ac-ft

N: 0.0 %
P: 0.0 %

P: 1.52 kg/yr
Q: 7.14 ac-ft

↓

Mass Removed
N: 0.00 kg/yr
P: 0.00 kg/yr
Q: 0.00 ac-ft

Summary Treatment Report Version: 5.3.2

Project: Blackstone Corporate Park

Analysis Type: Specified Removal Efficiency

Date:2/6/2026

BMP Types:

Catchment 1 - (Site Basin) Multiple BMP

Catchment 2 - (Offsite Compensation) None

Based on % removal values to the nearest percent

Routing Summary
Catchment 1 Routed to Outlet
Catchment 2 Routed to Catchment 1

Total nitrogen target removal met? **Yes**
Total phosphorus target removal met? **Yes**

Summary Report

Nitrogen

Surface Water Discharge

Total N pre load	147.25 kg/yr	
Total N post load	394.39 kg/yr	
Target N load reduction	80 %	
Target N discharge load	78.88 kg/yr	
Percent N load reduction	80 %	
Provided N discharge load	79.85 kg/yr	176.07 lb/yr
Provided N load removed	314.54 kg/yr	693.55 lb/yr

Phosphorus

Surface Water Discharge

Total P pre load	24.425 kg/yr	
Total P post load	56.634 kg/yr	
Target P load reduction	80 %	
Target P discharge load	11.327 kg/yr	
Percent P load reduction	96 %	
Provided P discharge load	2.202 kg/yr	4.85 lb/yr
Provided P load removed	54.432 kg/yr	120.023 lb/yr

Nutrient Loading - Site Basin

Total Nutrient Loading Basin	58.18 acres

(Site Basin West, Site Basin East, Lake Basin West, Lake Basin East)

Pre-Development Inputs

Total Pre-Development Catchment Area — *Total area in acres for the catchment in pre-development condition. Catchment is a contiguous area where all runoff will accumulate.* — 58.18 ac

Pre-Development Non-DCIA CN — *Weighted curve number for portion of catchment which is not DCIA*

TR-55, Table 2-2c, woods/grass combination, poor condition, HSG D, CN=86

Non-DCIA CN	86
Percent DCIA	0%

Pre-Development DCIA Percentage — *Percentage of total catchment area that is directly-connected impervious area. Note, areas wet all year are not included.*

Pre-Development Land Use

Agricultural - General (Per Table 9.2 ERP Hadnbook Volume I effective June 28, 2024)	
TN=	2.290
TP=	0.381

Pre-Development Loading

N Loading (kg/yr)	144.593
P Loading (kg/yr)	24.057

Post Development Inputs

Total Post Development Catchment Area — *Total area in acres for the catchment defined at a point where there is a discharge as well as a please for possible BMPs in the post development condition.* — 58.18 ac

Post Development Non-DCIA Curve Number — *The weighted runoff curve number for the portion of the catchment that is not directly connected impervious area in the post-development state. A composite weighted CN can be calculated using average annual runoff.*

Pervious Area	11.75 ac
CN for Pervious Area*	80.00
*TR-55, Table 2-2a, open space in good condition, HSG D	

Impervious Area which is Not DCIA, See below on DCIA percent (This taken as Impervious area less DCIA below)

Area	10.83 ac	58.18
CN for Impervious Areas	98	

Weighted CN=(Pervious Area)(CN Pervious Area) + (Impervious Area not DCIA Area)(CN Impervious Area) / (Pervious Area + Impervious Area)
88.6

Post Development DCIA Percentage — *The percentage of the total catchment area that is directly connected impervious area in the post development state.* ***Areas wet all year (detention ponds, etc.) are not included in the DCIA calculation.***
The area of internal roadways and sidewalks as well as impervious area (FF and pavement conc from outparcels 1 through 15)

FF Outparcels 1-15	6.78 ac
Parking and Roadway Outparcels 1-15	28.82 ac
Total DCIA	35.60 ac
Percent DCIA	61.2%

Wet Pond Area

The area at the top of the permanent pool or at the control elevation. This area will be subtracted from the overall post-development catchment area when the wet pond is part of the total catchment area. As an alternative, the wet pond area does not have to be included in there catchment area.

Wet Pond Area (Lake Area)	9.21 ac

Post Development Loading

N Loading (kg/yr)	384.180
P Loading (kg/yr)	55.230

Nutrient Loading - Off Site Compensating Nutrient Loading

Total Nutrient Loading Basin	1.96 acres

(Site Basin West, Site Basin East, Lake Basin West, Lake Basin East)

Pre-Development Inputs

Total Pre-Development Catchment Area — *Total area in acres for the catchment in pre-development condition. Catchment is a contiguous area where all runoff will accumulate.* — 1.96 ac

Pre-Development Non-DCIA CN — *Weighted curve number for portion of catchment which is not DCIA*

TR-55, Table 2-2c, woods/grass combination, poor condition, HSG D, CN=86

Non-DCIA CN	86
Percent DCIA	0%

Pre-Development DCIA Percentage — *Percentage of total catchment area that is directly-connected impervious area. Note, areas wet all year are not included.*

Pre-Development Land Use

Highway - General (Per Table 9.2 ERP Hadnbook Volume I effective June 28, 2024)	
TN=	1.250
TP=	0.173

Pre-Development Loading

N Loading (kg/yr)	2.659
P Loading (kg/yr)	0.368

Post Development Inputs

Total Post Development Catchment Area — *Total area in acres for the catchment defined at a point where there is a discharge as well as a please for possible BMPs in the post development condition.* — 1.96 ac

Post Development Non-DCIA Curve Number — *The weighted runoff curve number for the portion of the catchment that is not directly connected impervious area in the post-development state. A composite weighted CN can be calculated using average annual runoff.*

Pervious Area	0.00 ac
CN for Pervious Area*	80.00
*TR-55, Table 2-2a, open space in good condition, HSG D	

Impervious Area which is Not DCIA, See below on DCIA percent (This taken as Impervious area less DCIA below)

Area	1.96 ac
CN for Impervious Areas	98

3.92

Weighted CN=(Pervious Area)(CN Pervious Area) + (Impervious Area not DCIA Area)(CN Impervious Area) / (Pervious Area + Impervious Area)
98.0

Post Development DCIA Percentage — *The percentage of the total catchment area that is directly connected impervious area in the post development state.* ***Areas wet all year (detention ponds, etc.) are not included in the DCIA calculation.*** *The area of internal roadways and sidewalks as well as impervious area.*

Highway Pavement	1.96 ac
Total DCIA	1.96 ac
Percent DCIA	100.0%

Wet Pond Area

The area at the top of the permanent pool or at the control elevation. This area will be subtracted from the overall post-development catchment area when the wet pond is part of the total catchment area. As an alternative, the wet pond area does not have to be included in there catchment area.

Wet Pond Area (Lake Area)	0.00 ac

Post Development Loading

N Loading (kg/yr)	11.007
P Loading (kg/yr)	1.523

Blackstone Corporate Park

Retention Volume - BMP Trains Nutrient Loading

Required Retention Volume per BMP Train	0.81	ac-ft

Dry Retention Areas 1, 2, and 3

Stage (ft)	Area (ac)	Storage (ac-ft)
29.00	1.623	0.00
29.43	**1.748**	**0.82**
31.00	2.203	3.83

Provided =	**0.823**	**ac-ft**

Rainfall Map

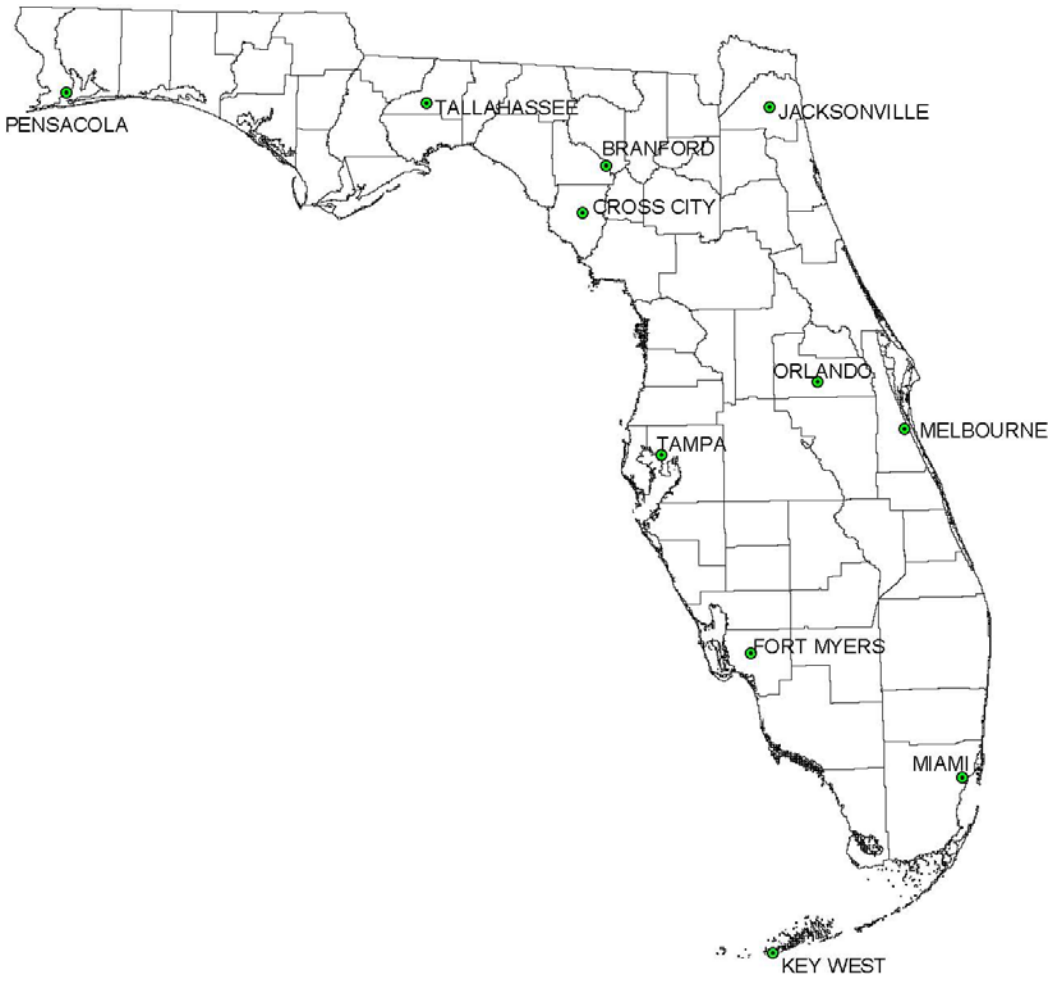


Figure 3-3. Locations of Selected Meteorological Sites.

TABLE 3-1

CHARACTERISTICS OF SELECTED METEOROLOGICAL SITES IN FLORIDA

GENERAL LOCATION	MONITORING SITE	MEAN ANNUAL RAINFALL (in/yr)	PERIOD OF RECORD
Panhandle	Pensacola Tallahassee	62.17 64.55	Pensacola: 33 years (1942-2005) Tallahassee: 45 years (1959-2005)
Big Bend Coastal	Cross City	55.14	47 years (1942-2000)
Northeast Coastal	Jacksonville	52.96	64 years (1942-2005)
North Florida	Branford	52.27	53 years (1945-2004)
Central	Orlando	50.03	62 years (1942-2005)
East Central Coastal	Melbourne	47.52	47 years (1942-2005)
West Central Coastal	Tampa	46.07	47 years (1949-2005)
Southwest Coastal	Ft. Myers	53.13	28 years (1960-2003)
Southeast Coastal	Miami	56.92	64 years (1942-2005)
Florida Keys	Key West	39.07	63 years (1942-2005)

Land Use

TABLE 4-1

GENERAL LAND USE CATEGORIES FOR RUNOFF CHARACTERIZATION DATA

GENERAL CATEGORY	DESCRIPTION
Low-Density Residential	Rural areas with lot sizes greater than 1 acre or less than one dwelling unit per acre; internal roadways associated with the homes are also included
Single-Family Residential	Typical detached home community with lot sizes generally less than 1 acre and dwelling densities greater than one dwelling unit per acre; duplexes constructed on one-third to one-half acre lots are also included in this category; internal roadways associated with the homes are also included
Multi-Family Residential	Residential land use consisting primarily of apartments, condominiums, and cluster-homes; internal roadways associated with the homes are also included
Low-Intensity Commercial	Areas which receive only a moderate amount of traffic volume where cars are parked during the day for extended periods of time; these areas include universities, schools, professional office sites, and small shopping centers; internal roadways associated with the development are also included
High-Intensity Commercial	Land use consisting of commercial areas with high levels of traffic volume and constant traffic moving in and out of the area; includes downtown areas, commercial sites, regional malls, and associated parking lots; internal roadways associated with the development are also included
Industrial	Land uses include manufacturing, shipping and transportation services, sewage treatment facilities, water supply plants, and solid waste disposal; internal roadways associated with the development are also included
Highway	Includes major road systems, such as interstate highways and major arteries and thoroughfares; roadway areas associated with residential, commercial, and industrial land use categories are already included in loading rates for these categories
Agriculture	Includes cattle, grazing, row crops, citrus, and related activities
Open/ Undeveloped	Includes open space, barren land, undeveloped land which may be occupied by native vegetation, rangeland, and power lines; this land does not include golf course areas which are heavily fertilized and managed; golf course areas have runoff characteristics most similar to single-family residential areas
Mining/ Extractive	Includes a wide variety of mining activities for resources such as phosphate, sand, gravel, clay, shell, etc.
Wetlands	Include a wide range of diverse wetland types, such as hardwood wetlands, cypress stands, grassed wetlands, freshwater marsh, and mixed wetland associations
Open Water/ Lakes	Land use consists of open water and lakes, rivers, reservoirs, and other open waterbodies

Pre-Development TR-55 Excerpt

PRE-DEVELOPMENT CURVE NUMBER

Table 2-2c Runoff curve numbers for other agricultural lands [1/]

Cover description		Curve numbers for hydrologic soil group			
Cover type	Hydrologic condition	A	B	C	D
Pasture, grassland, or range—continuous forage for grazing. [2/]	Poor	68	79	86	89
	Fair	49	69	79	84
	Good	39	61	74	80
Meadow—continuous grass, protected from grazing and generally mowed for hay.	—	30	58	71	78
Brush—brush-weed-grass mixture with brush the major element. [3/]	Poor	48	67	77	83
	Fair	35	56	70	77
	Good	30 [4/]	48	65	73
Woods—grass combination (orchard or tree farm). [5/]	Poor	57	73	82	86
	Fair	43	65	76	82
	Good	32	58	72	79
Woods. [6/]	Poor	45	66	77	83
	Fair	36	60	73	79
	Good	30 [4/]	55	70	77
Farmsteads—buildings, lanes, driveways, and surrounding lots.	—	59	74	82	86

1 Average runoff condition, and $I_a = 0.2S$.

2 *Poor:* <50%) ground cover or heavily grazed with no mulch.
Fair: 50 to 75% ground cover and not heavily grazed.
Good: > 75% ground cover and lightly or only occasionally grazed.

3 *Poor:* <50% ground cover.
Fair: 50 to 75% ground cover.
Good: >75% ground cover.

4 Actual curve number is less than 30; use CN = 30 for runoff computations.

5 CN's shown were computed for areas with 50% woods and 50% grass (pasture) cover. Other combinations of conditions may be computed from the CN's for woods and pasture.

6 *Poor:* Forest litter, small trees, and brush are destroyed by heavy grazing or regular burning.
Fair: Woods are grazed but not burned, and some forest litter covers the soil.
Good: Woods are protected from grazing, and litter and brush adequately cover the soil.

Post Development TR-55 Excerpt

POST DEVELOPMENT CURVE NUMBER

Table 2-2a Runoff curve numbers for urban areas [1/]

Cover description: Cover type and hydrologic condition	Average percent impervious area [2/]	Curve numbers for hydrologic soil group: A	B	C	D
Fully developed urban areas (vegetation established)					
Open space (lawns, parks, golf courses, cemeteries, etc.) [3/]:					
Poor condition (grass cover < 50%)		68	79	86	89
Fair condition (grass cover 50% to 75%)		49	69	79	84
Good condition (grass cover > 75%)		39	61	74	80
Impervious areas:					
Paved parking lots, roofs, driveways, etc. (excluding right-of-way)		98	98	98	98
Streets and roads:					
Paved; curbs and storm sewers (excluding right-of-way)		98	98	98	98
Paved; open ditches (including right-of-way)		83	89	92	93
Gravel (including right-of-way)		76	85	89	91
Dirt (including right-of-way)		72	82	87	89
Western desert urban areas:					
Natural desert landscaping (pervious areas only) [4/]		63	77	85	88
Artificial desert landscaping (impervious weed barrier, desert shrub with 1- to 2-inch sand or gravel mulch and basin borders)		96	96	96	96
Urban districts:					
Commercial and business	85	89	92	94	95
Industrial	72	81	88	91	93
Residential districts by average lot size:					
1/8 acre or less (town houses)	65	77	85	90	92
1/4 acre	38	61	75	83	87
1/3 acre	30	57	72	81	86
1/2 acre	25	54	70	80	85
1 acre	20	51	68	79	84
2 acres	12	46	65	77	82
Developing urban areas					
Newly graded areas (pervious areas only, no vegetation) [5/]		77	86	91	94
Idle lands (CN's are determined using cover types similar to those in table 2-2c).					

1 Average runoff condition, and $I_a = 0.2S$.

2 The average percent impervious area shown was used to develop the composite CN's. Other assumptions are as follows: impervious areas are directly connected to the drainage system, impervious areas have a CN of 98, and pervious areas are considered equivalent to open space in good hydrologic condition. CN's for other combinations of conditions may be computed using figure 2-3 or 2-4.

3 CN's shown are equivalent to those of pasture. Composite CN's may be computed for other combinations of open space cover type.

4 Composite CN's for natural desert landscaping should be computed using figures 2-3 or 2-4 based on the impervious area percentage (CN = 98) and the pervious area CN. The pervious area CN's are assumed equivalent to desert shrub in poor hydrologic condition.

5 Composite CN's to use for the design of temporary measures during grading and construction should be computed using figure 2-3 or 2-4 based on the degree of development (impervious area percentage) and the CN's for the newly graded pervious areas.

9.2.2 Calculating Predevelopment and Post-development Stormwater Nutrient Loading

a. To calculate the predevelopment and post development loadings before treatment of the annual mass loadings for TN and TP, multiply the predevelopment annual runoff volume (derived in Section 9.2.1) by the land use specific runoff characterization data (event mean concentrations or EMCs) for TN and TP. Applicants must use the most up-to-date verified EMC values, where available and incorporated by reference pursuant to rule 62.330.301(4), for their project region. Applicants also must comply with the applicable special basin or geographic area criteria in rule 62-330.301(1)(k), F.A.C., including any EMC values specified in the applicable Applicant's Handbook Volume II. EMC Values are listed in Table 9.2 for different types of land use categories. EMC values for the land uses must consider cover, soils, and topography and be representative of the latest assigned Florida Land Use and Cover Classification System (FLUCCS) code.

Table 9.2 Standardized Statewide Stormwater Nutrient EMC Values

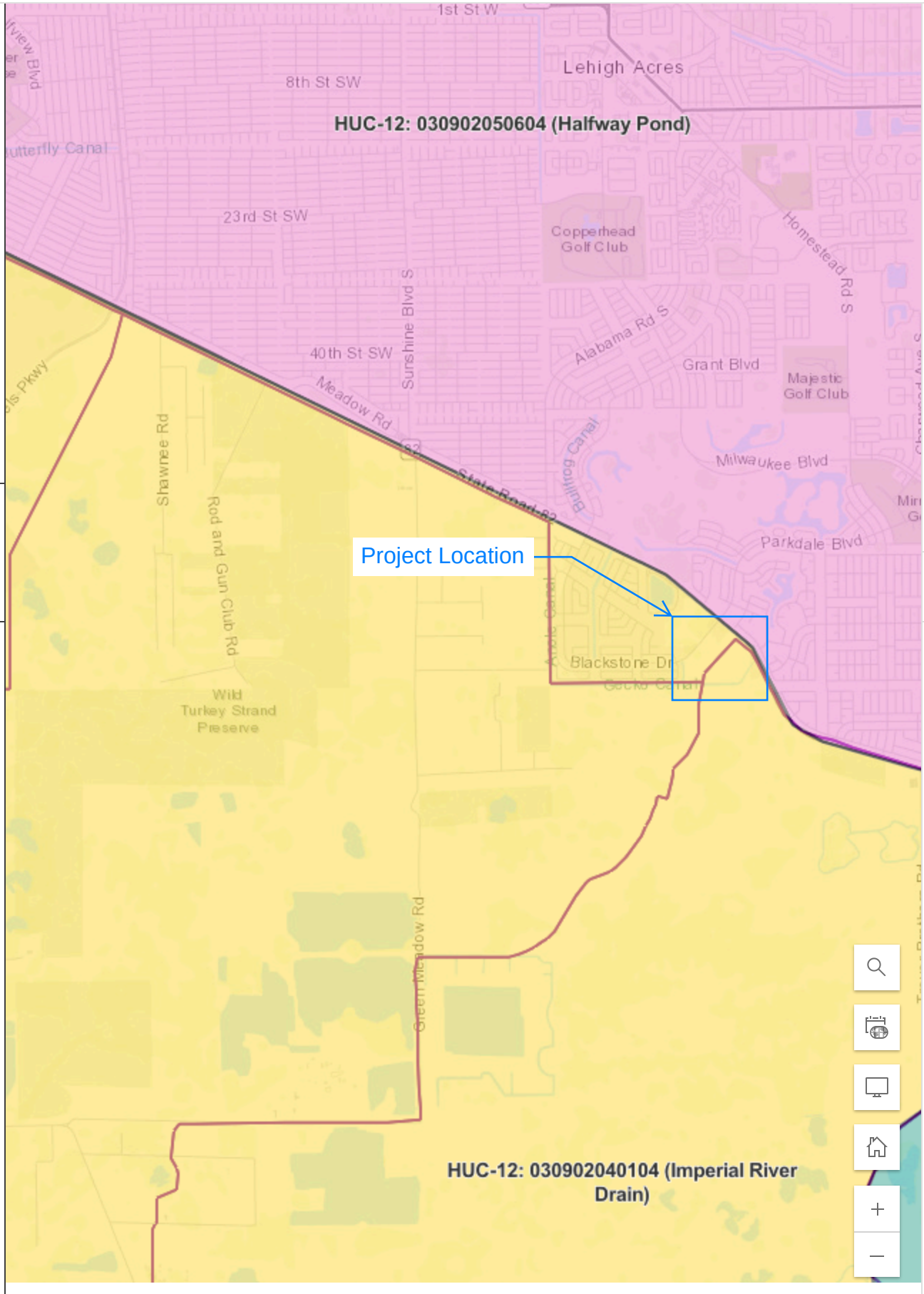
Land Use Category	Total N (mg/l)	Total P (mg/l)
Low Density Residential	1.65	0.270
Single Family	1.77	0.327
Multi-Family	1.84	0.520
Low Intensity Commercial	0.93	0.19
High Intensity Commercial	2.40	0.345
Light Industrial	1.20	0.260
Highway	1.25	0.173
Dry Prairie	2.025	0.184
Marl Prairie	0.684	0.012
Mesic Flatwoods	1.087	0.043
Ruderal/Upland Pine	1.694	0.162
Scrubby Flatwoods	1.155	0.027
Upland Hardwood	1.042	0.346
Upland Mixed Forest	0.606	1.166
Wet Flatwoods	1.213	0.021
Wet Prairie	1.095	0.015
Xeric Scrub	1.596	0.156
Rangeland/parkland	1.150	0.055
General Agricultural	2.29	0.381
Pasture	3.03	0.593
Citrus	2.11	0.180
Row Crops	2.50	0.577

b. At the time of the application, an applicant may propose to use TN and TP EMC values accepted by the Agency which denote EMC values derived from regional or local government studies. Any study conducted must be submitted with the permit application for the Agency records. If EMC values from a study are to be used, data collected must follow quality assurance provisions outlined in chapter 62-160, F.A.C., and include:

- Data collected at a representative variety of rainfall depths;
- Minimum of 10 rainfall events;
- Minimum of one year of data with seasonal variation;
- Use of autosamplers to allow for runoff to be sampled for the duration of the rainfall event;
- Volume or time weighted composite samples;

Appendix N: HUC Map

Florida's Mitigation Watersheds and Basins



University of South Florida, County of Lee, FL, Esri, HERE, Garmin, INCREMENT P, USGS, METI/N… Powered by Esri

ATWELL

PART IV #25-40

Recommended Action

Engineering Review:

By: _____ 3/3/2026 **Approved** _X_ **Denied** ___

District Field Review:

By: _____ **Approved** ___ **Denied** ___

District Manager: _____ **Date**: _____

Comments:

The referenced permit modification application for a previously approved Permit No. 25-40 Blackstone Corporate Park at Blackstone Dr. and State Road 82, in Lehigh Acres, submitted by Atwell, LLC, has been reviewed and is recommended for approval with the following stipulations/comments:

1. Any disturbed canal right-of-way must be restored with sod and maintained until fully established. Seeding is not permitted.

2. The permittee shall submit record drawings within thirty (30) days of project completion. The drawings must be signed and sealed and include sufficient details to demonstrate that the permitted facilities have been installed or constructed in substantial compliance with the LA-MSID-approved plans and design.

3. The permittee shall provide LA-MSID with copies of all applicable permits from other governing agencies prior to construction, including but not limited to a SFWMD Major Modification, FDOT Drainage Exception, Lee County DOT Right-of-Way Permit, Lee County Development Order, and FGUA Permits.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: PER #25-86 Beth Stacey Blvd Extension

Meeting Date: March 16, 2026

Agenda Item # 2E

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve PER #25-86 Beth Stacey Blvd Extension, with any listed stipulations.

What the action accomplishes: Allows Aquabella Development Group, LLC to extend Beth Stacey Blvd South and connect it to Milwaukee Blvd.

Fiscal Impact: Permit Fees: $1,500.00

Staff comments: Approved according to the revised plans signed/sealed February 23, 2026.

Staff review and recommended approval:

[x] **MANAGER** [ ] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

LEHIGH ACRES - MUNICIPAL SERVICES IMPROVEMENT DISTRICT
PERMIT

THIS PERMIT is granted this 16th day of March, 2026, by Lehigh Acres Municipal Services Improvement District (hereinafter referred to as "DISTRICT"), 601 East County Lane, Lehigh Acres, Florida 33936, to Aquabella Development Group, LLC, 10481 Six Mile Cypress Parkway, Fort Myers, FL 33966, hereinafter referred to as the "Permittee") as a non-exclusive permit for a roadway extension project to connect Beth Stacey Blvd. with Milwaukee Blvd., in Lehigh Acres, Florida.

W I T N E S S E T H:

1. In the event the DISTRICT is not the fee title owner of the real property (the "Property") to which this Permit is applicable, the Permittee agrees, prior to commencement, to obtain all necessary consents from the fee title owners of the Property.

2. This Permit does not constitute a waiver of the necessity by the Permittee to obtain such other necessary and appropriate permits required by other governmental bodies or agencies; therefore the Permittee, prior to commencement, is required to obtain any and all other applicable federal, state, and local permits required in connection with Permittee's use of the Property and agrees that at all times it will comply with the requirements of all federal, state and local laws, ordinances, rules and regulations applicable or pertaining to the use of the Property by the Permittee.

3. Permittee understands and agrees that the use of the Property pursuant to this Permit is subordinate to the rights and interests of the DISTRICT and to the extent applicable that of the fee title owner of the Property. Further, Permittee does hereby stipulate that the Permittee is not relying upon any representations whatsoever by the DISTRICT regarding the DISTRICT'S right, title, interest, or ownership as to the Property for which this Permit is applicable.

4. The Permittee, by accepting this permit, specifically agrees to allow authorized District Personnel, upon presentation of credentials or other documents as may be required by law, access to the premises, at reasonable times, where the permitted activity is located or conducted for the purpose of:
    a. Having access to and copying any records that must be kept under the conditions of the permit:
    b. Reviewing the facility, equipment, practices, or operations regulated or required under this permit; and
    c. Sampling or monitoring any substances or parameters at any location reasonably necessary to assure compliance with this permit or District rules.

5. The DISTRICT specifically reserves the right to maintain its facilities located over, under or upon the Property; to make improvements; add additional facilities; maintain, construct or alter its roads; maintain any facilities, devices, or improvements on the Property which aid in or are necessary to the DISTRICT'S operation; and the right to enter upon the Property at all times for such purposes. Permittee understands that in the exercise of such rights and interest, the DISTRICT may require Permittee, at no cost to DISTRICT, to relocate, alter, or remove the Permittee's facilities and equipment or other improvements made by Permittee pursuant to this Permit which interfere with or prevent the DISTRICT, in the DISTRICT'S opinion, from properly and faithfully constructing, improving and maintaining its facilities. If the Permittee should fail to relocate, alter, or remove the Permittee's facilities, equipment or other improvements made by the Permittee pursuant to this Permit within a reasonable time period following receipt of notice from the DISTRICT requesting same, then in that event the DISTRICT shall have the right to enter upon the lands and make such relocation, alterations or removal of the Permittee's facilities, equipment/or other improvements at no cost to DISTRICT.

6. Permittee agrees that it will not use the Property in any manner which interferes with the DISTRICT'S use of the Property or causes a hazardous condition to exist.

7. The DISTRICT assumes no responsibility for the ownership, operation and/or maintenance of the Permittee's facilities as permitted herein.

8. The Permittee agrees to backfill any excavation it makes within the Property and to repair or replace (i) any below-grade authorized facilities and features located therein, including but not limited to: water, sewer, stormwater or irrigation facilities, structures or appurtenances and (ii) as to at or above-grade facilities and features, this requirement shall only be applicable to grasses and sod, sidewalks, and roadway improvements (the term roadway improvements shall include but not be limited to curbing, roadway base, sub grade and asphaltic or concrete surfaces) which are removed or damaged as a direct result of said excavation. Such repair or replacement shall substantially restore these features to their condition as it existed immediately prior to the event precipitating the excavation.

9. All below-grade Permittee installations, including water and sewer lines, whether sub aqueous or not, together with appurtenant facilities, shall be located underground and have a minimum ground cover of thirty inches (30") below original design profile grade, including District waterways and roadways.

10. All Permittee activity related to sub aqueous utility, water and/or sewer lines, including appurtenant facilities, shall be conducted in such fashion that the primary functional purpose of the canal or lake body facility, specifically flood control, within which the activity is taking place is not adversely affected. More specifically, sub aqueous construction shall conform to the criteria given in the Lehigh Acres Municipal Services Improvement District (Lehigh Acres – MSID) Administrative Policy and Procedures.

11. Permittee shall, at no expense to DISTRICT and within a reasonable time following notice, adjust the positions and elevations of its facilities as may be required in connection with future improvements to, or construction of, works of the DISTRICT.

12. Permittee does hereby indemnify and hold harmless the DISTRICT, its Board of Commissioners, officers, personnel, and it's Engineers against any claims, losses, damages (including consequential), expenses, or legal fees that might arise out or result from the implementation of the proposed project by the Permittee or that may be caused by a failure of DISTRICT facilities or property.

13. The Permittee shall be obligated at the time of submission of its Permit application, to provide a detail of any other existing improvements located over, under, upon or through the Property and the Permittee's accompanying plans and specifications shall clearly indicate how the Permittee will comply with required clearances between the requested permitted activity and existing improvements located over, under and upon the Property, including compliance with other permitted users duly adopted and published standards.

14. Permittee shall reimburse the DISTRICT for its legal, engineering and other expenses incurred as a result of the implementation of the proposed project.

15. If Permittee should violate any of the terms or conditions of the Permit and shall not correct or remedy same within ten (10) days of receiving written notice of said violation from the DISTRICT, then in that event, the DISTRICT may, at its option, revoke, cancel and terminate this Permit.

16. This Permit may not be assigned by the Permittee without the prior written approval of the DISTRICT.

17. Unless otherwise adjusted, modified or terminated as set forth hereinabove, including the Conditions

attached hereto, this Permit shall continue in full force and effect so long as Permittee complies with the terms of this Permit.

18. In any litigation, including breach, enforcement or interpretation arising out of this Permit, the prevailing party to this Permit shall be entitled to recover reasonable attorney's fees, costs and expenses related to said action from the non-prevailing party.

19. That attached hereto are Supplemental Conditions to this Permit which are incorporated herein and made a part hereof.

LEHIGH ACRES – MUNICIPAL SERVICES IMPROVEMENT DISTRICT

BY: _____

NAME TYPED: David E. Lindsay

TITLE: District Manager

SUPPLEMENTAL CONDITIONS TO PERMIT NO. 25-86

A. GENERAL CONDITIONS

1. Notification shall be given by the Permittee to the District Engineer, AIM Engineering, at 239-332-4569 forty-eight (48) hours prior to commencement of the permitted activity. The District Engineer shall establish points of construction, if applicable, which will require inspection by the District Engineer prior to construction of the permitted activity. When the Permittee considers the permitted activity complete, the Permittee shall notify the District Engineer and schedule an on-site final inspection which shall be held in the presence of a representative of the Permittee and the District Engineer.

2. The permitted activity shall be implemented, constructed and/or installed in full accordance with the approved plans and specifications within six (6) full calendar years from the date of permit approval. Deviations from the approved plans or specifications shall be coordinated with and approved by the District Engineer prior to implementation of any such deviation.

3. When working in a District road right-of-way, not more than one-half (½) of the road or street shall be closed and traffic shall be controlled so as to provide for the safety and minimum hindrance to the public. All traffic control operations shall conform to the most current issue of the Florida Department of Transportation publication "Manual on Traffic Controls and safe Practices for Street and Highway Construction, Maintenance and Utility Operations."

4. The District Engineer shall be the final authority as to the quality and quantity of the material and works required to satisfy the terms and conditions of this Permit as they relate to District improvements and facilities.

5. Upon completion of the permitted activity and after its final review and acceptance by the District Engineer THE PERMITTEE SHALL DELIVER TO THE DISTRICT OFFICE ONE COMPLETE SET OF THE FINAL "RECORD DRAWINGS." FAILURE TO PROVIDE THE FINAL RECORD DRAWINGS MAY RESULT IN THE REVOCATION, CANCELLATION AND TERMINATION OF THIS PERMIT.

6. The above Conditions shall be continuing obligations of the Permittee and shall be complied with in the event of any subsequent maintenance, repair, replacement or modification of the permitted improvement.

Stipulations:

1). Permit #25-86 is approved according to the revised plans and calculations signed/sealed February 23rd, 2026.

2). Please submit a copy of your approved Lee County Development Order prior to starting roadway extension.

3). Maintenance of the entire roadway improvements and the on-site storm water management system including the storm sewer pipes, catch basins, lakes, control structure and outfall pipes into LAMSID canals are the responsibility of the Permittee.

4). An "As-Built" survey showing the completed improvements (including final asphalt elevations, all catch basins and storm pipe inverts, outfall structure elevations and all road ditch and berm elevations) must be submitted to Lehigh Acres - MSID within 30 days following completion of the improvements.

PART I

DATE SUBMITTED: 10 - 28 - 25

PERMIT #: 25 - 86

APPLICATION FOR A PERMIT

***Note**: The review of permit applications will be conducted per the Lehigh Acres Municipal Services Improvement District's Administrative Policy and Procedures

AGENT NAME: Atwell, LLC **PHONE #:** (239)939-5490

PROJECT NAME: Ibis Landing - Beth Stacey Boulevard

TOTAL ACREAGE: 34.87 (Basins 3 & 4) Only

Property Address (STRAP associated with this address will be used as the primary STRAP for this project): Beth Stacey Boulevard & Yellowtail Canal					
Mailing Address:	**10481 Six Mile Cypress Parkway**				
City:	**Fort Myers**	**State:**	**FL**	**Zip:**	**33966**

STRAP NUMBERS (*attach additional sheet if necessary*):

0	6	-	4	5	-	2	7	-	L	4	-	1	4	F	D	6	.	0	0	0	0
		-			-			-			-						.				
		-			-			-			-						.				
		-			-			-			-						.				
		-			-			-			-						.				
		-			-			-			-						.				
		-			-			-			-						.				
		-			-			-			-						.				

Brief description of proposed project:

Application for: The proposed application is for the ~~modification of the southeast residential area and of Ibis Landing~~ southern portion of Beth Stacey Blvd. including modification of the stormwater system. Updated surface water management calculations are provided. No modification of the previously approved control structures or outfall pipes is proposed.

Is the project proposed to be developed in phases? ☐ **Yes** ☒ **No**

Estimated # of Phases:	1
***Permit Fee:**	$ 1,500.00 **(to be verified by staff)**

PART II

OWNER/APPLICANT/ DEVELOPER INFORMATION

A.

Name of Applicant	Aquabella Development Group, LLC						
Mailing Address:	Street:	10481 Six Mile Cypress Parkway					
City:	Fort Myers	State:	FL		Zip:	33966	
Phone No.	Area Code:	(239)	Number:	278-1177	Ext.		
Fax No.	Area Code:		Number:	-			
E-mail Address:	scott.edwards@lennar.com						

B. Relationship of applicant to property*:

☒ Owner* ☐ Trustee* ☐ Option holder* ☐ Lessee*

☐ Contract Purchaser* ☐ Other (indicate)* _____

C.

Name of Developer:	Aquabella Development Group, LLC					
Mailing Address:	Street:	10481 Six Mile Cypress Parkway				
City:	Fort Myers	State:	FL		Zip:	33966
Phone No.	Area Code:	(239)	Number:	278-1177	Ext.	
Fax No.	Area Code:	( )	Number:	-		
E-mail Address:	scott.edwards@lennar.com					

D.

Name of Authorized Agent(s):	Atwell, LLC					
Mailing Address:	Street:	10511 Six Mile Cypress Parkway				
City:	Fort Myers	State:	FL		Zip:	33966
Phone No.	Area Code:	(239)	Number:	939-5490	Ext.	
Fax No.	Area Code:	(239)	Number:			
E-mail Address:	jsheppard@atwell.com & dave.underhill@atwell.com					

E.

Name of Property Owner:	Aquabella Development Group, LLC					
Mailing Address:	Street:	10481 Six Mile Cypress Parkway				
City:	Fort Myers	State:	FL		Zip:	33966
Phone No.	Area Code:	(239)	Number:	278-1177	Ext.	
Fax No.	Area Code:	( )	Number:	-		
E-mail Address:	scott.edwards@lennar.com					

Name of Property Owner:	Millrose Properties Florida, LLC					
Mailing Address:	Street:	5505 Waterford District Drive				
City:	Miami	State:	FL		Zip:	33126
Phone No.	Area Code:	(239)	Number:	278-1177	Ext.	
Fax No.	Area Code:	( )	Number:	-		
E-mail Address:	scott.edwards@lennar.com					

*A disclosure of the name of all persons or entities having an ownership interest in the property is required (attached).

PART III

DISCLOSURE OF OWNERSHIP INTEREST FORM:

PRIMARY STRAP NUMBER: 06 - 45 - 27 - L4 - 14CC4 . 0000

1. If the property is owned in the fee simple by an **INDIVIDUAL**, tenancy by the entirety, tenancy in common, or joint tenancy, list all parties with an ownership interest as well as the percentage of such interest.

Name and Address	Percentage of Ownership

2. If the property is owned by a **CORPORATION**, list the officers and stockholders and the percentage of stock owned by each.

Name, Address, Office	Percentage of Stock

3. If the property is in the name of a **TRUSTEE**, list the beneficiaries of the trust with percentages of interest.

Name and Address	Percentage of Interest

4. If the property is in the name of a **GENERAL PARTNERSHIP OR LIMITED LIABILITY COMPANY**, list the names of general and limited partners.

Name and Address	Percentage of Interest
Lennar Homes, LLC	100%
10481 Six Mile Cypress Parkway	
Fort Myers, FL 33966	

5. If there is a **CONTACT FOR PURCHASE**, whether contingent on this application Or not, and whether a Corporation, Trustee, or Partnership, list the names of the contract purchasers below, including the officers, stockholders, beneficiaries, or partners.

Name and Address	Percentage of Interest

Date of Contract (mm/dd/yy): / /

6. If any contingency clause or contract terms involve additional parties, list all individuals or officers, if a corporation, partnership, or trust.

Name and Address	Percentage of Interest

For any changes of ownership of changes in contracts for purchase subsequent to the date of the application, but prior to the date of final certificate for compliance, a supplemental disclosure of interest must be filed.

The above is a full disclosure of all parties of interest in this application, to the best of my knowledge and belief.

Aquabella Development Group, LLC
By: Lennar Homes, LLC, its Manager

SIGNATURE OF APPLICANT
Scott Edwards - Vice President - Lennar Homes, LLC
PRINTED OR TYPED NAME OF APPLICANT

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this 26th day of Sept. (m), 2025(y), by Scott Edwards who is personally know to me or has produced _____ as identification.

JENNIFER M. SHEPPARD
MY COMMISSION # HH 689789
EXPIRES: November 16, 2029

SIGNATURE OF NOTARY PUBLIC
Jennifer M. Sheppard
PRINTED NAME OF NOTARY PUBLIC

PART III

DISCLOSURE OF OWNERSHIP INTEREST FORM FORM:

PRIMARY STRAP NUMBER: 0 6 - 4 5 - 2 7 - 1 0 - 0 0 0 0 H . 0 0 0 0

2. If the property is owned in the fee simple by an **INDIVIDUAL**, tenancy by the entirety, tenancy in common, or joint tenancy, list all parties with an ownership interest as well as the percentage of such interest.

Name and Address	Percentage of Ownership

2. If the property is owned by a **CORPORATION**, list the officers and stockholders and the percentage of stock owned by each.

Name, Address, Office	Percentage of Stock

3. If the property is in the name of a **TRUSTEE**, list the beneficiaries of the trust with percentages of interest.

Name and Address	Percentage of Interest

7. If the property is in the name of a **GENERAL PARTNERSHIP OR LIMITED LIABILITY COMPANY**, list the names of general and limited partners.

Name and Address	Percentage of Interest
Millrose Properties Florida, LLC	100%
5505 Waterford District Drive	
Miami, FL 33126	

8. If there is a **CONTACT FOR PURCHASE**, whether contingent on this application Or not, and whether a Corporation, Trustee, or Partnership, list the names of the contract purchasers below, including the officers, stockholders, beneficiaries, or partners.

Name and Address	Percentage of Interest

Date of Contract (mm/dd/yy): / /

9. If any contingency clause or contract terms involve additional parties, list all individuals or officers, if a corporation, partnership, or trust.

Name and Address	Percentage of Interest

For any changes of ownership of changes in contracts for purchase subsequent to the date of the application, but prior to the date of final certificate for compliance, a supplemental disclosure of interest must be filed.

The above is a full disclosure of all parties of interest in this application, to the best of my knowledge and belief.

Millrose Properties Florida, LLC
By: U.S. Home, LLC, its Sole Manager

SIGNATURE OF APPLICANT

Scott Edwards - Vice President - U.S. Home, LLC
PRINTED OR TYPED NAME OF APPLICANT

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this 10th day of Sept. (m), 2025 (y), by Scott Edwards who is personally know to me or has produced ——— as identification.

JENNIFER M. SHEPPARD
MY COMMISSION # HH 699789
EXPIRES: November 16, 2029

SIGNATURE OF NOTARY PUBLIC

Jennifer M. Sheppard
PRINTED NAME OF NOTARY PUBLIC

PART IV

EXPLANATION

All Permit Application sections must be completed and submitted.

Permit Application fees and submittal items (see Part VI) must be submitted with the application.

Any and all work involving the lands and/or facilities of the District shall not commence without a Board approved permit.

All set policies, procedures, technical requirements, and fee schedules can be found and reviewed in the 'Administrative Policy and Procedures Guide'. The Guide is available at the District Office located at 601 East County Lane, Lehigh Acres, FL or online at la-msid.com/.

PART V

PROFESSIONAL CONSULTANT FORM

ATTORNEY

Name:	Charles Mann - Pavese Law Firm						
Mailing Address:	1833 Hendry Street						
City:	Fort Myers	State:	FL			Zip:	33901
Business No.	Area Code:	(239)	Number:	334-2195		Ext.	
Other Contact:							

ENGINEER

Name:	David R. Underhill, Jr. - Atwell, LLC						
Mailing Address:	10511 Six Mile Cypress Parkway						
City:	Fort Myers	State:	FL			Zip:	33966
Business No.	Area Code:	(239)	Number:	939-5490		Ext.	
Other Contact:	Jennifer Sheppard - jsheppard@atwell.com						

OTHER REPRESENTATIVE/PROFESSIONAL

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

PART VI

SUBMITTALS

NO. OF COPIES		SUBMITTAL DOCUMENT
SUB'D	REQ'D	
		ITEMS REQUIRED FOR INITIAL SUBMITTAL
✓	2	**Construction/Development Plans** signed and sealed by a Licensed Florida Professional Engineer including: 1. Location map showing the location of the proposed project in relation to adjacent streets and LA-MSID facilities. 2. Details of proposed works that fully communicate the work and/or use desired (i.e. Existing Conditions Survey, Proposed Site Plan, Paving Grading & Drainage Plan, Cross Sections, Element Details, and Erosion Control Plan sheets). 3. Sufficient survey data to determine elevations, property and ROW lines, limits of existing and proposed conditions, and notation of utilized vertical datum (NAVD/NGVD).
✓	2	**Stormwater Drainage Calculations** signed and sealed by a Licensed Florida Professional Engineer including, but not limited to, computer stormwater routing modeling data (ICPR4, Cascade, etc.) to verify compliance with the Design Criteria (input summary data below): **Water Quality Volume (Required):** **37.41** (ACRE-FT) **Water Quality Volume (Provided):** **42.18** (ACRE-FT) **Control Elevation:** **24 - 25.5** (FT-NGVD) **Minimum Road/Parking Elevation:** **26.5** (FT-NGVD) *(above the 10-year, 1-day design storm peak stage elevation)* **Minimum Perimeter Berm Elevation:** **27.35** (FT-NGVD) *(above the 25-year, 3-day design storm peak stage elevation)* **Minimum Finished Floor Elevation:** **28.0** (FT-NGVD) *(above the 100-year, 3-day design storm peak stage elevation and previously experienced high-water levels external to project)*
✓	2	**Flash Drive / CD** containing plans in both AutoCAD (.dwg file drawn in "State Plane Coordinates") and PDF formats.
✓	2	**Permit Application** (PARTS I & II)
✓	2	**Disclosure of Owner Form (signed and notarized)** (PART III)
✓	2	**Professional Consultant Form & Submittals** (PARTS V & VI)
✓	2	**Copy of Approved FDEP Notice of Intent (NOI)** for all sites one (1) acre and greater in size.
	2	**Copy of approved SWFMD permit**, for all sites required to get a permit, **or FDEP "10/2 Self-Certification"** approval.
✓	1	**Check for appropriate Permit Fees** made out to LA-MSID.

ATWELL

LETTER OF TRANSMITTAL

TO: Mr. Mike Cook
Lehigh Acres-Municipal Service Improvement District
601A East County Lane
Lehigh Acres, FL 33936

DATE: 11/5/2025
PHONE: 239.368.0044
FAX:

REFERENCE: Beth Stacey Boulevard - #25-86 **JOB NO:** BK342L

COPIES	DESCRIPTION
1	Check for $1,500.00 for review fee for the above project

REASON:
- ____ Your Comments
- ____ Your Approval
- ____ Your Information
- ____ Your Review
- ✓ Per Your Request

SENT VIA:
- ____ Regular Mail
- ✓ FedEx / UPS
- ____ Courier
- ____ Pick-up
- ____ E-mail

COMMENTS:

Thank you

Jennifer M. Sheppard
Senior Project Coordinator
Email: jsheppard@atwell.com

cc:

10511 Six Mile Cypress Pkwy, Suite 101, Fort Myers, FL 33966
www.atwell-group.com

Lennar Corporation
Lennar Homes, LLC
Lee/Collier Homebuilding
10481 Ben C. Pratt/6 Mile Cypress Pkwy
Ft. Myers, FL 33966

C925653772565377

PAGE: 1 of 1

DATE: November 4, 2025
CHECK NUMBER: 2565377
AMOUNT PAID: $1,500.00

00018 16885 CKS SD 25308 - 0002565377 NNNNNNNNNNN 3085100005203 KIP3C9 C
LEHIGH ACRES MUNICIPAL SERV IMPROVE
601 E COUNTY LN
LEHIGH ACRES FL 33936

Vendor No: 13301953

Date	CO. #	Invoice Number	Payment Advice	Gross Amount	Discount	Net Amount
10/30/25	023300	202530 150000 FEE	MV-Permit 25-86 Beth Stacy Ibi	$1,500.00	$0.00	$1,500.00
			MV-Permit 25-86 Beth Stacy Ibi			
		TOTALS		**$1,500.00**	**$0.00**	**$1,500.00**

Permit #25-86

PLEASE DETACH BEFORE DEPOSITING CHECK

Lennar Corporation
Lennar Homes, LLC
Lee/Collier Homebuilding
10481 Ben C. Pratt/6 Mile Cypress Pkwy
Ft. Myers, FL 33966

RECEIVED
NOV 14 2025
LEHIGH ACRES
MUNICIPAL SERVICES
IMPROVEMENT DISTRICT
COPY

CHECK NUMBER 2565377

70-2322
719

November 4, 2025

*** VOID AFTER 180 DAYS ***

PAY TO THE ORDER OF: LEHIGH ACRES MUNICIPAL SERV IMPROVE
601 E COUNTY LN
LEHIGH ACRES, FL 33936

CHECK AMOUNT
$1,500.00

EXACTLY *********1,500 DOLLARS AND 00 CENTS

Security features included. Details on back.

JPMorgan Chase Bank, N.A.
Chicago, IL

Authorized Signature

⑈2565377⑈ ⑆071923226⑆ 886293331⑈

February 23, 2026

Mr. Michael S. Cook, CDM
Lehigh Acres - Municipal Services Improvement District
601A East County Lane
Lehigh Acres, Florida 33936

Reference: **BETH STACEY BOULEVARD**
APPLICATION NO. 25-86

Dear Mr. Cook:

This letter is written in response to your review comments dated January 6, 2026. With this letter we are submitting the following additional information you requested:

1. Revised Engineering Plans
2. Stormwater Management Report

Presented below are responses to each review comment.

Comment 1: Please provide a cross-section of the proposed modifications to Copperhead Canal. At a minimum, the cross section should depict the existing canal conditions - including top of bank and toe of slope - and the proposed modifications, including the new top of bank, toe of slope, proposed swale, and roadway section. Elevations should be clearly indicated.

Response: Please see sections E-E and F-F on sheet 09 of the revised civil engineering plans.

Comment 2: Please include the proposed fill material specifications for the canal modifications.

Response: The proposed fill material will be USCS Soil Classification SP-Clean Sands with less than 5% fine or organic materials.

Comment 3: Please add a note to the design plans stating that any disturbed areas within the District's right-of-way must be restored with sod and maintained until fully established. Seeding will not be permitted.

Response: Please see sheet 08 of the revised civil engineering plans which requires disturbed areas to be sodded.

Comment 4: It is understood that a second 8-foot-wide sidewalk has been added along the roadway. Please provide supporting drainage calculations as needed, demonstrating that the stormwater management system can accommodate the additional impervious area.

Response: The additional impervious areas are represented in the revised land use calculations for basins 4 and 5. Please see the attached stormwater management report for the additional impervious areas and stormwater calculations.

Comment 5: According to 23-07 Stormwater Calcs, the minimum centerline road elevation is 27 feet, however on sheet 13, the section depicting station 408+50 has a pavement high point of 26.85 ft on the left, and 26.68 ft on the right. Station 408+0 is also below the minimum pavement centerline height requirement.

Response: **Permit 23-07 is based on the National Geodetic Vertical Datum of 1929. The engineering plans for Beth Stacey submitted in this application are based on the North American Vertical Datum of 1988. When converted, the minimum centerline road elevation goes from 27' NGVD29 to 25.8' NAVD88. Therefore, the elevations depicted on sections 408+50 and 408+0 meet the minimum centerline pavement requirement. Please see sheet 04 of the revised civil engineering plans for the Stormwater Summary Table and a Datum conversion example.**

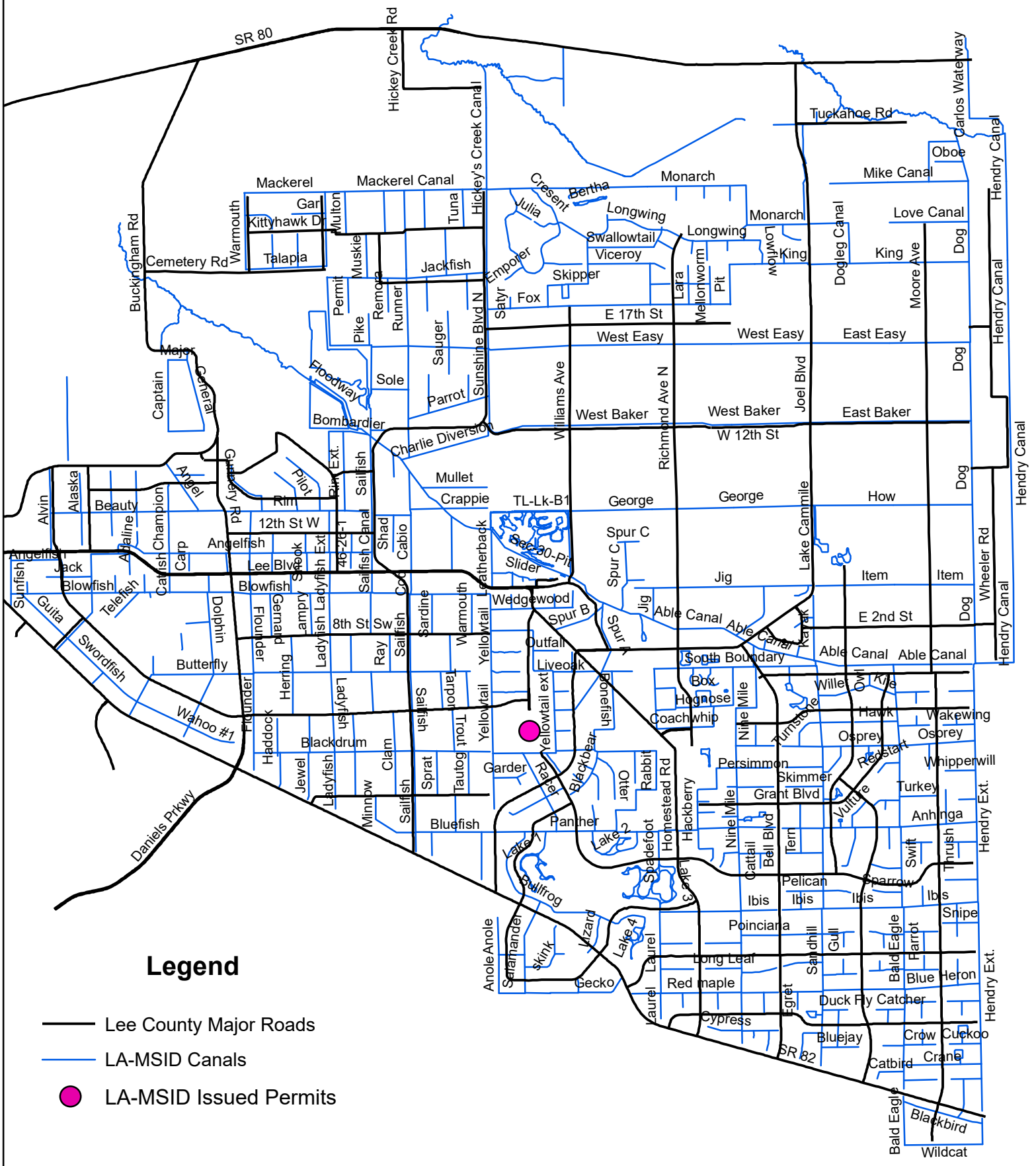
If you should have any questions or require additional information, please contact me at our Fort Myers office.

Sincerely,
ATWELL, LLC

Jennifer M. Sheppard
Senior Project Coordinator

:jms

Permit # 2025-86



Legend

- Lee County Major Roads
- LA-MSID Canals
- LA-MSID Issued Permits

IBIS LANDING

BETH STACEY BOULEVARD L.D.O. PLANS

SECTION 06, TOWNSHIP 45 SOUTH, RANGE 27 EAST
LEE COUNTY, FLORIDA



GENERAL LOCATION MAP
NOT TO SCALE

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199 FAX: (239) 278-3174

INDEX OF SHEETS

SHEET	TITLE
1	COVER SHEET
2	EXISTING CONDITIONS & AERIAL PHOTOGRAPH
3	MASTER SITE PLAN
4	MASTER DRAINAGE PLAN
5–8	PLAN & PROFILES
9	TYPICAL CROSS SECTIONS
10	PAVING, GRADING & DRAINAGE DETAILS
11	UTILITY DETAILS
12	BEST MANAGEMENT PRACTICE PLAN
13–18	CROSS SECTIONS

UTILITY OWNER CONTACTS

WATER & WASTEWATER:
FLORIDA GOVERNMENTAL UTILITY AUTHORITY
c/o ACCENTURE, 9400 SOUTHPARK CENTER LOOP, SUITE 400
ORLANDO, FL 32819
PHONE (407) 629-6900

ELECTRIC POWER SERVICE:
LEE COUNTY ELECTRIC CO-OP
P.O. BOX 3455
NORTH FORT MYERS, FLORIDA 33918
PHONE: (239) 995-2121
FAX: (239) 995-7904

TELEPHONE SERVICE:
SPRINT
2820 CARGO STREET BUILDING A
FORT MYERS, FLORIDA 33916
PHONE: (239) 336-2003
FAX: (239) 336-2026



SPECIFIC LOCATION MAP
NOT TO SCALE

PROJECT CONTACTS

ENGINEER:
DAVID R. UNDERHILL, P.E. PROJECT MANAGER
BANKS ENGINEERING
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

SURVEYOR:
BANKS ENGINEERING
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

DEVELOPER:
AQUABELLA DEVELOPMENT GROUP, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 278-1199

BANKS
ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

NO.	DATE	REVISION DESCRIPTION	BY
2	9-15-25	REVISED PER COUNTY COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

IBIS LANDING



N

0 250' 500' 750'
GRAPHIC SCALE 1"= 250'

IBIS LANDING

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

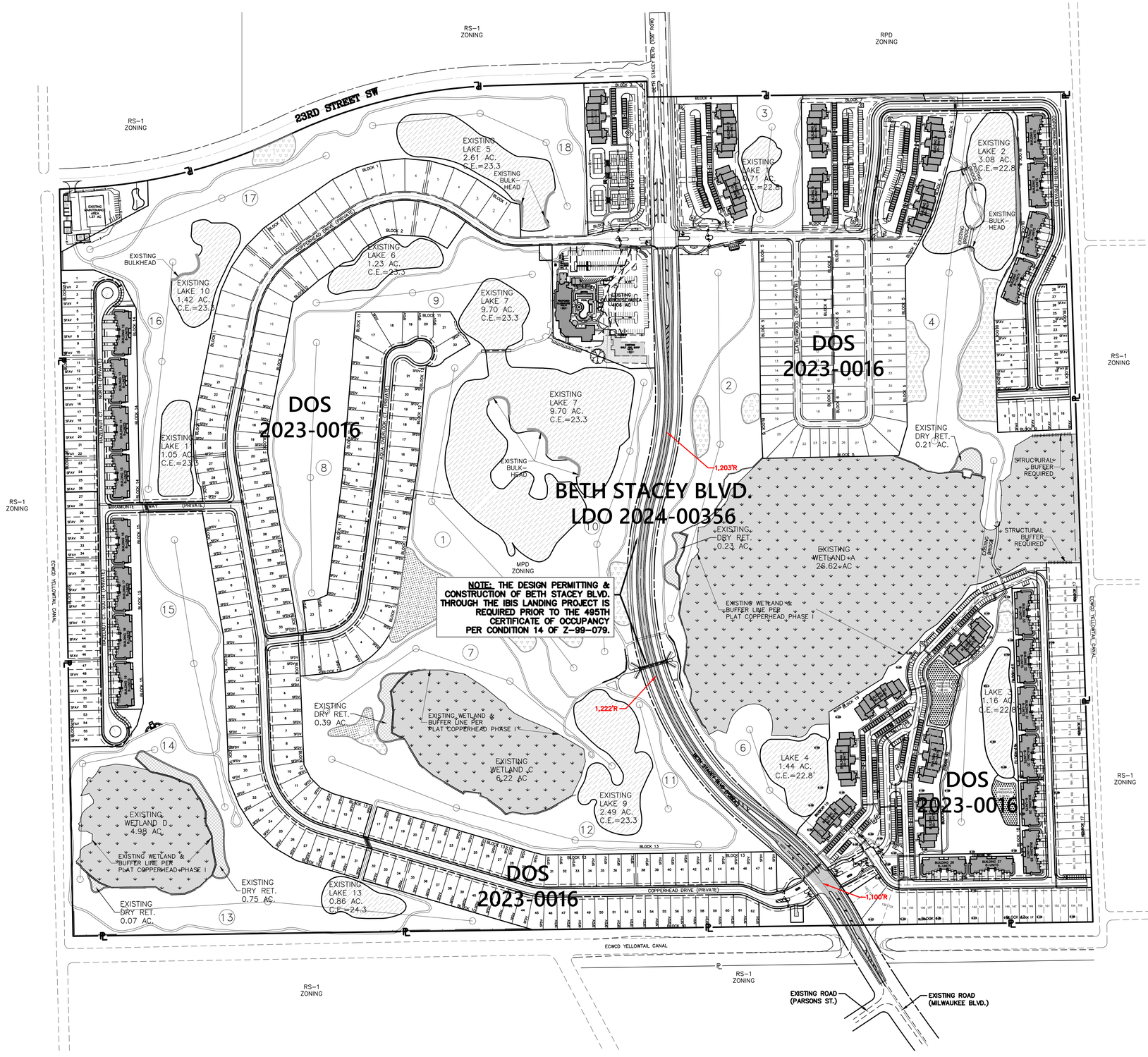
BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

EXISTING CONDITIONS & AERIAL PHOTOGRAPH
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	EX. CON-ARL	JLW	JLW	DRU	1"=250'	02



N

0 200' 400' 600'

GRAPHIC SCALE 1"= 200'

LEGEND:

- PROPERTY LINE
- RIGHT-OF-WAY LINE
- EXISTING STORM WATER MANAGEME
- PROPOSED ASPHALT PAVEMENT
- WETLAND AREA & BUFFER
- INDIGENOUS AREA PER #DOS 991203400D
- GOPHER TORTOISE PRESERVE

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

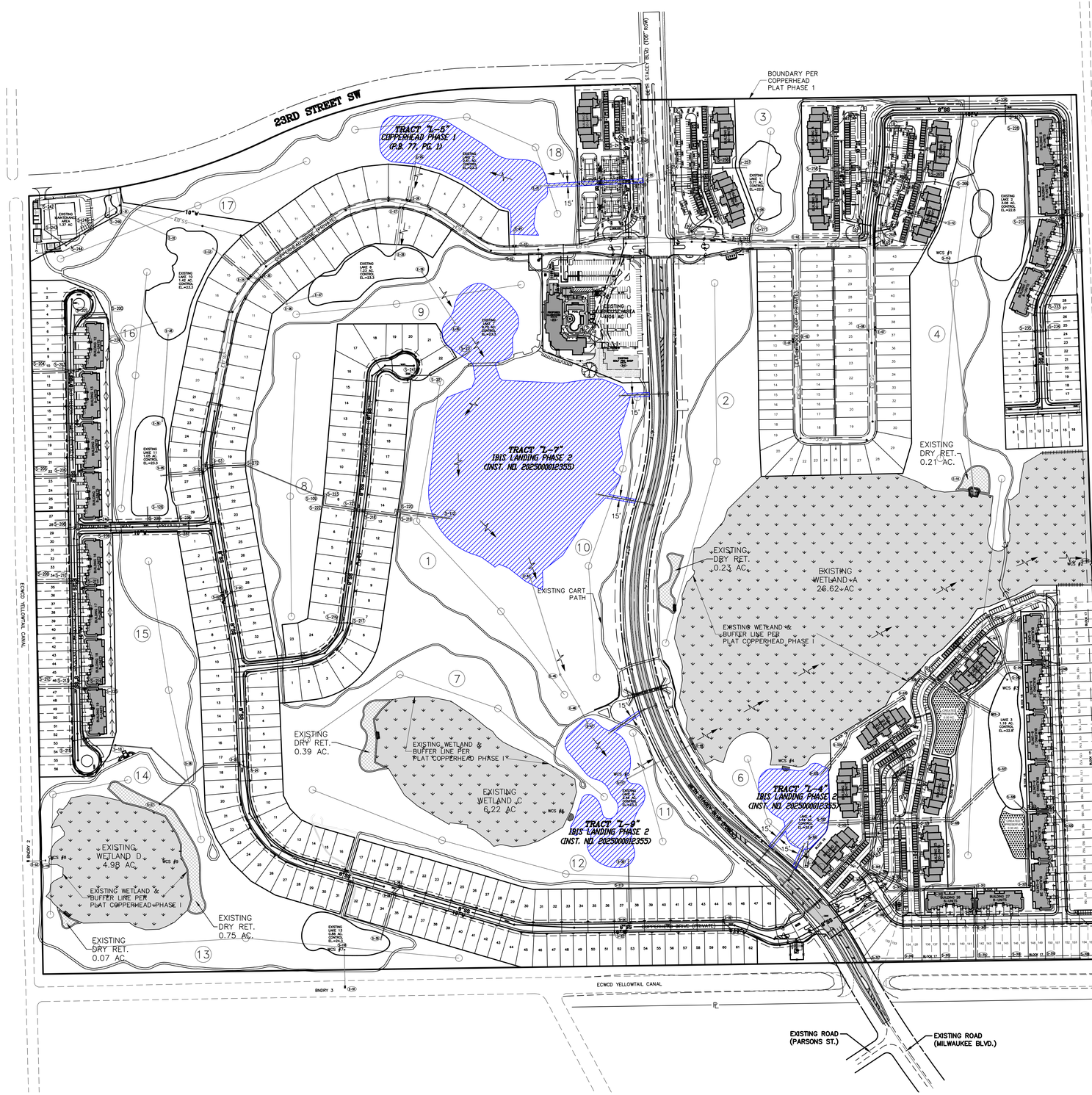
MASTER SITE PLAN

IBIS LANDING - BETH STACEY BLVD.

LEE COUNTY, FLORIDA

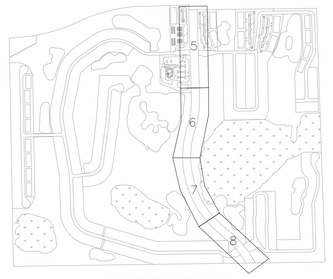
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	MSP	JLW	JLW	DRU	1"=200'	03

S:\JOBS\3XXX\3142L\ENGINEERING\DEVELOPMENT\BETH STACEY BLVD. LDO PLANS\3142L-BLVD-03-MSP.DWG 5/19/2025 9:23 AM JIMMY MCKAY



0 200' 400' 600'

GRAPHIC SCALE 1" = 200'



PROFILES KEY MAP

N.T.S.

LEGEND:

- PROPOSED STORM PIPE
- PROPOSED DRAINAGE STRUCTURE
- (S-1) PROPOSED DRAINAGE STRUCTURE W/IDENTIFICATION (SEE DETAIL SHEET)
- DRAINAGE FLOW ARROW
- A A TYPICAL CROSS SECTION
- GATE VALVE
- FIRE HYDRANT ASSEMBLY
- 90° BEND
- 45° BEND
- 22.5° BEND
- 11.25° BEND
- TEE
- 6"F 6" FORCE MAIN
- 8"F 8" FORCEMAIN
- E6"F EXISTING 6" FORCE MAIN
- E8"F EXISTING 8" FORCE MAIN
- 6"W 6" POTABLE WATER LINE
- 8"W 8" POTABLE WATER LINE
- 10"W 10" POTABLE WATER LINE
- 12"W 12" POTABLE WATER LINE
- E6"W EXISTING 6" POTABLE WATER LINE
- E8"W EXISTING 8" POTABLE WATER LINE
- E10"W EXISTING 10" POTABLE WATER LINE
- E12"W EXISTING 12" POTABLE WATER LINE
- +20.9 EXISTING GRADE
- 25.55 PROPOSED GRADE
- PROPOSED LEE COUNTY DRAINAGE EASEMENT

CONSTRUCTION NOTES:

1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH FLORIDA DEPT. OF TRANSPORTATION (F.D.O.T.) STANDARD SPECIFICATIONS FOR ROAD & BRIDGE CONSTRUCTION, LATEST EDITION.
2. CONTRACTOR TO DETERMINE & VERIFY LOCATION & ELEVATION OF ANY BURIED UTILITY LINES OR CABLES. CONTRACTOR IS RESPONSIBLE FOR THE REPAIR &/OR REPLACEMENT OF ANY UTILITY LINES OR CABLES DISTRIBUTED BY CONSTRUCTION OPERATIONS.
3. SUBGRADE & LIMEROCK BASE COURSES TO BE COMPACTED TO 98% MIN. OF THE MAX. DRY DENSITY AS DETERMINED BY AASHTO T-180 TESTING.
4. THE CONTRACTOR SHALL SUPPLY SATISFACTORY DENSITY TESTS OF SUBGRADE & LIMEROCK BASE MATERIALS TO THE ENGINEER PRIOR TO PAYMENT &/OR FINAL ACCEPTANCE. (ONE TEST PER COURSE PER 400 SQUARE YARDS).
5. CONTRACTOR SHALL PROVIDE & MAINTAIN TRAFFIC CONTROL IN ACCORDANCE WITH F.D.O.T. STANDARD SPECIFICATIONS & INDEXES & THE FEDERAL HIGHWAY ADMINISTRATION'S MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD), PART IV.
6. CONTRACTORS SHALL SOD ALL DISTURBED AREAS & OTHERWISE RESTORE ALL AREAS TO EXISTING CONDITION OR BETTER.
7. UPON COMPLETION OF ALL IMPROVEMENTS TO BETH STACEY BLVD. THE INITIAL STRIPPING SHALL BE ACCOMPLISHED BY PAINT. AFTER A 30 DAY "CURING" PERIOD THE ROADWAY WILL THEN BE RE-STRIPPED IN THERMOPLASTIC ALONG WITH THE PLACEMENT OF ALL REFLECTIVE PAVEMENT MARKERS (RPM'S).

SIGNING & PAVEMENT MARKING NOTES:

1. ALL SIGNING & STRIPPING WITHIN THE BETH STACEY BLVD. RIGHT OF WAY IS TO BE THERMOPLASTIC & IN ACCORDANCE WITH F.D.O.T. STANDARD INDEX 17346.
2. ALL REFLECTIVE PAVEMENT MARKERS (R.P.M.'S) SHALL BE IN ACCORDANCE WITH F.D.O.T. INDEX NO. 17352 AND F.D.O.T. STANDARD SPECIFICATIONS SECTION 706. ALL RPM'S SHALL BE CLASS "B" TYPE AND OFFSET 2" FROM SOLID LINES.
3. ALL EXISTING PAVEMENT MARKINGS CONFLICTING WITH NEW SRTIPING SHALL BE IMMEDIATELY REMOVED BY HYDROBLASTING TO ELIMINATE THE PREVIOUS MARKINGS REGARDLESS OF WEATHER AND LIGHT CONDITIONS.
4. ANY EXISTING SIGN TO REMAIN THAT IS DISTURBED DURING CONSTRUCTION OR RELOCATED SHALL BE RESET TO CURRENT STANDARDS FOR HEIGHT, OFFSET, & METHOD OF INSTALLATION. INSTALLATION OF ALL TRAFFIC SIGNS WITHIN THE BETH STACEY BLVD. R.O.W. SHALL COMPLY WITH THE CURRENT LEE COUNTY DESIGN STANDARDS FOR SIGN INSTALLATION.
5. CAUTION SHALL BE EXERCISED WHILE RELOCATING EXISTING SIGNS SO AS TO PREVENT DAMAGE TO THE SIGNS. IF THE SIGNS ARE DAMAGED BEYOND USE, AS DETERMINED BY FDOT, THEY SHALL BE REPLACED BY THE CONTRACTOR AT HIS/HER EXPENSE.
6. POSTED SPEED IS 35 MPH. DESIGN SPEED IS 40 MPH.
7. CONTRACTOR TO SECURE AND COMPLY WITH LEE COUNTY R/W USE PERMIT.
8. RPM SPACING ON DOUBLE YELLOW SOLID LINES SHALL BE 40 FEET.
9. RPM SPACING ON WHITE SOLID TURN LANE LINES SHALL BE 20'.
10. ALL DETECTABLE WARNING SURFACES SHALL BE CAST IN PLACE (WET SET & ANCHORED IN CONCRETE).
11. NO UTILITY STRUCTURES, VALVE BOXES OR METER BOXES ARE ALLOWED TO BE WITHIN THE SIDEWALK ALONG BARRETT ROAD.
12. ALL CROSSWALK STRIPING IS 4' SEPARATION FROM 24" STOP BAR. SEE DETAIL ON SHEET 10.

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523

ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690

WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

MASTER ENGINEERING PLAN

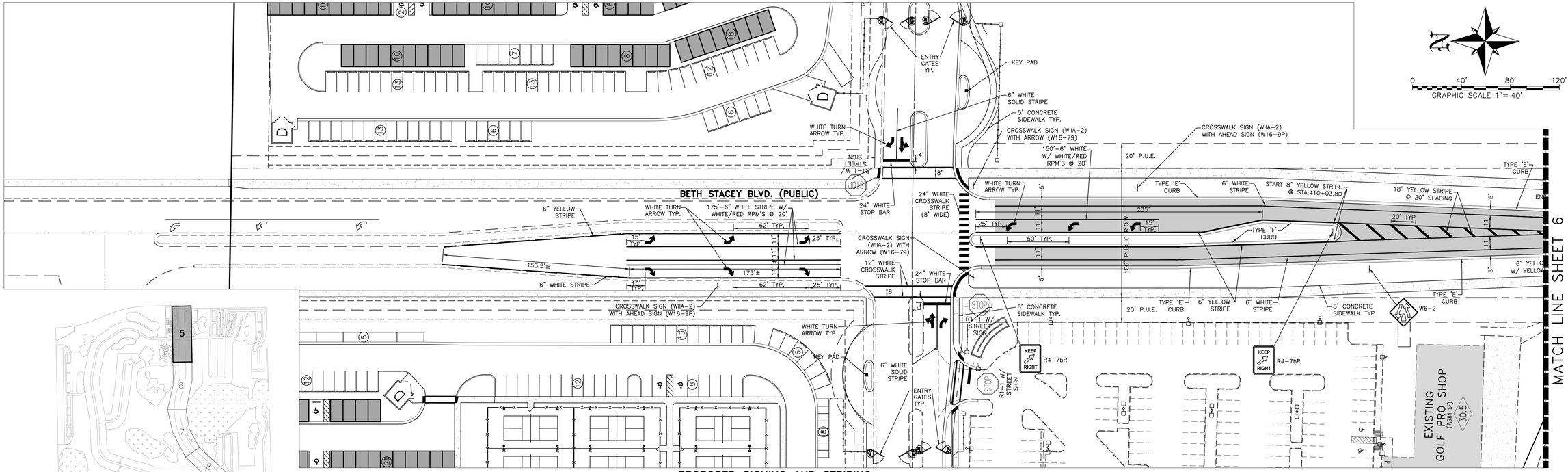
IBIS LANDING - BETH STACEY BLVD.

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	MEP	JLW	JLW	DRU	1"=200'	04

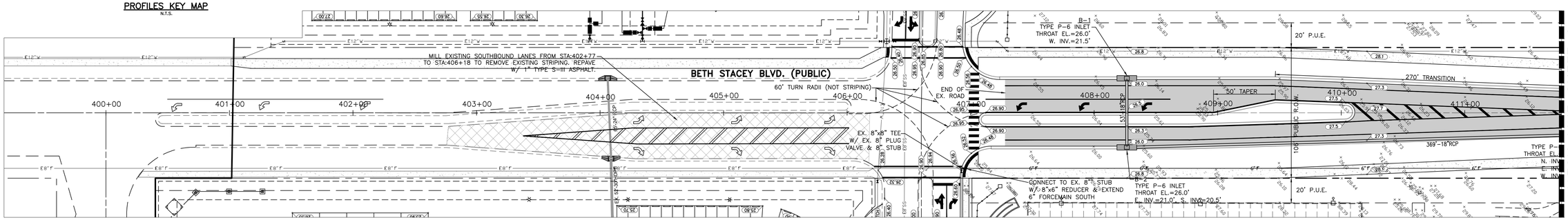
S:\JOBS\STATUS\3142L\ENGINEERING\DESIGN\PLANS\BETH STACEY BLVD. L.D.O. PLANS\3142L-BLVD-04-MEP.DWG 9/19/2025 9:54 AM JIMMY MCKAY

IBIS LANDING

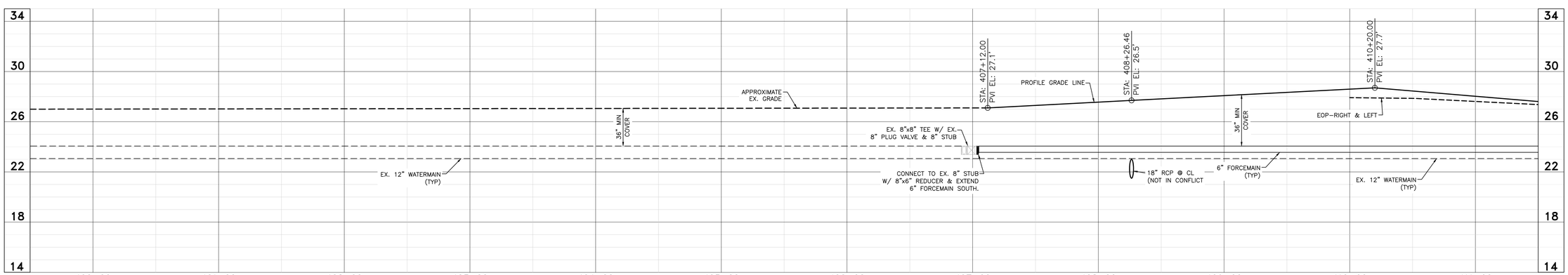


PROFILES KEY MAP
N.T.S.

PROPOSED SIGNING AND STRIPING
1" = 40'



PROPOSED PLAN & PROFILE
1" = 40'



ALL ELEVATIONS ARE BASED ON NORTH AMERICAN VERTICAL DATUM OF 1988

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
2	9-15-25	REVISED PER COUNTY COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

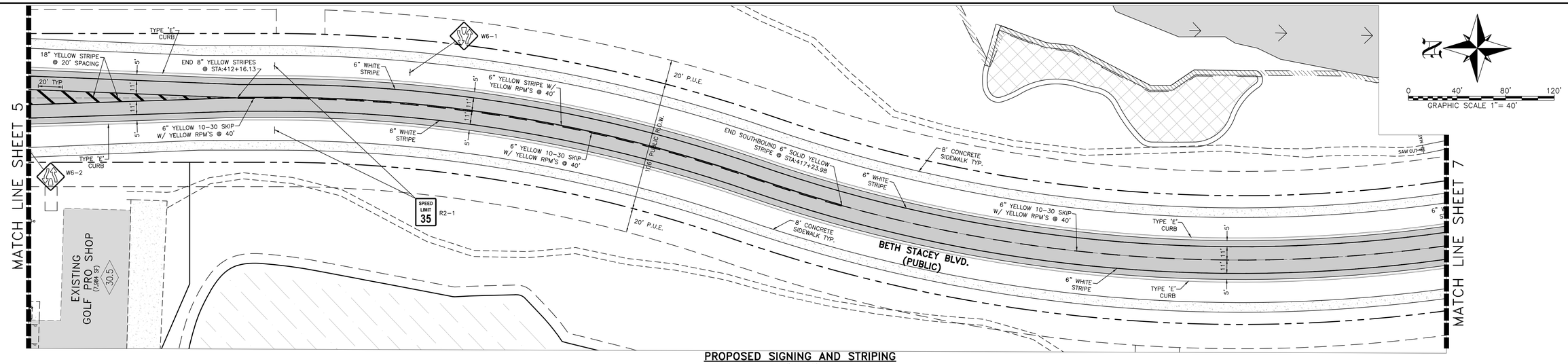
PLAN & PROFILE 1

IBIS LANDING - BETH STACEY BLVD.

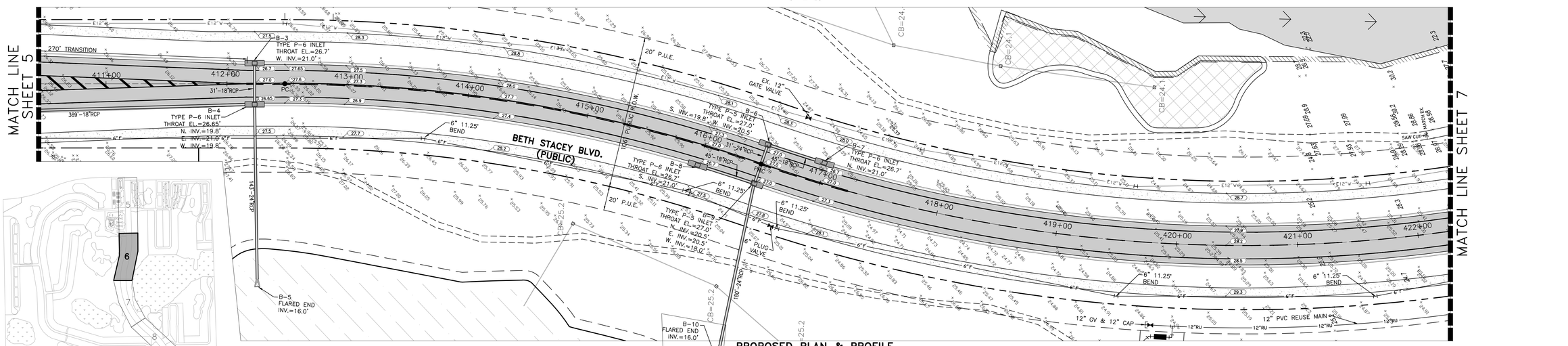
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	PP1	JLW	JLW	DRU	1"=4' V 1"=40' H	05

S:\JOBS\3142L\ENGINEERING\DESIGN\SHEETS\BETH STACEY BLVD. L.D.S. PLANS\3142L-BLVD-05-06-PP.DWG 9/9/2025 9:55 AM JIMMY MCKAY

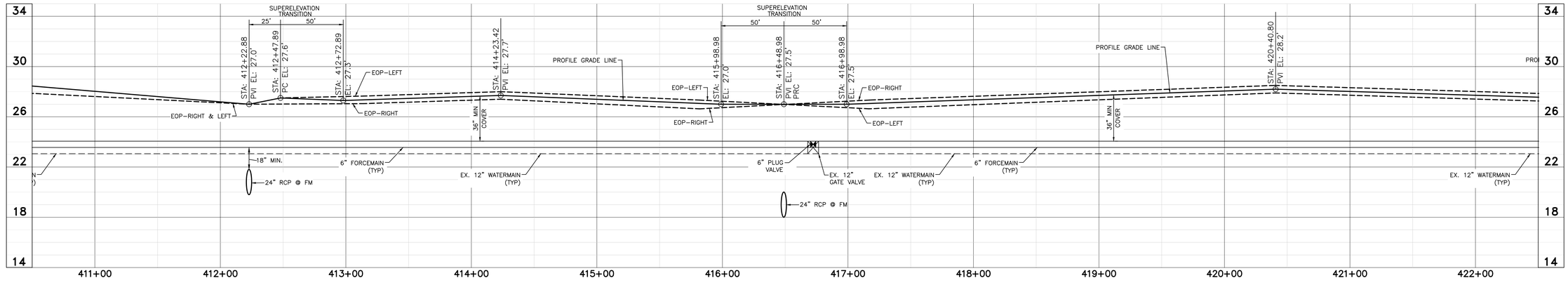


PROPOSED SIGNING AND STRIPING
1" = 40'



PROPOSED PLAN & PROFILE
1" = 40'

PROFILES KEY MAP
N.T.S.



ALL ELEVATIONS ARE BASED ON NORTH AMERICAN VERTICAL DATUM OF 1988

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
2	9-15-25	REVISED PER COUNTY COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

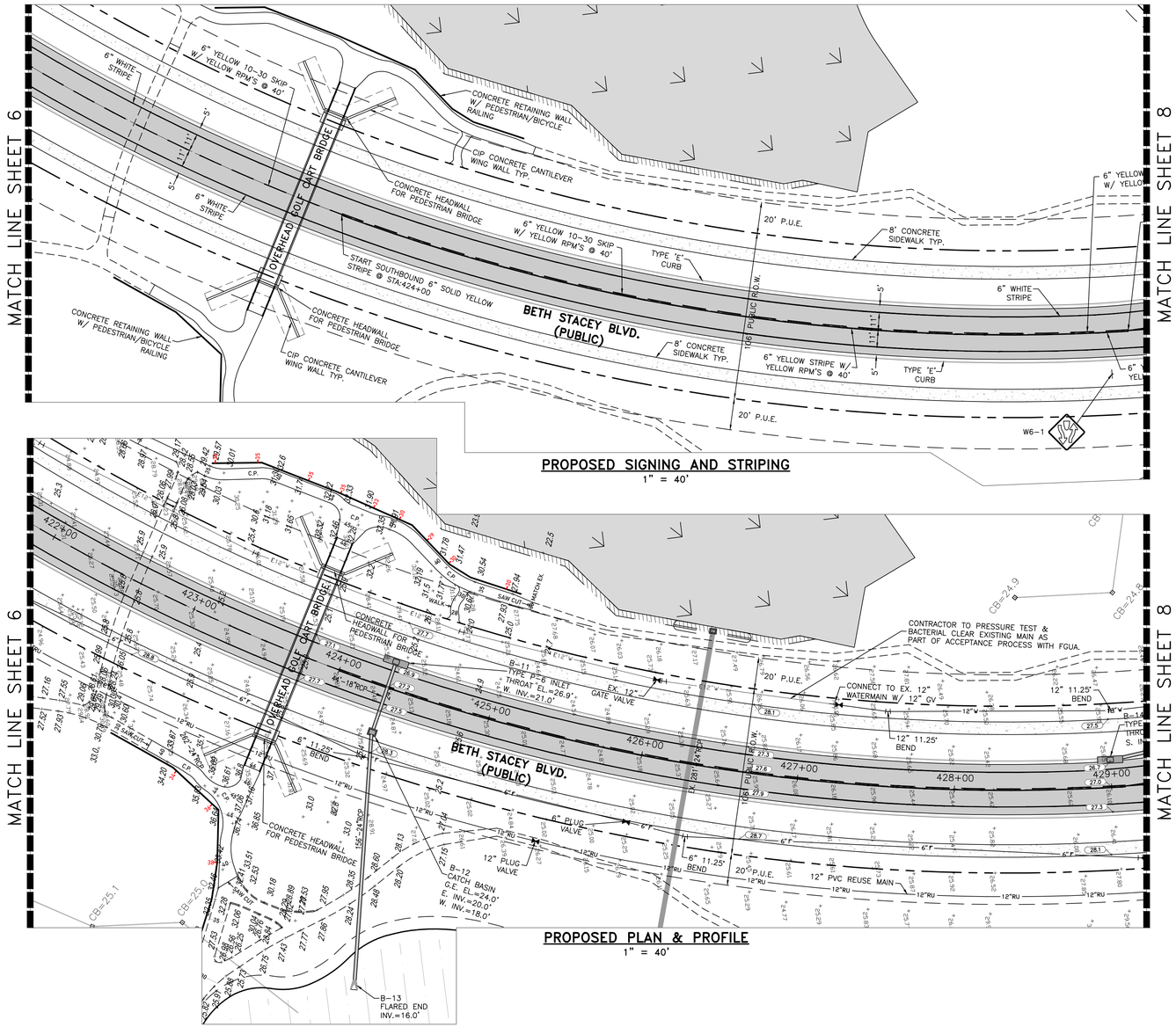
BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

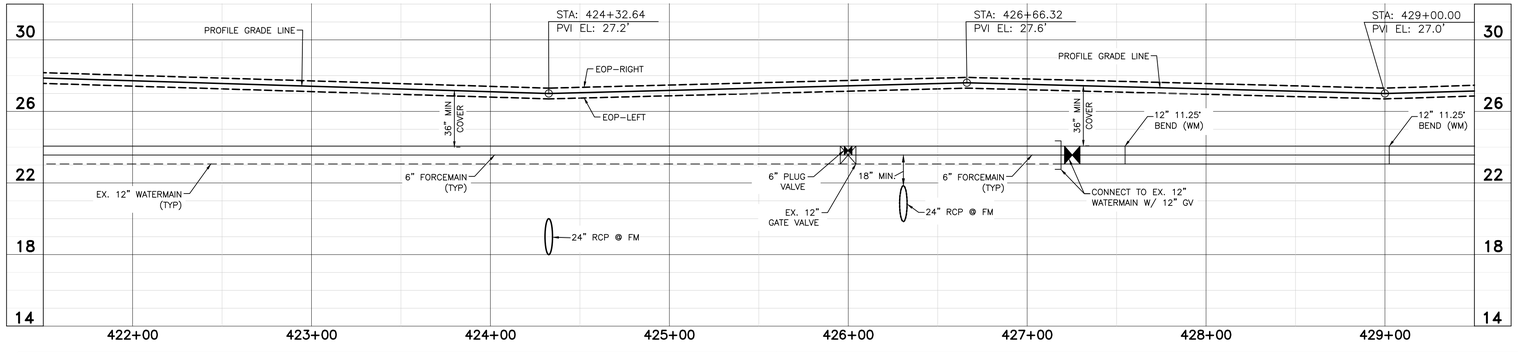
PLAN & PROFILE 2
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

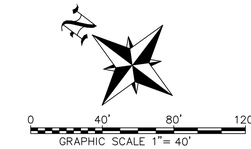
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	PP2	JLW	JLW	DRU	1"=4' V 1"=40' H	06

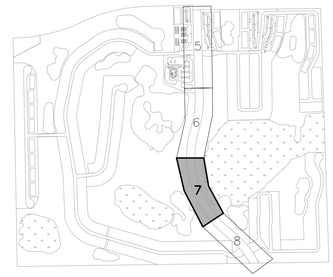


PROPOSED SIGNING AND STRIPING
1" = 40'

PROPOSED PLAN & PROFILE
1" = 40'







PROFILES KEY MAP
N.T.S.

NOTE: FOR GOLF CART BRIDGE DESIGN AND DETAILS SEE PLANS PREPARED BY ARCOS BRIDGE, INC.

ALL ELEVATIONS ARE BASED ON NORTH AMERICAN VERTICAL DATUM OF 1988

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
2	9-15-25	REVISED PER COUNTY COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

PLAN & PROFILE 3
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	PP3	JLW	JLW	DRU	1"=4' V 1"=40' H	07

IBIS LANDING

MATCH LINE SHEET 7

BETH STACEY BLVD. (PUBLIC)

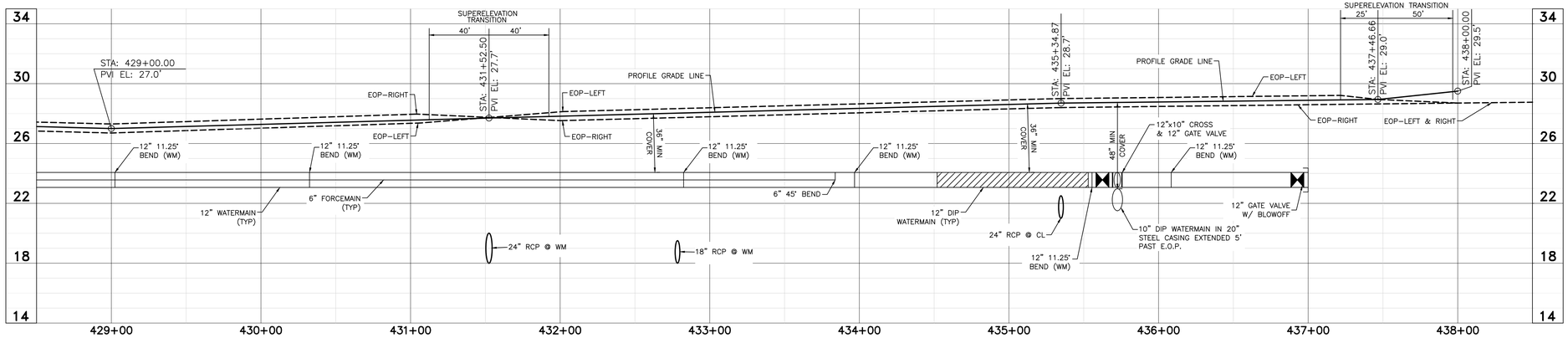
MILWAUKEE BLVD.

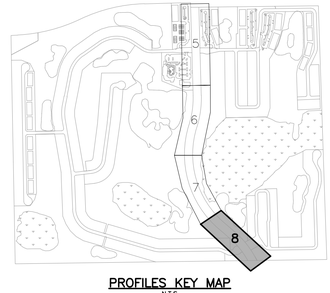
PARSONS ST.

0 40' 80' 120'
GRAPHIC SCALE 1" = 40'

PROPOSED SIGNING AND STRIPING
1" = 40'

PROPOSED PLAN & PROFILE
1" = 40'





PROFILES KEY MAP
N.T.S.

ALL ELEVATIONS ARE BASED ON NORTH AMERICAN VERTICAL DATUM OF 1988

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
2	9-15-25	REVISED PER COUNTY COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523

ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690

WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

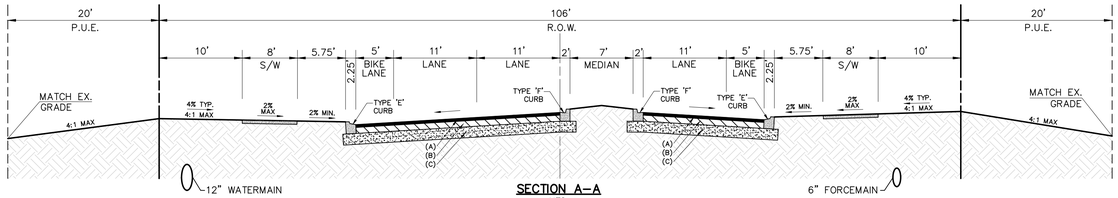
PLAN & PROFILE 4

IBIS LANDING - BETH STACEY BLVD.

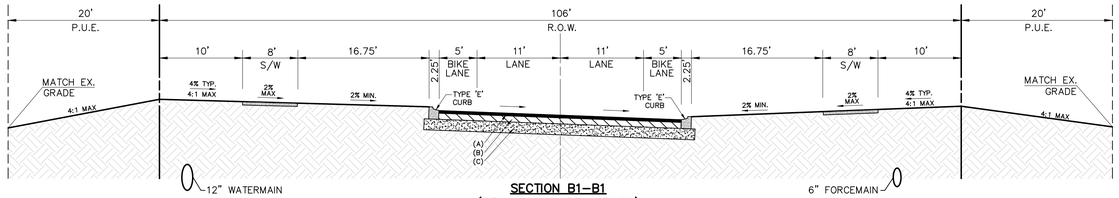
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	PP4	JLW	JLW	DRU	1"=4', V 1"=40' H	08

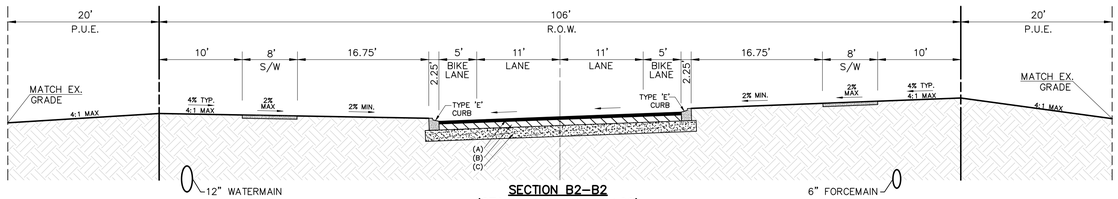
S:\JOBS\3XXX\3142L\ENGINEERING\CIVIL\PLANS\BETH STACEY BLVD. L.D.O. PLANS\3142L-BLVD-05-08-PP.DWG 9/19/2025 8:58 AM JIMMY WIECKY



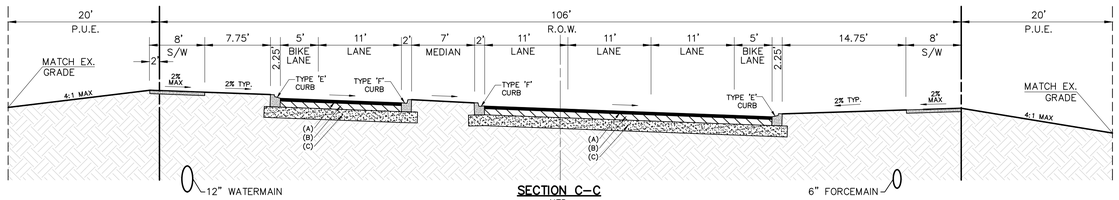
SECTION A–A
NTS



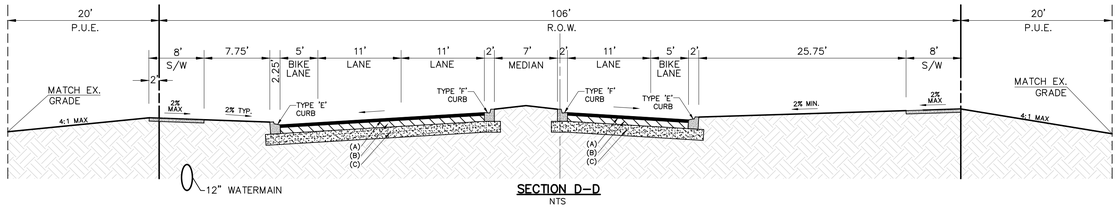
SECTION B1–B1
(STA: 412+00 TO 417+00)
NTS



SECTION B2–B2
(STA: 417+00 TO 429+00)
NTS



SECTION C–C
NTS



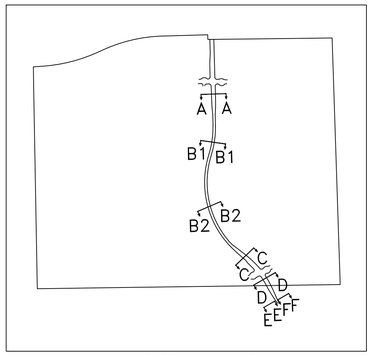
SECTION D–D
NTS

PAVEMENT DESIGN (BETH STACEY BLVD.)

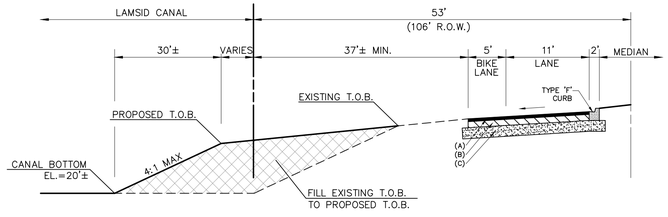
(A) 1.5" TYPE SP 12.5 ASPHALT W/ 1" TYPE SP 9.5 ASPHALT SURFACE COURSE.

(B) 8" LIMEROCK BASE (MIN. LBR=100) COMPACTED TO 98% OF MAXIMUM DENSITY BY AASHTO T-180.

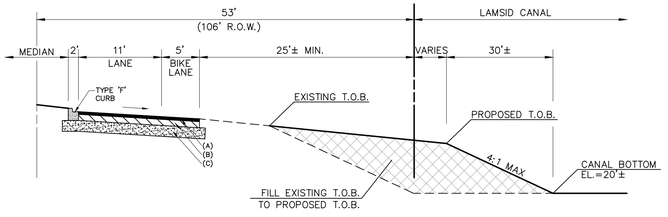
(C) 12" STABILIZED SUBGRADE TYPE B STABILIZATION (MIN. LBR=40) COMPACTED TO 98% OF MAXIMUM DENSITY BY AASHTO T-180.



SECTION KEY MAP
NTS



SECTION E–E
NTS



SECTION F–F
NTS

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

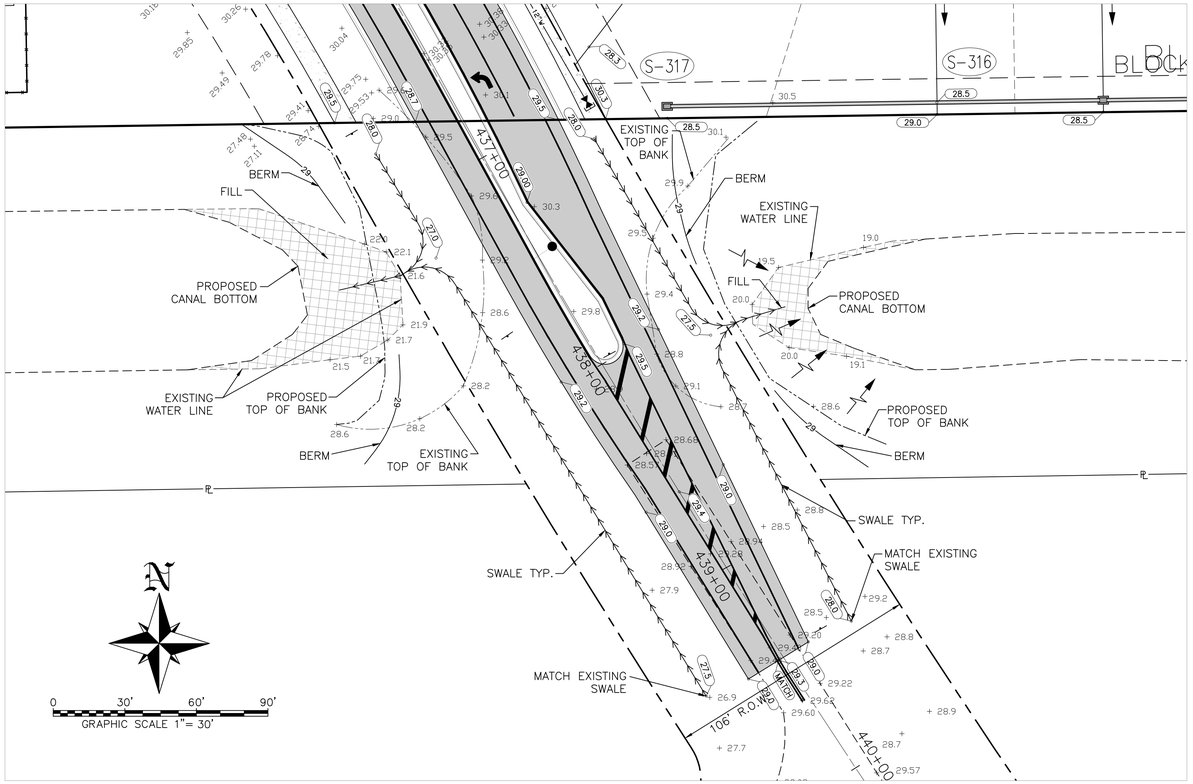
NO.	DATE	REVISION DESCRIPTION	BY
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM
DAVID R. UNDERHILL JR.
P.E. #47029

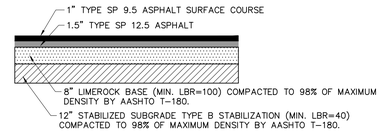
TYPICAL CROSS-SECTIONS
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	XSEC	JLW	JLW	DRU	NTS	09

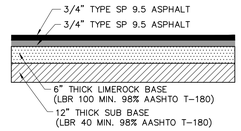
S:\JOBS\3XXX\3142L\ENGINEERING\DESIGN\JOBS\BETH STACEY BLVD. LEE. FLA\XS\3142L-BLVD-09-DESIGNS 6/9/2025 9:58 AM JIMMY MCKEWY



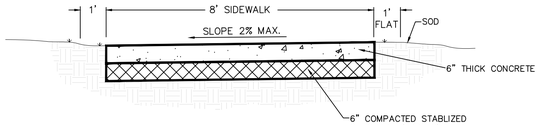
BETH STACEY BLVD. & MILWAUKEE BLVD TIE-IN
SCALE: 1"=30'



BETH STACEY R.O.W. PAVEMENT SECTION
N.T.S.



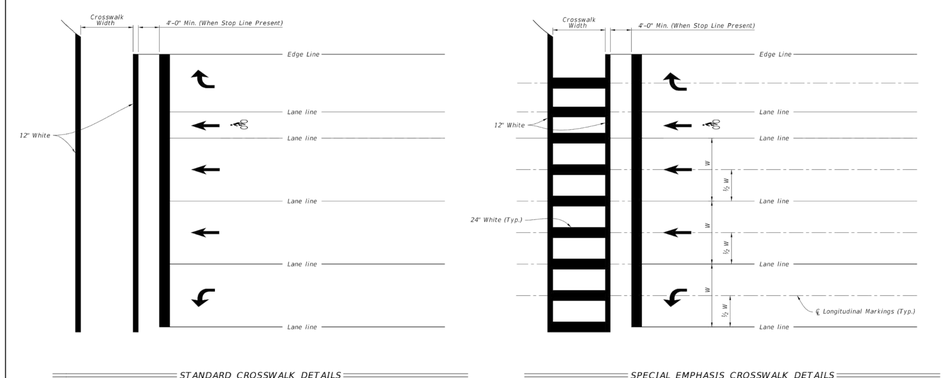
LOCAL STREET PAVEMENT SECTION
N.T.S.



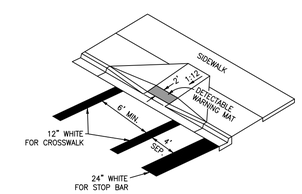
CONSTRUCTION NOTES:

1. TYPICAL: SCORE OR SAW CUT WALKS 1 1/2" DEEP, 5'-0" O.C.
2. CONCRETE: 6" THICK FDOT CLASS I, 3000 P.S.I. MIN.
3. SIDEWALK FINISH TO BE LIGHT BROOM.
4. ALL DISTURBED AREAS NEXT TO WALK TO BE RESODDED WITH BAHIA TO MATCH ORIGINAL GRASS.
5. SIDEWALK CROSS SLOPE SHALL NOT EXCEED 2% CROSS SLOPE OR 5% RUNNING SLOPE.
6. SIDEWALK CONSTRUCTION SHALL COMPLY WITH THE LATEST EDITION OF THE FLORIDA GREENBOOK AND FDOT DESIGN STANDARDS, INCLUDING CURB RAMPS AND DETECTABLE WARNING SURFACES.
7. NO UTILITY STRUCTURES, VALVE BOXES OR METER BOXES ARE ALLOWED TO BE WITHIN THE SIDEWALK ALONG BARRETT ROAD

BETH STACEY BLVD. SIDEWALK DETAIL
N.T.S.



NOTES:
1. For crosswalk width, exceed width of the adjacent sidewalk, but do not make width less than 6' for intersection crosswalks and 10' for midblock crosswalks. Measure width from the inside of the transverse crosswalk markings.
2. When the Special Emphasis Crosswalk is not perpendicular to the lane lines, make the longitudinal markings parallel to the lane lines.
3. Refer to Index 522-002 when Curb Ramps are present.



TYPICAL CROSSWALK W/ DETECTABLE WARNING MAT & STRIPING SEPARATION DETAIL
N.T.S.

LAST REVISION	DESCRIPTION:	FDOT FY 2023-24 STANDARD PLANS	PAVEMENT MARKINGS	INDEX	SHEET
11/01/21				711-001	9 of 13

CROSSWALK PAVEMENT MARKING DETAILS
N.T.S.

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

PAVING GRADING & DRAINAGE DETAILS
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	PGD-DT	JLW	JLW	DRU	NTS	10

IBIS LANDING

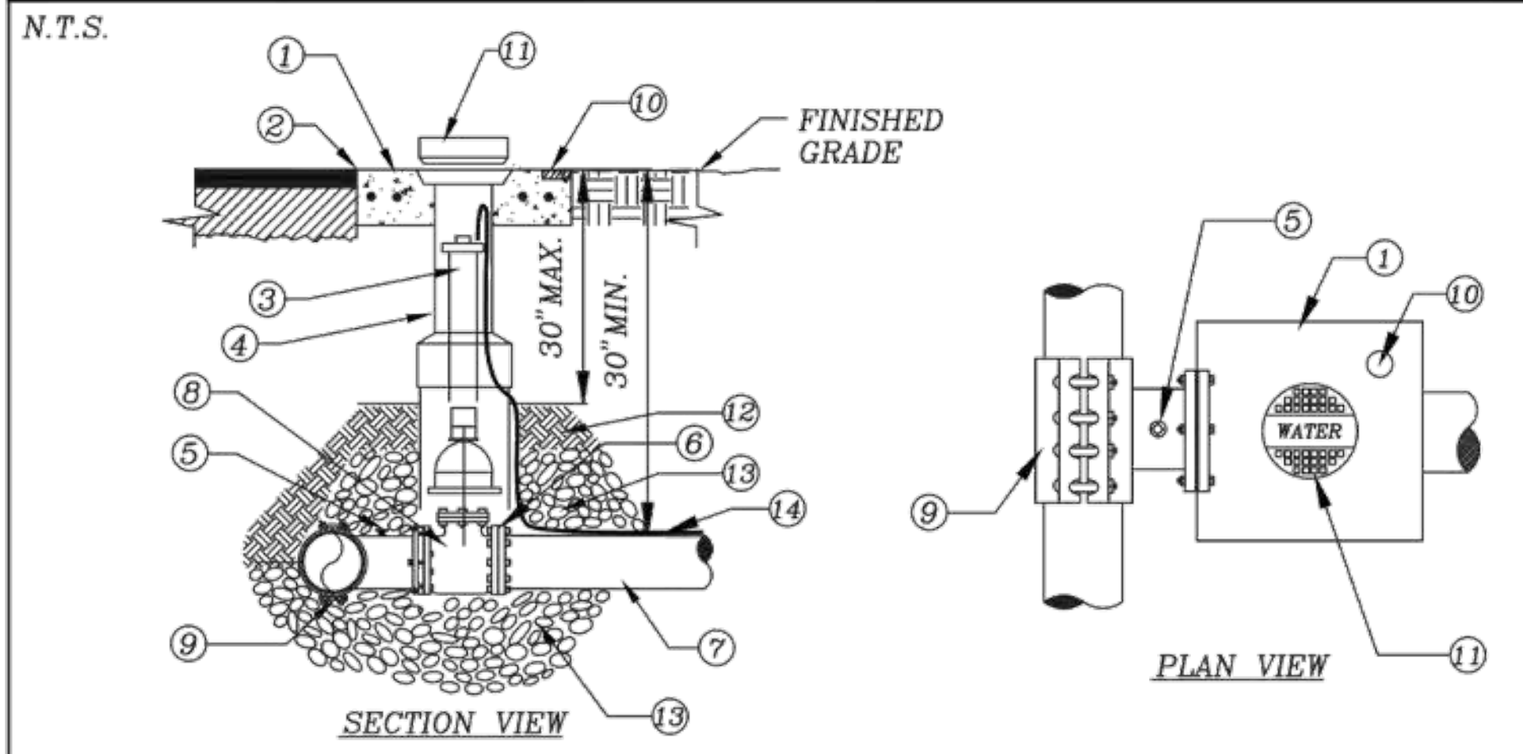
S:\USERS\STAFF\LISAGOODRICH\DESKTOP\LIVE PROJECTS\BETH STACEY BLVD. (L.D.) PLANS\3142L-BLVD-10-PGD-DT.DWG 5/19/2023 9:58 AM JIMMY MCKAY

3/6/2023 2:38:27 PM

STANDARD DETAIL NO. 6.1

TAPPING SLEEVE AND VALVE

N.T.S.



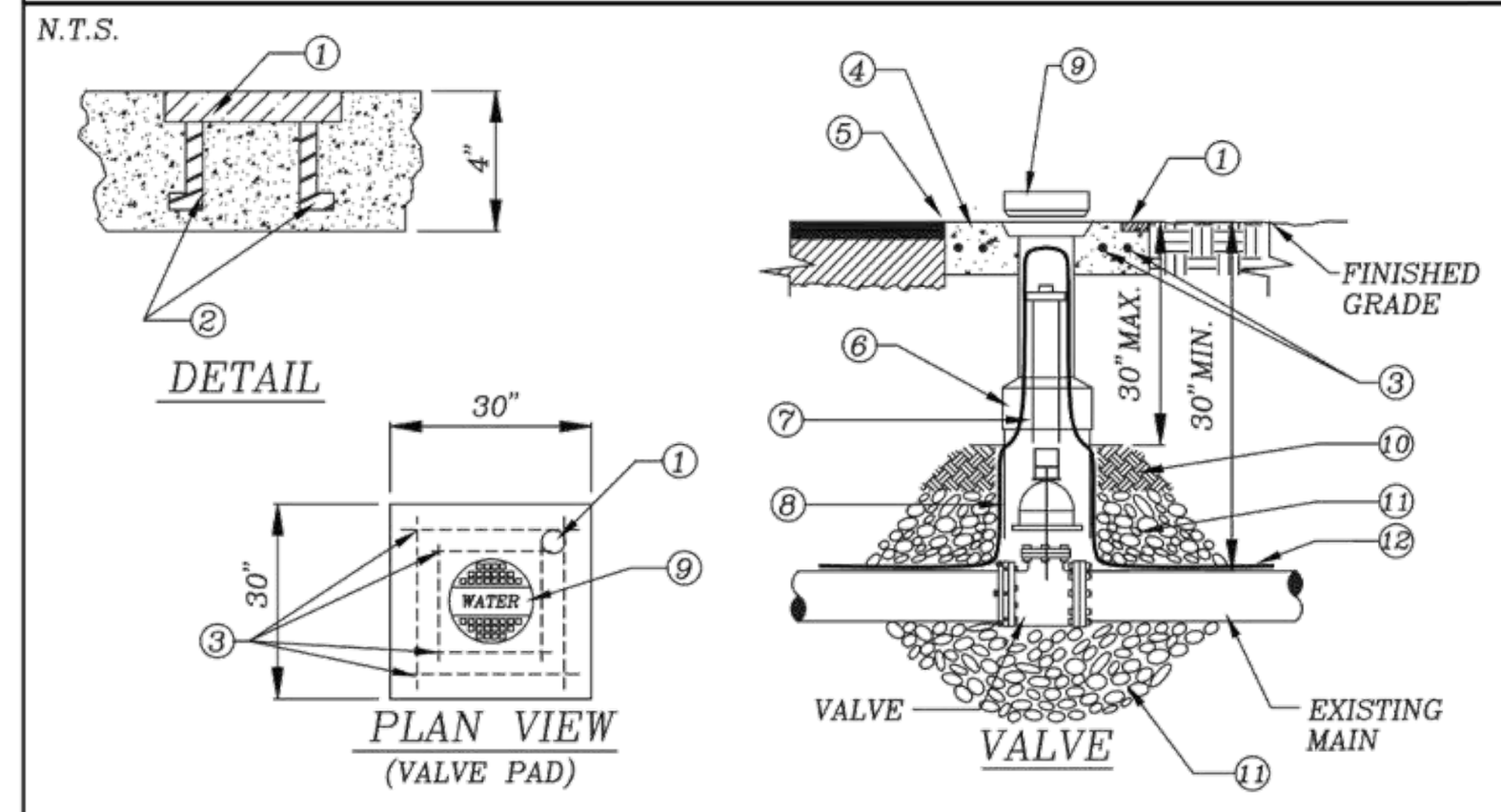
① CONCRETE PAD (SEE VALVE INSTALLATION DETAIL)
② SET TOP OF BOX FLUSH WITH FINISHED GRADE
③ EXTENSION STEM IF TOP OF OPERATING NUT IS MORE THAN 30" BELOW FINISH GRADE
④ VALVE BOX (SEE VALVE INSTALLATION DETAIL)
⑤ OUTLET FOR TESTING
⑥ MJ RESTRAINED
⑦ MAIN SIZE VARIES
⑧ TAPPING VALVE (SEE VALVE INSTALLATION DETAIL)
⑨ LCU APPROVED ALL STAINLESS STEEL PRESSURE RATED TAPPING SLEEVE. FOR SIZE ON SIZE TAPS A FLANGED TAPPING SLEEVE IS REQUIRED. SPRING LOADED TAPPING SLEEVE IS REQUIRED WHEN CONNECTING TO HDPE (SEE APPROVED MATERIAL LIST).
⑩ BRASS PLATE (SEE VALVE INSTALLATION DETAIL)
⑪ CAST IRON DROP COVER MARKED "WATER","SEWER", OR "REUSE"
⑫ COMPACTED SUITABLE EARTH BACKFILL
⑬ 3/4" GRANULAR MATERIAL #57 STONE
⑭ 12 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.2

VALVE INSTALLATION

N.T.S.



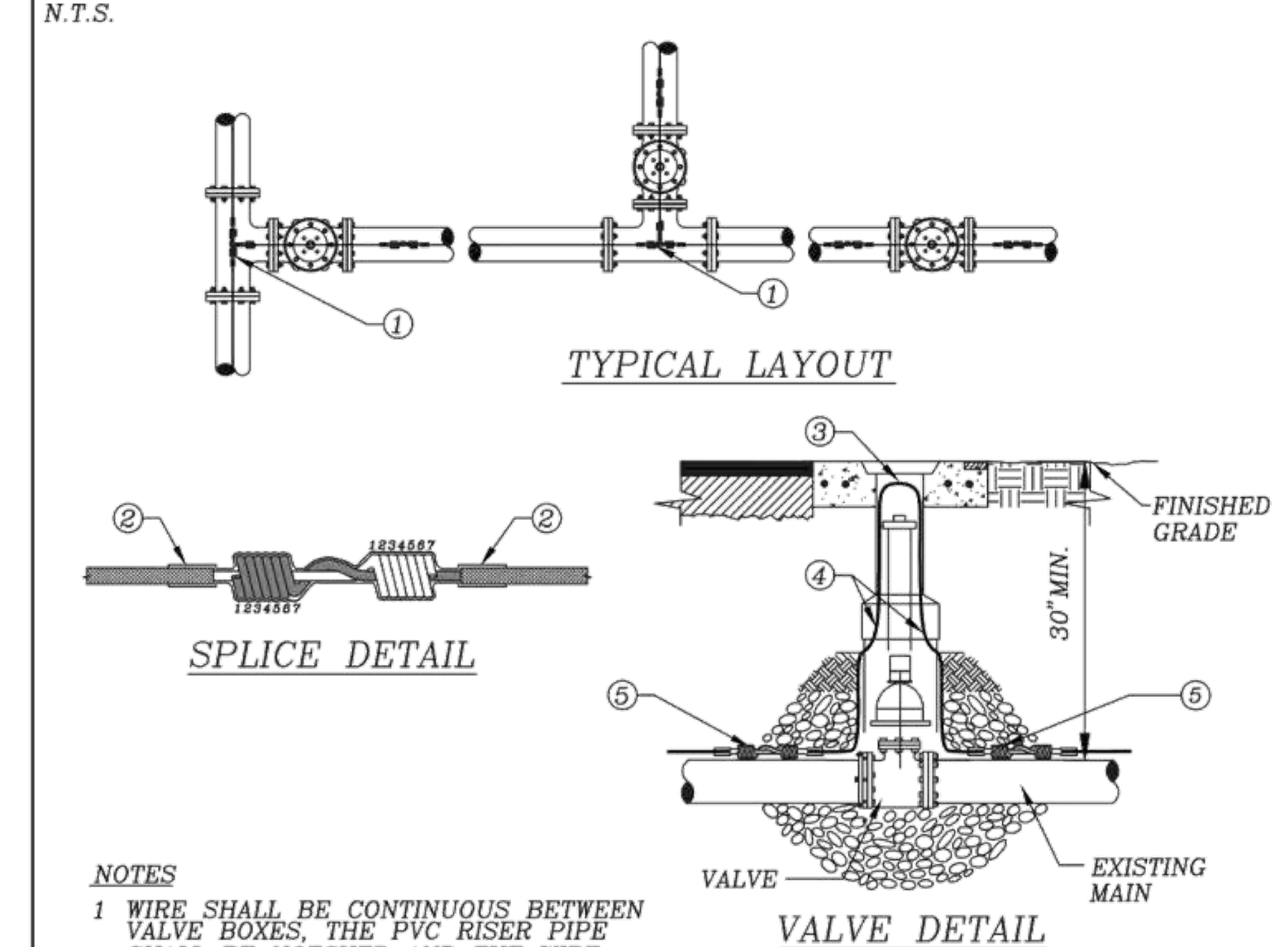
① BRASS PLATE: SIZE OF VALVE, VALVE TYPE No. OF TURNS & DIRECTION TO OPEN VALVE M.F.G. & YEAR INSTALL SYSTEM "WATER" OR "SEWER" OR "REUSE" OR "FIRE"
② ANCHOR
③ EIGHT (8) #4 BARS OVERLAP EACH CORNER BY 2"
④ 30"SQ X 4" THK.CONC.PAD SURROUNDING BOX, MIN. 3,000 P.S.I. POURED IN PLACE
⑤ SET TOP OF BOX FLUSH WITH FINISHED GRADE
⑥ HEAVY DUTY TRAFFIC BEARING CAST IRON VALVE BOX, ADJUSTABLE SCREW TYPE, 5 1/4" DIAMETER SHAFT THAT IS LCU APPROVED
⑦ EXTENSION STEM WITH 2" OPERATING NUT AS REQUIRED, IF NUT IS MORE THAN 30" BELOW FINISH GRADE
⑧ RISER NOT TO BEAR ON VALVE OR PIPE
⑨ CAST IRON DROP COVER MARKED "WATER" OR "SEWER" OR "REUSE OR "FIRE"
⑩ COMPACTED SUITABLE EARTH BACKFILL
⑪ 3/4" GRANULAR MATERIAL #57 STONE
⑫ 12 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.3

LOCATING WIRE FOR PRESSURE MAINS

N.T.S.



NOTES

1 WIRE SHALL BE CONTINUOUS BETWEEN VALVE BOXES, THE PVC RISER PIPE SHALL BE NOTCHED AND THE WIRE ATTACHED TO THE RISER PIPE TO ALLOW FOR THE WIRE TO FIT INSIDE THE CAST IRON VALVE BOX WITHOUT SHEARING THE WIRE.
2 LOCATING WIRE SHALL BE LAID ON TOP OF THE MAIN, AND SHALL BE TAPED TO IT AT 10' INTERVALS AND TAPED AT ALL FITTINGS. TAPE SHALL BE 10 MIL POLYETHYLENE.
3 CONTRACTOR SHALL CONDUCT A CONTINUITY TEST ON ALL LOCATING WIRE SPLICES. PRIOR TO SCHEDULING THE OFFICIAL FINAL INSPECTION WITH LCU.
4 ALL SPLICES SHALL UTILIZE A DIRECT BURY WATERPROOF SPLICE KIT. (SEE APPROVED MATERIAL LIST)

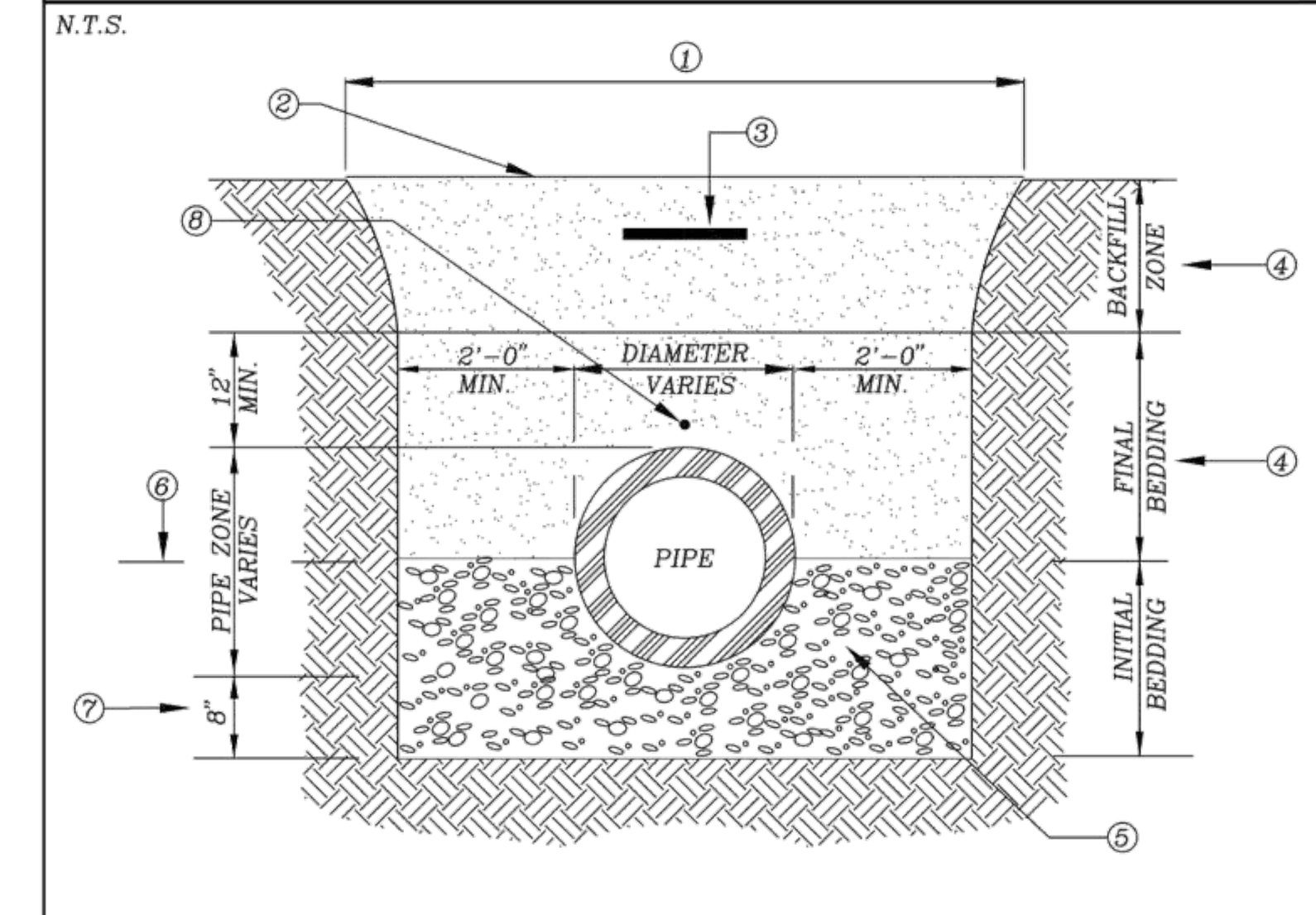
① SPLICE RUN TOGETHER, KEEP CLEAR OF FITTING, TYPICAL
② COVER SPLICE WITH 2 WRAPS OF 10 MIL. POLYETHYLENE TAPE
③ 6" LOOP OF BARE WIRE, 9" MAX. DEPTH TO TOP OF BARE WIRE
④ 12 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE
⑤ SPLICE (SEE DETAIL THIS SHEET)

REV: 12/10/2015

STANDARD DETAIL NO. 6.5

TRENCH CROSS SECTION

N.T.S.



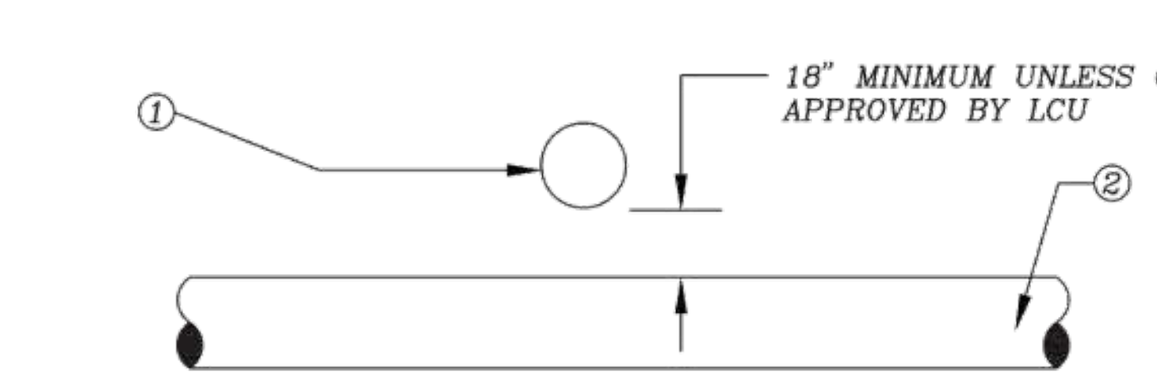
① EXCAVATED TRENCH
② FINISHED GRADE
③ MARKING TAPE 12" DEPTH MAX.
④ MATERIALS CLEAN DRY SAND, FINE LIME ROCK OR PIT SHELL MECHANICALLY COMPACTED IN 6" LIFTS TO AT LEAST 95% OF MAX. DENSITY, OR 98% IF UNDER PAVED AREA OF ROADWAY.
⑤ #57 STONE LOCATED UNDER PIPE OR EXISTING UNDISTURBED SUITABLE MATERIAL
⑥ SPRING LINE
⑦ STANDARD 8" MIN TRENCH UNDERCUT AND BACKFILL WITH #57 STONE COMPACTED IN 6" LIFTS ADDITIONAL UNDERCUT AND CRUSHED ROCK BEDDING FOUNDATION WHEN DIRECTED BY L.C.U. THE CONTRACTOR IS TO REMOVE UNSTABLE MATERIAL FROM THE TRENCH FOUNDATION. THEN INSTALL STABILIZED CRUSHED ROCK BACKFILL WITH #57 STONE COMPACTED IN 6" LIFTS
⑧ LOCATING WIRE IS REQUIRED FOR ALL PRESSURIZED PIPELINES.

REV: 12/10/2015

STANDARD DETAIL NO. 6.8

WATER AND SEWER CROSSING DETAIL

N.T.S.

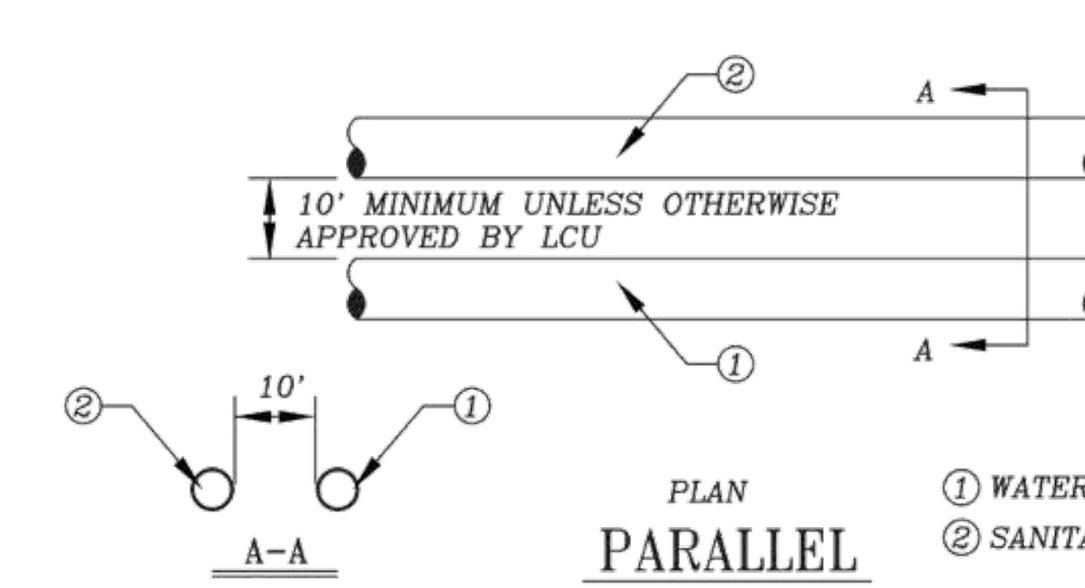


SECTION
CROSSINGS

① WATER MAIN
② SANITARY SEWER MAIN

NOTE: IF LESS THAN 18" CLEARANCE ENCASE WATER MAIN IN STEEL CASING

N.T.S.



PLAN
PARALLEL

① WATER MAIN
② SANITARY SEWER MAIN

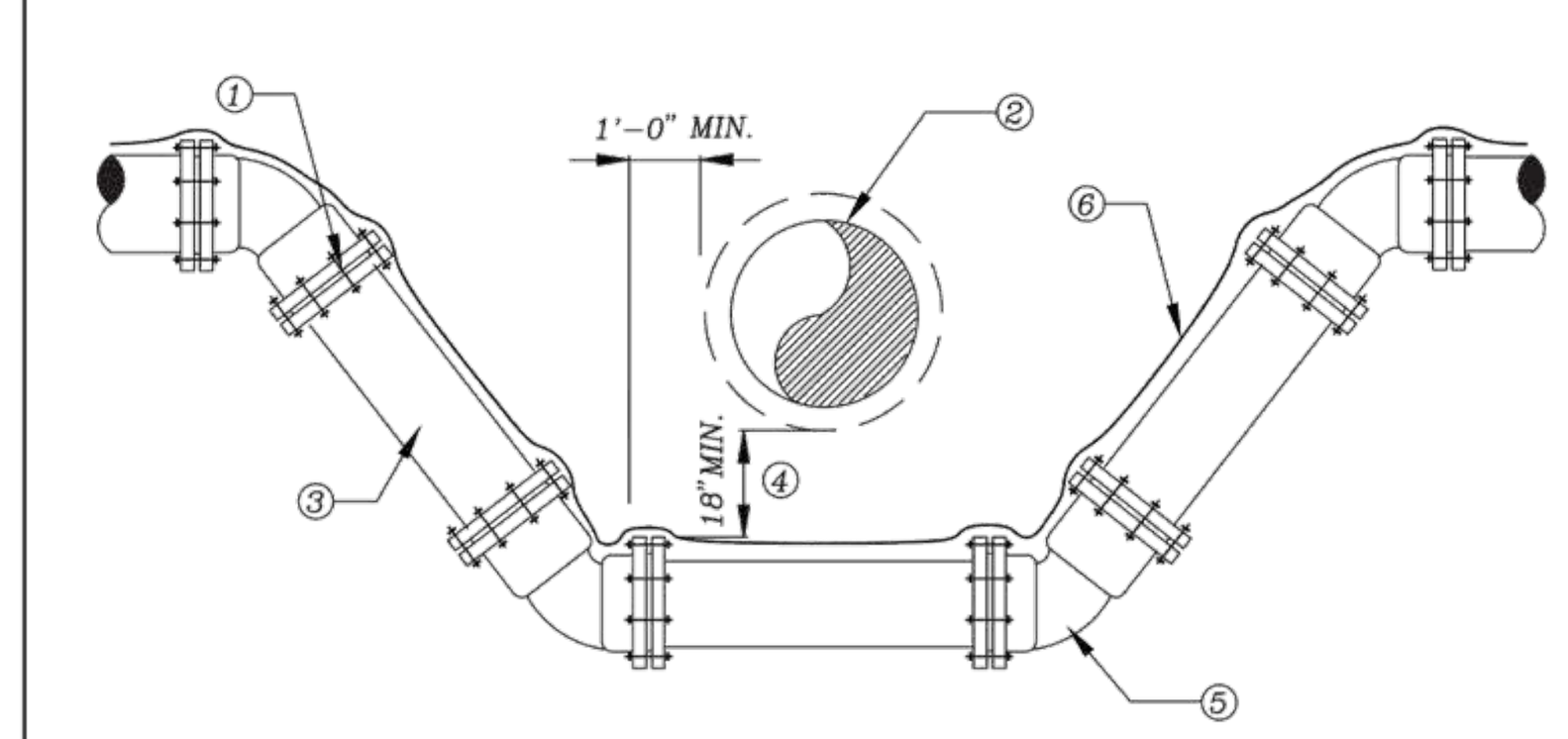
NOTE: IF LESS THAN 10' CLEARANCE ENCASE WATER MAIN IN STEEL CASING

REV: 12/10/2015

STANDARD DETAIL NO. 6.10

WATER PRESSURE LINE VERTICAL OFFSET W/ DUCTILE IRON MJ FITTINGS & PIPE

N.T.S.



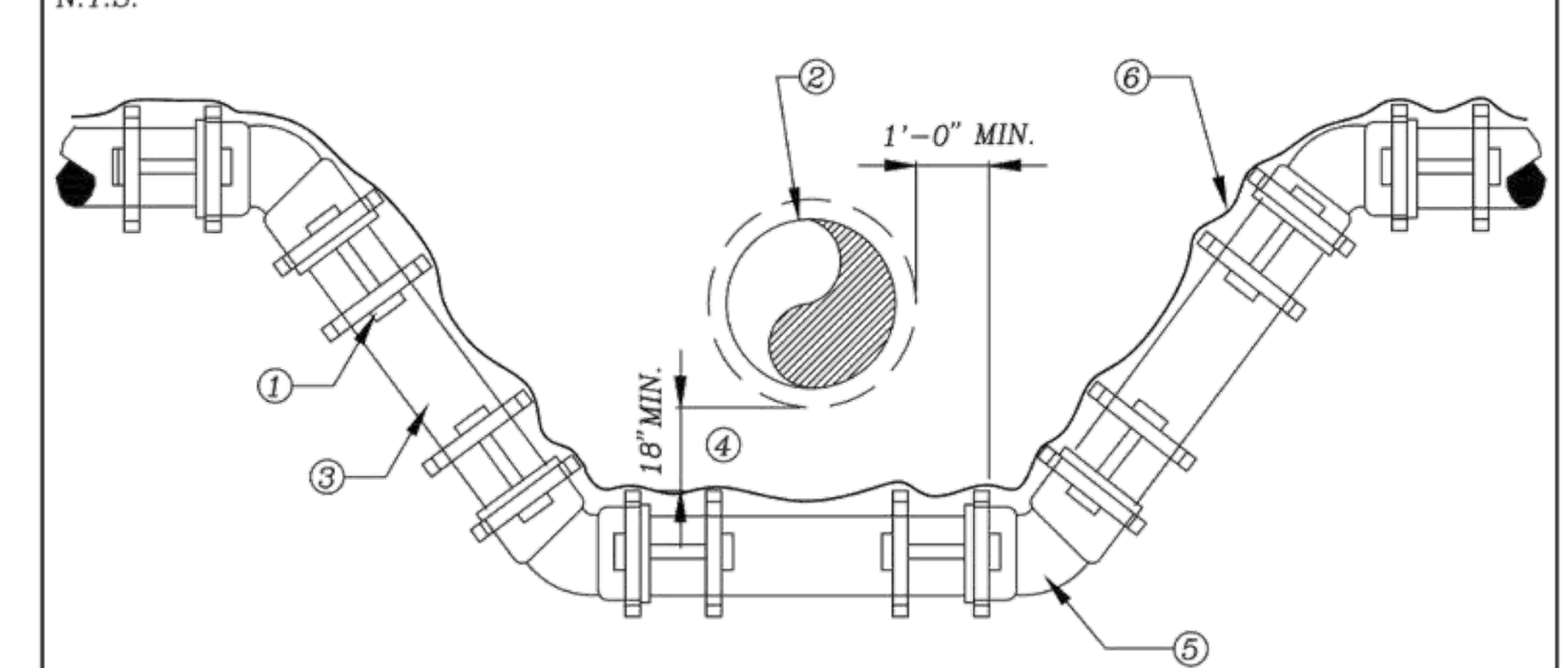
① LCU APPROVED JOINT RESTRAINT FOR D.I., M.J. PIPE AND FITTINGS PLACED IN ACCORDANCE WITH JOINT RESTRAINT SCHEDULE. (SEE DETAIL 6.12)
② CONFLICT PIPE
③ LCU APPROVED DUCTILE IRON PIPING PRESSURE CLASS 350
④ 18" MIN. UNLESS OTHERWISE APPROVED BY LCU
⑤ 45° MECHANICAL JOINT FITTING, TYPICAL
⑥ 12 GAUGE DOUBLE INSULATED COPPER WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.11

WASTE WATER FORCE MAIN VERTICAL OFFSET WITH FITTINGS, (P.V.C.)

N.T.S.



WASTE WATER FORCE MAIN

① LCU APPROVED JOINT RESTRAINT FOR P.V.C., M.J. PIPE AND FITTINGS PLACED IN ACCORDANCE WITH JOINT RESTRAINT SCHEDULE. (SEE DETAIL 6.12)
② CONFLICT PIPE
③ LCU APPROVED C-900 PVC DR 14
④ 18" MIN. UNLESS OTHERWISE APPROVED BY LCU
⑤ 45° MECHANICAL JOINT FITTING, TYPICAL
⑥ 12 GAUGE DOUBLE INSULATED COPPER WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.12

RESTRAINED LENGTH SCHEDULE

N.T.S.

DUCTILE IRON PIPE

PIPE SIZE (Inches)	MINIMUM RESTRAINED PIPE LENGTH (FEET) — HORIZONTAL BENDS 90°	45°	22-1/2°	11-1/4°	DEAD END/VALVE	HORIZONTAL TEE
4	17	7	4	2	29	6
6	23	10	5	2	40	17
8	29	12	6	3	53	29
10	35	14	7	4	63	38
12	41	17	8	4	74	49
16	51	21	11	5	94	68
24	69	29	14	7	131	105
30	81	34	17	8	156	129

PVC PIPE

PIPE SIZE (Inches)	MINIMUM RESTRAINED PIPE LENGTH (FEET) — HORIZONTAL BENDS 90°	45°	22-1/2°	11-1/4°	DEAD END/VALVE	HORIZONTAL TEE
4	20	8	4	2	45	8
6	29	12	6	3	63	25
8	36	15	8	4	83	43
10	44	18	9	5	99	58
12	51	21	11	5	116	74
16	63	26	13	7	149	103
24	87	36	18	9	208	158
30	102	42	21	10	248	194

A COMPLETE JOINT RESTRAINING SCHEDULE FOR ALL ENCOUNTERED VERTICAL & HORIZONTAL BENDS, VERTICAL OFFSETS, TEES, AND DEAD ENDS SHALL BE THE RESPONSIBILITY OF THE DESIGN ENGINEER.

LENGTH FIGURES BASED ON FOLLOWING:

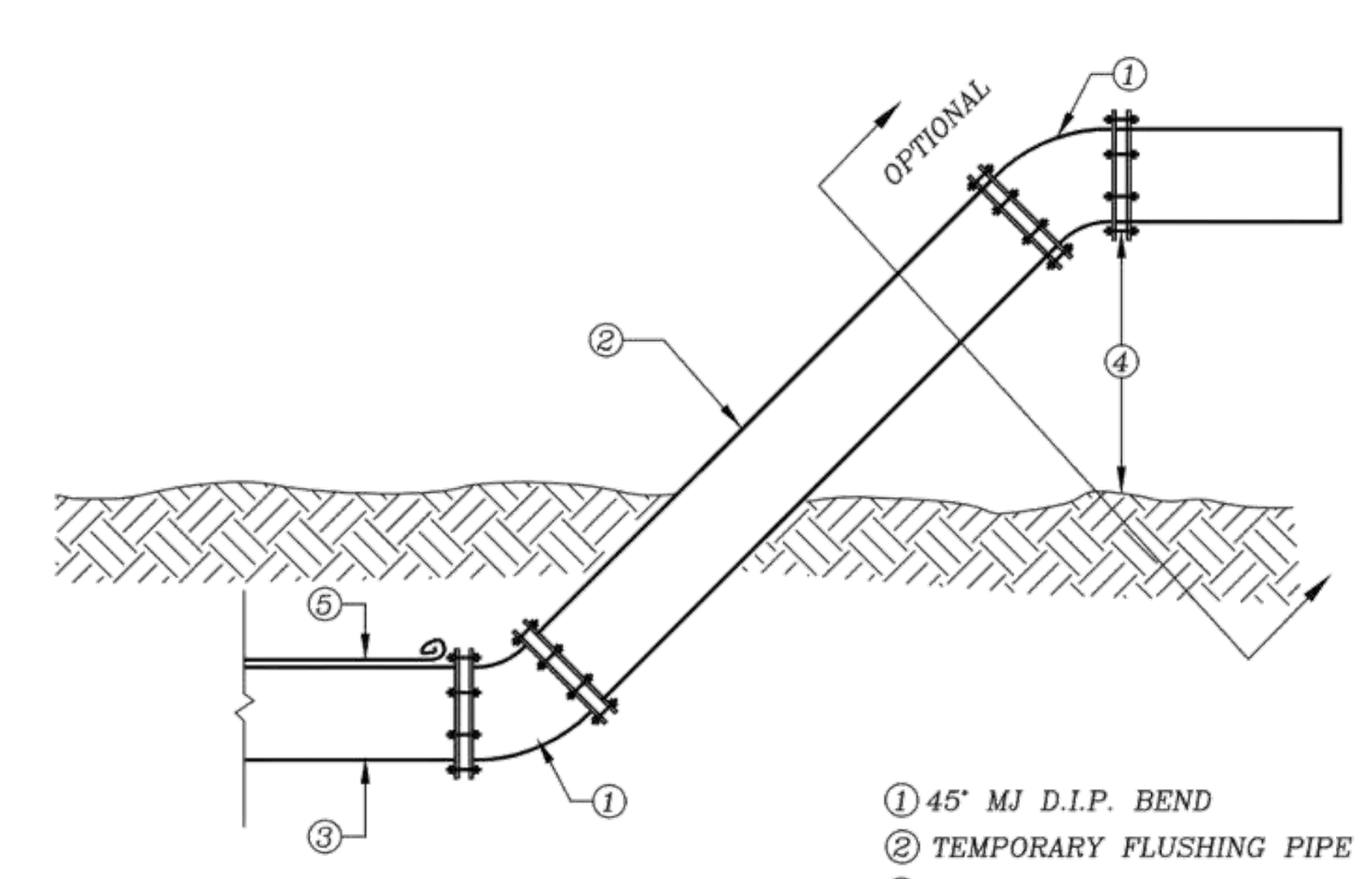
Pressure = 150 psi, FS = 1.5, trench type = 3, 30" cover on Bare pipe, Soil type = GP & SP

REV: 12/22/2022

STANDARD DETAIL NO. 6.13

WATER MAIN FLUSHING DETAIL

N.T.S.



① 45° MJ D.I.P. BEND
② TEMPORARY FLUSHING PIPE
③ END OF WATER MAIN
④ 24" MIN.

NOTES:

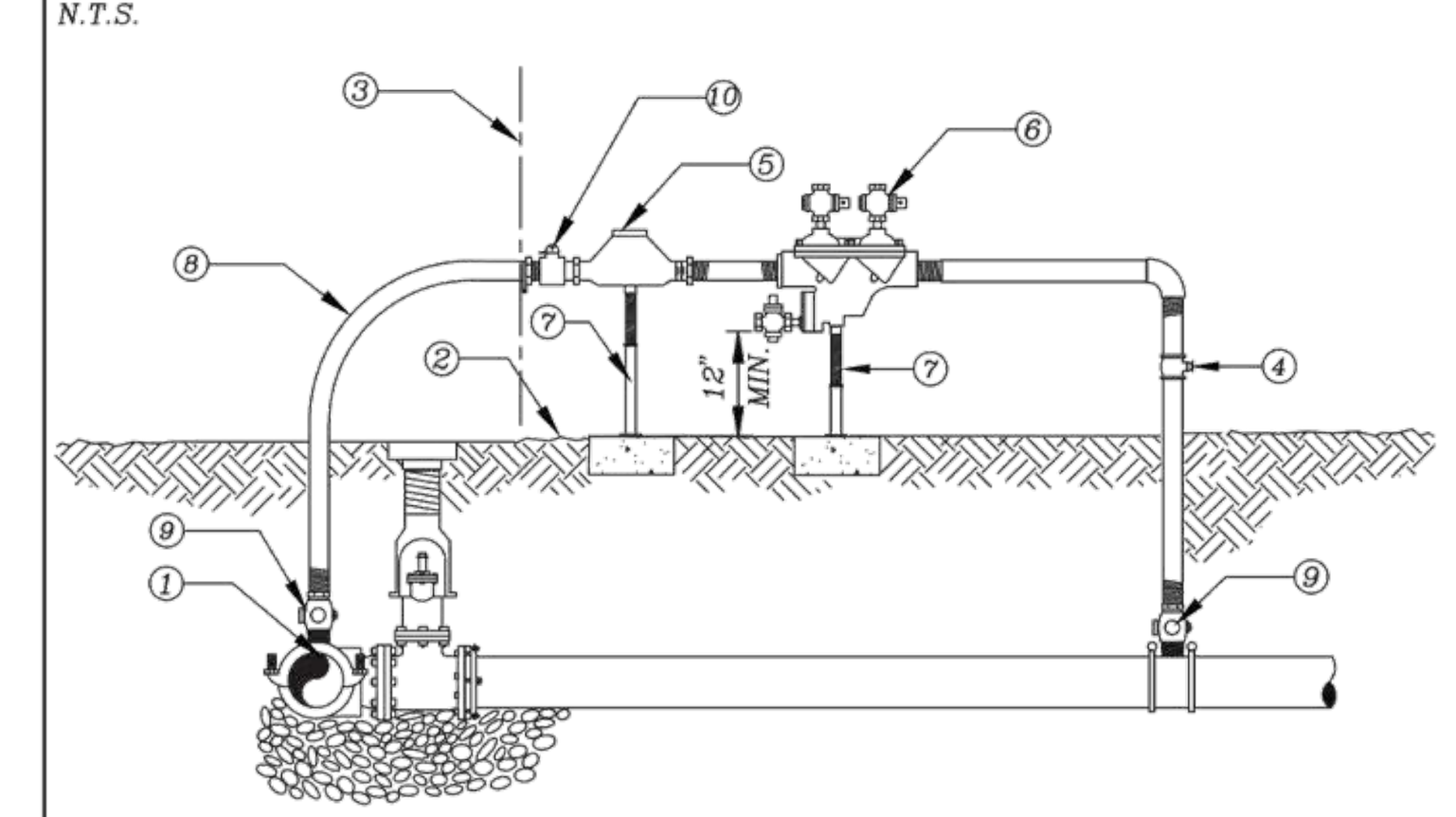
1. FLUSHING DETAIL TO BE USED ON WATER LINES 8" DIAMETER OR GREATER.
2. FLUSHING PIPE SHALL BE THE SAME DIAMETER AS WATER MAIN.
3. CONTRACTOR SHALL PROVIDE ADEQUATE DRAINAGE FOR FLUSHED WATER.
4. PROVIDE M.J. CAP IF ASSEMBLY IS LEFT IN PLACE UNATTENDED.
5. 12 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.15

TEMPORARY CONSTRUCTION JUMPER WITH METER

N.T.S.



NOTES:

CONTRACTOR/CUSTOMER SHALL APPLY AT LEE COUNTY UTILITIES FOR TEMPORARY WATER SERVICE FIVE DAYS PRIOR TO REQUIRED SERVICE DATE.
THE METER ONLY WILL BE FURNISHED BY LEE COUNTY UTILITIES. ALL FITTINGS, PIPING, VALVES AND MATERIALS SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR/CUSTOMER. THE APPROVED BACKFLOW PREVENTION DEVICE SHALL BE TESTED AND CERTIFIED BY LEE COUNTY UTILITIES APPROVED CERTIFIED BACKFLOW PREVENTION TECHNICIAN (FURNISHED BY CONTRACTOR/CUSTOMER) AT TIME OF METER INSTALLATION. CONTRACTOR/CUSTOMER SHALL PROTECT THE ASSEMBLY FROM DAMAGE. TEMPORARY CONSTRUCTION JUMPER/METER ASSEMBLY SHALL NOT BE REMOVED UNTIL SYSTEM IS ACCEPTED AND DEPARTMENT OF HEALTH CERTIFICATION AND CLEARANCE FOR SERVICE FORM HAS BEEN RECEIVED BY LEE COUNTY UTILITIES. WHEN THE NEW SYSTEM IS ACCEPTED, THE TEMPORARY CONSTRUCTION JUMPER METER ASSEMBLY MUST BE COMPLETELY REMOVED DOWN TO CORP STOPS.

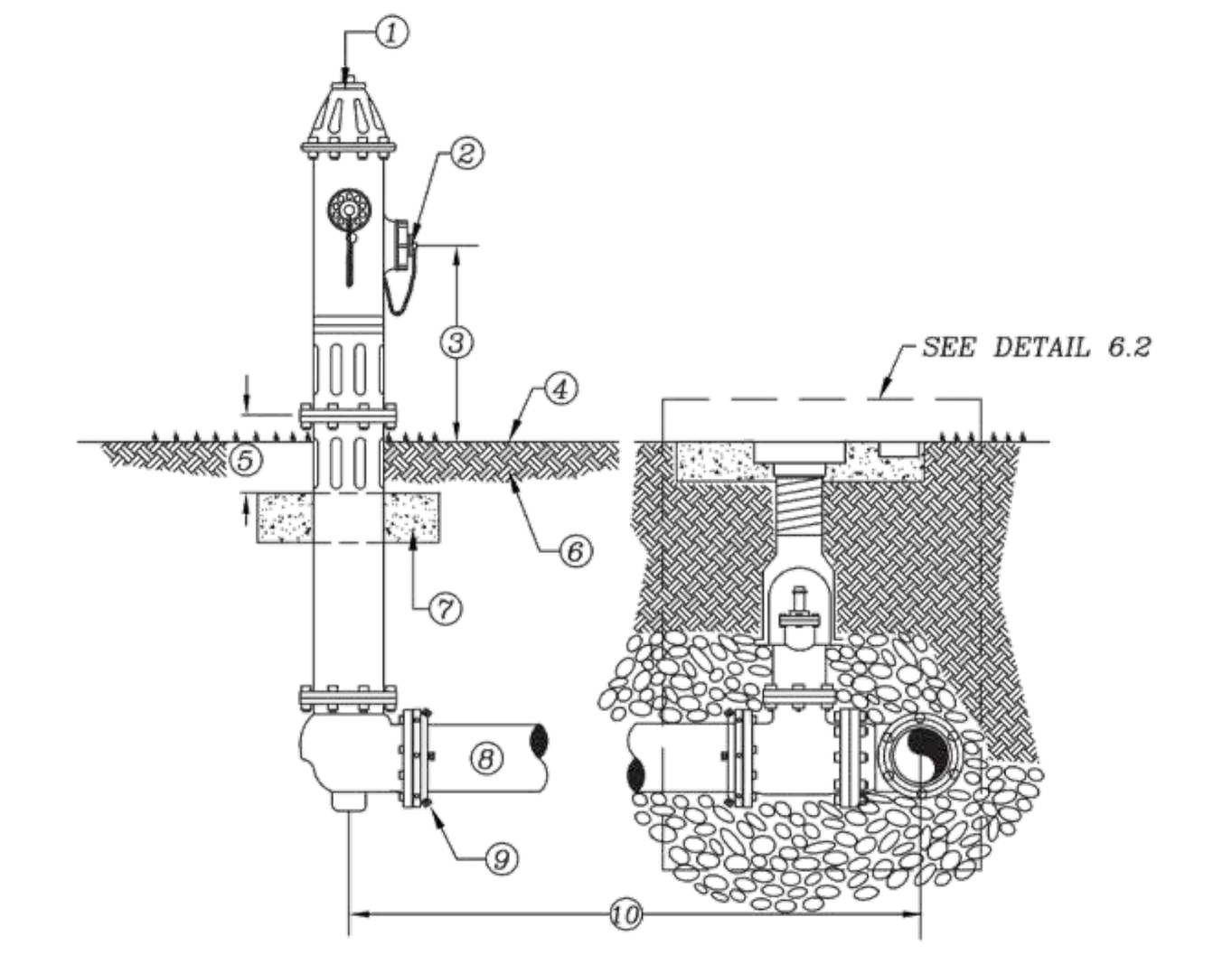
① TAPPING SLEEVE AND VALVE TO BE FULLY RESTRAINED ON EXISTING MAIN (SEE LCU STANDARD DETAIL)
② GRADE
③ PROPERTY LINE AND/OR RIGHT OF WAY LINE
④ BRONZE BALL VALVE
⑤ METER FURNISHED BY LEE COUNTY UTILITIES
⑥ REDUCED PRESSURE BACKFLOW PREVENTER
⑦ PIPE STAND (SEE LCU STANDARD DETAIL)
⑧ 2" DIA. HDPE DR9
⑨ 2" DIA. CORPORATION STOP
⑩ 2" DIA. CURB STOP

REV: 12/10/2015

STANDARD DETAIL NO. 6.24

FIRE HYDRANT ASSEMBLY

N.T.S.



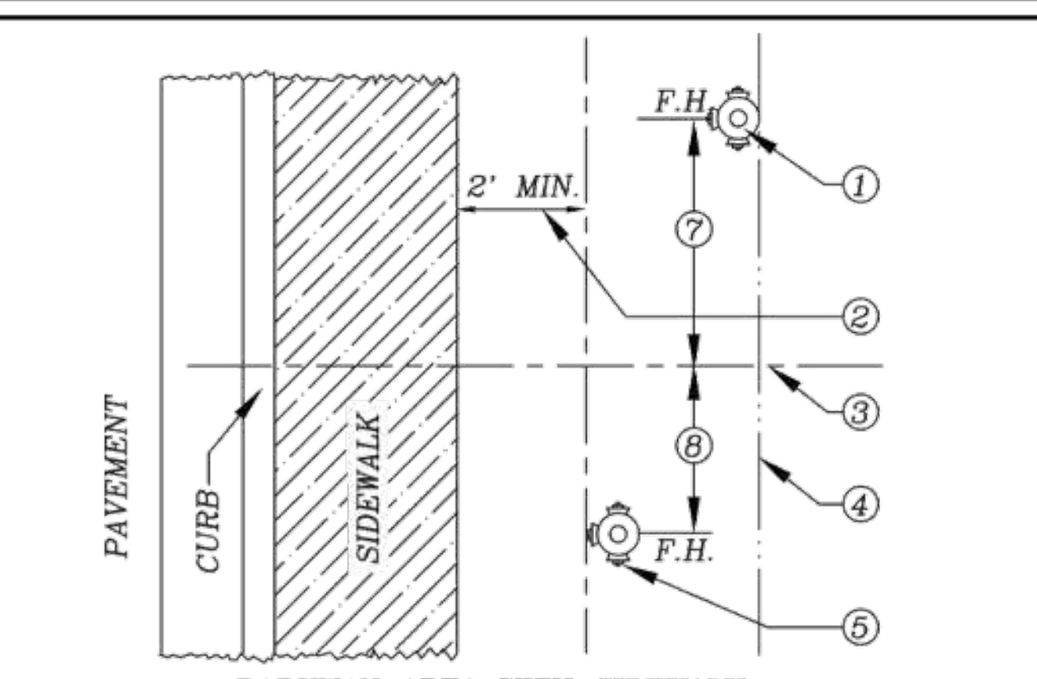
① FIRE HYDRANT: STANDARD A.W.W.A. SAFETY YELLOW (SEE APPROVED MATERIAL LIST)
② PUMPER CONNECTION FACING AS PER FIRE DISTRICT RECOMMENDATION
③ 18" MIN. – 24" MAX.
④ FINISHED GRADE
⑤ 6" MIN. CLEARANCE
⑥ COMPACTED FILL
⑦ 30" X 30" X 4" CONCRETE BREAK AWAY CONCRETE PAD, 3000 P.S.I.
⑧ PIPE TO BE D.I.P., PRESSURE CLASS 350
⑨ RESTRAINED JOINTS, FROM HYDRANT TO WATER MAIN
⑩ VARIABLE DISTANCE, IF GREATER THAN 100' A VALVE SHALL BE INSTALLED AT HYDRANT BASE

REV: 12/10/2015

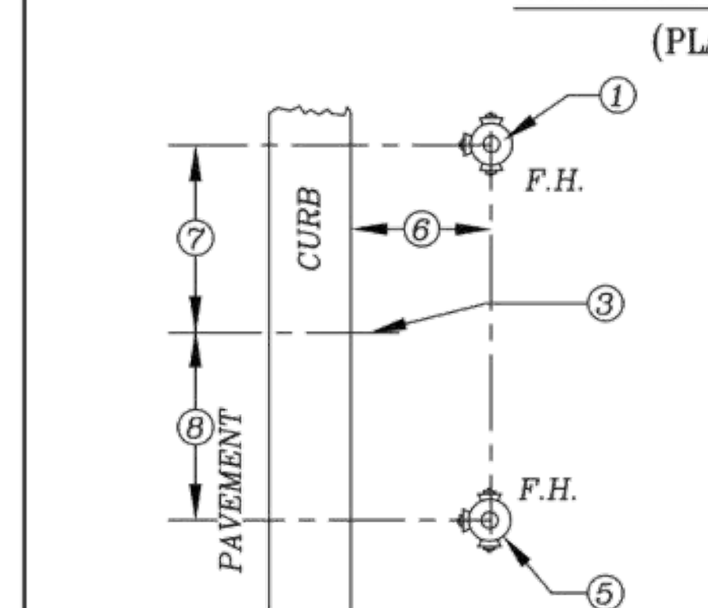
STANDARD DETAIL NO. 6.25

FIRE HYDRANT LOCATIONS/CLEARANCE

N.T.S.

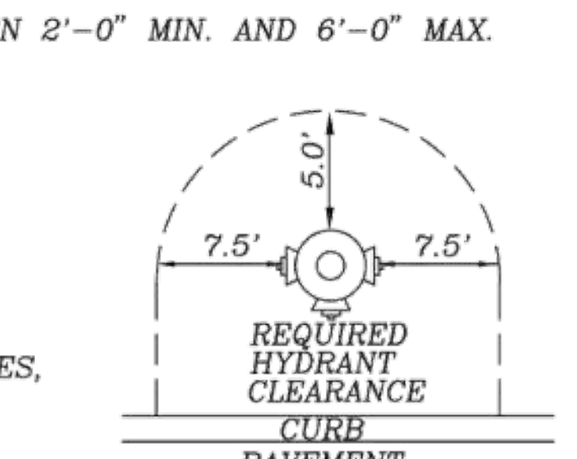


PARKWAY AREA WITH SIDEWALK
(PLAN VIEW)



PARKWAY AREA OR NO SIDEWALK
(PLAN VIEW)

① PREFERRED LOCATION
② F.H. TO BE LOCATED 2' MINIMUM FROM EDGE OF SIDEWALK
③ P.T. OR P.C. OF CURB RETURN
④ PROPERTY LINE RIGHT-OF-WAY OR LIMIT OF EASEMENT
⑤ ACCEPTABLE LOCATION WITH CURB RADIUS OF 20' OR MORE
⑥ BETWEEN 2'-0" MIN. AND 6'-0" MAX.
⑦ 6'-0"
⑧ 4'-0"



(PLAN VIEW)

NOTES

1. OBSTRUCTIONS SUCH AS UTILITY POLES, STREET SIGNS, IRRIGATION BOXES, FENCES, ETC. SHALL NOT BE PLACED BETWEEN CURB AND HYDRANT.
2. SOME LOCATIONS APPLY AT EITHER END OF CURB RETURNS.
3. DIMENSION SHOWN ON CONSTRUCTION DRAWINGS SUPERCEDE LOCATIONS SHOWN HERE.

REV: 12/10/2015

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

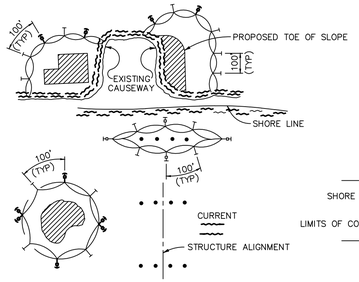
NO.	DATE	REVISION DESCRIPTION	BY

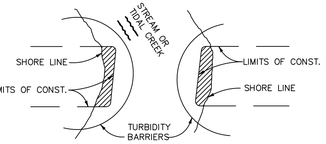
BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM
DAVID R. UNDERHILL JR.
P.E. #47029

UTILITY DETAILS
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	UTL-DT	JLW	JLW	DRU	NTS	11

S:\JOBS\31XX\3142L\ENGINEERING\DEVPLANS\BETH STACEY BLVD. L.D.O. PLANS\3142L-BLVD-11-UTL-DT.DWG 9/19/2025 9:58 AM JIMMY WHEWAY





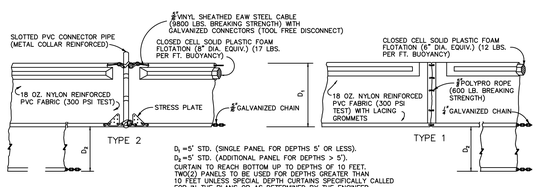
LEGEND

- PILE LOCATIONS
- DREDGE OR FILL AREA
- MOORING BUOY W/ANCHOR
- ANCHOR
- BARRIER MOVEMENT DUE TO CURRENT ACTION

NOTES:
1. TURBIDITY BARRIERS ARE TO BE USED IN ALL PERMANENT BODIES OF WATER REGARDLESS OF WATER DEPTH.
2. NUMBER AND SPACING OF ANCHORS DEPENDENT ON CURRENT VELOCITIES.
3. DEPLOYMENT OF BARRIER AROUND PILE LOCATIONS MAY VARY TO ACCOMMODATE CONSTRUCTION OPERATIONS.
4. NAVIGATION MAY REQUIRE SEGMENTING BARRIER DURING CONSTRUCTION OPERATIONS.
5. FOR ADDITIONAL INFORMATION SEE SECTION 104 OF THE STANDARD SPECIFICATIONS.

NOTE:
TURBIDITY BARRIERS FOR FLOWING STREAMS AND TIDAL CREEKS MAY BE EITHER FLOATING, OR STAKED TYPES OR ANY COMBINATIONS OF TYPES THAT WILL SUIT SITE CONDITIONS AND MEET EROSION CONTROL AND WATER QUALITY REQUIREMENTS. THE BARRIER TYPE(S) WILL BE AT THE CONTRACTORS OPTION UNLESS OTHERWISE SPECIFIED IN THE PLANS, HOWEVER PAYMENT WILL BE UNDER THE PAY ITEM(S) ESTABLISHED IN THE STAKED TURBIDITY BARRIER. POSTS IN STAKED PLANS FOR FLOATING TURBIDITY BARRIER AND/OR TURBIDITY BARRIERS TO BE INSTALLED IN VERTICAL POSITION UNLESS OTHERWISE DIRECTED BY THE ENGINEER.

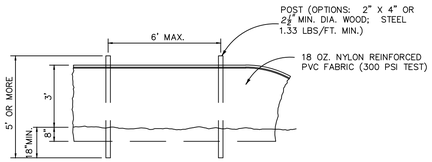
TURBIDITY BARRIER APPLICATION DETAIL
N.T.S.



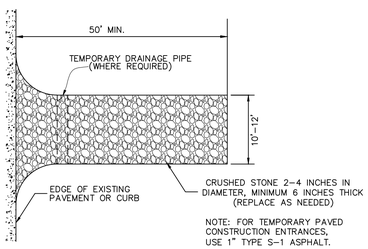
D_1 = 5' STD. (SINGLE PANEL FOR DEPTHS 5' OR LESS).
D_2 = 5' STD. (ADDITIONAL PANEL FOR DEPTHS > 5').
CURTAIN TO REACH BOTTOM UP TO DEPTHS OF 10 FEET, TWO(2) PANELS TO BE USED FOR DEPTHS GREATER THAN 10 FEET UNLESS SPECIAL DEPTH CURTAINS SPECIFICALLY CALLED FOR IN THE PLANS OR AS DETERMINED BY THE ENGINEER.

NOTICE: COMPONENTS OF TYPES ~ AND ~~ MAY BE SIMILAR OR IDENTICAL TO PROPRIETARY DESIGNS. ANY INFRINGEMENT ON THE PROPRIETARY RIGHTS OF THE DESIGNER SHALL BE THE SOLE RESPONSIBILITY OF THE USER. SUBSTITUTIONS FOR TYPES ~ AND ~~ SHALL BE AS APPROVED BY THE ENGINEER.

FLOATING TURBIDITY BARRIER DETAIL
N.T.S.

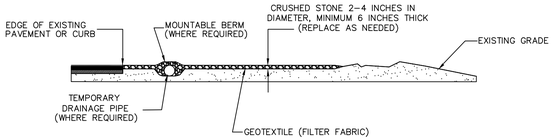


STAKED TURBIDITY BARRIER DETAIL
N.T.S.



NOTE: FOR TEMPORARY PAVED CONSTRUCTION ENTRANCES, USE 1" TYPE S-1 ASPHALT.

STABALIZED CONSTRUCTION ENTRANCE/EXIT PLAN VIEW
N.T.S.



STABALIZED CONSTRUCTION ENTRANCE/EXIT SECTION VIEW
N.T.S.

GENERAL NOTES

1. EROSION CONTROL DETAILS ARE APPLICABLE TO NEW CONSTRUCTION, RECONSTRUCTION AND RRR PROJECTS.
2. FOR SODDING ADJACENT TO DITCHES AND AT HEADWALLS, SEE INDEX NO. 281.
3. ALL FRONT SLOPES STEEPER THAN 3:1 ARE TO BE SODDED.



BETH STACY BLVD – 407+00.00

LOOKING SOUTH

BETH STACY BLVD – 410+50.00

LOOKING SOUTH

BETH STACY BLVD – 412+50.00

LOOKING SOUTH

BETH STACY BLVD – 407+50.00

LOOKING SOUTH

BETH STACY BLVD – 410+00.00

LOOKING SOUTH

BETH STACY BLVD – 412+00.00

LOOKING SOUTH

BETH STACY BLVD – 408+00.00

LOOKING SOUTH

BETH STACY BLVD – 409+00.00

LOOKING SOUTH

BETH STACY BLVD – 411+50.00

LOOKING SOUTH

BETH STACY BLVD – 408+50.00

LOOKING SOUTH

BETH STACY BLVD – 409+50.00

LOOKING SOUTH

BETH STACY BLVD – 411+00.00

LOOKING SOUTH

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BETH STACEY BLVD
CROSS SECTIONS (1)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's **below.** **Call** before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE AS TO THE ACCURACY OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND UNDERGROUND LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

DAVID R. UNDERHILL JR.

P.E. # 47029

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DU

PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
3142L	07/03/2025	N.A.V 1988

BOOK AND PAGE:

SHEET

13

\\VLFORT\Projects\Jobs\3142L\3142L\Engineering\DevPlans\3.0 Phase 2 Plans\3142-3.0-PH2-BETH STACEY BLVD SECTIONS.dwg, 9/9/2025 10:57:34 AM, NUNOTO (AURICULA, ATWELL, LLC

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BETH STACEY BLVD. CROSS SECTIONS (2)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's **below.** **Call** before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE AS TO THE ACCURACY OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL UTILITY LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

DAVID R. UNDERHILL JR.

P.E. # 47029

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DU

PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
3142L	07/03/2025	N.A.V 1988

BOOK AND PAGE:

SHEET

14

\\ATWORKS\Projects\Jobs\3142\3142L\Engineering\DwgFiles\3.0. Phase 2 Plans\3142-3.0.-PH2-BETH STACEY BLVD SECTIONS.dwg, 8/8/2025 10:57:37 AM, NUNCIO LAURICELLA, ATWELL, LLC

BETH STACY BLVD – 419+00.00

LOOKING SOUTH

BETH STACY BLVD – 421+00.00

LOOKING SOUTH

BETH STACY BLVD – 423+00.00

LOOKING SOUTH

BETH STACY BLVD – 419+50.00

LOOKING SOUTH

BETH STACY BLVD – 421+50.00

LOOKING SOUTH

BETH STACY BLVD – 423+50.00

LOOKING SOUTH

BETH STACY BLVD – 420+00.00

LOOKING SOUTH

BETH STACY BLVD – 422+00.00

LOOKING SOUTH

BETH STACY BLVD – 424+00.00

LOOKING SOUTH

BETH STACY BLVD – 420+50.00

LOOKING SOUTH

BETH STACY BLVD – 422+50.00

LOOKING SOUTH

BETH STACY BLVD – 424+50.00

LOOKING SOUTH

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BETH STACEY BLVD. CROSS SECTIONS (3)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's **below.** **Call** before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE AS TO THE ACCURACY OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL UTILITY LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

DAVID R. UNDERHILL JR.

P.E. # 47029

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DU

PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
3142L	07/03/2025	N.A.V 1988

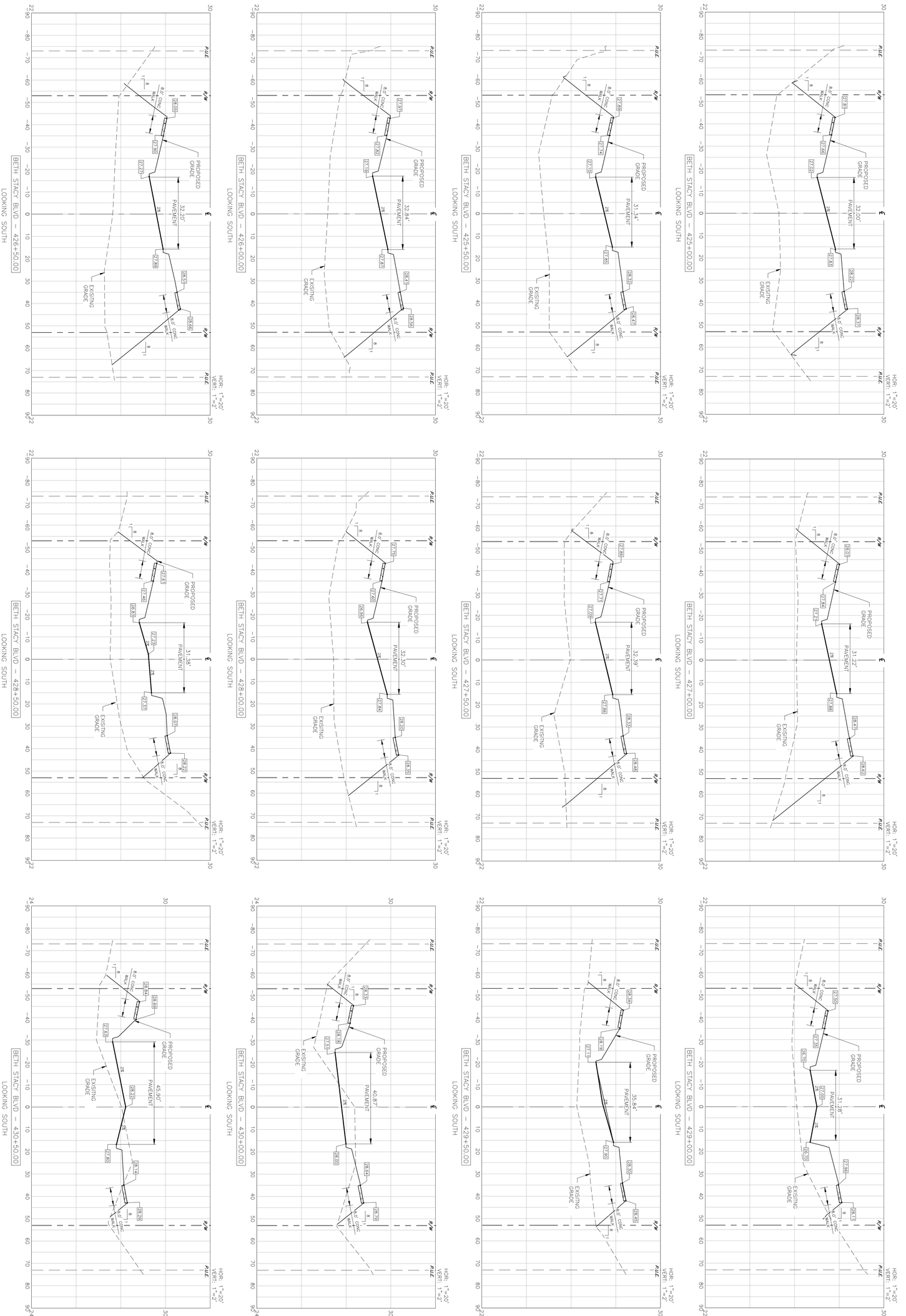
BOOK AND PAGE:

SHEET

15

\\ATWORKS\Projects\Lee\3142L\Engineering\Dwr\Plans\3142L - Phase 2 Plans\3142L-2-D-PH2-BETH STACEY BLVD SECTIONS.dwg, 8/8/2025 10:57:40 AM, NUNOTO LAURICELA, ATWELL, LLC

\\FLFOR2FS1\Projects\Jobs\31xx\3142\Engineering\DevPhase\D.O. Phase 2 Plans\3142-D.O.-PH2-BETH STACEY BLVD SECTIONS.dwg, 9/9/2025 10:57:45 AM, NUNOTO LAURICELLA, ATWELL, LLC



811
Know what's below.
Call before you dig.
ALL UTILITIES ARE SHOWN IN APPROXIMATE LOCATIONS. THE CONTRACTOR SHALL FIELD VERIFY THE LOCATIONS OF ALL UTILITIES PRIOR TO CONSTRUCTION AND SHALL PRESERVE AND PROTECT ALL UTILITIES FROM DAMAGE DURING CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

DAVID A. LINDEMUTH, JR.
P.E. # 67929

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DD
PROJECT NUMBER: 3142	COMPLETION DATE: 07/30/2025	SURVEY DATUM: NAD 83 1990
BOOK AND PAGE:		

SHEET 16

NO.	DATE	REVISION DESCRIPTION	BY

BETH STACEY BLVD.
CROSS SECTIONS (4)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BETH STACEY BLVD. CROSS SECTIONS (5)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE ARE FACTUAL AND CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

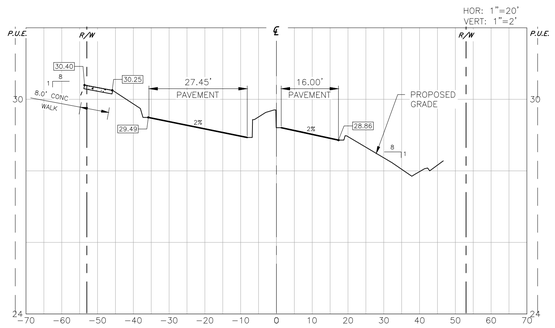
NOT RELEASED FOR CONSTRUCTION

DAVID R. UNDERHILL JR.
P.E. # 47029

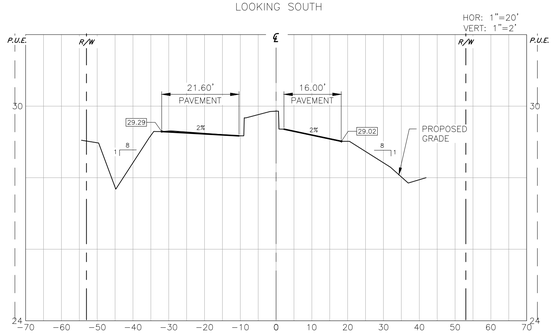
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DU
PROJECT NUMBER: 3142L	COMPLETION DATE: 07/03/2025	SURVEY DATUM: N.A.V 1988
BOOK AND PAGE:		

SHEET
17

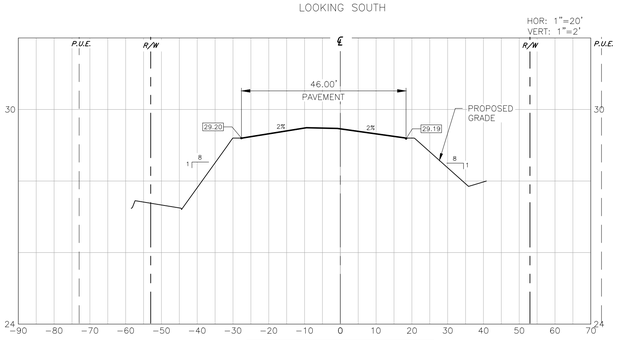
\\ATWORKS\Projects\Lakes\3142L\Engineering\DevPlans\D.O. Phase 2 Plans\3142-D.O.-PH2-BETH STACEY BLVD SECTIONS.dwg, 8/8/2025 10:07:46 AM, NUNO70 LAURIOLLA, ATWELL, LLC



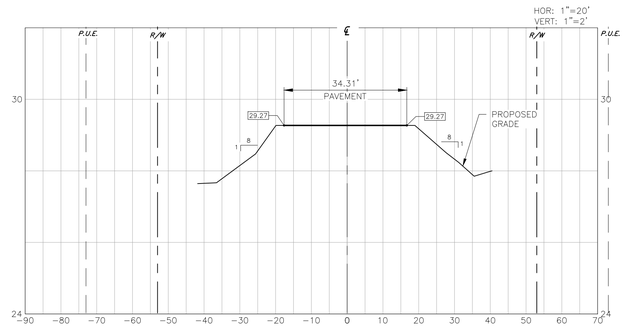
BETH STACY BLVD – 437+00.00
LOOKING SOUTH



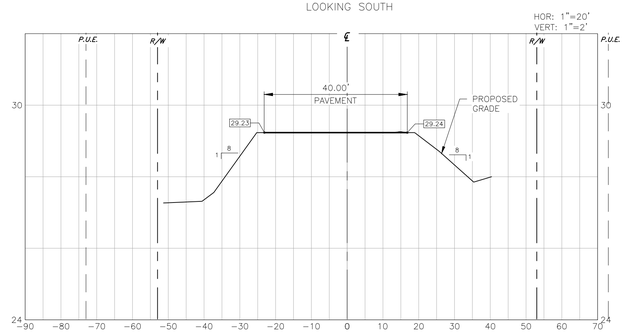
BETH STACY BLVD – 437+50.00
LOOKING SOUTH



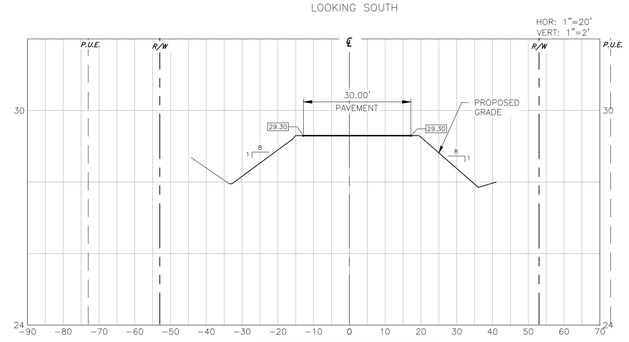
BETH STACY BLVD – 438+00.00
LOOKING SOUTH



BETH STACY BLVD – 439+00.00
LOOKING SOUTH



BETH STACY BLVD – 438+50.00
LOOKING SOUTH



BETH STACY BLVD – 439+40.56
LOOKING SOUTH

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BETH STACEY BLVD.
CROSS SECTIONS (6)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR: AQUABELLA DEVELOPMENT, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

DAVID R. UNDERHILL JR.
P.E. # 47029

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DU
PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
3142L	07/03/2025	N.A.V 1988
BOOK AND PAGE:		

SHEET
18

IBIS LANDING

BETH STACEY BOULEVARD L.D.O. PLANS

SECTION 06, TOWNSHIP 45 SOUTH, RANGE 27 EAST
LEE COUNTY, FLORIDA



GENERAL LOCATION MAP
NOT TO SCALE

UTILITY OWNER CONTACTS

WATER & WASTEWATER:
FLORIDA GOVERNMENTAL UTILITY AUTHORITY
c/o ACCENTURE, 9400 SOUTHPARK CENTER LOOP, SUITE 400
ORLANDO, FL 32819
PHONE (407) 629-6900

ELECTRIC POWER SERVICE:
LEE COUNTY ELECTRIC CO-OP
P.O. BOX 3455
NORTH FORT MYERS, FLORIDA 33918
PHONE: (239) 995-2121
FAX: (239) 995-7904

TELEPHONE SERVICE:
SPRINT
2820 CARGO STREET BUILDING A
FORT MYERS, FLORIDA 33916
PHONE: (239) 336-2003
FAX: (239) 336-2026

PROJECT CONTACTS

ENGINEER:
DAVID R. UNDERHILL, P.E. PROJECT MANAGER
BANKS ENGINEERING
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

SURVEYOR:
BANKS ENGINEERING
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490

DEVELOPER:
AQUABELLA DEVELOPMENT GROUP, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 278-1199

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199 FAX: (239) 278-3174



SPECIFIC LOCATION MAP
NOT TO SCALE

INDEX OF SHEETS

SHEET	TITLE
1	COVER SHEET
2	EXISTING CONDITIONS & AERIAL PHOTOGRAPH
3	MASTER SITE PLAN
4	MASTER DRAINAGE PLAN
5–8	PLAN & PROFILES
9	TYPICAL CROSS SECTIONS
10	PAVING, GRADING & DRAINAGE DETAILS
11	UTILITY DETAILS
12	BEST MANAGEMENT PRACTICE PLAN
13–18	CROSS SECTIONS

BANKS
ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

NO.	DATE	REVISION DESCRIPTION	BY
5	1-28-26	REVISED PER LAMSID COMMENTS	JLW
4	12-8-25	REVISED PER COUNTY COMMENTS	JLW
3	10-16-25	REVISED PER COUNTY COMMENTS	JLW
2	9-15-25	REVISED PER COUNTY COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

S:\JOBS\2024\2405\DWGS\ENGINEERING\DEVPLANS\BETH STACEY BLVD. L.D.O. PLANS\2405-BLVD-01-COVER.DWG 1/29/2026 1:23 PM JIMMY MCKAY
IBIS LANDING



N

0 250' 500' 750'
GRAPHIC SCALE 1"= 250'

IBIS LANDING

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

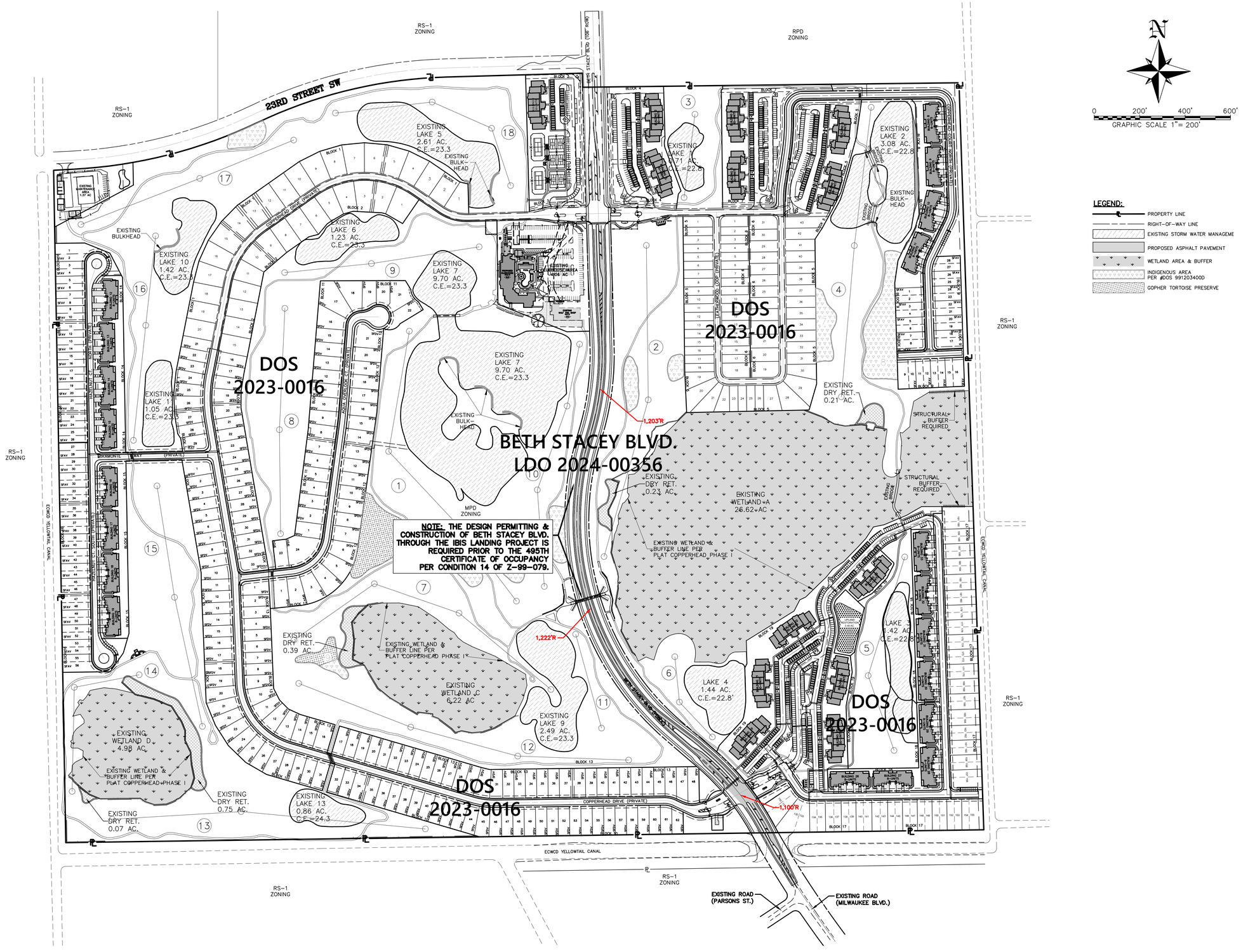
BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

EXISTING CONDITIONS & AERIAL PHOTOGRAPH
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	EX. CON-ARL	JLW	JLW	DRU	1"=250'	02



N

0 200' 400' 600'

GRAPHIC SCALE 1"= 200'

LEGEND:

- PROPERTY LINE
- RIGHT-OF-WAY LINE
- EXISTING STORM WATER MANAGEME
- PROPOSED ASPHALT PAVEMENT
- WETLAND AREA & BUFFER
- INDIGENOUS AREA PER #DOS 991203400D
- GOPHER TORTOISE PRESERVE

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
1	12-8-25	REVISED TURNLANE ON BETH STACEY BLVD	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

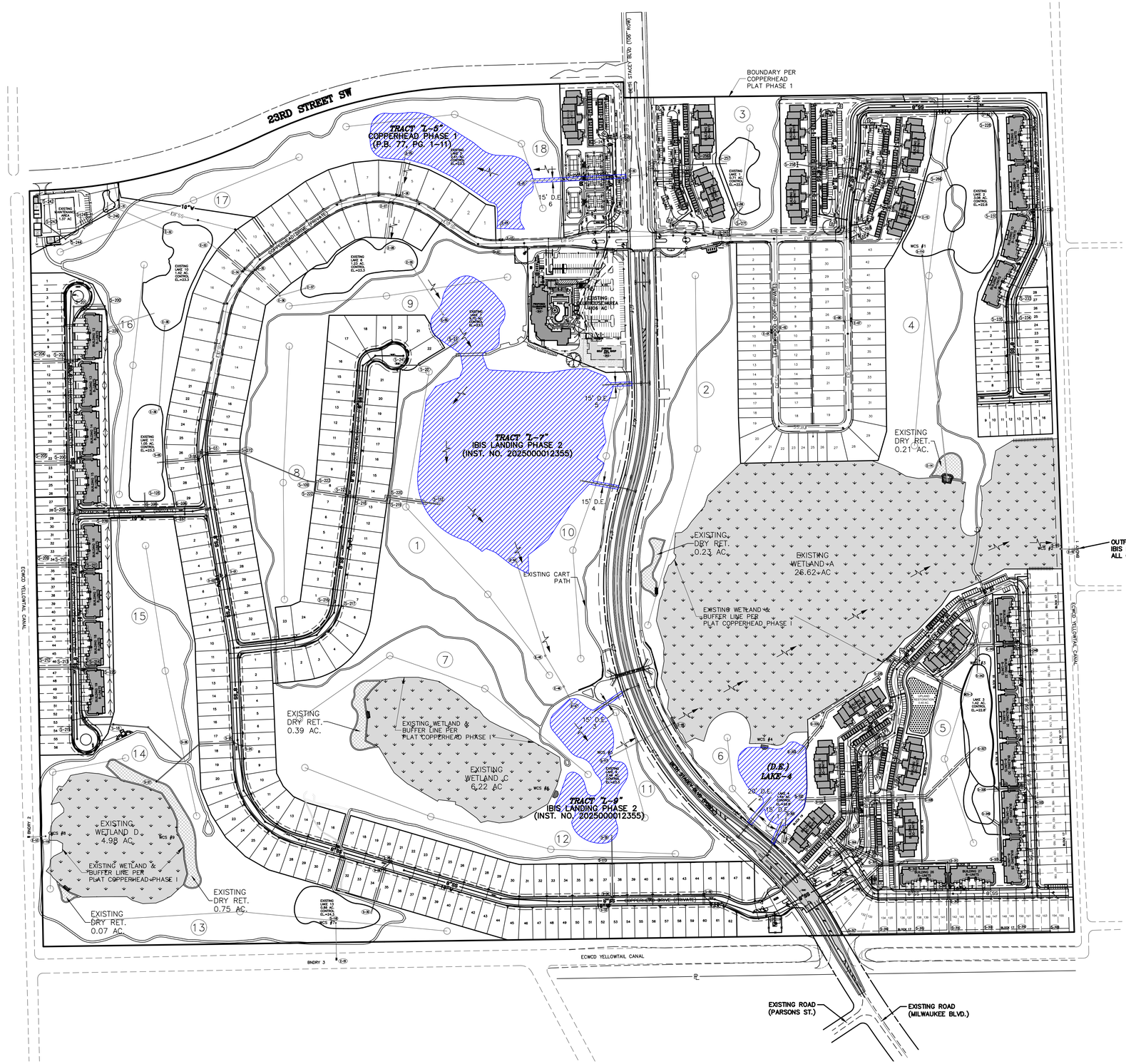
MASTER SITE PLAN

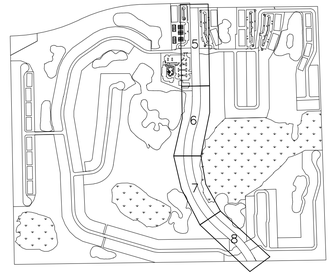
IBIS LANDING - BETH STACEY BLVD.

LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	MSP	JLW	JLW	DRU	1"=200'	03

S:\JOBS\3XXX\3142L\ENGINEERING\DEVELOPMENT\BETH STACEY BLVD. LDO PLANS\3142L-BLVD-03-MSP.DWG 1/29/2026 1:34 PM JIMMY MCKAY





PROFILES KEY MAP



STORMWATER MANAGEMENT SYSTEM
MINIMUM ELEVATIONS (NAVD)

Basin	WCS #	Control Elev.	5 Yr. Flood Stage	10 Yr. Flood Stage	Min. Flood Road	25 Yr. Flood Stage	Min. Flood Berm	100 Yr. Flood Stage	Min. Floor Elev.
B1	WCS 1	22.80	24.43	24.60	25.30	25.84	26.20*	26.24	26.80
B2	WCS 2	22.80	23.48	23.66	25.30	24.74	25.25	26.22	26.80
B3	WCS 3	22.80	24.23	24.41	25.30	25.61	26.15	26.22	26.80
B4	WCS 4	22.80	24.44	24.77	25.30	25.48	26.00	26.22	26.80
B5	WCS 5	23.30	24.34	24.67	25.30	26.12	26.65	26.74	27.10
B6	WCS 6	22.80	24.19	24.43	25.30	25.55	N/A	26.37	26.80
B7	WCS 7	24.30	25.93	26.24	26.30	27.32	27.85	28.07	28.10
B8	WCS 8	24.30	24.97	25.16	26.30	26.32	26.85	27.19	27.20
B9	WCS 9	24.30	24.97	25.16	26.30	26.32	26.85	27.19	27.20
B10	WCS 10	22.30	24.50	24.66	25.80	25.52	26.40	26.99	N/A

* EXISTING PERIMETER BERM WILL PROVIDE 4" OF FREEBOARD ALONG NORTH PROPERTY LINE. MINIMUM BERM HEIGHT IS 26.4' ALONG LA-MSID CANALS.

PLEASE NOTE: B2, B7, & B10 WILL HAVE BERM IMPROVEMENTS TO REACH REQUIRED 6" FREEBOARD.

5 YEAR 24 HR RAINFALL IS 5.20"
10 YEAR 24 HR TOTAL RAINFALL IS 6.00"
25 YEAR 72 HR TOTAL RAINFALL IS 10.20"
100 YEAR 72 HR TOTAL RAINFALL IS 14.20"

DATUM NOTE:

THIS TABLE IS ON THE NORTH AMERICAN VERTICAL DATUM OF 1988. THE DATUM CONVERSION FROM NGVD 29 TO NAVD 88 IS -1.20.

EXAMPLE: 24.00' (NGVD 29) - 1.20' = 22.80' (NAVD 88)

LEGEND:

- PROPOSED STORM PIPE
- PROPOSED DRAINAGE STRUCTURE
- (S-1) PROPOSED DRAINAGE STRUCTURE w/IDENTIFICATION (SEE DETAIL SHEET)
- DRAINAGE FLOW ARROW
- A A TYPICAL CROSS SECTION
- GATE VALVE
- FIRE HYDRANT ASSEMBLY
- 90° BEND
- 45° BEND
- 22.5° BEND
- 11.25° BEND
- TEE
- 6"F 6" FORCE MAIN
- 8"F 8" FORCEMAIN
- E6"F EXISTING 6" FORCE MAIN
- E8"F EXISTING 8" FORCE MAIN
- 6"W 6" POTABLE WATER LINE
- 8"W 8" POTABLE WATER LINE
- 10"W 10" POTABLE WATER LINE
- 12"W 12" POTABLE WATER LINE
- E6"W EXISTING 6" POTABLE WATER LINE
- E8"W EXISTING 8" POTABLE WATER LINE
- E10"W EXISTING 10" POTABLE WATER LINE
- E12"W EXISTING 12" POTABLE WATER LINE
- +20.0 EXISTING GRADE
- (25.35) PROPOSED GRADE
- PROPOSED LEE COUNTY DRAINAGE EASEMENT

CONSTRUCTION NOTES:

1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH FLORIDA DEPT. OF TRANSPORTATION (F.D.O.T.) STANDARD SPECIFICATIONS FOR ROAD & BRIDGE CONSTRUCTION, LATEST EDITION.
2. CONTRACTOR TO DETERMINE & VERIFY LOCATION & ELEVATION OF ANY BURIED UTILITY LINES OR CABLES. CONTRACTOR IS RESPONSIBLE FOR THE REPAIR &/OR REPLACEMENT OF ANY UTILITY LINES OR CABLES DISTRIBUTED BY CONSTRUCTION OPERATIONS.
3. SUBGRADE & LIMEROCK BASE COURSES TO BE COMPACTED TO 98% MIN. OF THE MAX. DRY DENSITY AS DETERMINED BY AASHTO T-180 TESTING.
4. THE CONTRACTOR SHALL SUPPLY SATISFACTORY DENSITY TESTS OF SUBGRADE & LIMEROCK BASE MATERIALS TO THE ENGINEER PRIOR TO PAYMENT &/OR FINAL ACCEPTANCE. (ONE TEST PER COURSE PER 400 SQUARE YARDS).
5. CONTRACTOR SHALL PROVIDE & MAINTAIN TRAFFIC CONTROL IN ACCORDANCE WITH F.D.O.T. STANDARD SPECIFICATIONS & INDEXES & THE FEDERAL HIGHWAY ADMINISTRATION'S MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD), PART IV.
6. CONTRACTORS SHALL SOD ALL DISTURBED AREAS & OTHERWISE RESTORE ALL AREAS TO EXISTING CONDITION OR BETTER.
7. UPON COMPLETION OF ALL IMPROVEMENTS TO BETH STACEY BLVD. THE INITIAL STRIPPING SHALL BE ACCOMPLISHED BY PAINT. AFTER A 30 DAY CURING PERIOD THE ROADWAY WILL THEN BE RE-STRIPPED IN THERMOPLASTIC ALONG WITH THE PLACEMENT OF ALL REFLECTIVE PAVEMENT MARKERS (RPM'S).
8. ANY DISTURBED AREAS WITHIN THE DISTRICT'S R.O.W. MUST BE RESTORED WITH SOD AND MAINTAINED FULLY ESTABLISHED. SEEDING WILL NOT BE PERMITTED.

SIGNING & PAVEMENT MARKING NOTES:

1. ALL SIGNING & STRIPPING WITHIN THE BETH STACEY BLVD. RIGHT OF WAY IS TO BE THERMOPLASTIC & IN ACCORDANCE WITH F.D.O.T. STANDARD INDEX 17346.
2. ALL REFLECTIVE PAVEMENT MARKERS (R.P.M.'S) SHALL BE IN ACCORDANCE WITH F.D.O.T. INDEX NO. 17352 AND F.D.O.T. STANDARD SPECIFICATIONS SECTION 706. ALL RPM'S SHALL BE CLASS "B" TYPE AND OFFSET 2" FROM SOLID LINES.
3. ALL EXISTING PAVEMENT MARKINGS CONFLICTING WITH NEW SRTIPING SHALL BE IMMEDIATELY REMOVED BY HYDROBLASTING TO ELIMINATE THE PREVIOUS MARKINGS REGARDLESS OF WEATHER AND LIGHT CONDITIONS.
4. ANY EXISTING SIGN TO REMAIN THAT IS DISTURBED DURING CONSTRUCTION OR RELOCATED SHALL BE RESET TO CURRENT STANDARDS FOR HEIGHT, OFFSET, & METHOD OF INSTALLATION. INSTALLATION OF ALL TRAFFIC SIGNS WITHIN THE BETH STACEY BLVD. R.O.W. SHALL COMPLY WITH THE CURRENT LEE COUNTY DESIGN STANDARDS FOR SIGN INSTALLATION.
5. CAUTION SHALL BE EXERCISED WHILE RELOCATING EXISTING SIGNS SO AS TO PREVENT DAMAGE TO THE SIGNS. IF THE SIGNS ARE DAMAGED BEYOND USE, AS DETERMINED BY FDOT, THEY SHALL BE REPLACED BY THE CONTRACTOR AT HIS/HER EXPENSE.
6. POSTED SPEED IS 35 MPH. DESIGN SPEED IS 40 MPH.
7. CONTRACTOR TO SECURE AND COMPLY WITH LEE COUNTY R/W USE PERMIT.
8. RPM SPACING ON DOUBLE YELLOW SOLID LINES SHALL BE 40 FEET.
9. RPM SPACING ON WHITE SOLID TURN LANE LINES SHALL BE 20'.
10. ALL DETECTABLE WARNING SURFACES SHALL BE CAST IN PLACE (WET SET & ANCHORED IN CONCRETE).
11. NO UTILITY STRUCTURES, VALVE BOXES OR METER BOXES ARE ALLOWED TO BE WITHIN THE SIDEWALK ALONG BARRETT ROAD.
12. ALL CROSSWALK STRIPING IS 4' SEPARATION FROM 24" STOP BAR. SEE DETAIL ON SHEET 10.

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
3	1-28-26	REVISED PER LAMSID COMMENTS	JLW
2	12-8-25	REVISED TURNLANE ON BETH STACEY BLVD	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523

ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690

WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

MASTER ENGINEERING PLAN

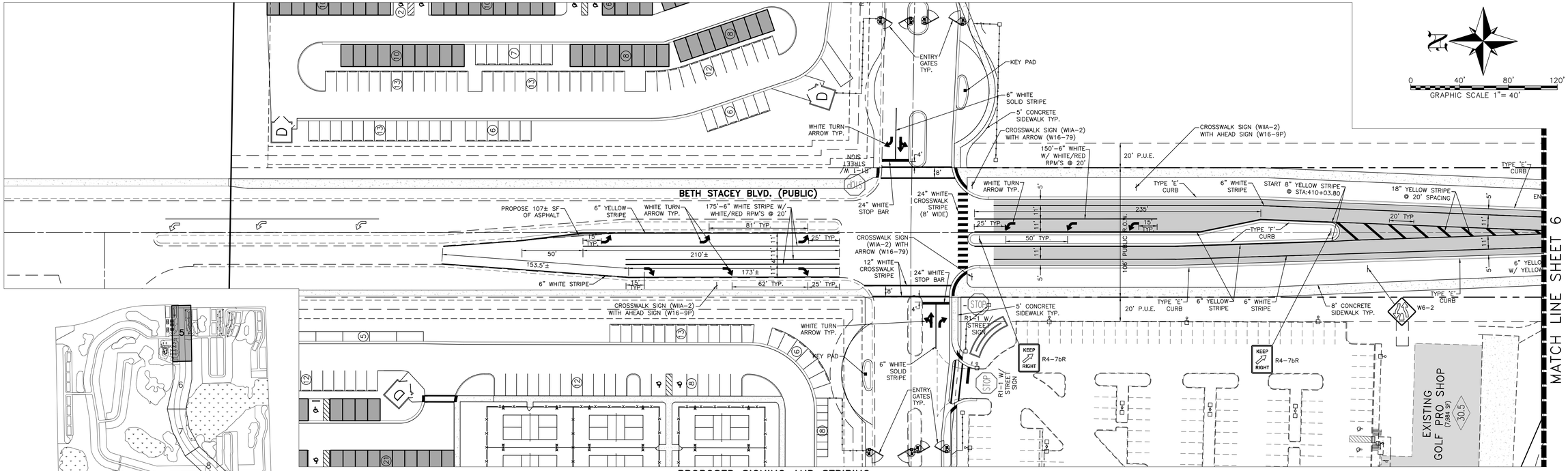
IBIS LANDING - BETH STACEY BLVD.

LEE COUNTY, FLORIDA

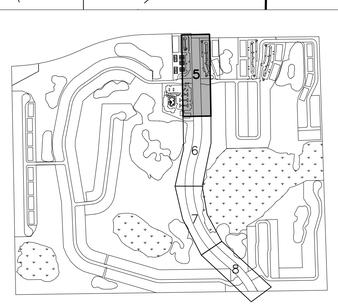
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	MEP	JLW	JLW	DRU	1"=200'	04

IBIS LANDING

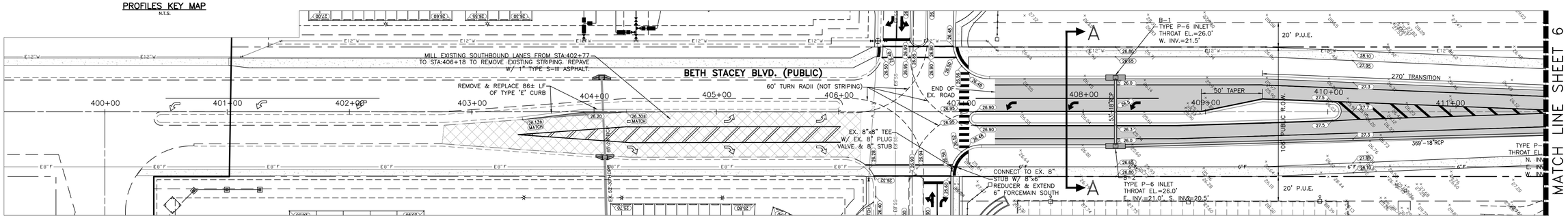
S:\USERS\DWG\3142L\DWGS\DWG\GEOPLAN\GEN 23RDST BLVD. L.D.O. PLANS\3142L-BLVD-04-MEP.DWG 1/29/2026 1:35 PM JIMMY MCKAY



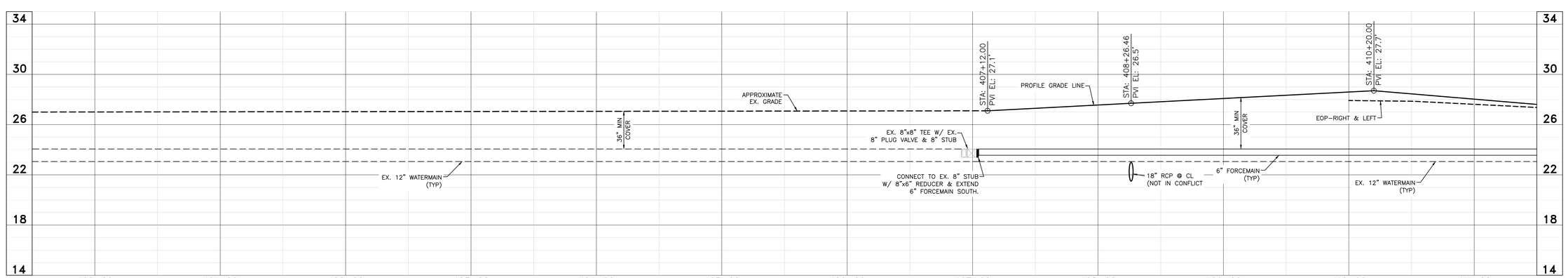
PROPOSED SIGNING AND STRIPING
1" = 40'



PROFILES KEY MAP
N.T.S.



PROPOSED PLAN & PROFILE
1" = 40'



ALL ELEVATIONS ARE BASED ON NORTH AMERICAN VERTICAL DATUM OF 1988

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
4	12-8-25	REVISED TURNLANE ON BETH STACEY BLVD	JLW
3	10-16-25	REVISED PER COUNTY COMMENTS	JLW
2	9-15-25	REVISED PER COUNTY COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

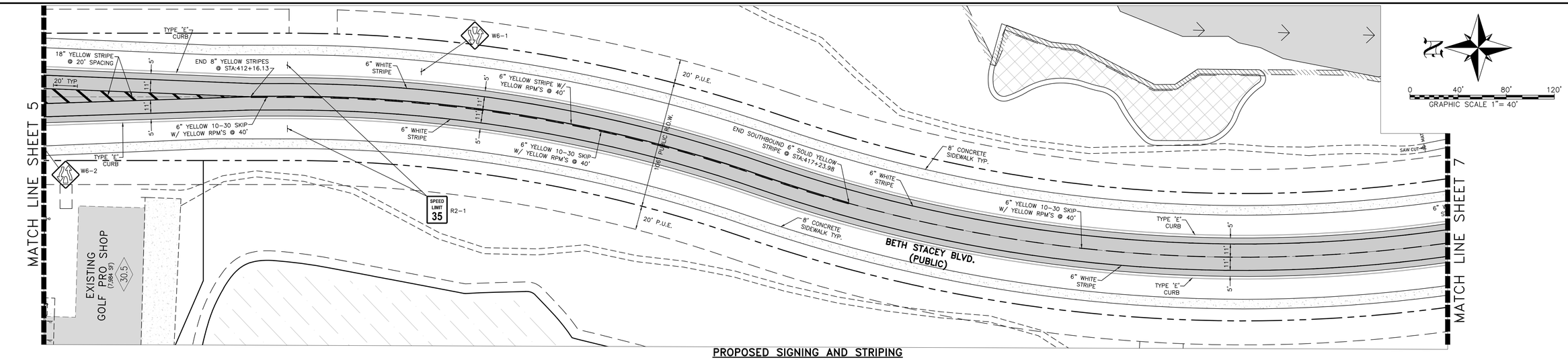
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

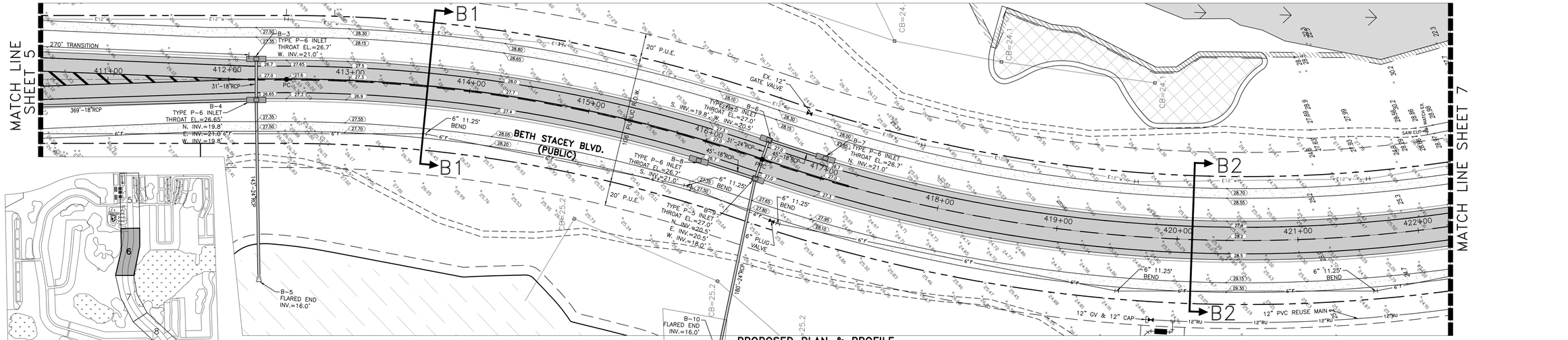
PLAN & PROFILE 1
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	PP1	JLW	JLW	DRU	1"=4' V 1"=40' H	05

IBIS LANDING

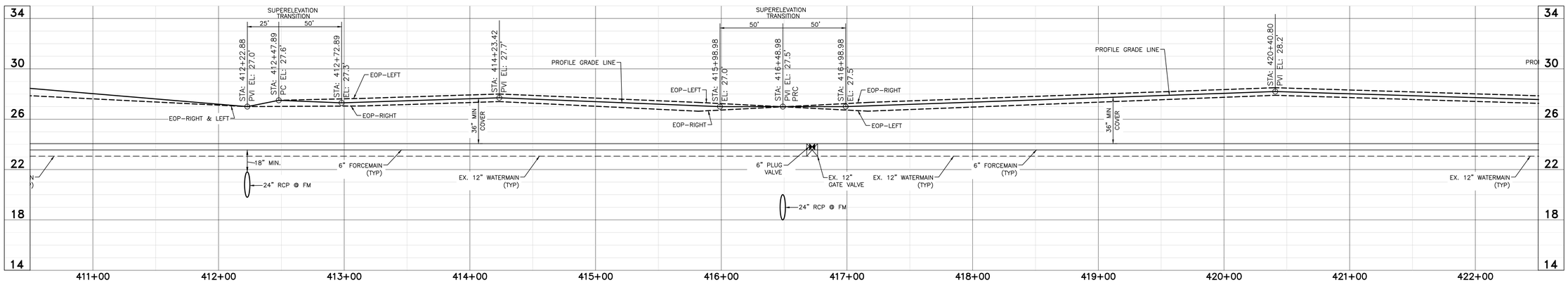


PROPOSED SIGNING AND STRIPING
1" = 40'



PROFILES KEY MAP
N.T.S.

PROPOSED PLAN & PROFILE
1" = 40'



ALL ELEVATIONS ARE BASED ON NORTH AMERICAN VERTICAL DATUM OF 1988

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
3	10-16-25	REVISED PER COUNTY COMMENTS	JLW
2	9-15-25	REVISED PER COUNTY COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523

ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690

WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

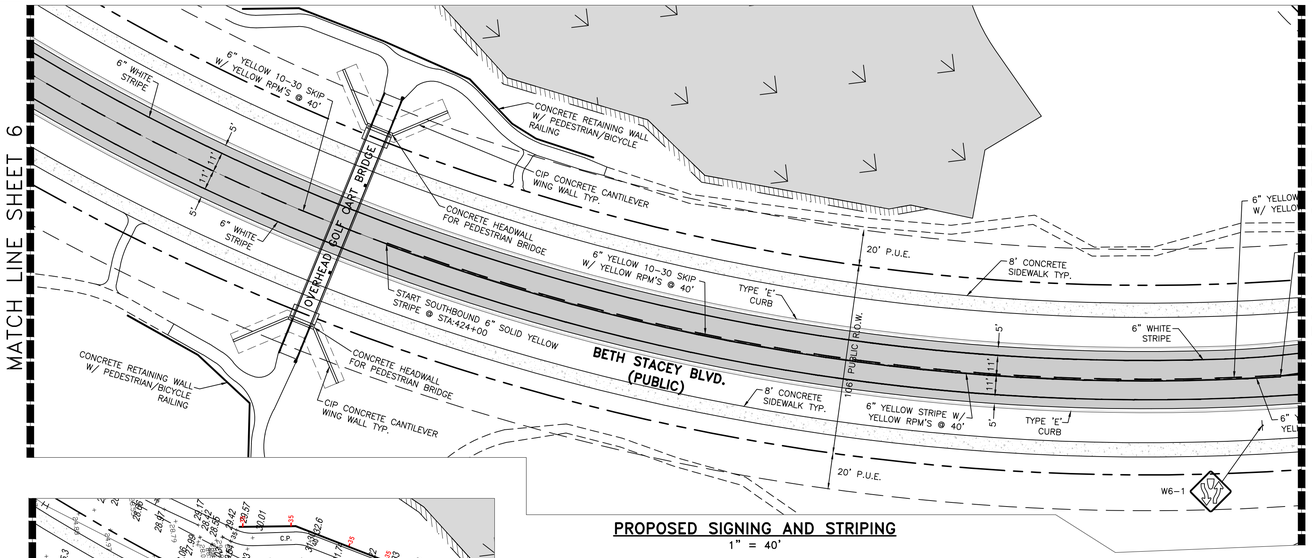
PLAN & PROFILE 2

IBIS LANDING - BETH STACEY BLVD.

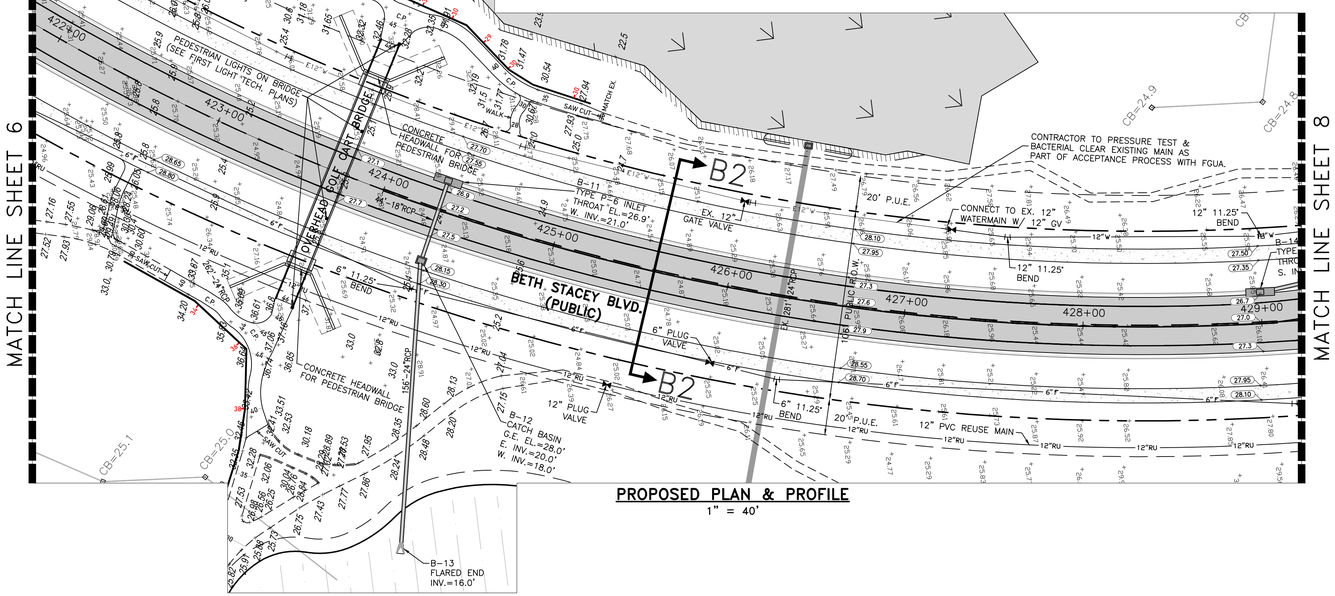
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	PP2	JLW	JLW	DRU	1"=4' V 1"=40' H	06

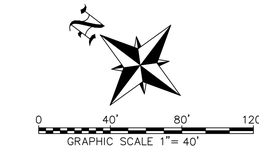
S:\JOBS\3142L\ENGINEERING\DESIGN\PLANS\BETH STACEY BLVD. I.L.O. PLANS\3142L-BLVD-05-30-PP.DWG 1/28/2026 1:36 PM JIMMY WICKAY

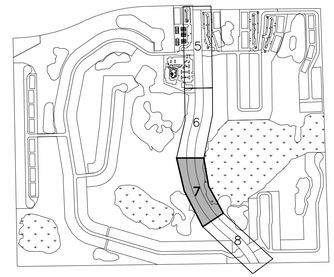


PROPOSED SIGNING AND STRIPING
1" = 40'



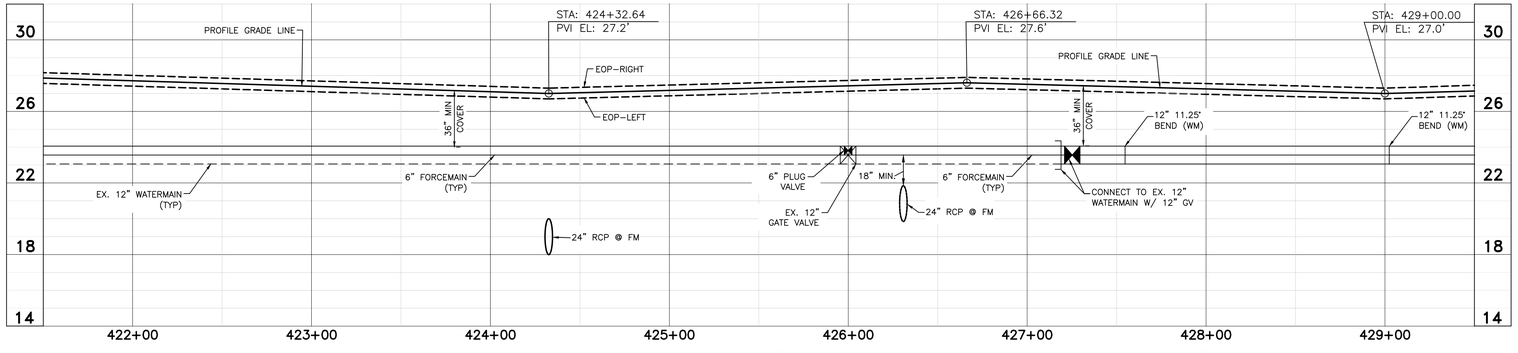
PROPOSED PLAN & PROFILE
1" = 40'





PROFILES KEY MAP
N.T.S.

NOTE: FOR GOLF CART BRIDGE DESIGN AND DETAILS SEE PLANS PREPARED BY ARCOS BRIDGE, INC.



ALL ELEVATIONS ARE BASED ON NORTH AMERICAN VERTICAL DATUM OF 1988

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
4	12-8-25	ADDED PEDESTRIAN LIGHTS ON BRIDGE	JLW
3	10-16-25	REVISED PER COUNTY COMMENTS	JLW
2	9-15-25	REVISED PER COUNTY COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

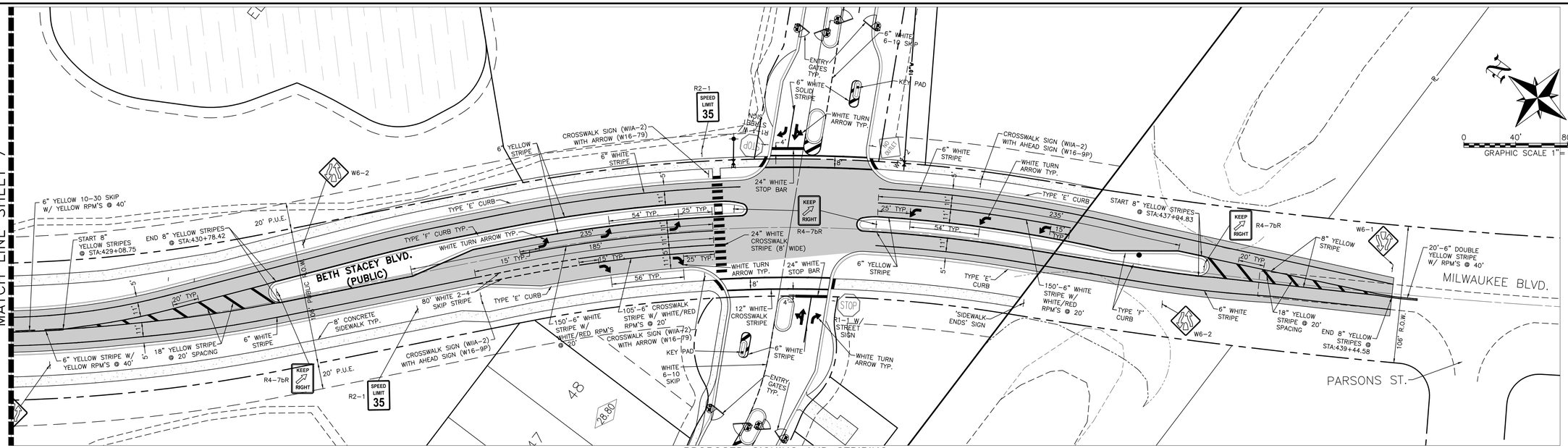
DAVID R. UNDERHILL JR.
P.E. #47029

PLAN & PROFILE 3
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

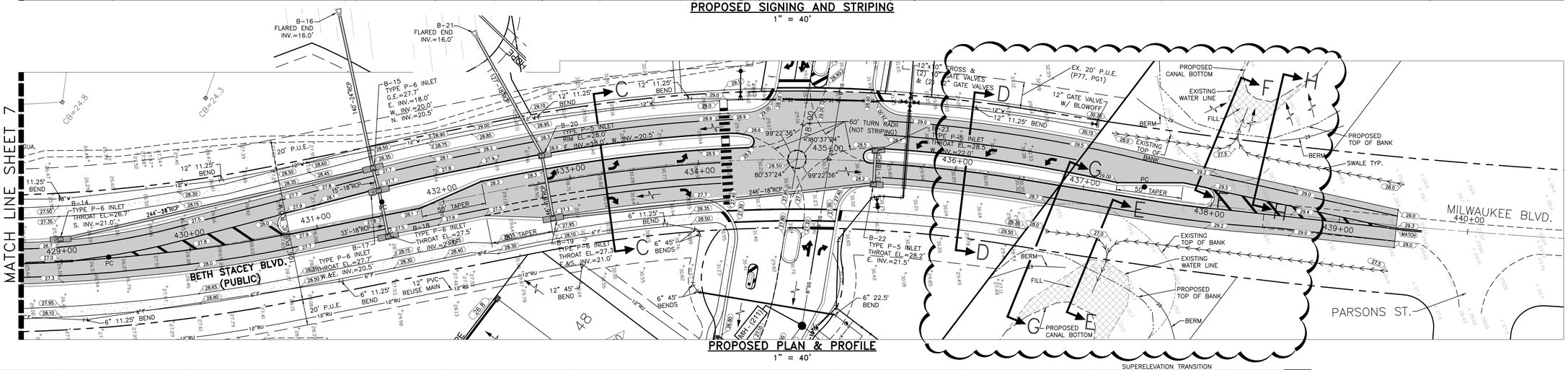
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	PP3	JLW	JLW	DRU	1"=4' V 1"=40' H	07

IBIS LANDING

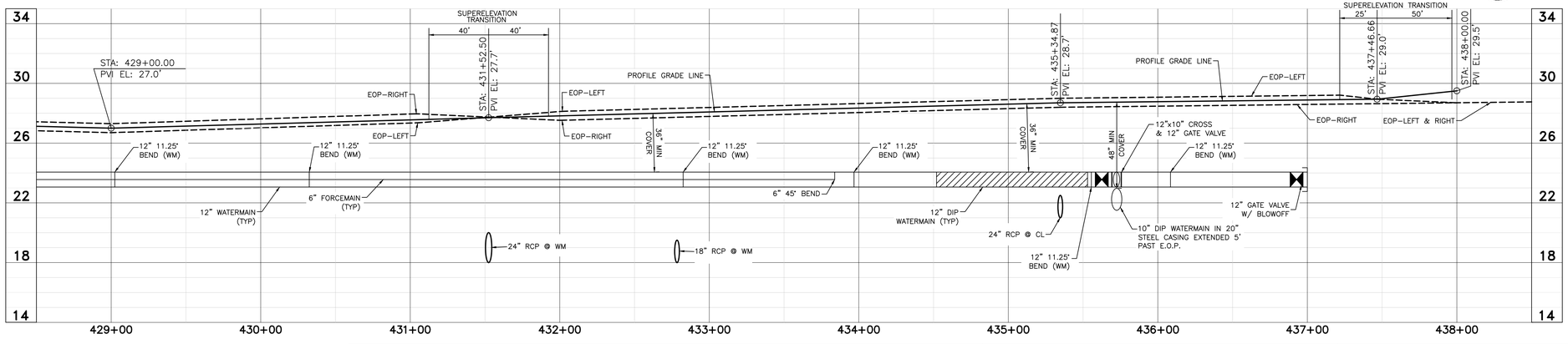
MATCH LINE SHEET 7

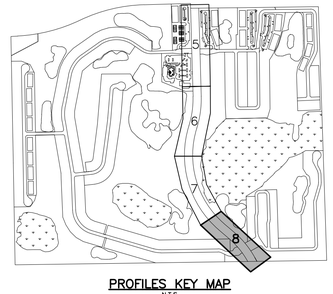


PROPOSED SIGNING AND STRIPING
1" = 40'



PROPOSED PLAN & PROFILE
1" = 40'





PROFILES KEY MAP
N.T.S.

ALL ELEVATIONS ARE BASED ON NORTH AMERICAN VERTICAL DATUM OF 1988

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY
4	1-28-26	REVISED PER LAMSID COMMENTS	JLW
3	10-16-25	REVISED PER COUNTY COMMENTS	JLW
2	9-15-25	REVISED PER COUNTY COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523

ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690

WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

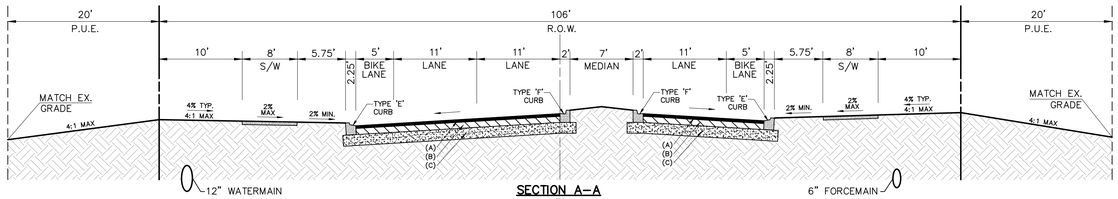
PLAN & PROFILE 4

IBIS LANDING - BETH STACEY BLVD.

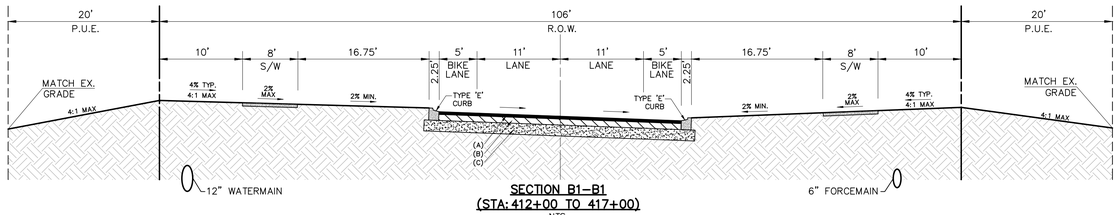
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	PP4	JLW	JLW	DRU	1"=4' V 1"=40' H	08

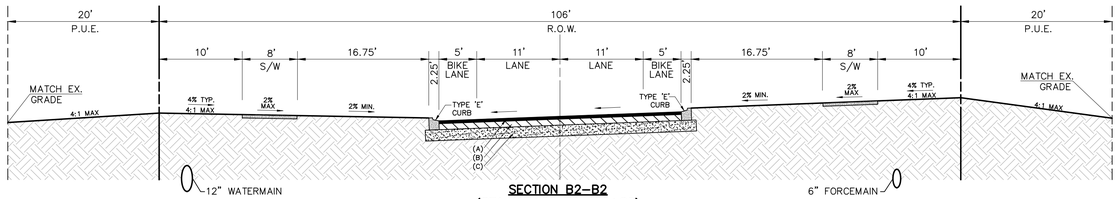
S:\USERS\VIVIA\DOCUMENTS\PROJECTS\IBIS BLVD. PLANS\PLAN-PROFILE-05-08-PP.DWG, 1/29/2026 1:39 PM JIMMY MECKLY



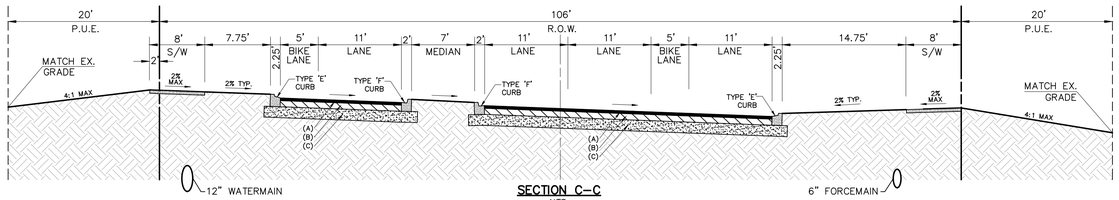
SECTION A–A
NTS



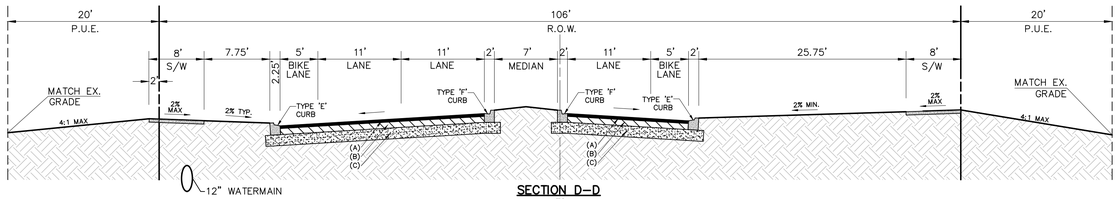
SECTION B1–B1
(STA: 412+00 TO 417+00)
NTS



SECTION B2–B2
(STA: 417+00 TO 429+00)
NTS



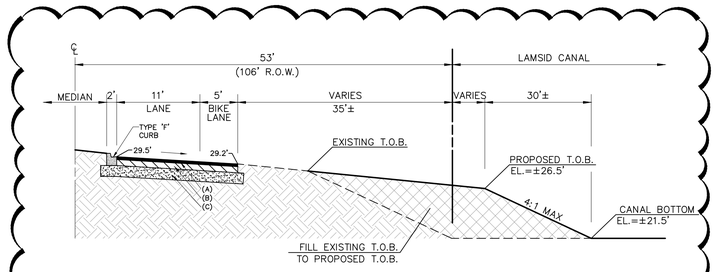
SECTION C–C
NTS



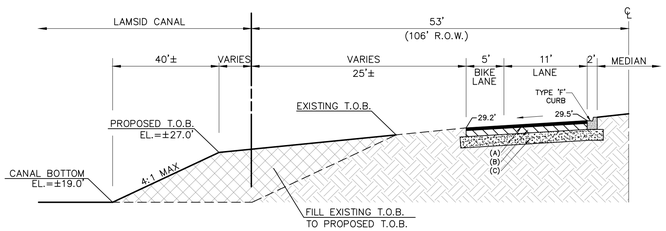
SECTION D–D
NTS

PAVEMENT DESIGN (BETH STACEY BLVD.)

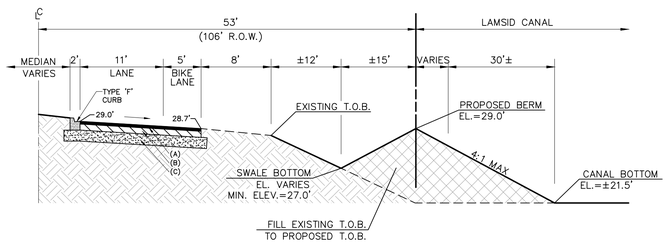
- (A) 1.5" TYPE SP 12.5 ASPHALT W/ 1" TYPE SP 9.5 ASPHALT SURFACE COURSE.
- (B) 8" LIMEROCK BASE (MIN. LBR=100) COMPACTED TO 98% OF MAXIMUM DENSITY BY AASHTO T-180.
- (C) 12" STABILIZED SUBGRADE TYPE B STABILIZATION (MIN. LBR=40) COMPACTED TO 98% OF MAXIMUM DENSITY BY AASHTO T-180.



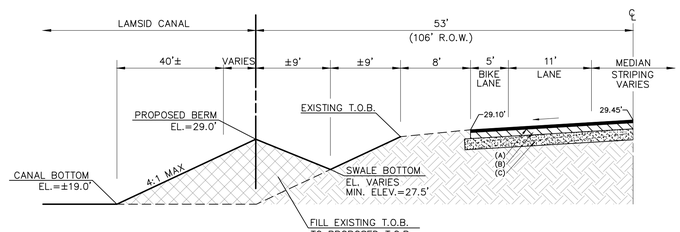
SECTION E–E
NTS



SECTION F–F
NTS

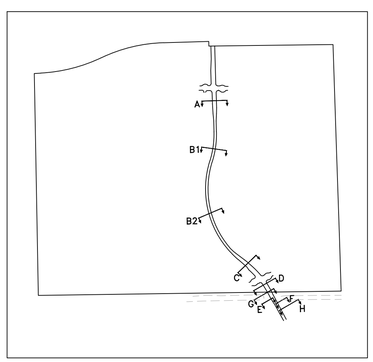


SECTION G–G
NTS



SECTION H–H
NTS

FILL NOTE: ALL FILL MATERIAL USED TO FILL THE ROADWAY AND LAMSID CANAL MUST BE USCS SOIL CLASSIFICATION SP–CLEAN SANDS WITH LESS THAN 5% FINE OR ORGANIC MATERIAL.



SECTION KEY MAP
NTS

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

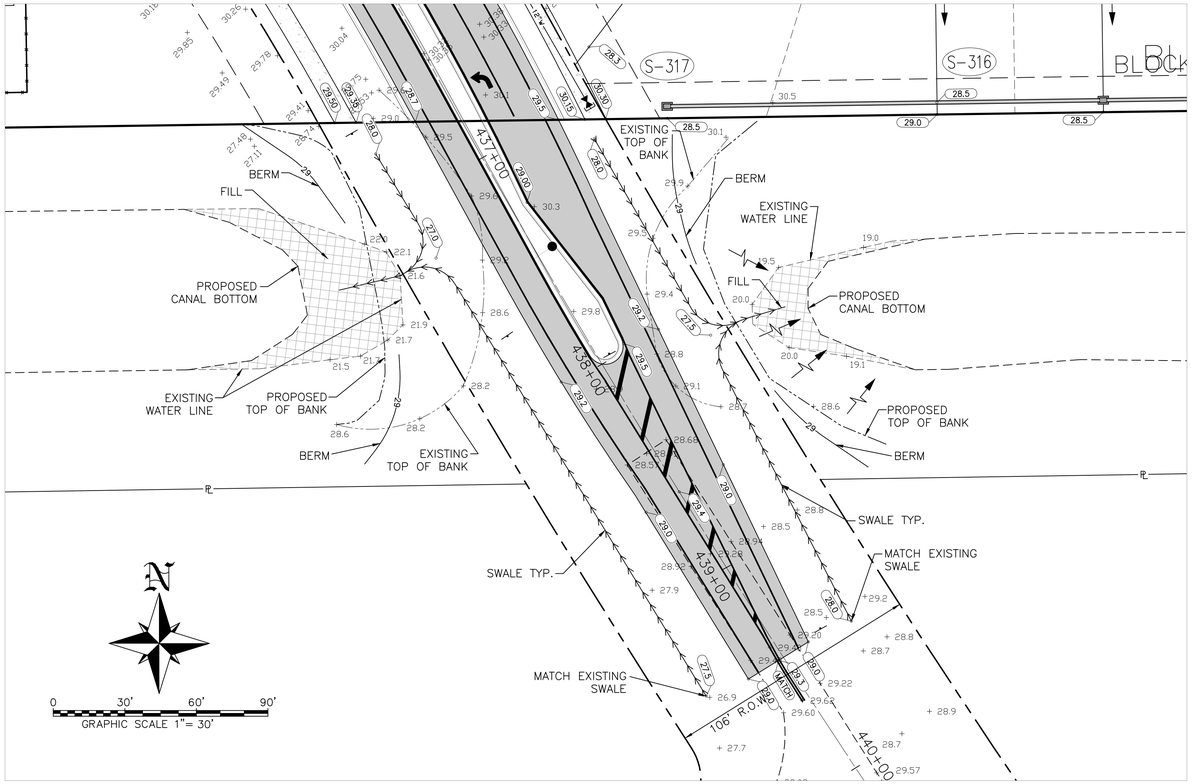
NO.	DATE	REVISION DESCRIPTION	BY
2	1-28-26	REVISED PER LAMSID COMMENTS	JLW
1	6-2-25	REVISED PER COMMENTS	JLW

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM
DAVID R. UNDERHILL JR.
P.E. #47029

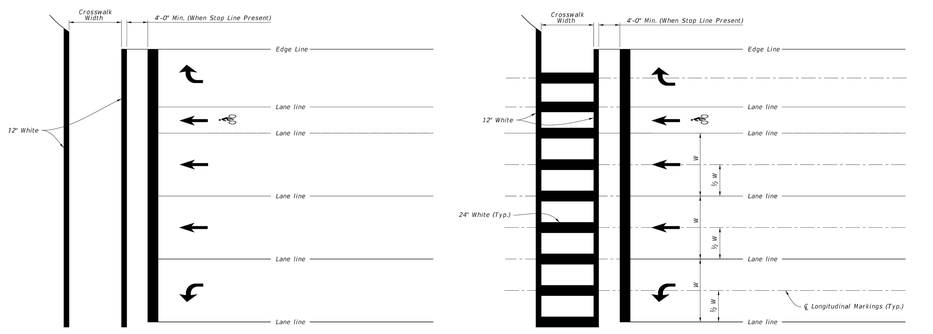
TYPICAL CROSS-SECTIONS
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	XSEC	JLW	JLW	DRU	NTS	09

S:\JOBS\3142L\ENGINEERING\GRAPHICS\BETH STACEY BLVD. L.D.S. PLANS\3142L-BLVD-04-XSEC.DWG 1/29/2026 1:38 PM JIMMY WEEKLY



BETH STACEY BLVD. & MILWAUKEE BLVD TIE-IN
SCALE: 1"=30'

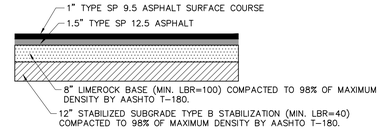


NOTES:

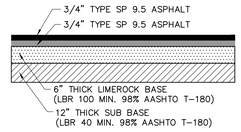
1. For crosswalk width, exceed width of the adjacent sidewalk, but do not make width less than 6' for intersection crosswalks and 10' for midblock crosswalks. Measure width from the inside of the transverse crosswalk markings.
2. When the Special Emphasis Crosswalk is not perpendicular to the lane lines, make the longitudinal markings parallel to the lane lines.
3. Refer to Index 522-002 when Curb Ramps are present.

LAST REVISION	DESCRIPTION:	FDOT FY 2023-24 STANDARD PLANS		INDEX	SHEET
11/01/21			PAVEMENT MARKINGS	711-001	9 of 13

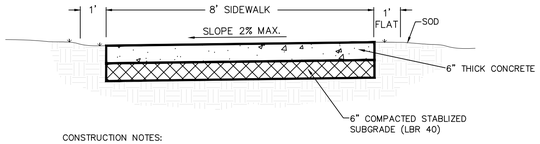
CROSSWALK PAVEMENT MARKING DETAILS
N.T.S.



BETH STACEY R.O.W. PAVEMENT SECTION
N.T.S.



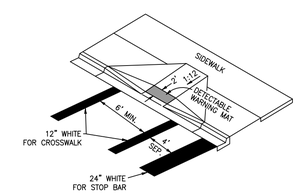
LOCAL STREET PAVEMENT SECTION
N.T.S.



CONSTRUCTION NOTES:

1. TYPICAL: SCORE OR SAW CUT WALKS 1 1/2" DEEP, 5'-0" O.C.
2. CONCRETE: 6" THICK FDOT CLASS I, 3000 P.S.I. MIN.
3. SIDEWALK FINISH TO BE LIGHT BROOM.
4. ALL DISTURBED AREAS NEXT TO WALK TO BE RESODDED WITH BAHIA TO MATCH ORIGINAL GRASS.
5. SIDEWALK CROSS SLOPE SHALL NOT EXCEED 2% CROSS SLOPE OR 5% RUNNING SLOPE.
6. SIDEWALK CONSTRUCTION SHALL COMPLY WITH THE LATEST EDITION OF THE FLORIDA GREENBOOK AND FDOT DESIGN STANDARDS, INCLUDING CURB RAMPS AND DETECTABLE WARNING SURFACES.
7. NO UTILITY STRUCTURES, VALVE BOXES OR METER BOXES ARE ALLOWED TO BE WITHIN THE SIDEWALK ALONG BARRETT ROAD

BETH STACEY BLVD. SIDEWALK DETAIL
N.T.S.



TYPICAL CROSSWALK W/ DETECTABLE WARNING MAT & STRIPING SEPARATION DETAIL
N.T.S.

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM
DAVID R. UNDERHILL JR. P.E. #47029

PAVING GRADING & DRAINAGE DETAILS
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	PGD-DT	JLW	JLW	DRU	NTS	10

IBIS LANDING

S:\USERS\STAFF\LISAGOODMAN\DESKTOP\LISA\BETH STACEY BLVD. L.D.O. PLANS\SHEETS\BS-10-PGD-DT.DWG 1/29/2024 1:38 PM JIMMY MCKAY

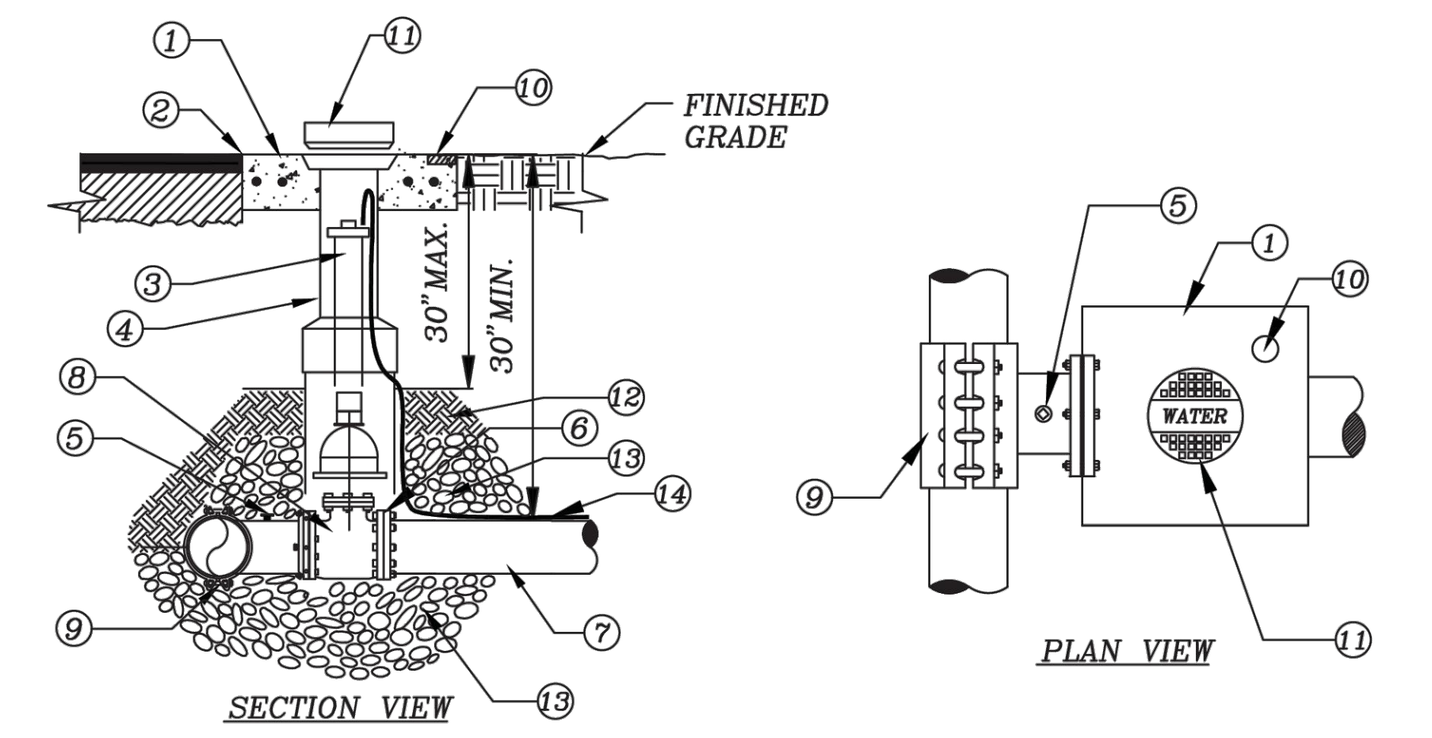
10/1/2022 2:56:01 PM

IBIS LANDING

STANDARD DETAIL NO. 6.1

TAPPING SLEEVE AND VALVE

N.T.S.



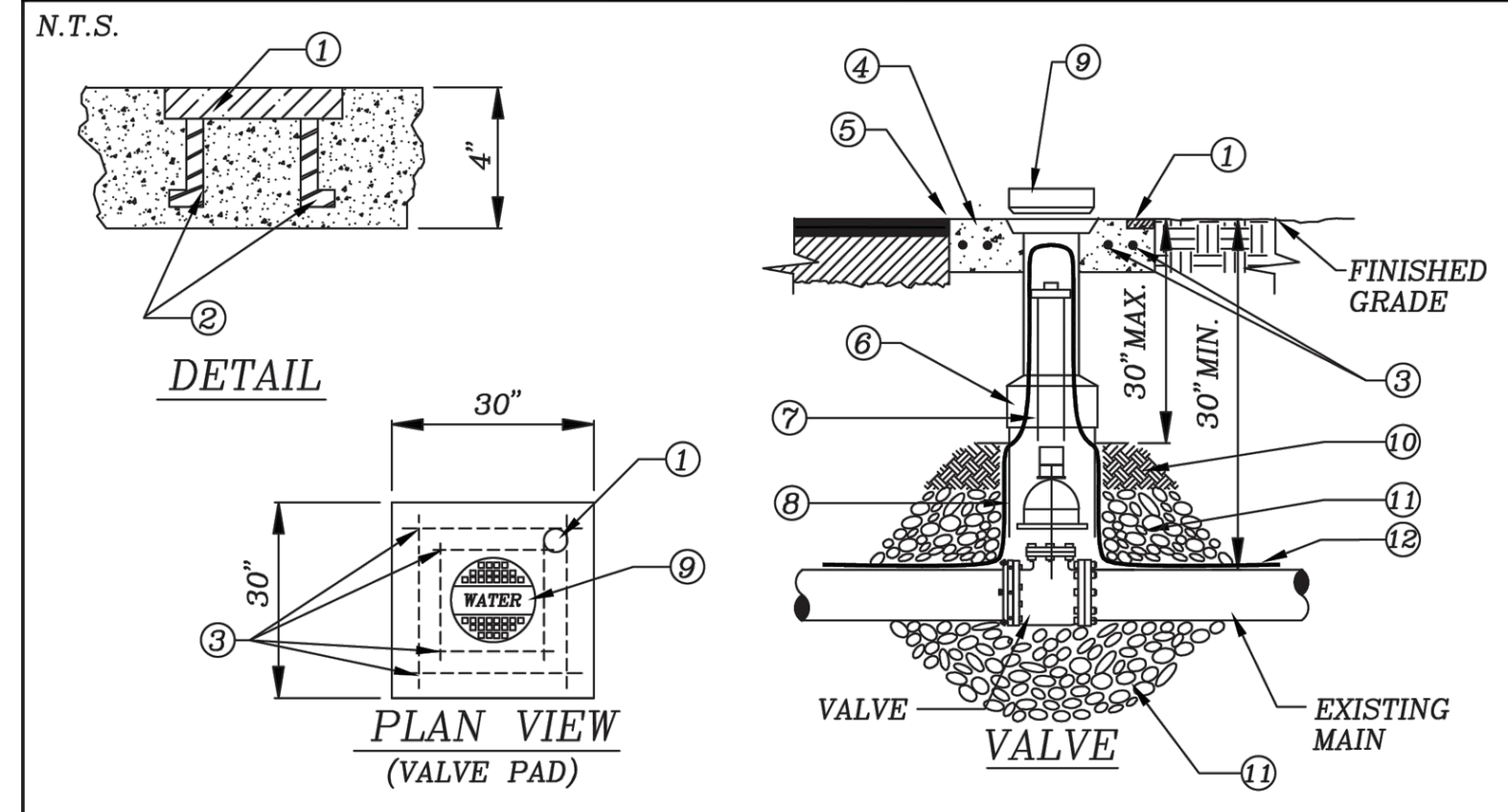
1. CONCRETE PAD (SEE VALVE INSTALLATION DETAIL)
2. SET TOP OF BOX FLUSH WITH FINISHED GRADE
3. EXTENSION STEM IF TOP OF OPERATING NUT IS MORE THAN 30" BELOW FINISH GRADE
4. VALVE BOX (SEE VALVE INSTALLATION DETAIL)
5. OUTLET FOR TESTING
6. MJ RESTRAINED
7. MAIN SIZE VARIES
8. TAPPING VALVE (SEE VALVE INSTALLATION DETAIL)
9. LCU APPROVED ALL STAINLESS STEEL PRESSURE RATED TAPPING SLEEVE. FOR SIZE ON SIZE TAPS A FLANGED TAPPING SLEEVE IS REQUIRED. SPRING LOADED TAPPING SLEEVE IS REQUIRED WHEN CONNECTING TO HDPE (SEE APPROVED MATERIAL LIST).
10. BRASS PLATE (SEE VALVE INSTALLATION DETAIL)
11. CAST IRON DROP COVER MARKED "WATER","SEWER", OR "REUSE"
12. COMPACTED SUITABLE EARTH BACKFILL
13. 3/4" GRANULAR MATERIAL #57 STONE
14. 12 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.2

VALVE INSTALLATION

N.T.S.



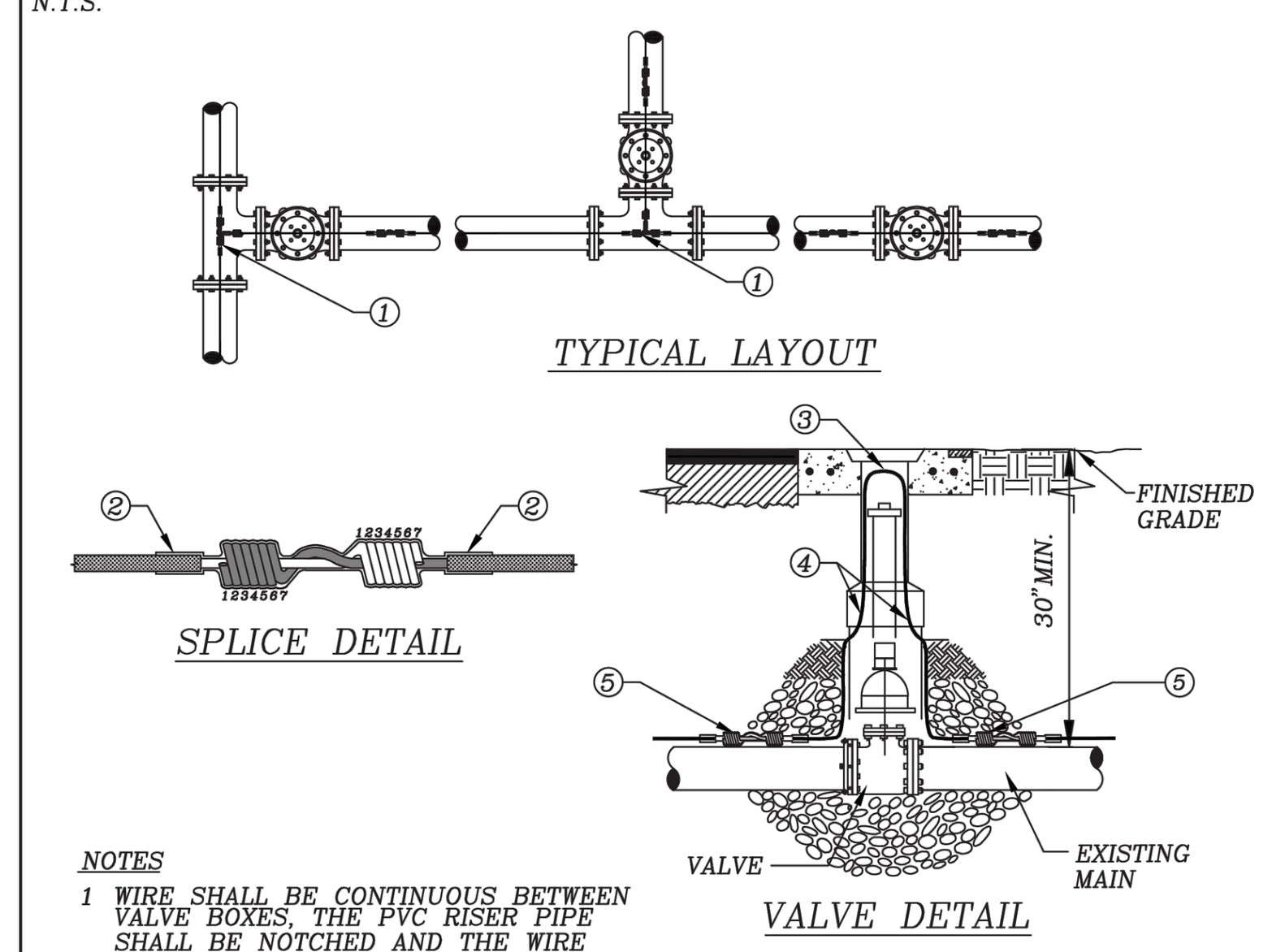
1. BRASS PLATE: SIZE OF VALVE, VALVE TYPE No. OF TURNS & DIRECTION TO OPEN VALVE M.F.G. & YEAR INSTALL SYSTEM "WATER" OR "SEWER" OR "REUSE" OR "FIRE"
2. ANCHOR
3. EIGHT (8) #4 BARS OVERLAP EACH CORNER BY 2"
4. 30"SQ X 4" THK.CONC.PAD SURROUNDING BOX, MIN. 3,000 P.S.I. POURED IN PLACE
5. SET TOP OF BOX FLUSH WITH FINISHED GRADE
6. HEAVY DUTY TRAFFIC BEARING CAST IRON VALVE BOX, ADJUSTABLE SCREW TYPE, 5 1/4" DIAMETER SHAFT THAT IS LCU APPROVED
7. EXTENSION STEM WITH 2" OPERATING NUT AS REQUIRED, IF NUT IS MORE THAN 30" BELOW FINISH GRADE
8. RISER NOT TO BEAR ON VALVE OR PIPE
9. CAST IRON DROP COVER MARKED "WATER" OR "SEWER" OR "REUSE OR "FIRE"
10. COMPACTED SUITABLE EARTH BACKFILL
11. 3/4" GRANULAR MATERIAL #57 STONE
12. 12 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.3

LOCATING WIRE FOR PRESSURE MAINS

N.T.S.



NOTES
1 WIRE SHALL BE CONTINUOUS BETWEEN VALVE BOXES, THE PVC RISER PIPE SHALL BE NOTCHED AND THE WIRE ATTACHED TO THE RISER PIPE TO ALLOW FOR THE WIRE TO FIT INSIDE THE CAST IRON VALVE BOX WITHOUT SHEARING THE WIRE.
2 LOCATING WIRE SHALL BE LAID ON TOP OF THE MAIN, AND SHALL BE TAPED TO IT AT 10' INTERVALS AND TAPED AT ALL FITTINGS. TAPE SHALL BE 10 MIL POLYETHYLENE.
3 CONTRACTOR SHALL CONDUCT A CONTINUITY TEST ON ALL LOCATING WIRE SPLICES. PRIOR TO SCHEDULING THE OFFICIAL FINAL INSPECTION WITH LCU.
4 ALL SPLICES SHALL UTILIZE A DIRECT BURY WATERPROOF SPLICE KIT. (SEE APPROVED MATERIAL LIST)

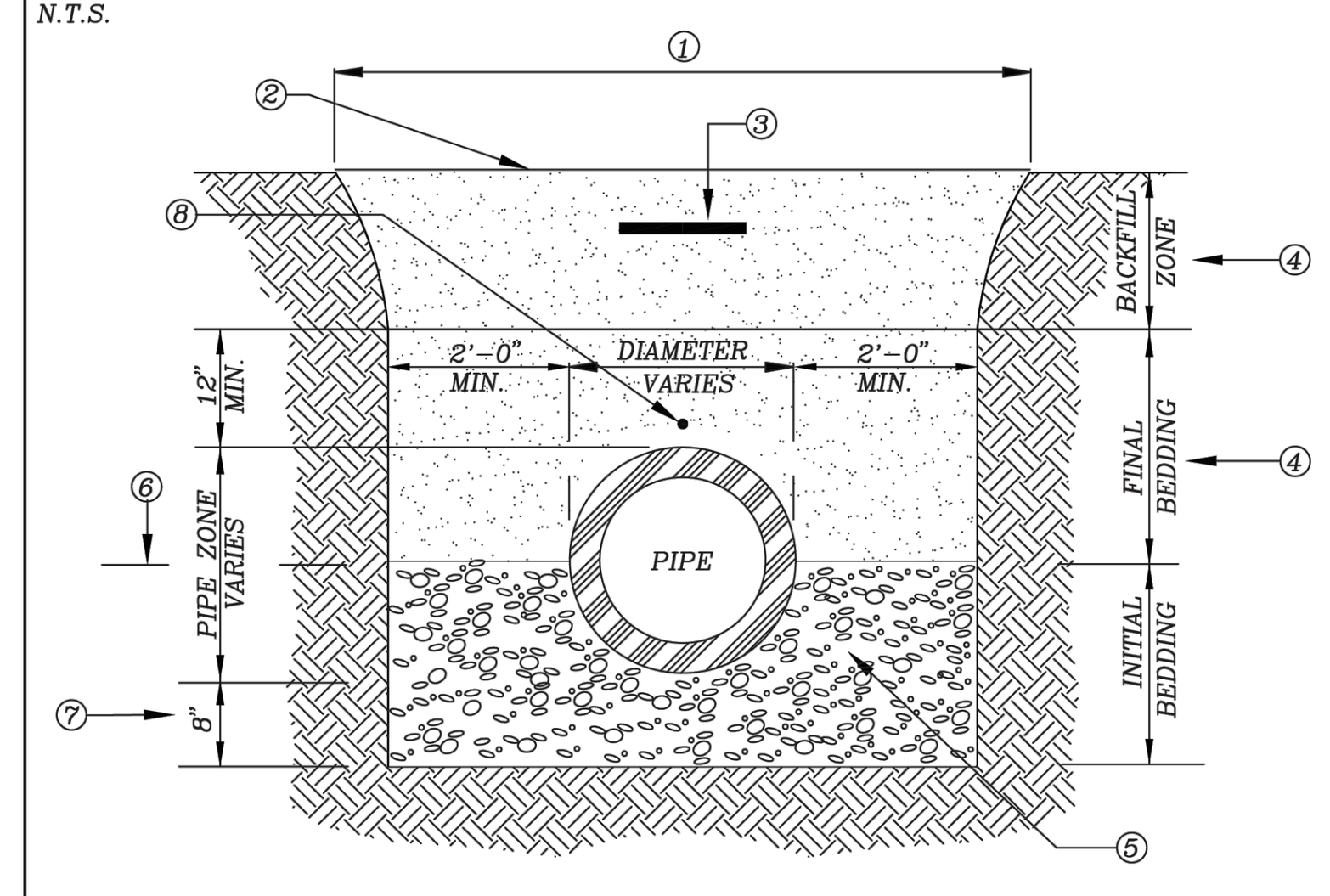
1. SPLICE RUN TOGETHER, KEEP CLEAR OF FITTING, TYPICAL
2. COVER SPLICE WITH 2 WRAPS OF 10 MIL. POLYETHYLENE TAPE
3. 6" LOOP OF BARE WIRE, 9" MAX. DEPTH TO TOP OF BARE WIRE
4. 12 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE
5. SPLICE (SEE DETAIL THIS SHEET)

REV: 12/10/2015

STANDARD DETAIL NO. 6.5

TRENCH CROSS SECTION

N.T.S.



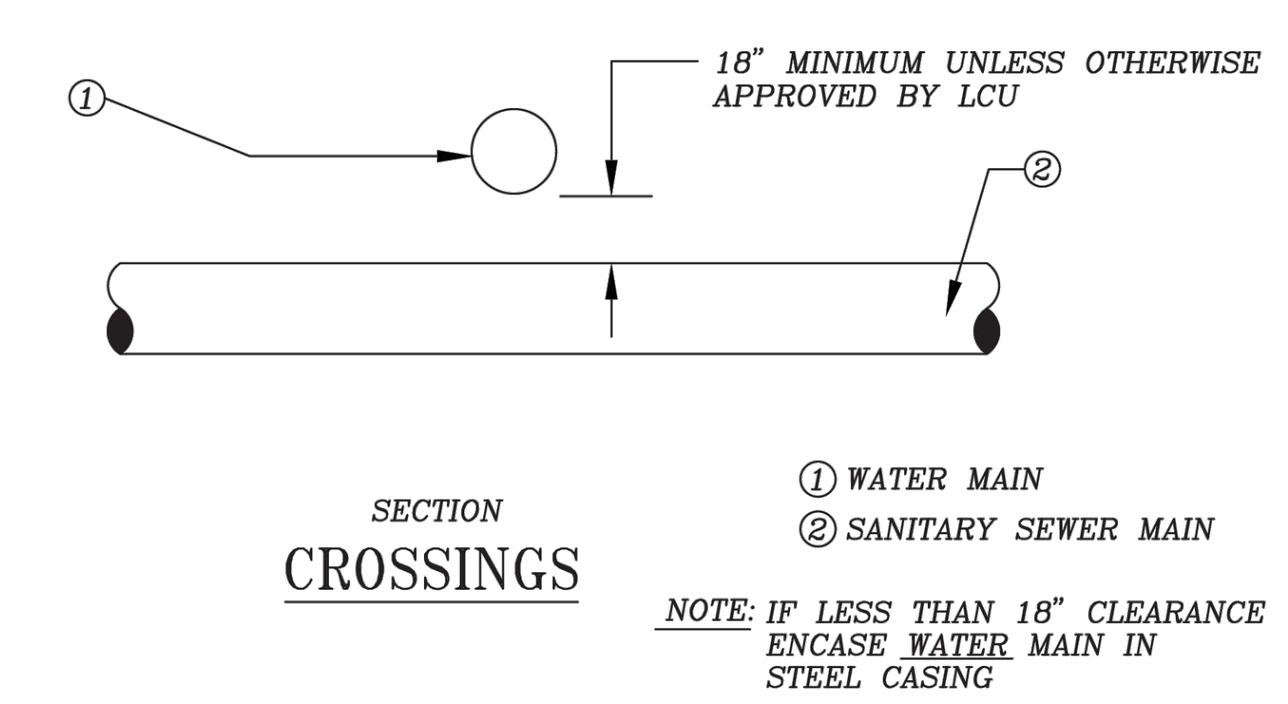
1. EXCAVATED TRENCH
2. FINISHED GRADE
3. MARKING TAPE 12" DEPTH MAX.
4. MATERIALS CLEAN DRY SAND, FINE LIME ROCK OR PIT SHELL MECHANICALLY COMPACTED IN 6" LIFTS TO AT LEAST 95% OF MAX. DENSITY, OR 98% IF UNDER PAVED AREA OF ROADWAY.
5. #57 STONE LOCATED UNDER PIPE OR EXISTING UNDISTURBED SUITABLE MATERIAL
6. SPRING LINE
7. STANDARD 8" MIN TRENCH UNDERCUT AND BACKFILL WITH #57 STONE COMPACTED IN 6" LIFTS ADDITIONAL UNDERCUT AND CRUSHED ROCK BEDDING FOUNDATION WHEN DIRECTED BY L.C.U. THE CONTRACTOR IS TO REMOVE UNSTABLE MATERIAL FROM THE TRENCH FOUNDATION. THEN INSTALL STABILIZED CRUSHED ROCK BACKFILL WITH #57 STONE COMPACTED IN 6" LIFTS
8. LOCATING WIRE IS REQUIRED FOR ALL PRESSURIZED PIPELINES.

REV: 12/10/2015

STANDARD DETAIL NO. 6.8

WATER AND SEWER CROSSING DETAIL

N.T.S.

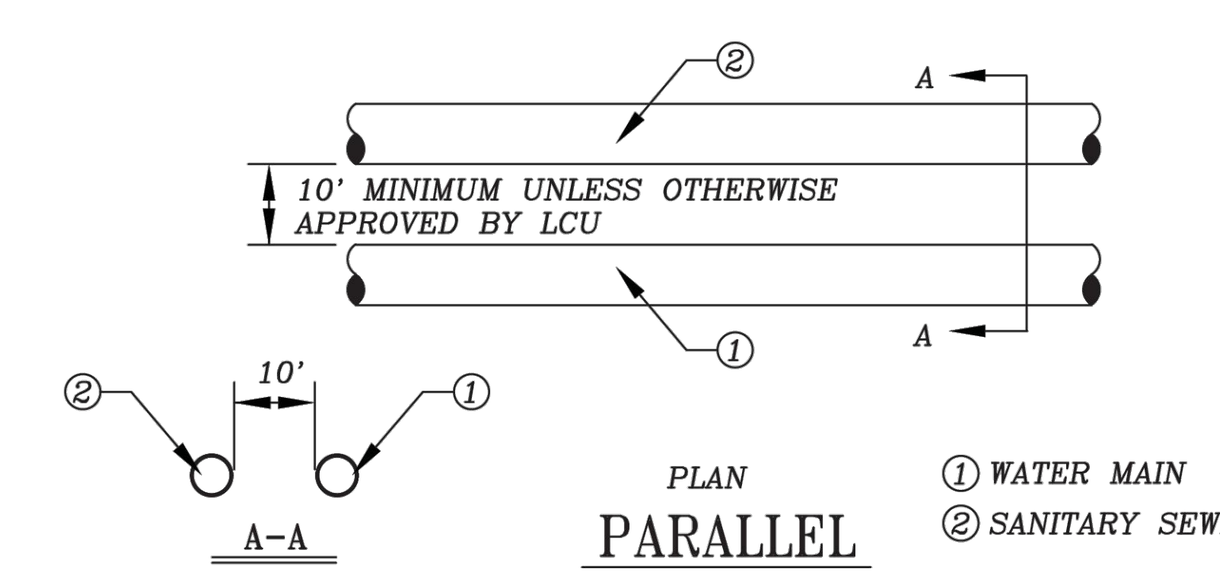


SECTION
CROSSINGS

1. WATER MAIN
2. SANITARY SEWER MAIN

NOTE: IF LESS THAN 18" CLEARANCE ENCASE WATER MAIN IN STEEL CASING

N.T.S.



PLAN
PARALLEL

1. WATER MAIN
2. SANITARY SEWER MAIN

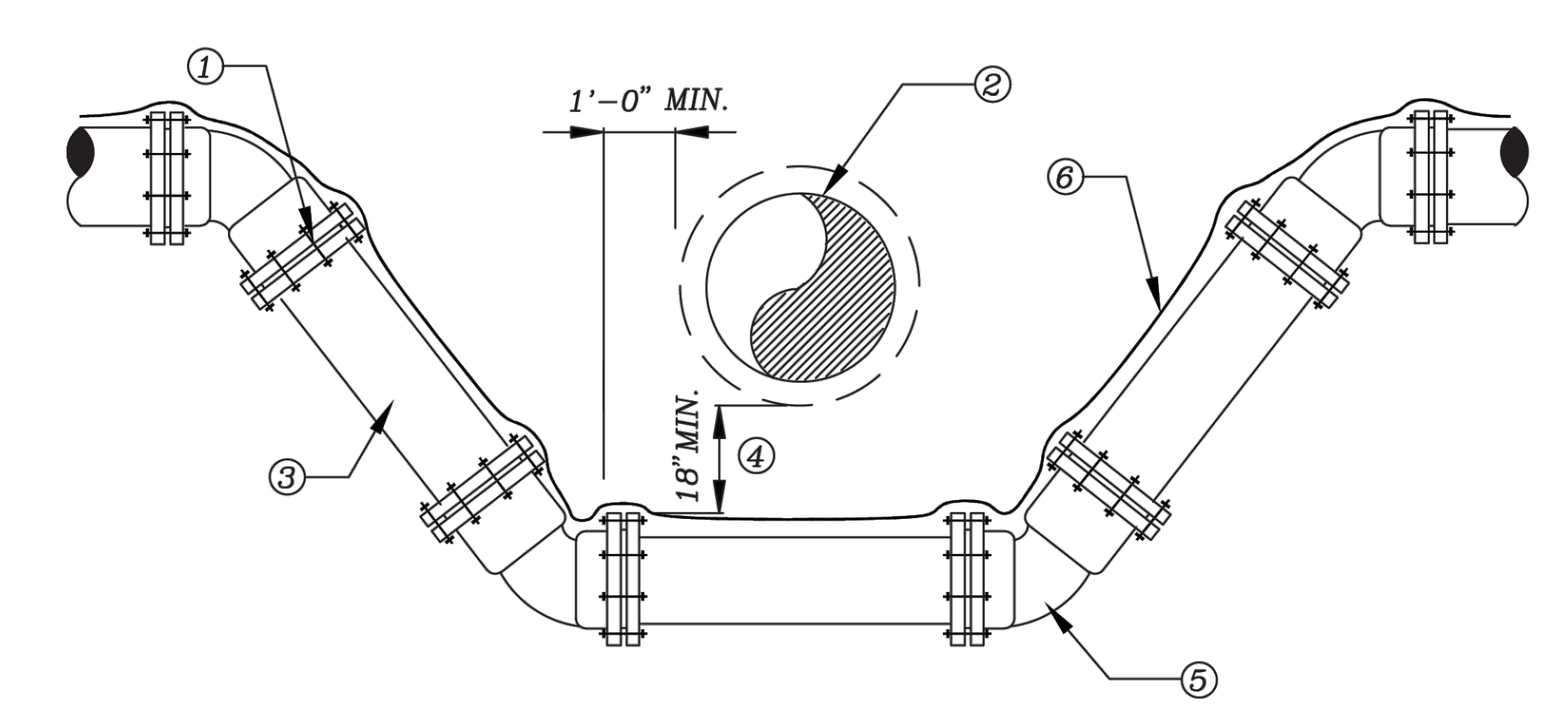
NOTE: IF LESS THAN 10' CLEARANCE ENCASE WATER MAIN IN STEEL CASING

REV: 12/10/2015

STANDARD DETAIL NO. 6.10

WATER PRESSURE LINE VERTICAL OFFSET W/ DUCTILE IRON MJ FITTINGS & PIPE

N.T.S.



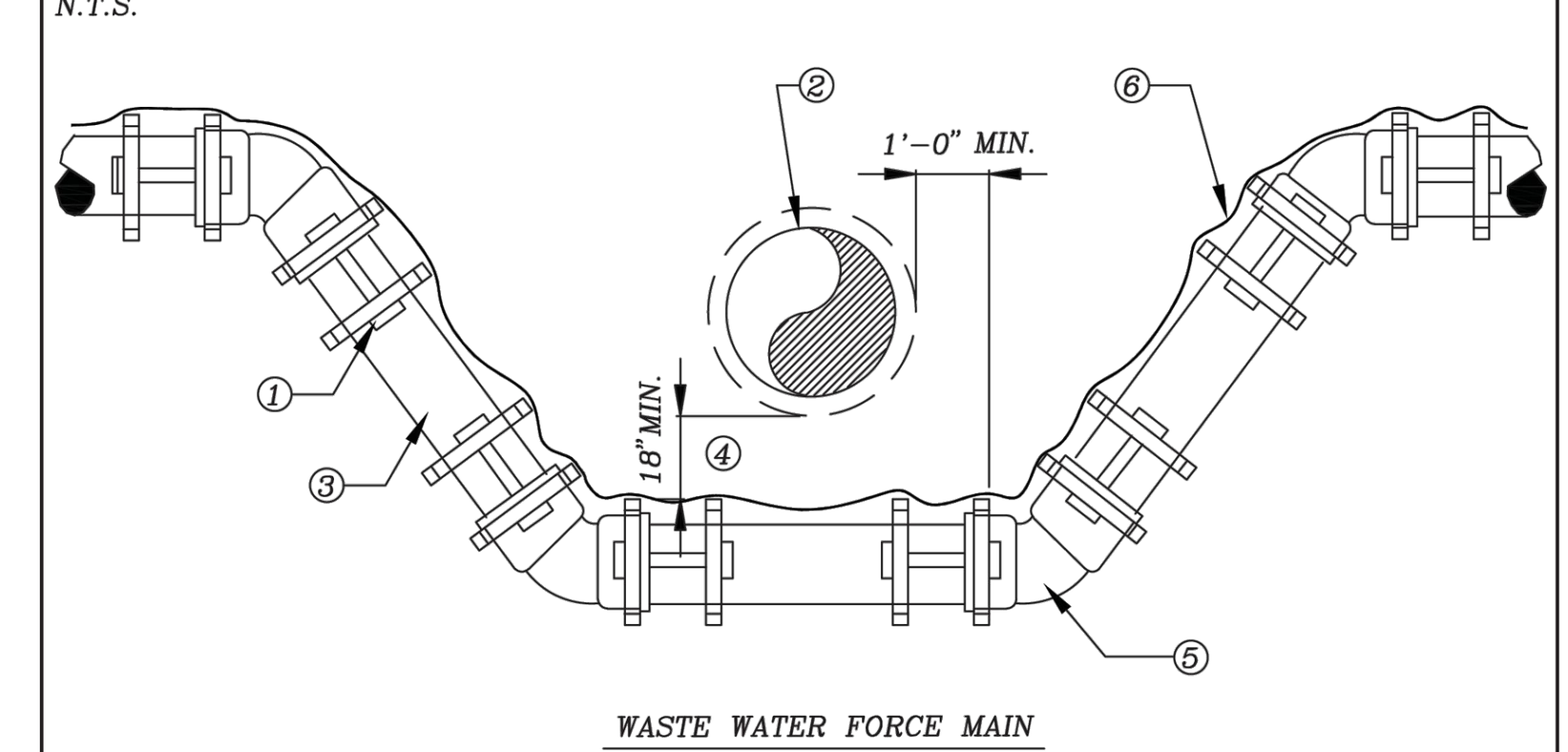
1. LCU APPROVED JOINT RESTRAINT FOR D.I., M.J. PIPE AND FITTINGS PLACED IN ACCORDANCE WITH JOINT RESTRAINT SCHEDULE. (SEE DETAIL 6.12)
2. CONFLICT PIPE
3. LCU APPROVED DUCTILE IRON PIPING PRESSURE CLASS 350
4. 18" MIN. UNLESS OTHERWISE APPROVED BY LCU
5. 45° MECHANICAL JOINT FITTING, TYPICAL
6. 12 GAUGE DOUBLE INSULATED COPPER WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.11

WASTE WATER FORCE MAIN VERTICAL OFFSET WITH FITTINGS, (P.V.C.)

N.T.S.



WASTE WATER FORCE MAIN

1. LCU APPROVED JOINT RESTRAINT FOR P.V.C., M.J. PIPE AND FITTINGS PLACED IN ACCORDANCE WITH JOINT RESTRAINT SCHEDULE. (SEE DETAIL 6.12)
2. CONFLICT PIPE
3. LCU APPROVED C-900 PVC DR 14
4. 18" MIN. UNLESS OTHERWISE APPROVED BY LCU
5. 45° MECHANICAL JOINT FITTING, TYPICAL
6. 12 GAUGE DOUBLE INSULATED COPPER WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.12

RESTRAINED LENGTH SCHEDULE

N.T.S.

DUCTILE IRON PIPE

PIPE SIZE	MINIMUM RESTRAINED PIPE LENGTH (FEET)					
	HORIZONTAL BENDS				DEAD	HORIZONTAL
(Inches)	90°	45°	22-1/2°	11-1/4°	END/VALVE	TEE
4	17	7	4	2	29	6
6	23	10	5	2	40	17
8	29	12	6	3	53	29
10	35	14	7	4	63	38
12	41	17	8	4	74	49
16	51	21	11	5	94	68
24	69	29	14	7	131	105
30	81	34	17	8	156	129

PVC PIPE

PIPE SIZE	MINIMUM RESTRAINED PIPE LENGTH (FEET)					
	HORIZONTAL BENDS				DEAD	HORIZONTAL
(Inches)	90°	45°	22-1/2°	11-1/4°	END/VALVE	TEE
4	20	8	4	2	45	8
6	29	12	6	3	63	25
8	36	15	8	4	83	43
10	44	18	9	5	99	58
12	51	21	11	5	116	74
16	63	26	13	7	149	103
24	87	36	18	9	208	158
30	102	42	21	10	248	194

A COMPLETE JOINT RESTRAINING SCHEDULE FOR ALL ENCOUNTERED VERTICAL & HORIZONTAL BENDS, VERTICAL OFFSETS, TEES, AND DEAD ENDS SHALL BE THE RESPONSIBILITY OF THE DESIGN ENGINEER.

LENGTH FIGURES BASED ON FOLLOWING:

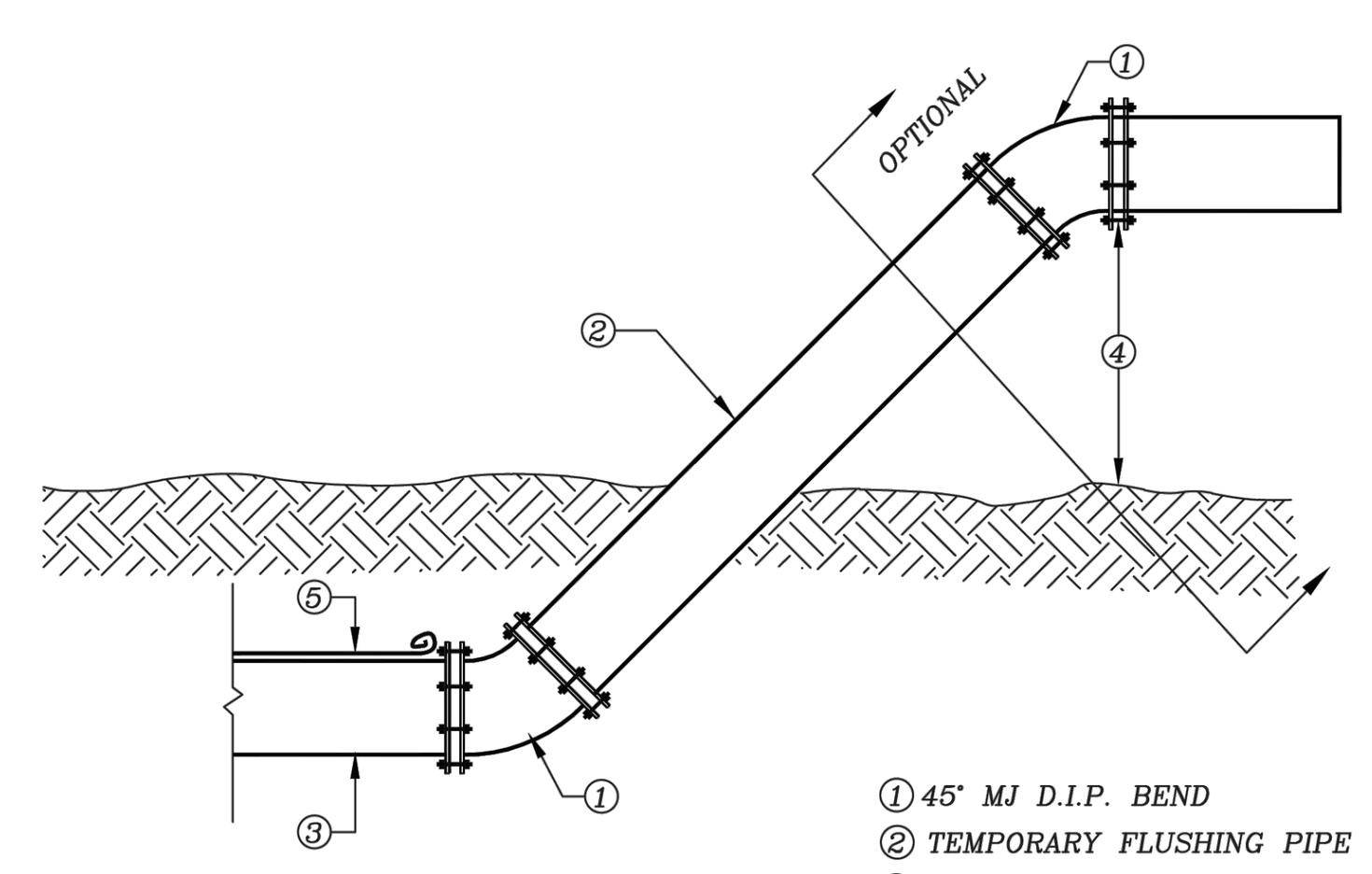
Pressure = 150 psi, FS = 1.5, trench type = 3, 30" cover on Bare pipe, Soil type = GP & SP

REV: 12/22/2022

STANDARD DETAIL NO. 6.13

WATER MAIN FLUSHING DETAIL

N.T.S.



1. 45° MJ D.I.P. BEND
2. TEMPORARY FLUSHING PIPE
3. END OF WATER MAIN
4. 24" MIN.

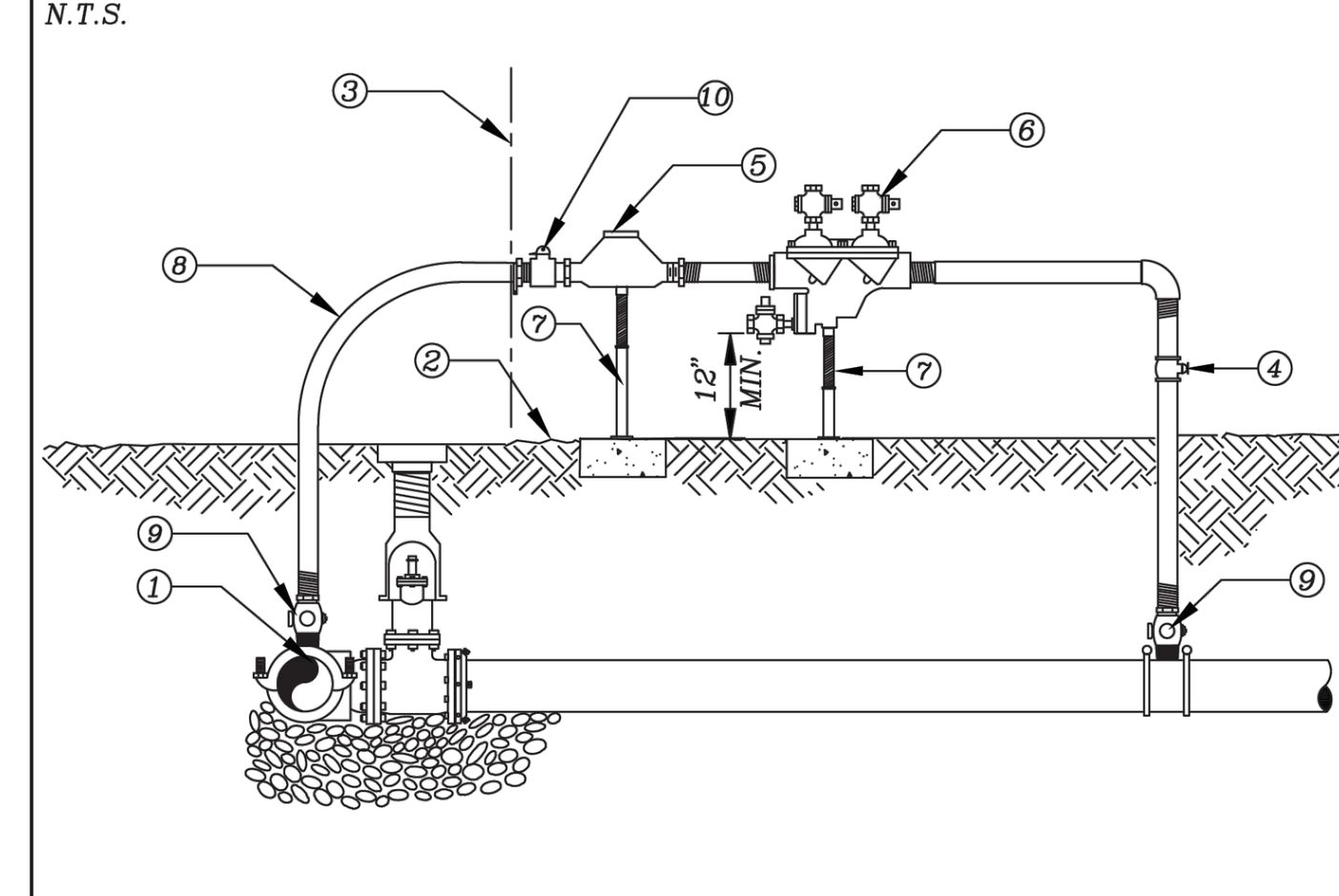
NOTES:
1. FLUSHING DETAIL TO BE USED ON WATER LINES 8" DIAMETER OR GREATER.
2. FLUSHING PIPE SHALL BE THE SAME DIAMETER AS WATER MAIN.
3. CONTRACTOR SHALL PROVIDE ADEQUATE DRAINAGE FOR FLUSHED WATER.
4. PROVIDE M.J. CAP IF ASSEMBLY IS LEFT IN PLACE UNATTENDED.
5. 12 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.15

TEMPORARY CONSTRUCTION JUMPER WITH METER

N.T.S.



NOTES:
CONTRACTOR/CUSTOMER SHALL APPLY AT LEE COUNTY UTILITIES FOR TEMPORARY WATER SERVICE FIVE DAYS PRIOR TO REQUIRED SERVICE DATE.
THE METER ONLY WILL BE FURNISHED BY LEE COUNTY UTILITIES. ALL FITTINGS, PIPING, VALVES AND MATERIALS SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR/CUSTOMER. THE APPROVED BACKFLOW PREVENTION DEVICE SHALL BE TESTED AND CERTIFIED BY LEE COUNTY UTILITIES APPROVED CERTIFIED BACKFLOW PREVENTION TECHNICIAN (FURNISHED BY CONTRACTOR/CUSTOMER) AT TIME OF METER INSTALLATION. CONTRACTOR/CUSTOMER SHALL PROTECT THE ASSEMBLY FROM DAMAGE. TEMPORARY CONSTRUCTION JUMPER/METER ASSEMBLY SHALL NOT BE REMOVED UNTIL SYSTEM IS ACCEPTED AND DEPARTMENT OF HEALTH CERTIFICATION AND CLEARANCE FOR SERVICE FORM HAS BEEN RECEIVED BY LEE COUNTY UTILITIES. WHEN THE NEW SYSTEM IS ACCEPTED, THE TEMPORARY CONSTRUCTION JUMPER METER ASSEMBLY MUST BE COMPLETELY REMOVED DOWN TO CORP STOPS.

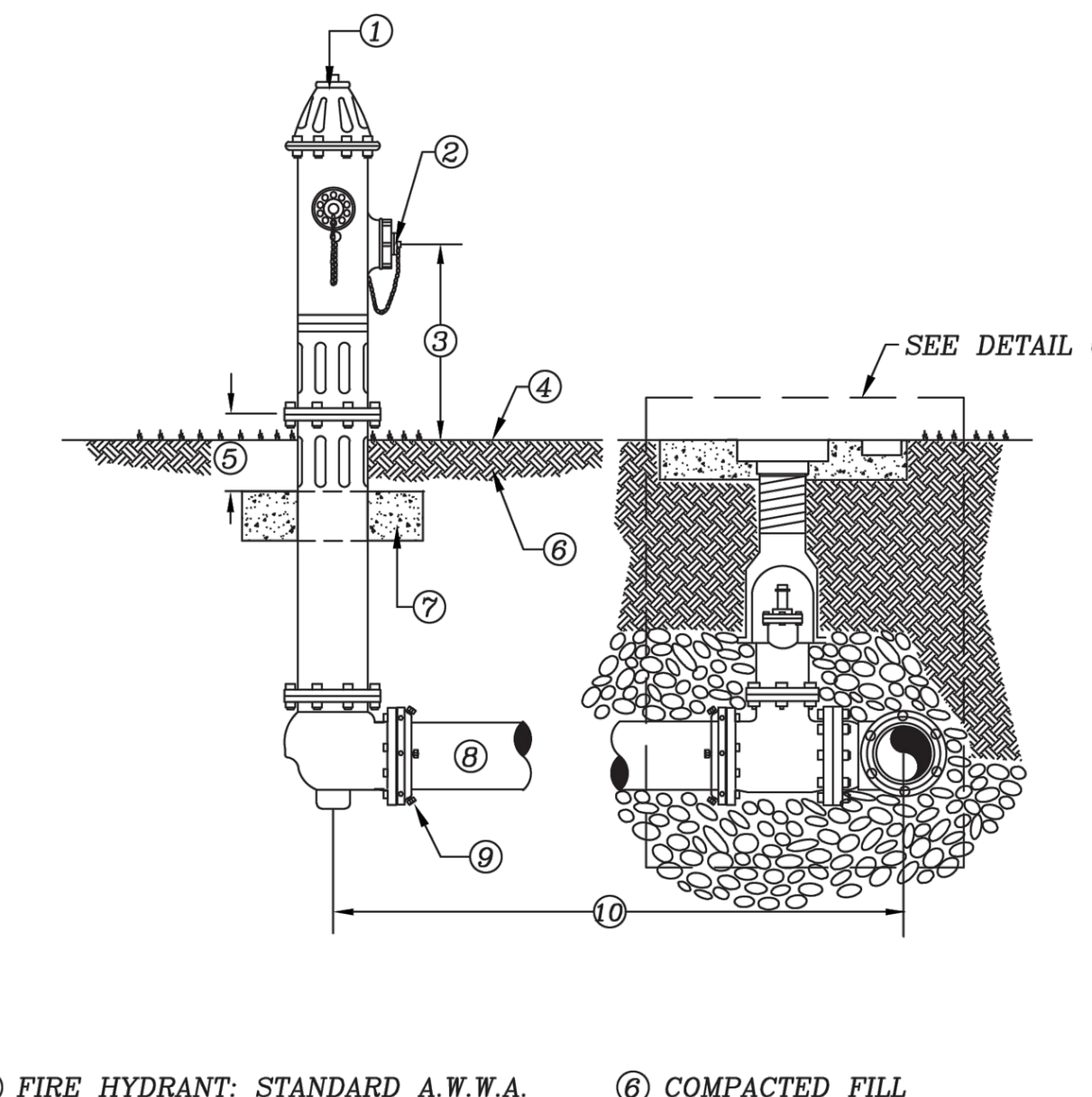
1. TAPPING SLEEVE AND VALVE TO BE FULLY RESTRAINED ON EXISTING MAIN (SEE LCU STANDARD DETAIL)
2. GRADE
3. PROPERTY LINE AND/OR RIGHT OF WAY LINE
4. BRONZE BALL VALVE
5. METER FURNISHED BY LEE COUNTY UTILITIES
6. REDUCED PRESSURE BACKFLOW PREVENTER
7. PIPE STAND (SEE LCU STANDARD DETAIL)
8. 2" DIA. HDPE DR9
9. 2" DIA. CORPORATION STOP
10. 2" DIA. CURB STOP

REV: 12/10/2015

STANDARD DETAIL NO. 6.24

FIRE HYDRANT ASSEMBLY

N.T.S.



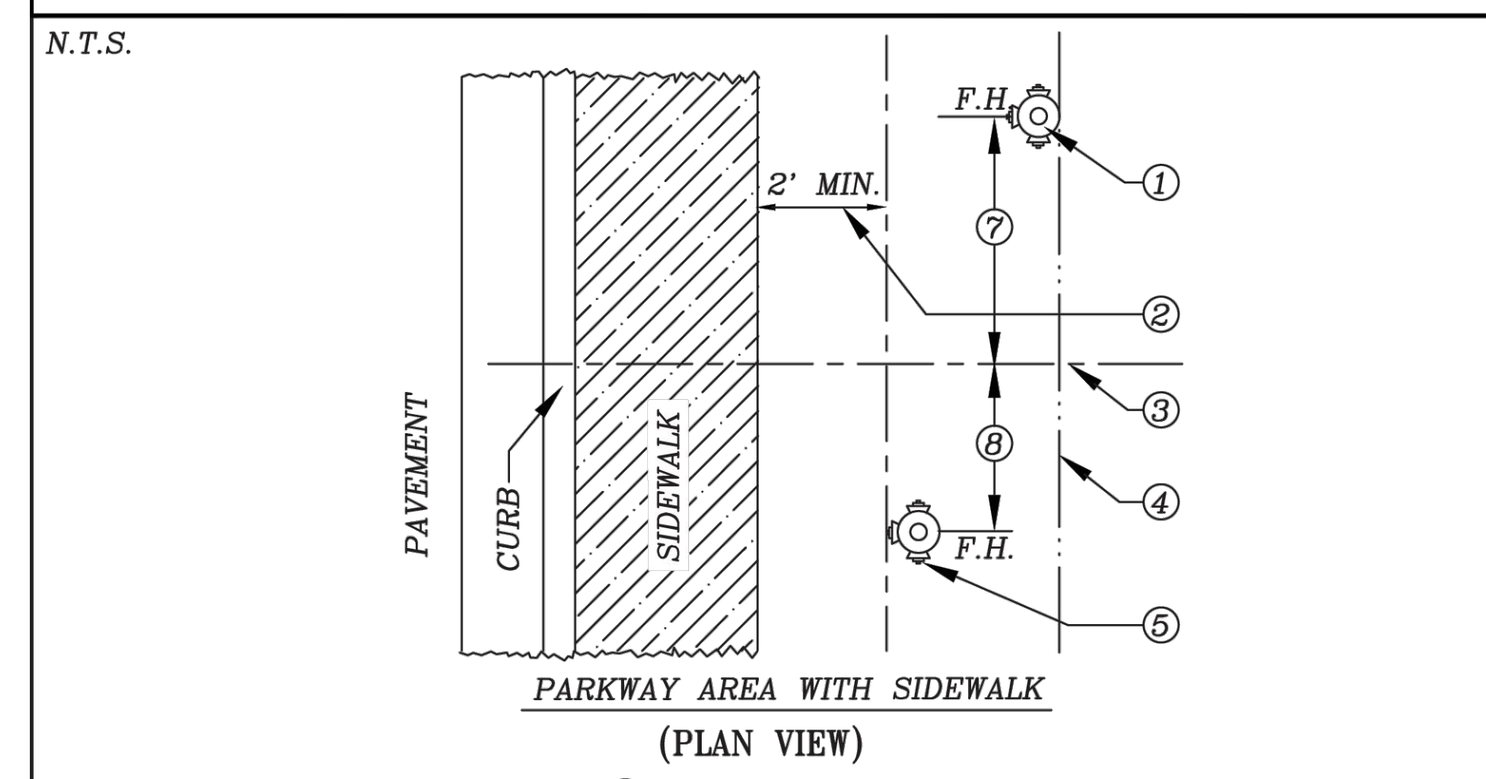
1. FIRE HYDRANT: STANDARD A.W.W.A. SAFETY YELLOW (SEE APPROVED MATERIAL LIST)
2. PUMPER CONNECTION FACING AS PER FIRE DISTRICT RECOMMENDATION
3. 18" MIN. – 24" MAX.
4. FINISHED GRADE
5. 6" MIN. CLEARANCE
6. COMPACTED FILL
7. 30" X 30" X 4" CONCRETE BREAK AWAY CONCRETE PAD, 3000 P.S.I.
8. PIPE TO BE D.I.P., PRESSURE CLASS 350
9. RESTRAINED JOINTS, FROM HYDRANT TO WATER MAIN
10. VARIABLE DISTANCE, IF GREATER THAN 100' A VALVE SHALL BE INSTALLED AT HYDRANT BASE

REV: 12/10/2015

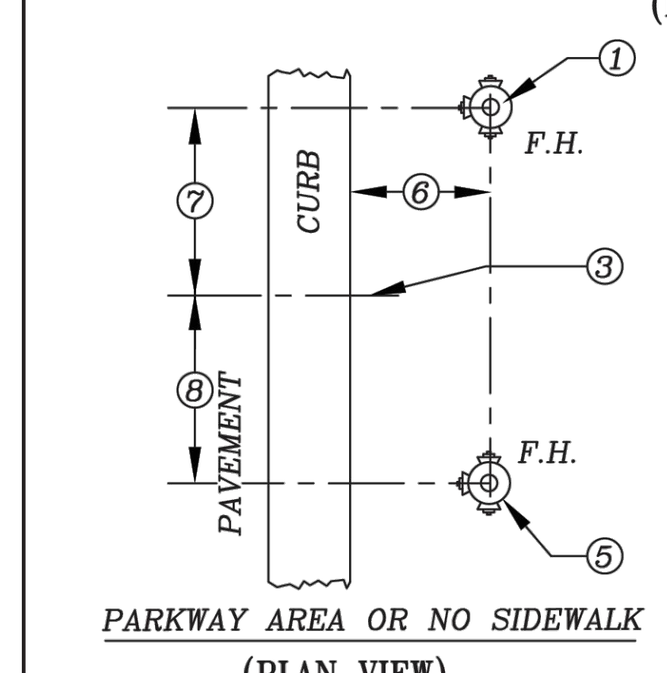
STANDARD DETAIL NO. 6.25

FIRE HYDRANT LOCATIONS/CLEARANCE

N.T.S.

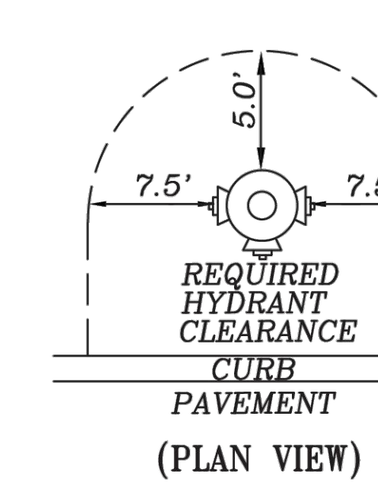


PARKWAY AREA WITH SIDEWALK
(PLAN VIEW)



PARKWAY AREA OR NO SIDEWALK
(PLAN VIEW)

1. PREFERRED LOCATION
2. F.H. TO BE LOCATED 2' MINIMUM FROM EDGE OF SIDEWALK
3. P.T. OR P.C. OF CURB RETURN
4. PROPERTY LINE RIGHT-OF-WAY OR LIMIT OF EASEMENT
5. ACCEPTABLE LOCATION WITH CURB RADIUS OF 20' OR MORE
6. BETWEEN 2'-0" MIN. AND 6'-0" MAX.
7. 6'-0"
8. 4'-0"



(PLAN VIEW)

NOTES
1. OBSTRUCTIONS SUCH AS UTILITY POLES, STREET SIGNS, IRRIGATION BOXES, FENCES, ETC. SHALL NOT BE PLACED BETWEEN CURB AND HYDRANT.
2. SOME LOCATIONS APPLY AT EITHER END OF CURB RETURNS.
3. DIMENSION SHOWN ON CONSTRUCTION DRAWINGS SUPERCEDE LOCATIONS SHOWN HERE.

REV: 12/10/2015

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC
10481 SIX MILE CYPRESS PARKWAY
FORT MYERS, FL. 33966
PHONE: (239) 278-1199
FAX: (239) 278-3174

NO.	DATE	REVISION DESCRIPTION	BY

BANKS ENGINEERING
Professional Engineers, Planners, & Land Surveyors
Serving The State Of Florida

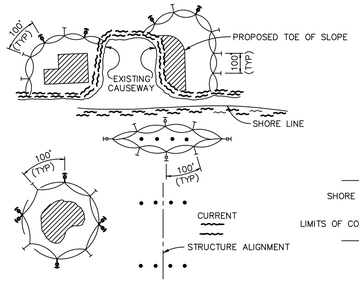
10511 SIX MILE CYPRESS PARKWAY
FORT MYERS, FLORIDA 33966
PHONE: (239) 939-5490 FAX: (239) 939-2523
ENGINEERING LICENSE # EB 6469
SURVEY LICENSE # LB 6690
WWW.BANKSENG.COM

DAVID R. UNDERHILL JR.
P.E. #47029

UTILITY DETAILS
IBIS LANDING - BETH STACEY BLVD.
LEE COUNTY, FLORIDA

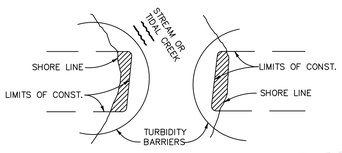
DATE	PROJECT	DRAWING	DESIGN	DRAWN	CHECKED	SCALE	SHEET
JUNE 2023	3142L	UTL-DT	JLW	JLW	DRU	NTS	11

S:\JOBS\31XX\3142L\ENGINEERING\DEVPLANS\BETH STACEY BLVD. L.D.O. PLANS\3142L-BLVD-11-UTL-DT.DWG 1/28/2026 1:38 PM JIMMY WHEWAY



LEGEND

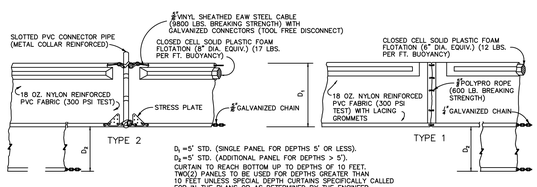
- PILE LOCATIONS
- DREDGE OR FILL AREA
- MOORING BUOY W/ANCHOR
- ANCHOR
- BARRIER MOVEMENT DUE TO CURRENT ACTION



NOTES:
1. TURBIDITY BARRIERS ARE TO BE USED IN ALL PERMANENT BODIES OF WATER REGARDLESS OF WATER DEPTH.
2. NUMBER AND SPACING OF ANCHORS DEPENDENT ON CURRENT VELOCITIES.
3. DEPLOYMENT OF BARRIER AROUND PILE LOCATIONS MAY VARY TO ACCOMMODATE CONSTRUCTION OPERATIONS.
4. NAVIGATION MAY REQUIRE SEGMENTING BARRIER DURING CONSTRUCTION OPERATIONS.
5. FOR ADDITIONAL INFORMATION SEE SECTION 104 OF THE STANDARD SPECIFICATIONS.

NOTE:
TURBIDITY BARRIERS FOR FLOWING STREAMS AND TIDAL CREEKS MAY BE EITHER FLOATING, OR STAKED TYPES OR ANY COMBINATIONS OF TYPES THAT WILL SUIT SITE CONDITIONS AND MEET EROSION CONTROL AND WATER QUALITY REQUIREMENTS. THE BARRIER TYPE(S) WILL BE AT THE CONTRACTORS OPTION UNLESS OTHERWISE SPECIFIED IN THE PLANS, HOWEVER PAYMENT WILL BE UNDER THE PAY ITEM(S) ESTABLISHED IN THE STAKED TURBIDITY BARRIER. POSTS IN STAKED PLANS FOR FLOATING TURBIDITY BARRIER AND/OR TURBIDITY BARRIERS TO BE INSTALLED IN VERTICAL POSITION UNLESS OTHERWISE DIRECTED BY THE ENGINEER.

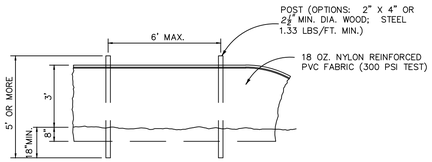
TURBIDITY BARRIER APPLICATION DETAIL
N.T.S.



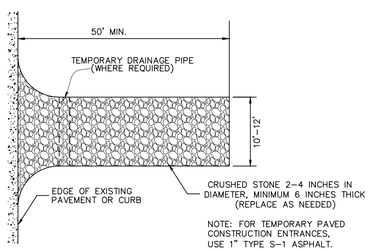
D_1 = 5' STD. (SINGLE PANEL FOR DEPTHS 5' OR LESS).
D_2 = 5' STD. (ADDITIONAL PANEL FOR DEPTHS > 5').
CURTAIN TO REACH BOTTOM UP TO DEPTHS OF 10 FEET, TWO(2) PANELS TO BE USED FOR DEPTHS GREATER THAN 10 FEET UNLESS SPECIAL DEPTH CURTAINS SPECIFICALLY CALLED FOR IN THE PLANS OR AS DETERMINED BY THE ENGINEER.

NOTICE: COMPONENTS OF TYPES ~ AND ~~ MAY BE SIMILAR OR IDENTICAL TO PROPRIETARY DESIGNS. ANY INFRINGEMENT ON THE PROPRIETARY RIGHTS OF THE DESIGNER SHALL BE THE SOLE RESPONSIBILITY OF THE USER. SUBSTITUTIONS FOR TYPES ~ AND ~~ SHALL BE AS APPROVED BY THE ENGINEER.

FLOATING TURBIDITY BARRIER DETAIL
N.T.S.

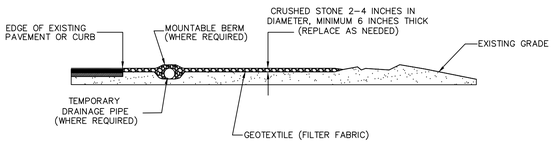


STAKED TURBIDITY BARRIER DETAIL
N.T.S.



NOTE: FOR TEMPORARY PAVED CONSTRUCTION ENTRANCES, USE 1" TYPE S-1 ASPHALT.

STABALIZED CONSTRUCTION ENTRANCE/EXIT PLAN VIEW
N.T.S.



STABALIZED CONSTRUCTION ENTRANCE/EXIT SECTION VIEW
N.T.S.

GENERAL NOTES

1. EROSION CONTROL DETAILS ARE APPLICABLE TO NEW CONSTRUCTION, RECONSTRUCTION AND RRR PROJECTS.
2. FOR SODDING ADJACENT TO DITCHES AND AT HEADWALLS, SEE INDEX NO. 281.
3. ALL FRONT SLOPES STEEPER THAN 3:1 ARE TO BE SODDED.



BETH STACY BLVD – 407+00.00

LOOKING SOUTH

BETH STACY BLVD – 410+50.00

LOOKING SOUTH

BETH STACY BLVD – 412+50.00

LOOKING SOUTH

BETH STACY BLVD – 407+50.00

LOOKING SOUTH

BETH STACY BLVD – 410+00.00

LOOKING SOUTH

BETH STACY BLVD – 412+00.00

LOOKING SOUTH

BETH STACY BLVD – 408+00.00

LOOKING SOUTH

BETH STACY BLVD – 409+00.00

LOOKING SOUTH

BETH STACY BLVD – 411+50.00

LOOKING SOUTH

BETH STACY BLVD – 408+50.00

LOOKING SOUTH

BETH STACY BLVD – 409+50.00

LOOKING SOUTH

BETH STACY BLVD – 411+00.00

LOOKING SOUTH

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BETH STACEY BLVD
CROSS SECTIONS (1)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's **below.** **Call** before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE AS TO THE ACCURACY OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND UNDERGROUND LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

DAVID R. UNDERHILL JR.

P.E. # 47029

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DU

PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
3142L	07/03/2025	N.A.V 1988

BOOK AND PAGE:

SHEET

13

\\VLFORT\Projects\Jobs\3142L\3142L\Engineering\DevPlans\3.0 Phase 2 Plans\3142-3.0-PH2-BETH STACEY BLVD SECTIONS.dwg, 9/9/2025 10:57:34 AM, NUNOTO (AURICULA, ATWELL, LLC

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BETH STACEY BLVD. CROSS SECTIONS (2)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's **below.** **Call** before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE AS TO THE ACCURACY OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL UTILITY LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

DAVID R. UNDERHILL JR.

P.E. # 47029

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DU

PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
3142L	07/03/2025	N.A.V 1988

BOOK AND PAGE:

SHEET

14

\\ATWORKS\Projects\Jobs\3142\3142L\Engineering\DwgFiles\3.0. Phase 2 Plans\3142-3.0.-PH2-BETH STACEY BLVD SECTIONS.dwg, 8/8/2025 10:57:37 AM, NUNCIO LAURICELLA, ATWELL, LLC

BETH STACY BLVD – 419+00.00

LOOKING SOUTH

BETH STACY BLVD – 421+00.00

LOOKING SOUTH

BETH STACY BLVD – 423+00.00

LOOKING SOUTH

BETH STACY BLVD – 419+50.00

LOOKING SOUTH

BETH STACY BLVD – 421+50.00

LOOKING SOUTH

BETH STACY BLVD – 423+50.00

LOOKING SOUTH

BETH STACY BLVD – 420+00.00

LOOKING SOUTH

BETH STACY BLVD – 422+00.00

LOOKING SOUTH

BETH STACY BLVD – 424+00.00

LOOKING SOUTH

BETH STACY BLVD – 420+50.00

LOOKING SOUTH

BETH STACY BLVD – 422+50.00

LOOKING SOUTH

BETH STACY BLVD – 424+50.00

LOOKING SOUTH

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BETH STACEY BLVD. CROSS SECTIONS (3)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's **below.** **Call** before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE AS TO THE ACCURACY OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL UTILITY LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

DAVID R. UNDERHILL JR.

P.E. # 47029

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DU

PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
3142L	07/03/2025	N.A.V 1988

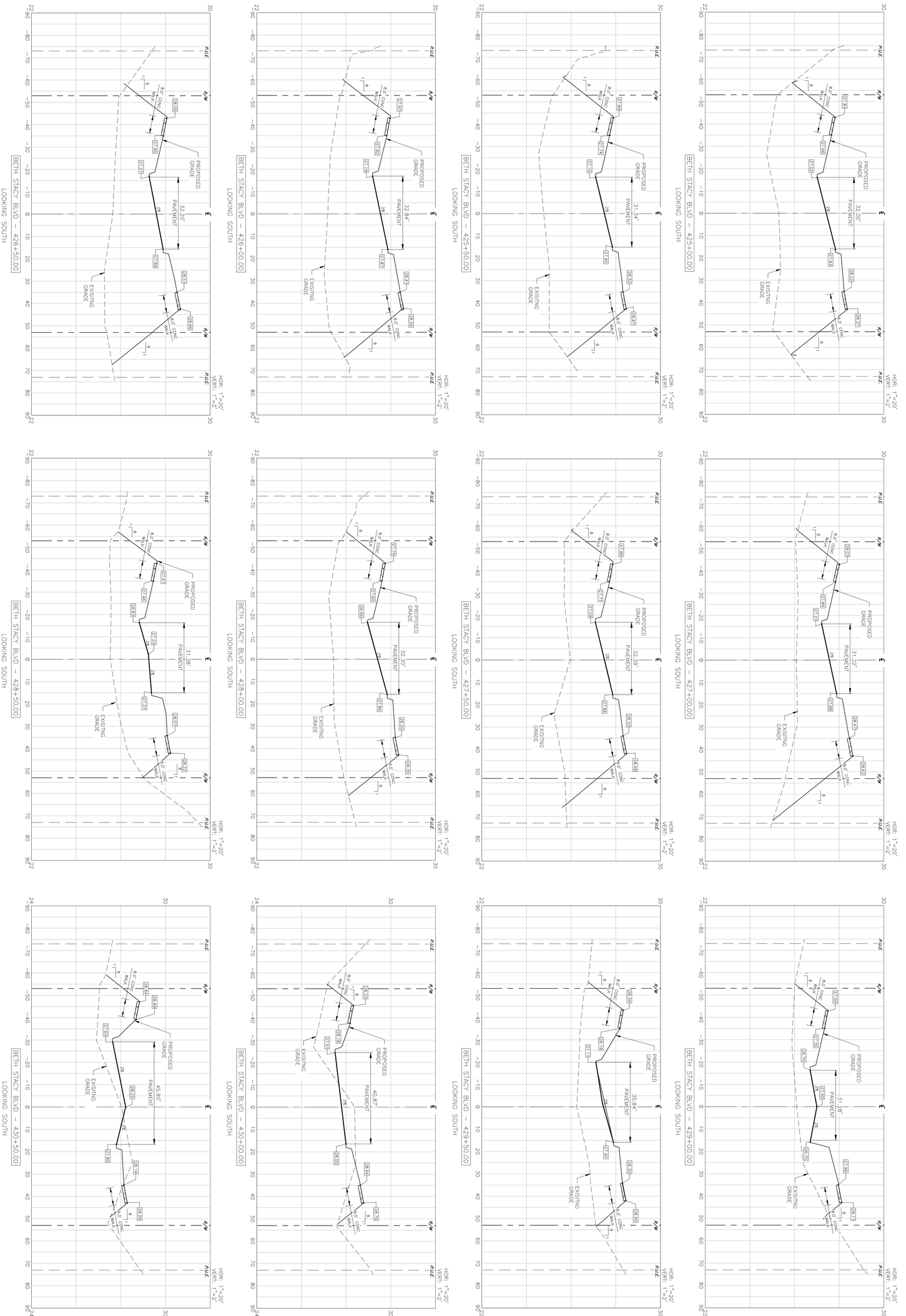
BOOK AND PAGE:

SHEET

15

\\ATWORKS\Projects\Lee\3142L\Engineering\Dwr\Plans\3142L - Phase 2 Plans\3142L-2-D-PH2-BETH STACEY BLVD SECTIONS.dwg, 8/8/2025 10:57:40 AM, NUNOTO LAURICELA, ATWELL, LLC

\\FLFOR2FS1\Projects\Jobs\31xx\3142\Engineering\DevPhase\D.O. Phase 2 Plans\3142-D.O.-PH2-BETH STACEY BLVD SECTIONS.dwg, 9/9/2025 10:57:45 AM, NUNOTO LAURICELLA, ATWELL, LLC



811
Know what's below.
Call before you dig.
ALL UTILITIES ARE SHOWN IN APPROXIMATE LOCATIONS. THE CONTRACTOR SHALL FIELD VERIFY THE LOCATIONS OF ALL UTILITIES PRIOR TO CONSTRUCTION AND SHALL PRESERVE AND PROTECT ALL UTILITIES FROM DAMAGE DURING CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

DAVID A. LINDEMUTH, JR.
P.E. # 67929

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DD
PROJECT NUMBER: 3142	COMPLETION DATE: 07/30/2025	SURVEY DATUM: NAD 83 1990
BOOK AND PAGE:		

SHEET 16

NO.	DATE	REVISION DESCRIPTION	BY

BETH STACEY BLVD.
CROSS SECTIONS (4)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR:

AQUABELLA DEVELOPMENT, LLC

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BETH STACEY BLVD. CROSS SECTIONS (5)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR:
AQUABELLA DEVELOPMENT, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE ARE FACTUAL AND CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

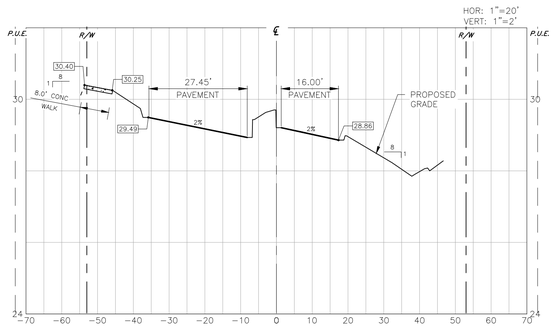
NOT RELEASED FOR CONSTRUCTION

DAVID R. UNDERHILL JR.
P.E. # 47029

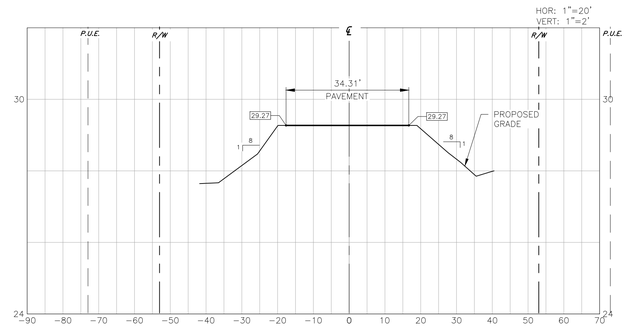
DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DU
PROJECT NUMBER: 3142L	COMPLETION DATE: 07/03/2025	SURVEY DATUM: N.A.V 1988
BOOK AND PAGE:		

SHEET
17

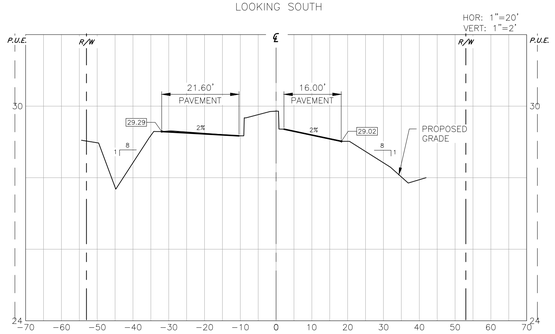
\\ATWORKS\Projects\Lakes\3142L\Engineering\DevPlans\D.O. Phase 2 Plans\3142-D.O.-PH2-BETH STACEY BLVD SECTIONS.dwg, 8/8/2025 10:07:46 AM, NUNO70 LAURIOLLA, ATWELL, LLC



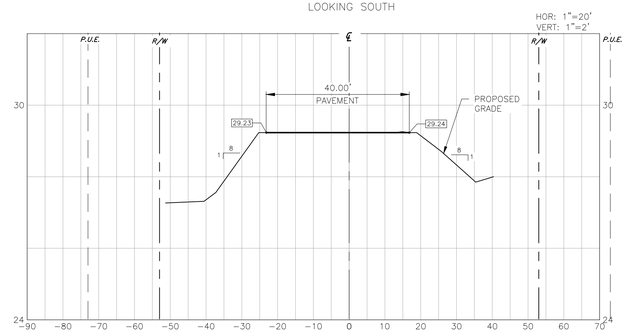
BETH STACY BLVD – 437+00.00
LOOKING SOUTH



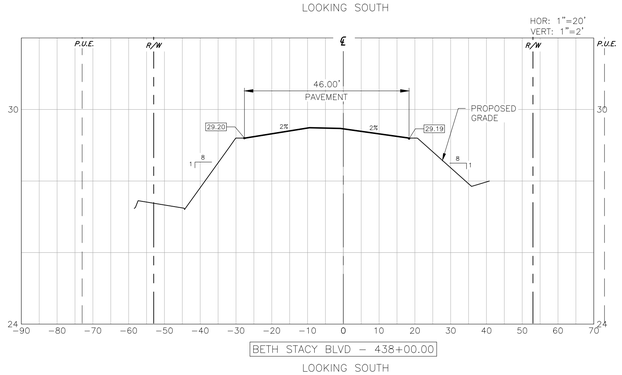
BETH STACY BLVD – 439+00.00
LOOKING SOUTH



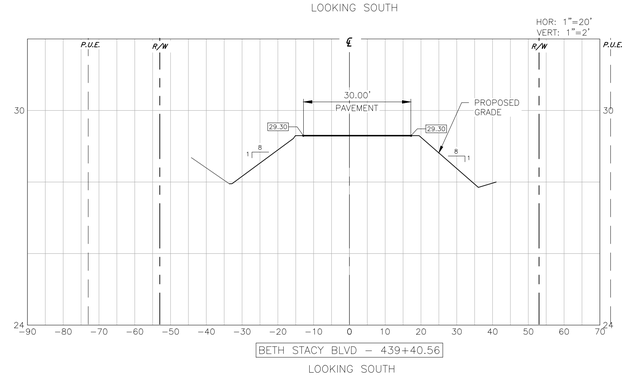
BETH STACY BLVD – 437+50.00
LOOKING SOUTH



BETH STACY BLVD – 438+50.00
LOOKING SOUTH



BETH STACY BLVD – 438+00.00
LOOKING SOUTH



BETH STACY BLVD – 439+40.56
LOOKING SOUTH

ATWELL
866.850.4200 www.atwell-group.com
10511 SIX MILE CYPRESS PKWY, SUITE101
FORT MYERS, FL 33966
239.939.5490

BETH STACEY BLVD.
CROSS SECTIONS (6)

IBIS LANDING - BETH STACEY BVLD

LEE COUNTY, FLORIDA

PREPARED FOR: AQUABELLA DEVELOPMENT, LLC

NO.	DATE	REVISION DESCRIPTION	BY

811 Know what's below. Call before you dig.

ALL UTILITIES ARE SHOWN BASED ON THE INFORMATION AVAILABLE TO THE ENGINEER. THERE IS NO GUARANTEE ALL FACILITIES ARE SHOWN OR THAT THE LOCATION, DEPTH, AND SIZE OF EACH FACILITY IS CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES AND SERVICE LINES PRIOR TO CONSTRUCTION.

NOT RELEASED FOR CONSTRUCTION

DAVID R. UNDERHILL JR.
P.E. # 47029

DRAFTED BY:	DESIGNED BY:	REVIEWED BY:
NL	NL	DU
PROJECT NUMBER:	COMPLETION DATE:	SURVEY DATUM:
3142L	07/03/2025	N.A.V 1988
BOOK AND PAGE:		

SHEET
18

ATWELL

DAVID R. UNDERHILL JR. LICENSE No. 47029 STATE OF FLORIDA PROFESSIONAL ENGINEER

IBIS LANDING (FKA COPPERHEAD)
WATER MANAGEMENT REPORT

PROJECT INFORMATION

This application is a request for a permit modification to the Ibis Landing Environmental Resource Permit (Permit Number 36-108291-P) for site plan revisions in the southeast corner of the project. Ibis Landing is a partially constructed 298.86-acre residential golf course project formerly known as Copperhead. The site is located along Beth Stacey Blvd, south of SW 23rd Street in Lehigh Acres, Lee County, Florida. The project is being developed by Lennar under the name Ibis Landing.

PROJECT BACKGROUND

On April 13, 2000, Permit Number 36-03765-P (Application Number 990415-2) was approved for construction and operation approval of a surface water management of a residential golf course development located on a 297.8-acre site known as Copperhead. The entire surface water management system, golf course and the first phase of residential development (approximately 100 lots) were constructed in 2001 and placed in service.

A new ERP (Permit Number 36-108291-P) was issued to reauthorize construction of the undeveloped residential areas in September 2023. The residential infrastructure for the western and northeastern areas of the project have been constructed over the last two years. Beth Stacy Blvd (Milwaukee Blvd) will be extended from north to south as part of the Ibis Landing project. This roadway will ultimately be owned by Lee County and will drain to the project's surface water management system.

SURFACE WATER MANAGEMENT SYSTEM

The existing onsite surface water management system consists of 9 primary surface water management basins which use the existing lakes and dry detention areas for treatment and storage. A 10th basin was proposed for the southern (offsite) portion of Beth Stacey Blvd that has not been constructed. The site is surrounded by Lehigh Acres Municipal Services Improvement District (LAMSID) Yellowtail Canal and Yellowtail Canal Extensions on the West, South and East sides. The project discharges to these canals which are part of the Orange River Watershed. The discharge rate is 30.1 cfm which is the allowable rate per LAMSID. No modifications to the previously permitted wetland preserves are proposed.

Stormwater calculations are only being provided for the basins that are being modified (Basins 2-5). No changes to the permitted control elevations are proposed. Per the permit, the eastern portion of the site (modified area) has a control elevation of 24.0' NGVD. The Ibis Landing Surface Water Management System provides the required water quality treatment and attenuation for the 25-year 3-day storm event. Water quality treatment volume is one inch plus an additional 50%. Water quality calculations are provided at the end of the calculations for the basins that discharge offsite (Basins 2).

The eastern portion of the site (Basins 1, 3, 4, 5 and 6) discharge to Basin 2, a 26.62 wetland which discharges to the east to the Yellowtail Canal. This permitted drainage pattern will be maintained. No modification to any weirs that discharge offsite are planned.

PROPOSED PROJECT MODIFICATIONS

This application proposes site plan revisions within Basins 3 and 4 in the southeast corner of the project. The existing lake in Basins 3 is being reshaped which results in a reduction in the lake size of 0.4 acres. However,

the existing lake in Basin 4 is being reshaped which results in a increase in the lake size of 0.3 acres. Basins 3 and Basin 4 are also connected to help equalize the stages in these basins. The outfall location in Basin 4 is being maintained and will operate as previously permitted/constructed. The outfall location is Basin 3 is being modified to accommodate the lake reshaping. The bleeder in Basin 3 was sized to account for both Basin 3 and Basin 4 water quality bleed down. Minor revisions to Beth Stacey Blvd are also proposed which consist of providing closed drainage (curb and gutter) for the roadway along with the addition of a second 8' concrete sidewalk along the roadway (which is within Basin 5). The proposed Basin 10, which provided storage for runoff from the southern portion of Beth Stacey Blvd will be eliminated and the roadway will drain to Basins 4 and 5 which increase the sizes of these basins. Approximately 250' offsite roadway will drain to the proposed roadway swales. Compensating water quality storage will be provided for this area of the offsite roadway within Basin 5.

WATER QUALITY

All basins provide the required water quality treatment prior to connecting to the onsite wetland or before discharging offsite. Water quality treatment is provided in the interconnected lakes and dry detention ponds throughout the site. The required water quality volume of 150% of the greater of one inch or 2.5" over the impervious areas is provided. Compensating water quality treatment in the amount of 2.5 inches times the offsite impervious area is provided for the offsite portion of Beth Stacey Blve.

Basin #	Total Area (AC)	Control Elev. (NGVD)	Sub-basins Included	Treatment Provided By	Final Outfall WCS
1	49.33	24.00	B1L1 & B1L2	Wet Detention	WCS 2
2	32.44	24.00	N/A	Dry Detention	WCS 2
3	16.53	24.00	N/A	Wet Detention	WCS 2
4	18.34	24.00	N/A	Wet Detention	WCS 2
5	131.83	24.50	B5L5, B5L6, B5L7, B5L9, B5L10, B5L11	Wet Detention	WCS 2
6	18.20	24.00	N/A	Dry Detention	WCS 2
7	9.40	25.50	N/A	Wet Detention	WCS 7
8	6.11	25.50	N/A	Dry Detention	WCS 8
9	16.68	25.50	N/A	Dry Detention	WCS 8

Basin	CE	Proposed Basin Area	Permitted WQ V (ac-ft)	Proposed Construction WQ V (ac-ft)
B1 (P1-A, P1-B, P2-A, P2-B, P2-C & P2-D)	24.00	49.33	3.81	N/A
B2 (WA)	24.00	32.44	2.97	N/A
B3 (P3)	24.00	16.53	1.82	2.07
B4 (P4)	24.00	18.34	1.76	2.51
B5 (P5, P6, P7, P8-A, P8-B, P9, P10, & P11)	24.50	131.83	23.00	N/A
B6 (WC-A, WC-B, & WC-C)	24.00	18.20	1.75	N/A
B7 (P13)	25.50	9.40	1.18	N/A
B8 (WD)	25.50	6.11	0.93	N/A
B9 (P12)	25.50	16.68	0.38	N/A
	Totals	**298.86**	**37.60**	**4.58**

The proposed project provides 150% of the 1 inch over the project area. 298.86 x 1.5 inches = ***37.36 ac-ft***

The proposed project provides 2.5 inches times the percentage of imperviousness for the offsite improvements. 0.22 ac x 2.5 inches = ***0.05 ac-ft***

Total Required = ***37.41 ac-ft***

Total Provided = ***42.18 ac-ft***

WATER QUANTITY AND MODEL

The project is divided into 9 Basins with Basin 1 and Basin 5 having multiple interconnected lakes/sub-basins. The land use, CN, and stage storage was calculated by sub-basin. An Interconnected Pond Routing model (ICPR 4) was created to simulate the flood routing for the existing SWM system design. The weirs used in the model for Basins 1, 4, 5, 6, & 9 are modeled as they exist. The weir between Lake 3 and the downstream Wetland is proposed to be constructed. This weir will provide pre-treatment for areas that connect to Wetlands. The weirs, for Basins 2, 7, and 8, that discharge offsite will operate as previously permitted/constructed to meet the design parameters of water quality, allowable bleed down, and allowable discharge for SFWMD.

The design elevations are as follows and the flood routing results are included for comparison in the tables on the next page.

	CONDITION	
	Permitted (App#230103-37076)	Proposed (Current)
BASIN	WA	B2
5-YR	24.68	24.66
10-YR	24.86	24.84
25-YR	25.94	25.97
100-YR	27.42	27.44
DISCHARGE	10.98	11.12

	CONDITION	
	Permitted (App#230103-37076)	Proposed (Current)
BASIN	P3	B3
5-YR	25.43	25.94
10-YR	25.61	26.07
25-YR	26.81	26.91
100-YR	27.42	27.44
DISCHARGE	N/A (Internal)	N/A

BASIN	P4	B4
5-YR	25.64	25.96
10-YR	25.97	26.09
25-YR	26.68	26.75
100-YR	27.42	27.44
DISCHARGE	N/A (Internal)	N/A (Internal)

BASIN	P5-P11,P14-P15	B5
5-YR	25.54	25.54
10-YR	25.87	25.87
25-YR	27.32	27.32
100-YR	27.94	27.95
DISCHARGE	N/A (Internal)	N/A

The next page contains a land use comparison between the total permitted land use from Permit No. 36-108291-P application 230103-37076 and the proposed modification.

LAND USE COMPARISON

	Permitted Total	% of Proposed
Basin	B2	
Building	0.00	0.0%
Impervious	0.16	0.5%
Pervious	7.15	22.0%
Dry Detentiion	0.44	1.4%
Wetland	24.69	76.1%
Total	**32.44**	100.0%
Basin	B3	
Building	3.24	19.6%
Impervious	4.59	27.8%
Pervious	6.83	41.3%
Lake	1.87	11.3%
Total	**16.53**	100.0%
Basin	B4	
Building	2.37	13.3%
Impervious	3.74	21.0%
Pervious	10.33	58.0%
Lake	1.37	7.7%
Total	**17.81**	100.0%
Basin	B5	
Building	16.24	12.4%
Impervious	24.18	18.5%
Pervious	71.75	54.8%
Dry Detention	0.07	0.1%
Lake	18.64	14.2%
Total	**130.88**	100.0%
Basin	B10	
Building	0.04	2.7%
Impervious	0.12	8.1%
Pervious	1.11	75.0%
Dry Detention	0.21	14.2%
Total	**1.48**	100.0%

Proposed Modification Total	% of Permitted
B2	
0.00	0.0%
0.16	0.5%
7.15	22.0%
0.44	1.4%
24.69	76.1%
32.44	100.0%
B3	
3.05	18.5%
4.71	28.5%
7.61	46.0%
1.16	7.0%
16.53	100.0%
B4	
1.36	7.4%
7.38	40.2%
8.16	44.5%
1.44	7.9%
18.34	100.0%
P5, P6, P7, P8-A, P8-B, P9, P10, & P11	
16.25	12.3%
24.73	18.8%
72.14	54.7%
0.07	0.1%
18.64	14.1%
131.83	100.0%

Basin 10 has been removed and added to Basins 4 and 5 as shown above.

0.53 ac of Basin 10 was added into Basin 4.

0.95 ac of Basin 10 was added into Basin 5.

OFFSITE DISCHARGES

CS	Contributing Basins	Contributing Area (AC)	Allowable Discharge* (cfs)	Model Discharge (cfs)
WCS-2	B1, B2, B3, B4, B5, & B6	266.67	12.54	11.12

* Based on 30.1 SCM

The project lies within the Lehigh Acres Municipal Services Improvement District which has an allowable discharge limit of 30.1 CSM. The proposed surface water management system for the 266.67-acre basin has an allowable discharge rate of 12.54 cfs. The total discharge is 11.12 cfs which is less than allowed.

TOTAL SITE

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	25.78	8.6%	0.00	0.00	0.00	25.78
LAKE BANK @ CE + 1	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE TOB	2.28	0.8%	2.28	0.00	0.00	0.00
DRY DETENTION	1.65	0.6%	1.65	0.00	0.00	0.00
PRESERVE	37.83	12.7%	37.83	0.00	0.00	0.00
ROW	32.59	10.9%	13.04	19.55	0.00	0.00
SF	63.92	21.4%	25.57	19.18	19.18	0.00
MF	25.26	8.5%	5.05	10.10	10.10	0.00
GOLF COURSE	96.62	32.3%	93.73	2.90	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	0.5%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	2.3%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**298.86**	**100%**	**185.38**	**55.01**	**32.56**	**25.92**

TOTAL PERVIOUS AREA	185.38	62.0%	
TOTAL IMPERVIOUS AREA	113.49	38.0%	*(INCLUDES WATER SURFACE AREA)*

TOTAL SITE

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 62.0% x 6.57 in.
S = 4.07 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 71

III. SURFACE STORAGE

Basin Area =	298.86	ac.			
Bldg. Coverage Area =	32.56	ac.			
Impervious =	55.01				
Lake Area (at Control)	25.92	ac.	***EL. =***	***24.00***	***(NGVD)***
Preserve Area (at control)	37.83	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less Preserved)=	147.55	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**25.92**	**0.00**	**0.00**	**0.00**	**25.92**	**0.00**
25.00	**28.06**	**0.00**	**0.00**	**0.00**	**28.06**	**26.99**
26.50	**30.34**	**0.00**	**0.00**	**0.00**	**30.34**	**70.79**
27.00	**32.62**	**7.57**	**29.51**	**27.51**	**97.20**	**102.68**
27.50	**32.93**	**15.13**	**59.02**	**49.51**	**156.59**	**166.12**
28.50	**32.93**	**18.92**	**132.79**	**55.01**	**239.65**	**364.25**

IV. WATER QUALITY DETENTION VOLUME

TOTAL SITE

Total Area =	298.86	ac.
Bldg. Coverage =	32.56	ac.
Water Surface Area =	25.92	ac.
Pervious Area =	185.38	ac.

First one (1) Inch of runoff over basin:

= 1" x 298.86 ac. x1'/12" = **24.91** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	240.38	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	55.01	ac.
% Impervious = Impervious Area / Site Area	22.9%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**13.01**	**ac-ft**
Required Water Quality Detention Volume	**24.91**	**ac-ft**
Additional 50% Water Quality Detention Volume	**12.45**	**ac-ft**
Total Required Water Quality Detention Volume	**37.36**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**37.36**	**ac-ft**

Between El. 24.00 and 25.36

WCS Stage-Storage Table

Stage	Site Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	25.92	0.00
25.00	28.06	26.99
26.50	30.34	70.79
27.00	97.20	102.68
27.50	156.59	166.12
28.50	239.65	364.25

Water Quality Min. Weir Elevation = **25.36** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event:

= 55.0 CSM x WCS Area x 1 SM/640 ac.

= **25.68** **cfs**

METHOD OF DETERMINATION:		
Orange River Watershed (Approved ERP Appl # 060503-10)	55.0	CSM

Basin B2

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 1	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE TOB	0.00	0.0%	0.00	0.00	0.00	0.00
DRY DETENTION	0.44	1.4%	0.44	0.00	0.00	0.00
PRESERVE	26.62	82.1%	26.62	0.00	0.00	0.00
ROW	0.00	0.0%	0.00	0.00	0.00	0.00
SF	0.00	0.0%	0.00	0.00	0.00	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	5.38	16.6%	5.22	0.16	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**32.44**	**100%**	**32.28**	**0.16**	**0.00**	**0.00**

TOTAL PERVIOUS AREA	32.28	99.5%
TOTAL IMPERVIOUS AREA	0.16	0.5%

Basin B2

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** in.

Available Soil Storage:
S = (% of Pervicus Area) * (Soil Storage Depth)
S = 99.5% x 6.57 in.
S = 6.53 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 60

III. SURFACE STORAGE

Basin Area =	32.44	ac.			
Bldg. Coverage Area =	0.00	ac.			
Dry Detention TOS (at CE + 1)	0.44	ac.	*EL. =*	***25.00***	***(NGVD)***
Dry Detention TOB (at CE + 2)	0.54	ac.	*EL. =*	***26.00***	***(NGVD)***
Golf Course	5.22	ac.			
Preserve Area (at control)	26.62	ac.	*EL. =*	***24.00***	***(NGVD)***
Pervious Area =	32.28	ac.			

Stage-Storage Table

Stage	Dry Detention	Preserve Area	Golf Course	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac-ft
24.00	0.00	26.62	0.00	26.62	0.00
25.00	0.44	26.62	0.00	27.06	26.84
26.50	0.44	26.62	1.04	28.10	68.21
27.00	0.44	26.62	2.09	29.15	82.53
27.50	0.44	26.62	2.61	29.67	97.23

IV. WATER QUALITY DETENTION VOLUME
The permitted 2.97 ac-ft of water quality was provided in the existing condition and no modification to the basin is proposed.

V. ALLOWABLE DISCHARGE

BASIN	**2**		
Contributing Flows			
Basin	1	49.33	
Basin	2	32.44	
Basin	3	16.53	
Basin	4	18.34	
Basin	5	131.83	
Basin	6	18.20	
Total Area =		266.68	ac.

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event.

=	55.0	CSM x WCS Area x 1 SM/640 ac.
=	**22.92**	**cfs**

METHOD OF DETERMINATION:		
55 CSM allowed in Orange River Watershed & Approved ERP Appl # 934015-2	55.0	CSM

VI. WEIR DESIGN

Total Basin

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**266.68 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**11.11 Ac-FT**
½" Discharge Rate:	=		=	**5.72 CFS**
Water Quality Stage:	=		=	**24.56 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**22.22 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation)	=	**24.23 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**24.39 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)	=	**0.39 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 5.72 **CFS**

Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 7.39 Length of weir **A 1.50 ft wide rectangular weir was used to meet the maximum discharge rate**

Basin B3L3

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.16	7.0%	0.00	0.00	0.00	1.16
LAKE BANK @ CE + 1	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE TOB	0.14	0.8%	0.14	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	2.62	15.8%	1.05	1.57	0.00	0.00
SF	5.83	35.3%	2.33	1.75	1.75	0.00
MF	3.25	19.7%	0.65	1.30	1.30	0.00
GOLF COURSE	3.26	19.7%	3.16	0.10	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.53**	**100%**	**7.61**	**4.72**	**3.05**	**1.16**

TOTAL PERVIOUS AREA	7.61	46.0%	
TOTAL IMPERVIOUS AREA	8.93	54.0%	*(INCLUDES WATER SURFACE AREA)*

Basin B3L3

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:

= (Average Developed Grade)-(Wet Season Water Table)

= 27.50 - 24.00

= 3.50 ft.

Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:

S = (% of Pervious Area) * (Soil Storage Depth)

S = 46.0% x 6.57 in.

S = 3.02 in.

Curve Number:

Cn = 1000/(S + 10)

Cn = 77

III. SURFACE STORAGE

Basin Area =	16.53	ac.			
Bldg. Coverage Area =	3.05	ac.			
Lake Area (at Control)	1.16	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.30	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.44	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.57	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	3.16	ac.			
Pervious Area (less bank & GC) =	4.03	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.16**	**0.00**	**0.00**	**0.00**	**1.16**	**0.00**
25.00	**1.30**	**0.00**	**0.00**	**0.00**	**1.30**	**1.23**
26.50	**1.44**	**0.00**	**0.00**	**0.00**	**1.44**	**3.28**
27.00	**1.57**	**0.63**	**0.81**	**2.36**	**5.37**	**4.98**
27.50	**1.57**	**1.26**	**1.61**	**4.25**	**8.70**	**8.50**
28.50	**1.57**	**1.58**	**3.63**	**4.72**	**11.50**	**18.60**

IV. WATER QUALITY DETENTION VOLUME

BASIN	**3**		
Total Area =		16.53	ac.
Bldg. Coverage =		3.05	ac.
Water Surface Area =		1.16	ac.
Pervious Area =		7.61	ac.

First one (1) Inch of runoff over basin:

= 1" x	16.53	ac. x1'/12" =	**1.38**	**ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	12.32	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	4.72	ac.
% Impervious = Impervious Area / Site Area	38.3%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.23**	**ac-ft**
Required Water Quality Detention Volume	**1.38**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.69**	**ac-ft**
Total Required Water Quality Detention Volume	**2.07**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.07**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.16	0.00
25.00	1.30	1.23
26.50	1.44	3.28
27.00	1.57	4.03
27.50	1.57	4.82
28.50	1.57	6.39

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.16 ac + 1.44 ac (Lake 3 and Lake 4) = 2.60 ac, the WQ is provided at 25.76 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin (The required bleed down for B4L4 is accounted for in this basin)

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**34.88 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**1.45 Ac-FT**
½" Discharge Rate:	=		=	**0.75 CFS**
Water Quality Stage:	=		=	**25.76 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)=		**3.12 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation) =		**24.07 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2 =		**24.92 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)=		**0.92 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 0.75 **CFS**

Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 0.27 Length of weir **A 0.27 ft wide rectangular weir was used to meet the maximum bleeder rate**

Basin B4L4

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.44	7.9%	0.00	0.00	0.00	1.44
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	9.88	53.9%	3.95	5.93	0.00	0.00
SF	1.42	7.7%	0.57	0.43	0.43	0.00
MF	2.34	12.8%	0.47	0.94	0.94	0.00
GOLF COURSE	2.87	15.6%	2.78	0.09	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**18.34**	**100%**	**8.16**	**7.38**	**1.36**	**1.44**

TOTAL PERVIOUS AREA	8.16	44.5%	
TOTAL IMPERVIOUS AREA	10.18	55.5%	*(INCLUDES WATER SURFACE AREA)*

Basin B4L4

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** in.

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 44.5% x 6.57 in.
S = 2.92 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 77

III. SURFACE STORAGE

Basin Area =	18.34	ac.			
Bldg. Coverage Area =	1.36	ac.			
Lake Area (at Control)	1.44	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.57	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.70	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.83	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	0.00	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less bank & GC) =	4.99	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.44**	**0.00**	**0.00**	**0.00**	**1.44**	**0.00**
25.00	**1.57**	**0.00**	**0.00**	**0.00**	**1.57**	**1.51**
26.50	**1.70**	**0.00**	**0.00**	**0.00**	**1.70**	**3.96**
27.00	**1.83**	**0.56**	**1.00**	**3.69**	**7.07**	**6.15**
27.50	**1.83**	**1.11**	**2.00**	**6.64**	**11.58**	**10.82**
28.50	**1.83**	**1.39**	**4.49**	**7.38**	**15.09**	**24.15**

IV. WATER QUALITY DETENTION VOLUME

BASIN **4**

Total Area =	18.34	ac.
Bldg. Coverage =	1.36	ac.
Water Surface Area =	1.44	ac.
Pervious Area =	8.16	ac.

First one (1) Inch of runoff over basin:

= 1" x 18.34 ac. x1'/12" = **1.53** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	15.54	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	7.38	ac.
% Impervious = Impervious Area / Site Area	47.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.67**	**ac-ft**
Required Water Quality Detention Volume	**1.67**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.84**	**ac-ft**
Total Required Water Quality Detention Volume	**2.51**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.51**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.44	0.00
25.00	1.57	1.51
26.50	1.70	3.96
27.00	1.83	4.84
27.50	1.83	5.76
28.50	1.83	7.59

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.16 ac + 1.44 ac (Lake 3 and Lake 4) = 2.60 ac, the WQ is provided at 25.76 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

The Existing weir, as approved in ERP 36-03765-P (App 990415-2) was utilized.

Basin B5L5

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.61	15.7%	0.00	0.00	0.00	2.61
LAKE BANK @ CE + 1	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE TOB	0.23	1.4%	0.23	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	0.66	4.0%	0.26	0.40	0.00	0.00
SF	2	12.1%	0.80	0.60	0.60	0.00
MF	3.99	24.1%	0.80	1.60	1.60	0.00
GOLF COURSE	6.62	39.9%	6.42	0.20	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.58**	**100%**	**8.98**	**2.79**	**2.20**	**2.61**

TOTAL PERVIOUS AREA	8.98	54.2%	
TOTAL IMPERVIOUS AREA	7.60	45.8%	*(INCLUDES WATER SURFACE AREA)*

Basin B5L5

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 54.2% x 6.57 in.
S = 3.56 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	16.58	ac.			
Bldg. Coverage Area =	2.20	ac.			
Lake Area (at Control)	2.61	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.84	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.08	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	3.31	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	6.62	ac.			
Pervious Area (less bank & GC) =	1.86	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.61**	**0.00**	**0.00**	**0.00**	**2.61**	**0.00**
25.50	**2.84**	**0.00**	**0.00**	**0.00**	**2.84**	**2.73**
27.00	**3.08**	**0.00**	**0.00**	**0.00**	**3.08**	**7.17**
27.50	**3.31**	**1.32**	**0.37**	**1.40**	**6.40**	**9.54**
28.00	**3.31**	**2.65**	**0.74**	**2.51**	**9.22**	**13.44**
29.00	**3.31**	**3.31**	**1.68**	**2.79**	**11.09**	**23.60**

Lake B5L6

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.23	9.6%	0.00	0.00	0.00	1.23
LAKE BANK @ CE + 1	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE TOB	0.16	1.2%	0.16	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	1.40	10.9%	0.56	0.84	0.00	0.00
SF	5.62	43.7%	2.25	1.69	1.69	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	4.14	32.2%	4.01	0.12	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**12.86**	**100%**	**7.29**	**2.65**	**1.69**	**1.23**

TOTAL PERVIOUS AREA	7.29	56.7%	
TOTAL IMPERVIOUS AREA	5.57	43.3%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L6

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 56.7% x 6.57 in.
S = 3.72 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 73

III. SURFACE STORAGE

Basin Area =	12.86	ac.			
Bldg. Coverage Area =	1.69	ac.			
Lake Area (at Control)	1.23	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	0.16	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	0.16	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	1.39	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.01	ac.			
Pervious Area (less bank & GC) =	2.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	1.23	0.00	0.00	0.00	1.23	0.00
25.50	1.31	0.00	0.00	0.00	1.31	1.27
27.00	1.39	0.00	0.00	0.00	1.39	3.29
27.50	1.39	0.80	0.56	1.33	4.08	4.66
28.00	1.39	1.61	1.12	2.39	6.50	7.30
29.00	1.39	2.01	2.53	2.65	8.57	14.84

Lake B5L7

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	9.70	20.4%	0.00	0.00	0.00	9.70
LAKE BANK @ CE + 1	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE TOB	0.71	1.5%	0.71	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	3.50	7.4%	1.40	2.10	0.00	0.00
SF	7.01	14.8%	2.80	2.10	2.10	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	18.16	38.2%	17.61	0.54	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	14.7%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**47.51**	**100%**	**25.36**	**7.55**	**4.90**	**9.70**

TOTAL PERVIOUS AREA	25.36	53.4%	
TOTAL IMPERVIOUS AREA	22.15	46.6%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L7

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.4% x 6.57 in.
S = 3.50 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	47.51	ac.			
Bldg. Coverage Area =	4.90	ac.			
Lake Area (at Control)	9.70	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	10.41	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	11.13	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	11.84	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	18.16	ac.			
Pervious Area (less bank & GC) =	5.60	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**9.70**	**0.00**	**0.00**	**0.00**	**9.70**	**0.00**
25.50	**10.41**	**0.00**	**0.00**	**0.00**	**10.41**	**10.06**
27.00	**11.13**	**0.00**	**0.00**	**0.00**	**11.13**	**26.21**
27.50	**11.84**	**3.63**	**1.12**	**3.77**	**20.37**	**34.09**
28.00	**11.84**	**7.26**	**2.24**	**6.79**	**28.14**	**46.21**
29.00	**11.84**	**9.08**	**5.04**	**7.55**	**33.51**	**77.04**

Lake B5L9

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.49	11.4%	0.00	0.00	0.00	2.49
LAKE BANK @ CE + 1	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE TOB	0.28	1.3%	0.28	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.60	11.9%	1.04	1.56	0.00	0.00
SF	6.92	31.6%	2.77	2.08	2.08	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	9.06	41.3%	8.79	0.27	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**21.92**	**100%**	**13.45**	**3.91**	**2.08**	**2.49**

TOTAL PERVIOUS AREA	13.45	61.3%	
TOTAL IMPERVIOUS AREA	8.47	38.7%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L9

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 61.3% x 6.57 in.
S = 4.03 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 71

III. SURFACE STORAGE

Basin Area =	21.92	ac.			
Bldg. Coverage Area =	2.08	ac.			
Lake Area (at Control)	2.49	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.77	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.05	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	2.77	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	9.06	ac.			
Pervious Area (less bank & GC) =	3.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.49**	**0.00**	**0.00**	**0.00**	**2.49**	**0.00**
25.50	**2.77**	**0.00**	**0.00**	**0.00**	**2.77**	**2.63**
27.00	**3.05**	**0.00**	**0.00**	**0.00**	**3.05**	**7.00**
27.50	**3.05**	**1.81**	**0.76**	**1.95**	**7.58**	**9.66**
28.00	**3.05**	**3.63**	**1.52**	**3.52**	**11.72**	**14.49**
29.00	**3.05**	**4.53**	**3.43**	**3.91**	**14.92**	**27.81**

Lake B5L10

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.42	9.3%	0.00	0.00	0.00	1.42
LAKE BANK @ CE + 1	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE TOB	0.17	1.1%	0.17	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	0.61	4.0%	0.24	0.37	0.00	0.00
SF	1.37	9.0%	0.55	0.41	0.41	0.00
MF	4.10	26.9%	0.82	1.64	1.64	0.00
GOLF COURSE	5.85	38.5%	5.68	0.18	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	9.0%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**15.22**	**100%**	**8.06**	**3.07**	**2.53**	**1.56**

TOTAL PERVIOUS AREA	8.06	53.0%	
TOTAL IMPERVIOUS AREA	7.16	47.0%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L10

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.0% x 6.57 in.
S = 3.48 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	15.22	ac.			
Bldg. Coverage Area =	2.53	ac.			
Lake Area (at Control)	1.56	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.72	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.89	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.92	ac.	***EL. =***	***26.50***	***(NGVD)***
Golf Course	5.68	ac.			
Pervious Area (less bank & GC) =	1.89	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.56**	**0.00**	**0.00**	**0.00**	**1.56**	**0.00**
25.50	**1.72**	**0.00**	**0.00**	**0.00**	**1.72**	**1.64**
27.00	**1.89**	**0.00**	**0.00**	**0.00**	**1.89**	**4.35**
27.50	**1.92**	**1.14**	**0.38**	**1.54**	**4.96**	**6.06**
28.00	**1.92**	**2.27**	**0.75**	**2.76**	**7.71**	**9.23**
29.00	**1.92**	**2.84**	**1.70**	**3.07**	**9.52**	**17.84**

Lake B5L11

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.05	5.9%	0.00	0.00	0.00	1.05
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.94	16.6%	1.18	1.76	0.00	0.00
SF	6.61	37.3%	2.64	1.98	1.98	0.00
MF	2.20	12.4%	0.44	0.88	0.88	0.00
GOLF COURSE	4.55	25.6%	4.41	0.14	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**17.74**	**100%**	**9.06**	**4.76**	**2.86**	**1.05**

TOTAL PERVIOUS AREA	9.06	51.1%	
TOTAL IMPERVIOUS AREA	8.68	48.9%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L11

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 51.1% x 6.57 in.
S = 3.35 In.

Curve Number:
Cn = 1000/(S + 10)
Cn = 75

III. SURFACE STORAGE

Basin Area =	17.74	ac.			
Bldg. Coverage Area =	2.86	ac.			
Lake Area (at Control)	1.05	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.18	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.31	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.44	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.41	ac.			
Pervious Area (less bank & GC) =	4.26	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.05**	**0.00**	**0.00**	**0.00**	**1.05**	**0.00**
25.50	**1.18**	**0.00**	**0.00**	**0.00**	**1.18**	**1.12**
27.00	**1.31**	**0.00**	**0.00**	**0.00**	**1.31**	**2.98**
27.50	**1.44**	**0.88**	**0.85**	**2.38**	**5.56**	**4.70**
28.00	**1.44**	**1.76**	**1.70**	**4.29**	**9.20**	**8.39**
29.00	**1.44**	**2.21**	**3.83**	**4.76**	**12.25**	**19.11**

IV. WATER QUALITY DETENTION VOLUME

BASIN **5**

Total Area =	131.83	ac.
Bldg. Coverage =	16.25	ac.
Water Surface Area =	18.64	ac.
Pervious Area =	72.21	ac.

First one (1) Inch of runoff over basin:

= 1" x 131.83 ac. x1'/12" = **10.99** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	96.94	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	24.73	ac.
% Impervious = Impervious Area / Site Area	25.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**6.02**	**ac-ft**
Required Water Quality Detention Volume	**10.99**	**ac-ft**
Additional 50% Water Quality Detention Volume	**5.49**	**ac-ft**

Beth Stacy Offsite Imperviousness

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	0.22	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	0.22	ac.
% Impervious = Impervious Area / Site Area	100.0%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**0.05**	**ac-ft**
Total Required Water Quality Detention Volume	**16.52**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**16.52**	**ac-ft**

Between El. 24.00 and 25.35

Stage-Storage Table for Water Quality Calculations

Stage (NGVD)	Lake Storage Area ac	Lake Storage Volume ac-ft
24.50	18.64	0.00
25.50	20.24	19.44
27.00	21.85	51.01
27.50	22.95	62.20
28.00	22.95	73.68
29.00	22.95	96.63

23 ac-ft of total wq volume is provided

Water Quality Min. Weir Elevation = **25.35** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**131.83 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**5.49 Ac-FT**
½" Discharge Rate:	=		=	**2.83 CFS**
Water Quality Stage:	=		=	**25.67 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**17.51 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	eatment Vol. After 24 Hr. / Total Basin Stora (+ Control Elevation)	=	**24.84 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**25.26 Ft-NAVD**
Average Height During Bleed Down:	=	Ave. Elev. During Bleed Down - Control Elev.	=	**0.76 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 2.83 **CFS**

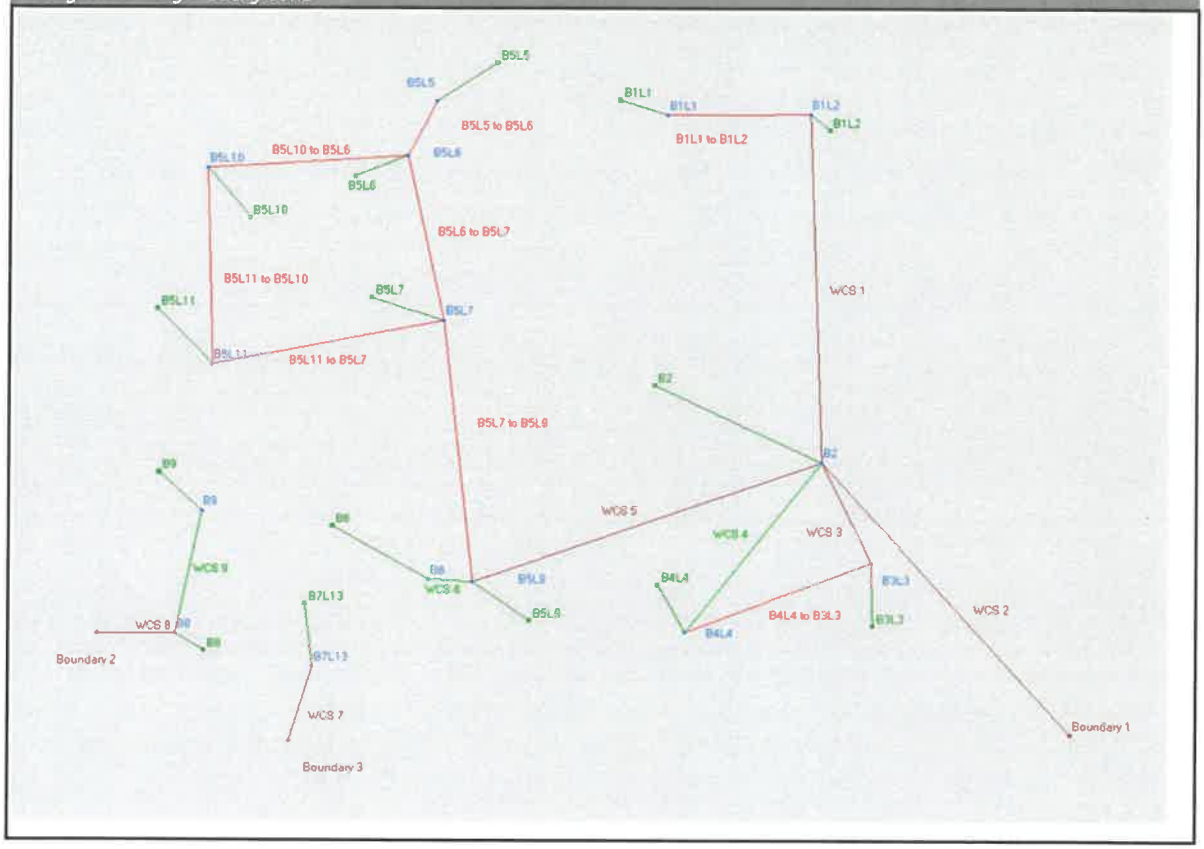
Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 1.37 Length of weir **A 1.33 ft wide rectangular weir was used to meet the maximum bleeder rate**

Background Image: Background



Impervious: Scenario 1 [Set]

Land Cover Zone	% Impervious	% DCIA	% Direct	Ia/S [dec]	Ia Impervious [in]	Ia Pervious [in]
Impervious & Pervious	0.00	0.00	0.00	0.00	0.000	0.000

Curve Number: Scenario 1 [Set]

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 60	60.0
Impervious & Pervious	Calculated CN 63	63.0
Impervious & Pervious	Calculated CN 64	64.0
Impervious & Pervious	Calculated CN 70	70.0
Impervious & Pervious	Calculated CN 71	71.0
Impervious & Pervious	Calculated CN 72	72.0

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 73	73.0
Impervious & Pervious	Calculated CN 74	74.0
Impervious & Pervious	Calculated CN 75	75.0
Impervious & Pervious	Calculated CN 76	76.0
Impervious & Pervious	Calculated CN 77	77.0

Manual Basin: B2

Scenario: Scenario1
Node: B2
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 60.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 32.4400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
32.4400	Impervious & Pervious	Calculated CN 60	

Comment:

Manual Basin: B3L3

Scenario: Scenario1
Node: B3L3
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5300 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5300	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B4L4

Scenario: Scenario1
Node: B4L4

Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 18.3400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
18.3400	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B5L10

Scenario: Scenario1
Node: B5L10
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 15.2200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
15.2200	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L11

Scenario: Scenario1
Node: B5L11
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 17.7400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
17.7400	Impervious & Pervious	Calculated CN 75	

Comment:

Manual Basin: B5L5

Scenario: Scenario1
Node: B5L5
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5800 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5800	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L6

Scenario: Scenario1
Node: B5L6
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 12.8600 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
12.8600	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L7

Scenario: Scenario1
Node: B5L7
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 47.5100 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
47.5100	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L9

Scenario: Scenario1
Node: B5L9
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 21.9200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
21.9200	Impervious & Pervious	Calculated CN 71	

Comment:

Node: B2

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	26.6200	1159567
25.00	27.0600	1178734
26.50	28.1000	1224036
27.00	29.1500	1269774
27.50	29.6700	1292425

Comment:

Node: B3L3

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft

Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.1600	50530
25.00	1.3000	56628
26.50	1.4400	62726
27.00	5.3700	233917
27.50	8.7000	378972
28.50	11.5000	500940

Comment:

Node: B4L4

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.4400	62726
25.00	1.5700	68389
26.50	1.7000	74052
27.00	7.0700	307969
27.50	11.5800	504425
28.50	15.0900	657320

Comment:

Node: B5L10

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.5600	67954
25.50	1.7200	74923
27.00	1.8900	82328
27.50	4.9600	216058
28.00	7.7100	335848
29.00	9.5200	414691

Comment:

Node: B5L11

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.0500	45738
25.50	1.1800	51401
27.00	1.3100	57064
27.50	5.5600	242194
28.00	9.2000	400752
29.00	12.2500	533610

Comment:

Node: B5L5

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.6100	113692
25.50	2.8400	123710
27.00	3.0800	134165
27.50	6.4000	278784
28.00	9.2200	401623
29.00	11.0900	483080

Comment:

Node: B5L6

Scenario: Scenario1
Type: Stage/Area

Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.2300	53579
25.50	1.3100	57064
27.00	1.3900	60548
27.50	4.0800	177725
28.00	6.5000	283140
29.00	8.5700	373309

Comment:

Node: B5L7

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	9.7000	422532
25.50	10.4100	453460
27.00	11.1300	484823
27.50	20.3700	887317
28.00	28.1400	1225778
29.00	33.5100	1459696

Comment:

Node: B5L9

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.4900	108464
25.50	2.7700	120661
27.00	3.0500	132858

Stage [ft]	Area [ac]	Area [ft2]
27.50	7.5800	330185
28.00	11.7200	510523
29.00	14.9200	649915

Comment:

Node: Boundary 1

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 23.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	23.00
0	0	0	60.0000	24.00
0	0	0	80.0000	25.00
0	0	0	100.0000	24.50
0	0	0	115.0000	23.30
0	0	0	336.0000	23.00

Comment:

Pipe Link: B4L4 to B3L3

Scenario: Scenario1
From Node: B3L3
To Node: B4L4
Link Count: 1
Flow Direction: Both
Damping: 0.0000 ft
Length: 731.00 ft
FHWA Code: 0
Entr Loss Coef: 1
Exit Loss Coef: 1
Bend Loss Coef: 0
Bend Location: 0.00 dec
Energy Switch: Energy

Upstream		Downstream	
Invert:	21.00 ft	Invert:	21.00 ft
Manning's N:	0.0130	Manning's N:	0.0130
Geometry:	Circular	Geometry:	Circular
Max Depth:	3.00 ft	Max Depth:	3.00 ft
Bottom Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000
Top Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L10 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.70 ft	Invert:	20.60 ft
From Node:	B5L10	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry:	Circular	Geometry:	Circular
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	450.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L10		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.15 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L10	Geometry:	Circular	Geometry:	Circular
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	320.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	21.00 ft	Invert:	20.50 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry:	Circular	Geometry:	Circular
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	430.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	1	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			

Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L5 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft
From Node:	B5L5	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	385.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L6 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.40 ft
From Node:	B5L6	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	220.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L7 to B5L9		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft

From Node:	B5L7	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L9	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.50 ft	Max Depth:	2.50 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	585.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Drop Structure Link: WCS 2		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B2	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	Boundary 1	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	65.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Sharp Crested Vertical	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	24.00 ft	Ref Node:	
Control Elevation:	24.00 ft	Discharge Coefficients	
Max Depth:	999.00 ft	Weir Default:	3.200
Max Width:	1.50 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600

Orifice Table:

Weir Comment:

Drop Structure Comment:

Drop Structure Link: WCS 3		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B3L3	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B2	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	237.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Sharp Crested Vertical	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	24.00 ft	Ref Node:	
Control Elevation:	24.00 ft	Discharge Coefficients	
Max Depth:	0.25 ft	Weir Default:	3.200
Max Width:	0.27 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component		Bottom Clip	
Weir:	2	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Horizontal		

Geometry Type:	Rectangular
Invert:	25.76 ft
Control Elevation:	25.76 ft
Max Depth:	3.00 ft
Max Width:	5.00 ft
Fillet:	0.00 ft

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	3.200
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Drop Structure Comment:

Weir Link: WCS 4

Scenario:	Scenario1
From Node:	B4L4
To Node:	B2
Link Count:	1
Flow Direction:	Both
Damping:	0.0000 ft
Weir Type:	Broad Crested Vertical
Geometry Type:	Rectangular
Invert:	26.00 ft
Control Elevation:	26.00 ft
Max Depth:	999.00 ft
Max Width:	30.00 ft
Fillet:	0.00 ft

Bottom Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Top Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	2.800
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Comment:

Simulation: 100yr3d

Scenario:	Scenario1
Run Date/Time:	9/18/2025 10:11:53 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	216.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr

Ia/S: 0.20 dec

Smp/Man Basin Rain Opt: Global

Rainfall Name: ~SFWMD-72
Rainfall Amount: 14.20 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 10yr 1d

Scenario: Scenario1
Run Date/Time: 9/23/2025 3:10:07 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec

IA Recovery Time: 12.0000 hr

Ia/S: 0.20 dec

dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FLMOD
		Rainfall Amount:	6.00 in
		Storm Duration:	24.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 25yr3d

Scenario: Scenario1
Run Date/Time: 9/23/2025 3:10:16 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:	
Unit Hydrograph Folder:	

Lookup Tables

Boundary Stage Set:	
Extern Hydrograph Set:	
Curve Number Set:	Scenario 1
Green-Ampt Set:	
Vertical Layers Set:	
Impervious Set:	Scenario 1

Tolerances & Options

Time Marching:	SAOR
Max Iterations:	6
Over-Relax Weight Fact:	0.5 dec
dZ Tolerance:	0.0010 ft
Max dZ:	1.0000 ft
Link Optimizer Tol:	0.0001 ft
IA Recovery Time:	24.0000 hr
Ia/S:	0.20 dec
Smp/Man Basin Rain Opt:	Global
Rainfall Name:	~SFWMD-72
Rainfall Amount:	10.20 in
Storm Duration:	72.0000 hr
Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (1D):	100 ft2
Energy Switch (1D):	Energy

Comment:

Simulation: 5yr1d

Scenario:	Scenario1
Run Date/Time:	9/23/2025 3:10:43 PM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 12.0000 hr

Ia/S: 0.20 dec

Smp/Man Basin Rain Opt: Global

Rainfall Name: ~FLMOD
Rainfall Amount: 5.20 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	10yr 1d	B2	24.84	42.0862
Scenario1	10yr 1d	B3L3	26.07	13.1869
Scenario1	10yr 1d	B4L4	26.09	13.2166
Scenario1	10yr 1d	B5L10	25.86	13.0674
Scenario1	10yr 1d	B5L11	25.87	12.8033
Scenario1	10yr 1d	B5L5	25.73	15.8862
Scenario1	10yr 1d	B5L6	25.72	24.0696
Scenario1	10yr 1d	B5L7	25.71	24.1362
Scenario1	10yr 1d	B5L9	25.64	24.5112
Scenario1	25yr3d	B2	25.98	74.1436
Scenario1	25yr3d	B3L3	26.91	60.4359
Scenario1	25yr3d	B4L4	26.75	60.2153
Scenario1	25yr3d	B5L10	27.32	61.3671
Scenario1	25yr3d	B5L11	27.32	60.8888
Scenario1	25yr3d	B5L5	27.19	64.9794
Scenario1	25yr3d	B5L6	27.16	64.4186
Scenario1	25yr3d	B5L7	27.08	72.0602
Scenario1	25yr3d	B5L9	26.77	72.7019
Scenario1	5yr1d	B2	24.66	50.1783
Scenario1	5yr1d	B3L3	25.94	15.5401
Scenario1	5yr1d	B4L4	25.96	15.5473
Scenario1	5yr1d	B5L10	25.54	13.0172
Scenario1	5yr1d	B5L11	25.54	12.8222
Scenario1	5yr1d	B5L5	25.45	15.5878
Scenario1	5yr1d	B5L6	25.44	24.0950
Scenario1	5yr1d	B5L7	25.44	24.1700
Scenario1	5yr1d	B5L9	25.40	24.4366

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	100yr3d	B2	27.44	216.0017
Scenario1	100yr3d	B3L3	27.44	216.0017
Scenario1	100yr3d	B4L4	27.44	216.0017
Scenario1	100yr3d	B5L10	27.94	64.2212
Scenario1	100yr3d	B5L11	27.93	64.2212
Scenario1	100yr3d	B5L5	27.95	72.0934
Scenario1	100yr3d	B5L6	27.92	72.1017
Scenario1	100yr3d	B5L7	27.86	72.2184
Scenario1	100yr3d	B5L9	27.59	80.0184

Sim	Link Name	Maximum Flow Rate [cfs]	Time to Maximum Flow Rate [hrs]
25yr3d	WCS 2	11.12	109.5011

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	0.0000	24.00	0.00
25yr3d	B2	0.2525	24.00	0.00
25yr3d	B2	0.5010	24.00	0.00
25yr3d	B2	0.7552	24.00	0.00
25yr3d	B2	1.0052	24.00	0.00
25yr3d	B2	1.2552	24.00	0.00
25yr3d	B2	1.5052	24.00	0.00
25yr3d	B2	1.7552	24.00	0.00
25yr3d	B2	2.0052	24.00	0.00
25yr3d	B2	2.2552	24.00	0.00
25yr3d	B2	2.5052	24.00	0.00
25yr3d	B2	2.7552	24.00	0.00
25yr3d	B2	3.0052	24.00	0.00
25yr3d	B2	3.2552	24.00	0.00
25yr3d	B2	3.5052	24.00	0.00
25yr3d	B2	3.7552	24.00	0.00
25yr3d	B2	4.0052	24.00	0.00
25yr3d	B2	4.2552	24.00	0.00
25yr3d	B2	4.5052	24.00	0.00
25yr3d	B2	4.7552	24.00	0.00
25yr3d	B2	5.0052	24.00	0.00
25yr3d	B2	5.2552	24.00	0.00
25yr3d	B2	5.5052	24.00	0.00
25yr3d	B2	5.7552	24.00	0.00
25yr3d	B2	6.0052	24.00	0.00
25yr3d	B2	6.2552	24.00	0.00
25yr3d	B2	6.5052	24.00	0.00
25yr3d	B2	6.7552	24.00	0.00
25yr3d	B2	7.0052	24.00	0.00
25yr3d	B2	7.2552	24.00	0.00
25yr3d	B2	7.5052	24.00	0.00
25yr3d	B2	7.7552	24.00	0.00
25yr3d	B2	8.0052	24.00	0.00
25yr3d	B2	8.2552	24.00	0.00
25yr3d	B2	8.5052	24.00	0.00
25yr3d	B2	8.7552	24.00	0.00
25yr3d	B2	9.0052	24.00	0.00
25yr3d	B2	9.2552	24.00	0.00
25yr3d	B2	9.5052	24.00	0.00
25yr3d	B2	9.7552	24.00	0.00
25yr3d	B2	10.0052	24.00	0.00
25yr3d	B2	10.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	10.5052	24.00	0.00
25yr3d	B2	10.7552	24.00	0.00
25yr3d	B2	11.0052	24.00	0.00
25yr3d	B2	11.2552	24.00	0.00
25yr3d	B2	11.5052	24.00	0.00
25yr3d	B2	11.7552	24.00	0.00
25yr3d	B2	12.0052	24.00	0.00
25yr3d	B2	12.2552	24.00	0.00
25yr3d	B2	12.5052	24.00	0.00
25yr3d	B2	12.7552	24.00	0.00
25yr3d	B2	13.0052	24.00	0.00
25yr3d	B2	13.2552	24.00	0.00
25yr3d	B2	13.5052	24.00	0.00
25yr3d	B2	13.7552	24.00	0.00
25yr3d	B2	14.0052	24.00	0.00
25yr3d	B2	14.2552	24.00	0.00
25yr3d	B2	14.5052	24.00	0.00
25yr3d	B2	14.7552	24.00	0.00
25yr3d	B2	15.0052	24.00	0.00
25yr3d	B2	15.2552	24.00	0.00
25yr3d	B2	15.5052	24.00	0.00
25yr3d	B2	15.7552	24.00	0.00
25yr3d	B2	16.0052	24.00	0.00
25yr3d	B2	16.2552	24.00	0.00
25yr3d	B2	16.5052	24.00	0.00
25yr3d	B2	16.7552	24.00	0.00
25yr3d	B2	17.0052	24.00	0.00
25yr3d	B2	17.2552	24.00	0.00
25yr3d	B2	17.5052	24.00	0.00
25yr3d	B2	17.7552	24.00	0.00
25yr3d	B2	18.0052	24.00	0.00
25yr3d	B2	18.2552	24.00	0.00
25yr3d	B2	18.5052	24.00	0.00
25yr3d	B2	18.7552	24.00	0.00
25yr3d	B2	19.0052	24.00	0.00
25yr3d	B2	19.2552	24.00	0.00
25yr3d	B2	19.5052	24.00	0.00
25yr3d	B2	19.7552	24.00	0.00
25yr3d	B2	20.0052	24.00	0.00
25yr3d	B2	20.2552	24.00	0.00
25yr3d	B2	20.5052	24.00	0.00
25yr3d	B2	20.7552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	21.0052	24.00	0.00
25yr3d	B2	21.2552	24.00	0.00
25yr3d	B2	21.5052	24.00	0.00
25yr3d	B2	21.7552	24.00	0.00
25yr3d	B2	22.0052	24.00	0.00
25yr3d	B2	22.2552	24.00	0.00
25yr3d	B2	22.5052	24.00	0.00
25yr3d	B2	22.7552	24.00	0.00
25yr3d	B2	23.0052	24.00	0.00
25yr3d	B2	23.2552	24.00	0.00
25yr3d	B2	23.5052	24.00	0.00
25yr3d	B2	23.7552	24.00	0.00
25yr3d	B2	24.0052	24.00	0.00
25yr3d	B2	24.2552	24.00	0.00
25yr3d	B2	24.5052	24.00	0.00
25yr3d	B2	24.7552	24.00	0.00
25yr3d	B2	25.0052	24.00	0.00
25yr3d	B2	25.2552	24.00	0.00
25yr3d	B2	25.5052	24.00	0.00
25yr3d	B2	25.7552	24.00	0.00
25yr3d	B2	26.0052	24.00	0.00
25yr3d	B2	26.2552	24.00	0.00
25yr3d	B2	26.5052	24.00	0.00
25yr3d	B2	26.7552	24.00	0.00
25yr3d	B2	27.0052	24.00	0.00
25yr3d	B2	27.2552	24.00	0.00
25yr3d	B2	27.5052	24.00	0.00
25yr3d	B2	27.7552	24.00	0.00
25yr3d	B2	28.0052	24.00	0.00
25yr3d	B2	28.2552	24.00	0.00
25yr3d	B2	28.5052	24.00	0.00
25yr3d	B2	28.7552	24.00	0.00
25yr3d	B2	29.0052	24.00	0.00
25yr3d	B2	29.2552	24.00	0.00
25yr3d	B2	29.5052	24.00	0.00
25yr3d	B2	29.7552	24.00	0.00
25yr3d	B2	30.0052	24.00	0.00
25yr3d	B2	30.2552	24.00	0.00
25yr3d	B2	30.5052	24.00	0.00
25yr3d	B2	30.7552	24.00	0.00
25yr3d	B2	31.0052	24.00	0.00
25yr3d	B2	31.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	31.5052	24.00	0.00
25yr3d	B2	31.7552	24.00	0.00
25yr3d	B2	32.0052	24.00	0.00
25yr3d	B2	32.2552	24.00	0.00
25yr3d	B2	32.5052	24.00	0.00
25yr3d	B2	32.7552	24.00	0.00
25yr3d	B2	33.0052	24.01	0.00
25yr3d	B2	33.2552	24.01	0.00
25yr3d	B2	33.5052	24.01	0.00
25yr3d	B2	33.7552	24.01	0.00
25yr3d	B2	34.0052	24.01	0.00
25yr3d	B2	34.2552	24.01	0.00
25yr3d	B2	34.5052	24.01	0.00
25yr3d	B2	34.7552	24.01	0.00
25yr3d	B2	35.0052	24.01	0.00
25yr3d	B2	35.2552	24.01	0.00
25yr3d	B2	35.5052	24.01	0.00
25yr3d	B2	35.7552	24.01	0.00
25yr3d	B2	36.0052	24.01	0.00
25yr3d	B2	36.2552	24.01	0.00
25yr3d	B2	36.5052	24.01	0.00
25yr3d	B2	36.7552	24.01	0.00
25yr3d	B2	37.0052	24.01	0.00
25yr3d	B2	37.2552	24.01	0.00
25yr3d	B2	37.5052	24.01	0.00
25yr3d	B2	37.7552	24.01	0.00
25yr3d	B2	38.0052	24.02	0.00
25yr3d	B2	38.2552	24.02	0.00
25yr3d	B2	38.5052	24.02	0.00
25yr3d	B2	38.7552	24.02	0.00
25yr3d	B2	39.0052	24.02	0.00
25yr3d	B2	39.2552	24.02	0.00
25yr3d	B2	39.5052	24.02	0.00
25yr3d	B2	39.7552	24.02	0.00
25yr3d	B2	40.0052	24.02	0.00
25yr3d	B2	40.2552	24.02	0.00
25yr3d	B2	40.5052	24.02	0.00
25yr3d	B2	40.7552	24.02	0.01
25yr3d	B2	41.0052	24.02	0.01
25yr3d	B2	41.2552	24.03	0.01
25yr3d	B2	41.5052	24.03	0.01
25yr3d	B2	41.7552	24.03	0.01

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	42.0052	24.03	0.01
25yr3d	B2	42.2552	24.03	0.01
25yr3d	B2	42.5052	24.03	0.01
25yr3d	B2	42.7552	24.03	0.01
25yr3d	B2	43.0052	24.03	0.01
25yr3d	B2	43.2552	24.03	0.01
25yr3d	B2	43.5052	24.03	0.01
25yr3d	B2	43.7552	24.04	0.01
25yr3d	B2	44.0052	24.04	0.01
25yr3d	B2	44.2552	24.04	0.01
25yr3d	B2	44.5052	24.04	0.01
25yr3d	B2	44.7552	24.04	0.01
25yr3d	B2	45.0052	24.04	0.02
25yr3d	B2	45.2552	24.04	0.02
25yr3d	B2	45.5052	24.04	0.02
25yr3d	B2	45.7552	24.05	0.02
25yr3d	B2	46.0052	24.05	0.02
25yr3d	B2	46.2552	24.05	0.02
25yr3d	B2	46.5052	24.05	0.02
25yr3d	B2	46.7552	24.05	0.02
25yr3d	B2	47.0052	24.05	0.02
25yr3d	B2	47.2552	24.05	0.02
25yr3d	B2	47.5052	24.06	0.03
25yr3d	B2	47.7552	24.06	0.03
25yr3d	B2	48.0052	24.06	0.03
25yr3d	B2	48.2552	24.06	0.03
25yr3d	B2	48.5052	24.06	0.03
25yr3d	B2	48.7552	24.06	0.03
25yr3d	B2	49.0052	24.07	0.03
25yr3d	B2	49.2552	24.07	0.04
25yr3d	B2	49.5052	24.07	0.04
25yr3d	B2	49.7552	24.07	0.04
25yr3d	B2	50.0052	24.07	0.04
25yr3d	B2	50.2552	24.07	0.04
25yr3d	B2	50.5052	24.08	0.05
25yr3d	B2	50.7552	24.08	0.05
25yr3d	B2	51.0052	24.08	0.05
25yr3d	B2	51.2552	24.08	0.05
25yr3d	B2	51.5052	24.08	0.05
25yr3d	B2	51.7552	24.09	0.06
25yr3d	B2	52.0052	24.09	0.06
25yr3d	B2	52.2552	24.09	0.06

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	52.5052	24.09	0.07
25yr3d	B2	52.7552	24.10	0.07
25yr3d	B2	53.0052	24.10	0.07
25yr3d	B2	53.2552	24.10	0.07
25yr3d	B2	53.5052	24.10	0.08
25yr3d	B2	53.7552	24.11	0.08
25yr3d	B2	54.0052	24.11	0.08
25yr3d	B2	54.2552	24.11	0.09
25yr3d	B2	54.5036	24.12	0.09
25yr3d	B2	54.7536	24.12	0.10
25yr3d	B2	55.0002	24.13	0.10
25yr3d	B2	55.2530	24.13	0.11
25yr3d	B2	55.5053	24.14	0.11
25yr3d	B2	55.7502	24.14	0.11
25yr3d	B2	56.0013	24.15	0.12
25yr3d	B2	56.2520	24.15	0.13
25yr3d	B2	56.5031	24.16	0.13
25yr3d	B2	56.7519	24.16	0.14
25yr3d	B2	57.0016	24.17	0.15
25yr3d	B2	57.2508	24.18	0.15
25yr3d	B2	57.5036	24.19	0.16
25yr3d	B2	57.7505	24.20	0.17
25yr3d	B2	58.0032	24.21	0.18
25yr3d	B2	58.2526	24.22	0.19
25yr3d	B2	58.5033	24.23	0.20
25yr3d	B2	58.7534	24.25	0.21
25yr3d	B2	59.0014	24.26	0.22
25yr3d	B2	59.2509	24.28	0.24
25yr3d	B2	59.5018	24.31	0.25
25yr3d	B2	59.7503	24.35	0.27
25yr3d	B2	60.0002	24.41	0.29
25yr3d	B2	60.2501	24.51	0.32
25yr3d	B2	60.5011	24.62	0.37
25yr3d	B2	60.7510	24.72	0.42
25yr3d	B2	61.0019	24.81	0.48
25yr3d	B2	61.2507	24.90	0.56
25yr3d	B2	61.5007	24.97	0.65
25yr3d	B2	61.7504	25.04	0.75
25yr3d	B2	62.0004	25.10	0.85
25yr3d	B2	62.2533	25.15	0.97
25yr3d	B2	62.5031	25.20	1.09
25yr3d	B2	62.7504	25.24	1.21

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	63.0028	25.28	1.35
25yr3d	B2	63.2555	25.32	1.49
25yr3d	B2	63.5061	25.35	1.63
25yr3d	B2	63.7556	25.39	1.78
25yr3d	B2	64.0051	25.42	1.93
25yr3d	B2	64.2527	25.45	2.09
25yr3d	B2	64.5019	25.48	2.25
25yr3d	B2	64.7519	25.51	2.42
25yr3d	B2	65.0019	25.53	2.59
25yr3d	B2	65.2552	25.56	2.76
25yr3d	B2	65.5011	25.58	2.94
25yr3d	B2	65.7502	25.60	3.12
25yr3d	B2	66.0002	25.62	3.30
25yr3d	B2	66.2544	25.65	3.49
25yr3d	B2	66.5002	25.66	3.67
25yr3d	B2	66.7519	25.68	3.87
25yr3d	B2	67.0019	25.70	4.06
25yr3d	B2	67.2519	25.72	4.26
25yr3d	B2	67.5019	25.74	4.45
25yr3d	B2	67.7519	25.76	4.65
25yr3d	B2	68.0019	25.77	4.86
25yr3d	B2	68.2519	25.79	5.06
25yr3d	B2	68.5019	25.80	5.27
25yr3d	B2	68.7519	25.82	5.48
25yr3d	B2	69.0019	25.83	5.69
25yr3d	B2	69.2519	25.84	5.90
25yr3d	B2	69.5019	25.86	6.11
25yr3d	B2	69.7519	25.87	6.33
25yr3d	B2	70.0019	25.88	6.54
25yr3d	B2	70.2519	25.89	6.76
25yr3d	B2	70.5019	25.90	6.98
25yr3d	B2	70.7519	25.91	7.19
25yr3d	B2	71.0019	25.92	7.41
25yr3d	B2	71.2519	25.93	7.63
25yr3d	B2	71.5019	25.93	7.85
25yr3d	B2	71.7519	25.94	8.08
25yr3d	B2	72.0019	25.95	8.30
25yr3d	B2	76.0019	25.97	11.81
25yr3d	B2	80.0019	25.97	15.11
25yr3d	B2	84.0019	25.96	18.34
25yr3d	B2	88.0019	25.95	21.65
25yr3d	B2	92.0019	25.93	25.03

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	96.0019	25.90	28.45
25yr3d	B2	100.0019	25.87	31.88
25yr3d	B2	104.0019	25.83	35.40
25yr3d	B2	108.0019	25.79	39.04
25yr3d	B2	112.0019	25.74	42.70
25yr3d	B2	116.0019	25.69	46.26
25yr3d	B2	120.0019	25.64	49.67
25yr3d	B2	124.0019	25.59	52.93
25yr3d	B2	128.0019	25.55	56.05
25yr3d	B2	132.0019	25.50	59.03
25yr3d	B2	136.0019	25.45	61.86
25yr3d	B2	140.0019	25.40	64.55
25yr3d	B2	144.0019	25.35	67.10
25yr3d	B2	148.0019	25.31	69.53
25yr3d	B2	152.0019	25.27	71.84
25yr3d	B2	156.0019	25.23	74.06
25yr3d	B2	160.0019	25.19	76.17
25yr3d	B2	164.0019	25.16	78.20
25yr3d	B2	168.0019	25.13	80.14
25yr3d	B2	172.0019	25.10	82.01
25yr3d	B2	176.0019	25.07	83.80
25yr3d	B2	180.0019	25.04	85.52
25yr3d	B2	184.0019	25.02	87.17
25yr3d	B2	188.0019	24.99	88.77
25yr3d	B2	192.0019	24.97	90.30
25yr3d	B2	196.0019	24.94	91.78
25yr3d	B2	200.0019	24.92	93.20
25yr3d	B2	204.0019	24.90	94.57
25yr3d	B2	208.0019	24.88	95.90
25yr3d	B2	212.0019	24.86	97.18
25yr3d	B2	216.0019	24.84	98.41
25yr3d	B2	220.0019	24.82	99.61
25yr3d	B2	224.0019	24.80	100.76
25yr3d	B2	228.0019	24.78	101.88
25yr3d	B2	232.0019	24.76	102.96
25yr3d	B2	236.0019	24.75	104.00
25yr3d	B2	240.0019	24.73	105.01
25yr3d	B2	244.0019	24.72	105.99
25yr3d	B2	248.0019	24.70	106.93
25yr3d	B2	252.0019	24.69	107.85
25yr3d	B2	256.0019	24.67	108.74
25yr3d	B2	260.0019	24.66	109.60

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	264.0019	24.64	110.43
25yr3d	B2	268.0019	24.63	111.24
25yr3d	B2	272.0019	24.62	112.02
25yr3d	B2	276.0019	24.61	112.78
25yr3d	B2	280.0019	24.59	113.52
25yr3d	B2	284.0019	24.58	114.23
25yr3d	B2	288.0019	24.57	114.93
25yr3d	B2	292.0019	24.56	115.60
25yr3d	B2	296.0019	24.55	116.26
25yr3d	B2	300.0019	24.54	116.89
25yr3d	B2	304.0019	24.53	117.51
25yr3d	B2	308.0019	24.52	118.11
25yr3d	B2	312.0019	24.51	118.69
25yr3d	B2	316.0019	24.50	119.25
25yr3d	B2	320.0019	24.49	119.80
25yr3d	B2	324.0019	24.48	120.34
25yr3d	B2	328.0019	24.47	120.86
25yr3d	B2	332.0019	24.46	121.36
25yr3d	B2	336.0019	24.45	121.85
25yr3d	B2	340.0019	24.44	122.32
25yr3d	B2	344.0019	24.44	122.79
25yr3d	B2	348.0019	24.43	123.24
25yr3d	B2	352.0019	24.42	123.67
25yr3d	B2	356.0019	24.41	124.10
25yr3d	B2	360.0019	24.40	124.51

TOTAL SITE

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	25.78	8.6%	0.00	0.00	0.00	25.78
LAKE BANK @ CE + 1	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE TOB	2.28	0.8%	2.28	0.00	0.00	0.00
DRY DETENTION	1.65	0.6%	1.65	0.00	0.00	0.00
PRESERVE	37.83	12.7%	37.83	0.00	0.00	0.00
ROW	32.59	10.9%	13.04	19.55	0.00	0.00
SF	63.92	21.4%	25.57	19.18	19.18	0.00
MF	25.26	8.5%	5.05	10.10	10.10	0.00
GOLF COURSE	96.62	32.3%	93.73	2.90	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	0.5%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	2.3%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**298.86**	**100%**	**185.38**	**55.01**	**32.56**	**25.92**

TOTAL PERVIOUS AREA	185.38	62.0%	
TOTAL IMPERVIOUS AREA	113.49	38.0%	*(INCLUDES WATER SURFACE AREA)*

TOTAL SITE

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:

=	(Average Developed Grade)-(Wet Season Water Table)		
=	27.50	-	24.00
=	3.50	ft.	

Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:

S =	(% of Pervious Area) * (Soil Storage Depth)			
S =	62.0%	x	6.57	in.
S =	**4.07**	**in.**		

Curve Number:

Cn =	1000/(S + 10)
Cn =	**71**

III. SURFACE STORAGE

Basin Area =	298.86	ac.			
Bldg. Coverage Area =	32.56	ac.			
Impervious =	55.01				
Lake Area (at Control)	25.92	ac.	***EL. =***	***24.00***	***(NGVD)***
Preserve Area (at control)	37.83	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less Preserved)=	147.55	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**25.92**	**0.00**	**0.00**	**0.00**	**25.92**	**0.00**
25.00	**28.06**	**0.00**	**0.00**	**0.00**	**28.06**	**26.99**
26.50	**30.34**	**0.00**	**0.00**	**0.00**	**30.34**	**70.79**
27.00	**32.62**	**7.57**	**29.51**	**27.51**	**97.20**	**102.68**
27.50	**32.93**	**15.13**	**59.02**	**49.51**	**156.59**	**166.12**
28.50	**32.93**	**18.92**	**132.79**	**55.01**	**239.65**	**364.25**

IV. WATER QUALITY DETENTION VOLUME

TOTAL SITE

Total Area =	298.86	ac.
Bldg. Coverage =	32.56	ac.
Water Surface Area =	25.92	ac.
Pervious Area =	185.38	ac.

First one (1) Inch of runoff over basin:

= 1" x 298.86 ac. x1'/12" = **24.91** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	240.38	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	55.01	ac.
% Impervious = Impervious Area / Site Area	22.9%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**13.01**	**ac-ft**
Required Water Quality Detention Volume	**24.91**	**ac-ft**
Additional 50% Water Quality Detention Volume	**12.45**	**ac-ft**
Total Required Water Quality Detention Volume	**37.36**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**37.36**	**ac-ft**

Between El. 24.00 and 25.36

WCS Stage-Storage Table

Stage	Site Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	25.92	0.00
25.00	28.06	26.99
26.50	30.34	70.79
27.00	97.20	102.68
27.50	156.59	166.12
28.50	239.65	364.25

Water Quality Min. Weir Elevation = **25.36** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event:

= 55.0 CSM x WCS Area x 1 SM/640 ac.

= **25.68** **cfs**

METHOD OF DETERMINATION:		
Orange River Watershed (Approved ERP Appl # 060503-10)	55.0	CSM

Basin B2

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 1	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE TOB	0.00	0.0%	0.00	0.00	0.00	0.00
DRY DETENTION	0.44	1.4%	0.44	0.00	0.00	0.00
PRESERVE	26.62	82.1%	26.62	0.00	0.00	0.00
ROW	0.00	0.0%	0.00	0.00	0.00	0.00
SF	0.00	0.0%	0.00	0.00	0.00	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	5.38	16.6%	5.22	0.16	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**32.44**	**100%**	**32.28**	**0.16**	**0.00**	**0.00**

TOTAL PERVIOUS AREA	32.28	99.5%
TOTAL IMPERVIOUS AREA	0.16	0.5%

Basin B2

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** in.

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 99.5% x 6.57 in.
S = 6.53 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 60

III. SURFACE STORAGE

Basin Area =	32.44	ac.			
Bldg. Coverage Area =	0.00	ac.			
Dry Detention TOS (at CE + 1)	0.44	ac.	*EL. =*	***25.00***	***(NGVD)***
Dry Detention TOB (at CE + 2)	0.54	ac.	*EL. =*	***26.00***	***(NGVD)***
Golf Course	5.22	ac.			
Preserve Area (at control)	26.62	ac.	*EL. =*	***24.00***	***(NGVD)***
Pervious Area =	32.28	ac.			

Stage-Storage Table

Stage (NGVD)	Dry Detention ac	Preserve Area ac	Golf Course ac	Total Area ac	Total Volume ac-ft
24.00	**0.00**	**26.62**	**0.00**	**26.62**	**0.00**
25.00	**0.44**	**26.62**	**0.00**	**27.06**	**26.84**
26.50	**0.44**	**26.62**	**1.04**	**28.10**	**68.21**
27.00	**0.44**	**26.62**	**2.09**	**29.15**	**82.53**
27.50	**0.44**	**26.62**	**2.61**	**29.67**	**97.23**

IV. WATER QUALITY DETENTION VOLUME
The permitted 2.97 ac-ft of water quality was provided in the existing condition and no modification to the basin is proposed.

V. ALLOWABLE DISCHARGE

BASIN	2	
Contributing Flows		
Basin	1	49.33
Basin	2	32.44
Basin	3	16.53
Basin	4	18.34
Basin	5	131.83
Basin	6	18.20
Total Area =		266.68 ac.

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event:

= 55.0 CSM x WCS Area x 1 SM/640 ac.

= **22.92 cfs**

METHOD OF DETERMINATION:		
55 CSM allowed in Orange River Watershed & Approved ERP Appl # 994015-2	55.0	CSM

VI. WEIR DESIGN

Total Basin

Design Control Structure to Discharge 1/2" in 24 Hours.

Basin Area:	=		=	**266.68 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**11.11 Ac-FT**
½" Discharge Rate:	=		=	**5.72 CFS**
Water Quality Stage:	=		=	**24.56 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**22.22 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation)	=	**24.23 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**24.39 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)	=	**0.39 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 5.72 CFS

Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 7.39 Length of weir **A 1.50 ft wide rectangular weir was used to meet the maximum discharge rate**

Basin B3L3

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.16	7.0%	0.00	0.00	0.00	1.16
LAKE BANK @ CE + 1	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE TOB	0.14	0.8%	0.14	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	2.62	15.8%	1.05	1.57	0.00	0.00
SF	5.83	35.3%	2.33	1.75	1.75	0.00
MF	3.25	19.7%	0.65	1.30	1.30	0.00
GOLF COURSE	3.26	19.7%	3.16	0.10	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.53**	**100%**	**7.61**	**4.72**	**3.05**	**1.16**

TOTAL PERVIOUS AREA	7.61	46.0%	
TOTAL IMPERVIOUS AREA	8.93	54.0%	*(INCLUDES WATER SURFACE AREA)*

Basin B3L3

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:

= (Average Developed Grade)-(Wet Season Water Table)

= 27.50 - 24.00

= 3.50 ft.

Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:

S = (% of Pervious Area) * (Soil Storage Depth)

S = 46.0% x 6.57 in.

S = 3.02 in.

Curve Number:

Cn = 1000/(S + 10)

Cn = 77

III. SURFACE STORAGE

Basin Area =	16.53	ac.			
Bldg. Coverage Area =	3.05	ac.			
Lake Area (at Control)	1.16	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.30	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.44	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.57	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	3.16	ac.			
Pervious Area (less bank & GC) =	4.03	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.16**	**0.00**	**0.00**	**0.00**	**1.16**	**0.00**
25.00	**1.30**	**0.00**	**0.00**	**0.00**	**1.30**	**1.23**
26.50	**1.44**	**0.00**	**0.00**	**0.00**	**1.44**	**3.28**
27.00	**1.57**	**0.63**	**0.81**	**2.36**	**5.37**	**4.98**
27.50	**1.57**	**1.26**	**1.61**	**4.25**	**8.70**	**8.50**
28.50	**1.57**	**1.58**	**3.63**	**4.72**	**11.50**	**18.60**

IV. WATER QUALITY DETENTION VOLUME

BASIN	**3**		
Total Area =		16.53	ac.
Bldg. Coverage =		3.05	ac.
Water Surface Area =		1.16	ac.
Pervious Area =		7.61	ac.

First one (1) Inch of runoff over basin:

= 1" x 16.53 ac. x1'/12" = **1.38** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	12.32	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	4.72	ac.
% Impervious = Impervious Area / Site Area	38.3%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.23**	**ac-ft**
Required Water Quality Detention Volume	**1.38**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.69**	**ac-ft**
Total Required Water Quality Detention Volume	**2.07**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.07**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.16	0.00
25.00	1.30	1.23
26.50	1.44	3.28
27.00	1.57	4.03
27.50	1.57	4.82
28.50	1.57	6.39

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.16 ac + 1.44 ac (Lake 3 and Lake 4) = 2.60 ac, the WQ is provided at 25.76 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin (The required bleed down for B4L4 is accounted for in this basin)

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**34.88 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**1.45 Ac-FT**
½" Discharge Rate:	=		=	**0.75 CFS**
Water Quality Stage:	=		=	**25.76 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)=		**3.12 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation) =		**24.07 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2 =		**24.92 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)=		**0.92 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 0.75 **CFS**

Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 0.27 Length of weir **A 0.27 ft wide rectangular weir was used to meet the maximum bleeder rate**

Basin B4L4

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.44	7.9%	0.00	0.00	0.00	1.44
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	9.88	53.9%	3.95	5.93	0.00	0.00
SF	1.42	7.7%	0.57	0.43	0.43	0.00
MF	2.34	12.8%	0.47	0.94	0.94	0.00
GOLF COURSE	2.87	15.6%	2.78	0.09	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**18.34**	**100%**	**8.16**	**7.38**	**1.36**	**1.44**

TOTAL PERVIOUS AREA	8.16	44.5%	
TOTAL IMPERVIOUS AREA	10.18	55.5%	*(INCLUDES WATER SURFACE AREA)*

Basin B4L4

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:

= (Average Developed Grade)-(Wet Season Water Table)

= 27.50 - 24.00

= 3.50 ft.

Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:

S = (% of Pervious Area) * (Soil Storage Depth)

S = 44.5% x 6.57 in.

S = 2.92 in.

Curve Number:

Cn = 1000/(S + 10)

Cn = 77

III. SURFACE STORAGE

Basin Area =	18.34	ac.			
Bldg. Coverage Area =	1.36	ac.			
Lake Area (at Control)	1.44	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.57	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.70	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.83	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	0.00	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less bank & GC) =	4.99	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.44**	**0.00**	**0.00**	**0.00**	**1.44**	**0.00**
25.00	**1.57**	**0.00**	**0.00**	**0.00**	**1.57**	**1.51**
26.50	**1.70**	**0.00**	**0.00**	**0.00**	**1.70**	**3.96**
27.00	**1.83**	**0.56**	**1.00**	**3.69**	**7.07**	**6.15**
27.50	**1.83**	**1.11**	**2.00**	**6.64**	**11.58**	**10.82**
28.50	**1.83**	**1.39**	**4.49**	**7.38**	**15.09**	**24.15**

IV. WATER QUALITY DETENTION VOLUME

BASIN	**4**	
Total Area =	18.34	ac.
Bldg. Coverage =	1.36	ac.
Water Surface Area =	1.44	ac.
Pervious Area =	8.16	ac.

First one (1) Inch of runoff over basin:

= 1" x 18.34 ac. x1'/12" = **1.53** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	15.54	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	7.38	ac.
% Impervious = Impervious Area / Site Area	47.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.67**	**ac-ft**
Required Water Quality Detention Volume	**1.67**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.84**	**ac-ft**
Total Required Water Quality Detention Volume	**2.51**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.51**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.44	0.00
25.00	1.57	1.51
26.50	1.70	3.96
27.00	1.83	4.84
27.50	1.83	5.76
28.50	1.83	7.59

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.16 ac + 1.44 ac (Lake 3 and Lake 4) = 2.60 ac, the WQ is provided at 25.76 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

The Existing weir, as approved in ERP 36-03765-P (App 990415-2) was utilized.

Basin B5L5

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.61	15.7%	0.00	0.00	0.00	2.61
LAKE BANK @ CE + 1	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE TOB	0.23	1.4%	0.23	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	0.66	4.0%	0.26	0.40	0.00	0.00
SF	2	12.1%	0.80	0.60	0.60	0.00
MF	3.99	24.1%	0.80	1.60	1.60	0.00
GOLF COURSE	6.62	39.9%	6.42	0.20	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.58**	**100%**	**8.98**	**2.79**	**2.20**	**2.61**

TOTAL PERVIOUS AREA	8.98	54.2%	
TOTAL IMPERVIOUS AREA	7.60	45.8%	*(INCLUDES WATER SURFACE AREA)*

Basin B5L5

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 54.2% x 6.57 in.
S = 3.56 In.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	16.58	ac.			
Bldg. Coverage Area =	2.20	ac.			
Lake Area (at Control)	2.61	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.84	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.08	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	3.31	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	6.62	ac.			
Pervious Area (less bank & GC) =	1.86	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.61**	**0.00**	**0.00**	**0.00**	**2.61**	**0.00**
25.50	**2.84**	**0.00**	**0.00**	**0.00**	**2.84**	**2.73**
27.00	**3.08**	**0.00**	**0.00**	**0.00**	**3.08**	**7.17**
27.50	**3.31**	**1.32**	**0.37**	**1.40**	**6.40**	**9.54**
28.00	**3.31**	**2.65**	**0.74**	**2.51**	**9.22**	**13.44**
29.00	**3.31**	**3.31**	**1.68**	**2.79**	**11.09**	**23.60**

Lake B5L6

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.23	9.6%	0.00	0.00	0.00	1.23
LAKE BANK @ CE + 1	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE TOB	0.16	1.2%	0.16	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	1.40	10.9%	0.56	0.84	0.00	0.00
SF	5.62	43.7%	2.25	1.69	1.69	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	4.14	32.2%	4.01	0.12	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**12.86**	**100%**	**7.29**	**2.65**	**1.69**	**1.23**

TOTAL PERVIOUS AREA	7.29	56.7%	
TOTAL IMPERVIOUS AREA	5.57	43.3%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L6

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 56.7% x 6.57 in.
S = 3.72 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 73

III. SURFACE STORAGE

Basin Area =	12.86	ac.			
Bldg. Coverage Area =	1.69	ac.			
Lake Area (at Control)	1.23	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	0.16	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	0.16	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	1.39	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.01	ac.			
Pervious Area (less bank & GC) =	2.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.23**	**0.00**	**0.00**	**0.00**	**1.23**	**0.00**
25.50	**1.31**	**0.00**	**0.00**	**0.00**	**1.31**	**1.27**
27.00	**1.39**	**0.00**	**0.00**	**0.00**	**1.39**	**3.29**
27.50	**1.39**	**0.80**	**0.56**	**1.33**	**4.08**	**4.66**
28.00	**1.39**	**1.61**	**1.12**	**2.39**	**6.50**	**7.30**
29.00	**1.39**	**2.01**	**2.53**	**2.65**	**8.57**	**14.84**

Lake B5L7

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	9.70	20.4%	0.00	0.00	0.00	9.70
LAKE BANK @ CE + 1	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE TOB	0.71	1.5%	0.71	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	3.50	7.4%	1.40	2.10	0.00	0.00
SF	7.01	14.8%	2.80	2.10	2.10	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	18.16	38.2%	17.61	0.54	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	14.7%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**47.51**	**100%**	**25.36**	**7.55**	**4.90**	**9.70**

TOTAL PERVIOUS AREA	25.36	53.4%	
TOTAL IMPERVIOUS AREA	22.15	46.6%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L7

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.4% x 6.57 in.
S = 3.50 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	47.51	ac.			
Bldg. Coverage Area =	4.90	ac.			
Lake Area (at Control)	9.70	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	10.41	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	11.13	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	11.84	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	18.16	ac.			
Pervious Area (less bank & GC) =	5.60	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	9.70	0.00	0.00	0.00	9.70	0.00
25.50	10.41	0.00	0.00	0.00	10.41	10.06
27.00	11.13	0.00	0.00	0.00	11.13	26.21
27.50	11.84	3.63	1.12	3.77	20.37	34.09
28.00	11.84	7.26	2.24	6.79	28.14	46.21
29.00	11.84	9.08	5.04	7.55	33.51	77.04

Lake B5L9

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.49	11.4%	0.00	0.00	0.00	2.49
LAKE BANK @ CE + 1	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE TOB	0.28	1.3%	0.28	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.60	11.9%	1.04	1.56	0.00	0.00
SF	6.92	31.6%	2.77	2.08	2.08	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	9.06	41.3%	8.79	0.27	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**21.92**	**100%**	**13.45**	**3.91**	**2.08**	**2.49**

TOTAL PERVIOUS AREA	13.45	61.3%	
TOTAL IMPERVIOUS AREA	8.47	38.7%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L9

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 61.3% x 6.57 in.
S = 4.03 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 71

III. SURFACE STORAGE

Basin Area =	21.92	ac.			
Bldg. Coverage Area =	2.08	ac.			
Lake Area (at Control)	2.49	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.77	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.05	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	2.77	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	9.06	ac.			
Pervious Area (less bank & GC) =	3.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.49**	**0.00**	**0.00**	**0.00**	**2.49**	**0.00**
25.50	**2.77**	**0.00**	**0.00**	**0.00**	**2.77**	**2.63**
27.00	**3.05**	**0.00**	**0.00**	**0.00**	**3.05**	**7.00**
27.50	**3.05**	**1.81**	**0.76**	**1.95**	**7.58**	**9.66**
28.00	**3.05**	**3.63**	**1.52**	**3.52**	**11.72**	**14.49**
29.00	**3.05**	**4.53**	**3.43**	**3.91**	**14.92**	**27.81**

Lake B5L10

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.42	9.3%	0.00	0.00	0.00	1.42
LAKE BANK @ CE + 1	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE TOB	0.17	1.1%	0.17	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	0.61	4.0%	0.24	0.37	0.00	0.00
SF	1.37	9.0%	0.55	0.41	0.41	0.00
MF	4.10	26.9%	0.82	1.64	1.64	0.00
GOLF COURSE	5.85	38.5%	5.68	0.18	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	9.0%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**15.22**	**100%**	**8.06**	**3.07**	**2.53**	**1.56**

TOTAL PERVIOUS AREA	8.06	53.0%	
TOTAL IMPERVIOUS AREA	7.16	47.0%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L10

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.0% x 6.57 in.
S = 3.48 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	15.22	ac.			
Bldg. Coverage Area =	2.53	ac.			
Lake Area (at Control)	1.56	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.72	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.89	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.92	ac.	***EL. =***	***26.50***	***(NGVD)***
Golf Course	5.68	ac.			
Pervious Area (less bank & GC) =	1.89	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.56**	**0.00**	**0.00**	**0.00**	**1.56**	**0.00**
25.50	**1.72**	**0.00**	**0.00**	**0.00**	**1.72**	**1.64**
27.00	**1.89**	**0.00**	**0.00**	**0.00**	**1.89**	**4.35**
27.50	**1.92**	**1.14**	**0.38**	**1.54**	**4.96**	**6.06**
28.00	**1.92**	**2.27**	**0.75**	**2.76**	**7.71**	**9.23**
29.00	**1.92**	**2.84**	**1.70**	**3.07**	**9.52**	**17.84**

Lake B5L11

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.05	5.9%	0.00	0.00	0.00	1.05
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.94	16.6%	1.18	1.76	0.00	0.00
SF	6.61	37.3%	2.64	1.98	1.98	0.00
MF	2.20	12.4%	0.44	0.88	0.88	0.00
GOLF COURSE	4.55	25.6%	4.41	0.14	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**17.74**	**100%**	**9.06**	**4.76**	**2.86**	**1.05**

TOTAL PERVIOUS AREA	9.06	51.1%	
TOTAL IMPERVIOUS AREA	8.68	48.9%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L11

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 51.1% x 6.57 in.
S = 3.35 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 75

III. SURFACE STORAGE

Basin Area =	17.74	ac.			
Bldg. Coverage Area =	2.86	ac.			
Lake Area (at Control)	1.05	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.18	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.31	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.44	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.41	ac.			
Pervious Area (less bank & GC) =	4.26	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.05**	**0.00**	**0.00**	**0.00**	**1.05**	**0.00**
25.50	**1.18**	**0.00**	**0.00**	**0.00**	**1.18**	**1.12**
27.00	**1.31**	**0.00**	**0.00**	**0.00**	**1.31**	**2.98**
27.50	**1.44**	**0.88**	**0.85**	**2.38**	**5.56**	**4.70**
28.00	**1.44**	**1.76**	**1.70**	**4.29**	**9.20**	**8.39**
29.00	**1.44**	**2.21**	**3.83**	**4.76**	**12.25**	**19.11**

IV. WATER QUALITY DETENTION VOLUME

BASIN **5**

Total Area =	131.83	ac.
Bldg. Coverage =	16.25	ac.
Water Surface Area =	18.64	ac.
Pervious Area =	72.21	ac.

First one (1) inch of runoff over basin:

= 1" x 131.83 ac. x1'/12" = **10.99** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	96.94	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	24.73	ac.
% Impervious = Impervious Area / Site Area	25.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**6.02**	**ac-ft**
Required Water Quality Detention Volume	**10.99**	**ac-ft**
Additional 50% Water Quality Detention Volume	**5.49**	**ac-ft**

Beth Stacy Offsite Imperviousness

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	0.22	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	0.22	ac.
% Impervious = Impervious Area / Site Area	100.0%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**0.05**	**ac-ft**
Total Required Water Quality Detention Volume	**16.52**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**16.52**	**ac-ft**

Between El. 24.00 and 25.35

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Lake Storage Volume
(NGVD)	ac	ac-ft
24.50	18.64	0.00
25.50	20.24	19.44
27.00	21.85	51.01
27.50	22.95	62.20
28.00	22.95	73.68
29.00	22.95	96.63

23 ac-ft of total wq volume is provided

Water Quality Min. Weir Elevation = **25.35** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**131.83 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**5.49 Ac-FT**
½" Discharge Rate:	=		=	**2.83 CFS**
Water Quality Stage:	=		=	**25.67 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**17.51 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	eatment Vol. After 24 Hr. / Total Basin Stora (+ Control Elevation)	=	**24.84 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**25.26 Ft-NAVD**
Average Height During Bleed Down:	=	ıve. Elev. During Bleed Down - Control Elev	=	**0.76 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q = 2.83 **CFS**

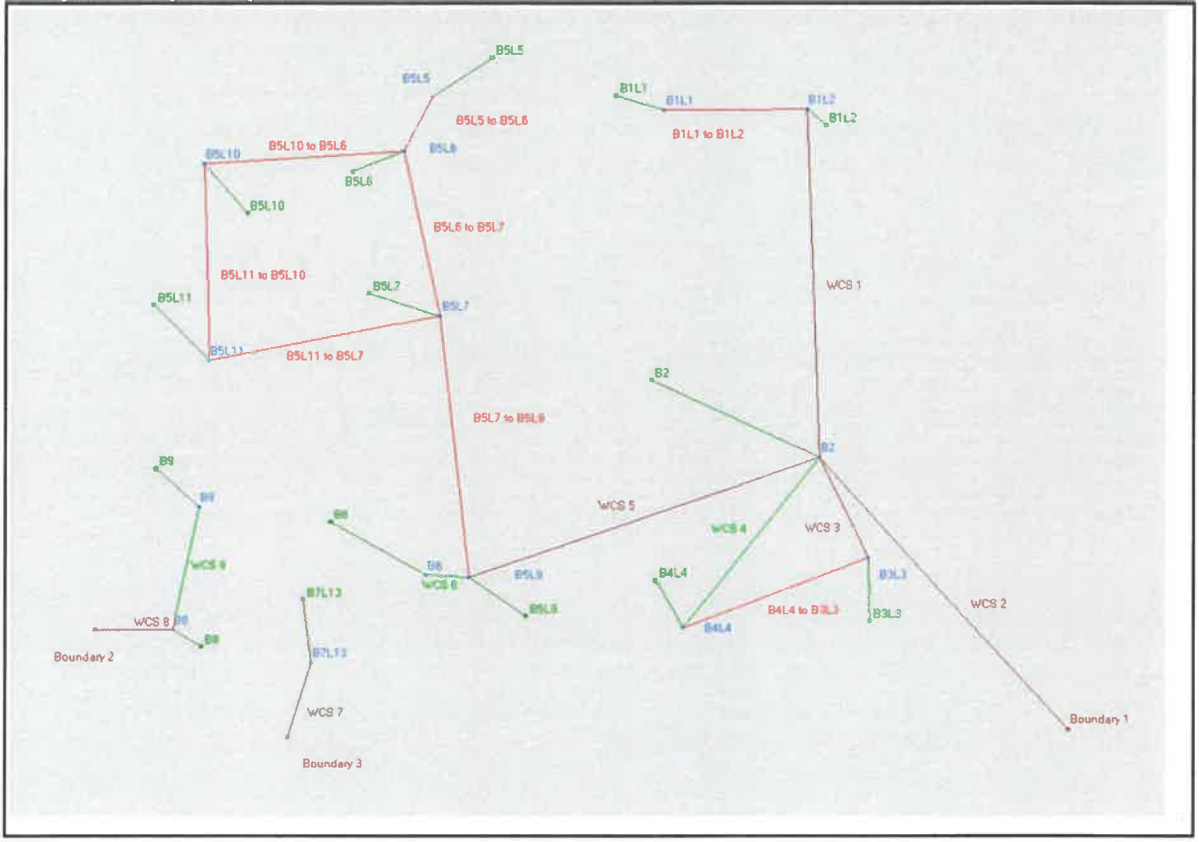
Weir Length (L)

H = height above weir

$L = Q/(3.13)(H^{1.5})$

L= 1.37 Length of weir **A 1.33 ft wide rectangular weir was used to meet the maximum bleeder rate**

Background Image: Background



Impervious: Scenario 1 [Set]

Land Cover Zone	% Impervious	% DCIA	% Direct	Ia/S [dec]	Ia Impervious [in]	Ia Pervious [in]
Impervious & Pervious	0.00	0.00	0.00	0.00	0.000	0.000

Curve Number: Scenario 1 [Set]

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 60	60.0
Impervious & Pervious	Calculated CN 63	63.0
Impervious & Pervious	Calculated CN 64	64.0
Impervious & Pervious	Calculated CN 70	70.0
Impervious & Pervious	Calculated CN 71	71.0
Impervious & Pervious	Calculated CN 72	72.0

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 73	73.0
Impervious & Pervious	Calculated CN 74	74.0
Impervious & Pervious	Calculated CN 75	75.0
Impervious & Pervious	Calculated CN 76	76.0
Impervious & Pervious	Calculated CN 77	77.0

Manual Basin: B2

Scenario: Scenario1
Node: B2
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 60.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 32.4400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
32.4400	Impervious & Pervious	Calculated CN 60	

Comment:

Manual Basin: B3L3

Scenario: Scenario1
Node: B3L3
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5300 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5300	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B4L4

Scenario: Scenario1
Node: B4L4

Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 18.3400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
18.3400	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B5L10

Scenario: Scenario1
Node: B5L10
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 15.2200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
15.2200	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L11

Scenario: Scenario1
Node: B5L11
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 17.7400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
17.7400	Impervious & Pervious	Calculated CN 75	

Comment:

Manual Basin: B5L5

Scenario: Scenario1
Node: B5L5
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5800 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5800	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L6

Scenario: Scenario1
Node: B5L6
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 12.8600 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
12.8600	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L7

Scenario: Scenario1
Node: B5L7
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 47.5100 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
47.5100	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L9

Scenario: Scenario1
Node: B5L9
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 21.9200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
21.9200	Impervious & Pervious	Calculated CN 71	

Comment:

Node: B2

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	26.6200	1159567
25.00	27.0600	1178734
26.50	28.1000	1224036
27.00	29.1500	1269774
27.50	29.6700	1292425

Comment:

Node: B3L3

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft

Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.1600	50530
25.00	1.3000	56628
26.50	1.4400	62726
27.00	5.3700	233917
27.50	8.7000	378972
28.50	11.5000	500940

Comment:

Node: B4L4

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.4400	62726
25.00	1.5700	68389
26.50	1.7000	74052
27.00	7.0700	307969
27.50	11.5800	504425
28.50	15.0900	657320

Comment:

Node: B5L10

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.5600	67954
25.50	1.7200	74923
27.00	1.8900	82328
27.50	4.9600	216058
28.00	7.7100	335848
29.00	9.5200	414691

Comment:

Node: B5L11

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.0500	45738
25.50	1.1800	51401
27.00	1.3100	57064
27.50	5.5600	242194
28.00	9.2000	400752
29.00	12.2500	533610

Comment:

Node: B5L5

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.6100	113692
25.50	2.8400	123710
27.00	3.0800	134165
27.50	6.4000	278784
28.00	9.2200	401623
29.00	11.0900	483080

Comment:

Node: B5L6

Scenario: Scenario1
Type: Stage/Area

Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.2300	53579
25.50	1.3100	57064
27.00	1.3900	60548
27.50	4.0800	177725
28.00	6.5000	283140
29.00	8.5700	373309

Comment:

Node: B5L7

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	9.7000	422532
25.50	10.4100	453460
27.00	11.1300	484823
27.50	20.3700	887317
28.00	28.1400	1225778
29.00	33.5100	1459696

Comment:

Node: B5L9

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.4900	108464
25.50	2.7700	120661
27.00	3.0500	132858

Stage [ft]	Area [ac]	Area [ft2]
27.50	7.5800	330185
28.00	11.7200	510523
29.00	14.9200	649915

Comment:

Node: Boundary 1

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 23.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	23.00
0	0	0	60.0000	24.00
0	0	0	80.0000	25.00
0	0	0	100.0000	24.50
0	0	0	115.0000	23.30
0	0	0	336.0000	23.00

Comment:

Pipe Link: B4L4 to B3L3

Scenario: Scenario1
From Node: B3L3
To Node: B4L4
Link Count: 1
Flow Direction: Both
Damping: 0.0000 ft
Length: 731.00 ft
FHWA Code: 0
Entr Loss Coef: 1
Exit Loss Coef: 1
Bend Loss Coef: 0
Bend Location: 0.00 dec
Energy Switch: Energy

Upstream		Downstream	
Invert:	21.00 ft	Invert:	21.00 ft
Manning's N:	0.0130	Manning's N:	0.0130
Geometry: Circular		Geometry: Circular	
Max Depth:	3.00 ft	Max Depth:	3.00 ft
Bottom Clip		Bottom Clip	
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000
Top Clip		Top Clip	
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L10 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.70 ft	Invert:	20.60 ft
From Node:	B5L10	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	450.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L10		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.15 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L10	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	320.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	21.00 ft	Invert:	20.50 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	430.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	1	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			

Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L5 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft
From Node:	B5L5	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	385.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L6 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.40 ft
From Node:	B5L6	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	220.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L7 to B5L9		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft

From Node:	B5L7	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L9	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.50 ft	Max Depth:	2.50 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	585.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Drop Structure Link: WCS 2		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B2	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	Boundary 1	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	65.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Sharp Crested Vertical	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	24.00 ft	Op Table:	
Control Elevation:	24.00 ft	Ref Node:	
Max Depth:	999.00 ft	Discharge Coefficients	
Max Width:	1.50 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600

Orifice Table:

Weir Comment:

Drop Structure Comment:

Drop Structure Link: WCS 3		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B3L3	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B2	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	237.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Sharp Crested Vertical	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	24.00 ft	Ref Node:	
Control Elevation:	24.00 ft	Discharge Coefficients	
Max Depth:	0.25 ft	Weir Default:	3.200
Max Width:	0.27 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component		Bottom Clip	
Weir:	2	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Horizontal		

Geometry Type:	Rectangular
Invert:	25.76 ft
Control Elevation:	25.76 ft
Max Depth:	3.00 ft
Max Width:	5.00 ft
Fillet:	0.00 ft

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	3.200
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Drop Structure Comment:

Weir Link: WCS 4

Scenario:	Scenario1
From Node:	B4L4
To Node:	B2
Link Count:	1
Flow Direction:	Both
Damping:	0.0000 ft
Weir Type:	Broad Crested Vertical
Geometry Type:	Rectangular
Invert:	26.00 ft
Control Elevation:	26.00 ft
Max Depth:	999.00 ft
Max Width:	30.00 ft
Fillet:	0.00 ft

Bottom Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Top Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	2.800
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Comment:

Simulation: 100yr3d

Scenario:	Scenario1
Run Date/Time:	9/18/2025 10:11:53 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	216.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr

Ia/S: 0.20 dec

Smp/Man Basin Rain Opt: Global

Rainfall Name: ~SFWMD-72
Rainfall Amount: 14.20 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 10yr 1d

Scenario: Scenario1
Run Date/Time: 9/23/2025 3:10:07 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec

IA Recovery Time: 12.0000 hr

Ia/S: 0.20 dec

dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FLMOD
		Rainfall Amount:	6.00 in
		Storm Duration:	24.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 25yr3d

Scenario: Scenario1
Run Date/Time: 9/23/2025 3:10:16 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:	
Unit Hydrograph Folder:	

Lookup Tables

Boundary Stage Set:	
Extern Hydrograph Set:	
Curve Number Set:	Scenario 1
Green-Ampt Set:	
Vertical Layers Set:	
Impervious Set:	Scenario 1

Tolerances & Options

Time Marching:	SAOR
Max Iterations:	6
Over-Relax Weight Fact:	0.5 dec
dZ Tolerance:	0.0010 ft
Max dZ:	1.0000 ft
Link Optimizer Tol:	0.0001 ft

IA Recovery Time:	24.0000 hr
Ia/S:	0.20 dec
Smp/Man Basin Rain Opt:	Global
Rainfall Name:	~SFWMD-72
Rainfall Amount:	10.20 in
Storm Duration:	72.0000 hr
Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (1D):	100 ft2
Energy Switch (1D):	Energy

Comment:

Simulation: 5yr1d

Scenario:	Scenario1
Run Date/Time:	9/23/2025 3:10:43 PM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 12.0000 hr

Ia/S: 0.20 dec

Smp/Man Basin Rain Opt: Global

Rainfall Name: ~FLMOD
Rainfall Amount: 5.20 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	10yr 1d	B2	24.84	42.0862
Scenario1	10yr 1d	B3L3	26.07	13.1869
Scenario1	10yr 1d	B4L4	26.09	13.2166
Scenario1	10yr 1d	B5L10	25.86	13.0674
Scenario1	10yr 1d	B5L11	25.87	12.8033
Scenario1	10yr 1d	B5L5	25.73	15.8862
Scenario1	10yr 1d	B5L6	25.72	24.0696
Scenario1	10yr 1d	B5L7	25.71	24.1362
Scenario1	10yr 1d	B5L9	25.64	24.5112
Scenario1	25yr3d	B2	25.98	74.1436
Scenario1	25yr3d	B3L3	26.91	60.4359
Scenario1	25yr3d	B4L4	26.75	60.2153
Scenario1	25yr3d	B5L10	27.32	61.3671
Scenario1	25yr3d	B5L11	27.32	60.8888
Scenario1	25yr3d	B5L5	27.19	64.9794
Scenario1	25yr3d	B5L6	27.16	64.4186
Scenario1	25yr3d	B5L7	27.08	72.0602
Scenario1	25yr3d	B5L9	26.77	72.7019
Scenario1	5yr1d	B2	24.66	50.1783
Scenario1	5yr1d	B3L3	25.94	15.5401
Scenario1	5yr1d	B4L4	25.96	15.5473
Scenario1	5yr1d	B5L10	25.54	13.0172
Scenario1	5yr1d	B5L11	25.54	12.8222
Scenario1	5yr1d	B5L5	25.45	15.5878
Scenario1	5yr1d	B5L6	25.44	24.0950
Scenario1	5yr1d	B5L7	25.44	24.1700
Scenario1	5yr1d	B5L9	25.40	24.4366

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	100yr3d	B2	27.44	216.0017
Scenario1	100yr3d	B3L3	27.44	216.0017
Scenario1	100yr3d	B4L4	27.44	216.0017
Scenario1	100yr3d	B5L10	27.94	64.2212
Scenario1	100yr3d	B5L11	27.93	64.2212
Scenario1	100yr3d	B5L5	27.95	72.0934
Scenario1	100yr3d	B5L6	27.92	72.1017
Scenario1	100yr3d	B5L7	27.86	72.2184
Scenario1	100yr3d	B5L9	27.59	80.0184

Sim	Link Name	Maximum Flow Rate [cfs]	Time to Maximum Flow Rate [hrs]
25yr3d	WCS 2	11.12	109.5011

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	0.0000	24.00	0.00
25yr3d	B2	0.2525	24.00	0.00
25yr3d	B2	0.5010	24.00	0.00
25yr3d	B2	0.7552	24.00	0.00
25yr3d	B2	1.0052	24.00	0.00
25yr3d	B2	1.2552	24.00	0.00
25yr3d	B2	1.5052	24.00	0.00
25yr3d	B2	1.7552	24.00	0.00
25yr3d	B2	2.0052	24.00	0.00
25yr3d	B2	2.2552	24.00	0.00
25yr3d	B2	2.5052	24.00	0.00
25yr3d	B2	2.7552	24.00	0.00
25yr3d	B2	3.0052	24.00	0.00
25yr3d	B2	3.2552	24.00	0.00
25yr3d	B2	3.5052	24.00	0.00
25yr3d	B2	3.7552	24.00	0.00
25yr3d	B2	4.0052	24.00	0.00
25yr3d	B2	4.2552	24.00	0.00
25yr3d	B2	4.5052	24.00	0.00
25yr3d	B2	4.7552	24.00	0.00
25yr3d	B2	5.0052	24.00	0.00
25yr3d	B2	5.2552	24.00	0.00
25yr3d	B2	5.5052	24.00	0.00
25yr3d	B2	5.7552	24.00	0.00
25yr3d	B2	6.0052	24.00	0.00
25yr3d	B2	6.2552	24.00	0.00
25yr3d	B2	6.5052	24.00	0.00
25yr3d	B2	6.7552	24.00	0.00
25yr3d	B2	7.0052	24.00	0.00
25yr3d	B2	7.2552	24.00	0.00
25yr3d	B2	7.5052	24.00	0.00
25yr3d	B2	7.7552	24.00	0.00
25yr3d	B2	8.0052	24.00	0.00
25yr3d	B2	8.2552	24.00	0.00
25yr3d	B2	8.5052	24.00	0.00
25yr3d	B2	8.7552	24.00	0.00
25yr3d	B2	9.0052	24.00	0.00
25yr3d	B2	9.2552	24.00	0.00
25yr3d	B2	9.5052	24.00	0.00
25yr3d	B2	9.7552	24.00	0.00
25yr3d	B2	10.0052	24.00	0.00
25yr3d	B2	10.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	10.5052	24.00	0.00
25yr3d	B2	10.7552	24.00	0.00
25yr3d	B2	11.0052	24.00	0.00
25yr3d	B2	11.2552	24.00	0.00
25yr3d	B2	11.5052	24.00	0.00
25yr3d	B2	11.7552	24.00	0.00
25yr3d	B2	12.0052	24.00	0.00
25yr3d	B2	12.2552	24.00	0.00
25yr3d	B2	12.5052	24.00	0.00
25yr3d	B2	12.7552	24.00	0.00
25yr3d	B2	13.0052	24.00	0.00
25yr3d	B2	13.2552	24.00	0.00
25yr3d	B2	13.5052	24.00	0.00
25yr3d	B2	13.7552	24.00	0.00
25yr3d	B2	14.0052	24.00	0.00
25yr3d	B2	14.2552	24.00	0.00
25yr3d	B2	14.5052	24.00	0.00
25yr3d	B2	14.7552	24.00	0.00
25yr3d	B2	15.0052	24.00	0.00
25yr3d	B2	15.2552	24.00	0.00
25yr3d	B2	15.5052	24.00	0.00
25yr3d	B2	15.7552	24.00	0.00
25yr3d	B2	16.0052	24.00	0.00
25yr3d	B2	16.2552	24.00	0.00
25yr3d	B2	16.5052	24.00	0.00
25yr3d	B2	16.7552	24.00	0.00
25yr3d	B2	17.0052	24.00	0.00
25yr3d	B2	17.2552	24.00	0.00
25yr3d	B2	17.5052	24.00	0.00
25yr3d	B2	17.7552	24.00	0.00
25yr3d	B2	18.0052	24.00	0.00
25yr3d	B2	18.2552	24.00	0.00
25yr3d	B2	18.5052	24.00	0.00
25yr3d	B2	18.7552	24.00	0.00
25yr3d	B2	19.0052	24.00	0.00
25yr3d	B2	19.2552	24.00	0.00
25yr3d	B2	19.5052	24.00	0.00
25yr3d	B2	19.7552	24.00	0.00
25yr3d	B2	20.0052	24.00	0.00
25yr3d	B2	20.2552	24.00	0.00
25yr3d	B2	20.5052	24.00	0.00
25yr3d	B2	20.7552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	21.0052	24.00	0.00
25yr3d	B2	21.2552	24.00	0.00
25yr3d	B2	21.5052	24.00	0.00
25yr3d	B2	21.7552	24.00	0.00
25yr3d	B2	22.0052	24.00	0.00
25yr3d	B2	22.2552	24.00	0.00
25yr3d	B2	22.5052	24.00	0.00
25yr3d	B2	22.7552	24.00	0.00
25yr3d	B2	23.0052	24.00	0.00
25yr3d	B2	23.2552	24.00	0.00
25yr3d	B2	23.5052	24.00	0.00
25yr3d	B2	23.7552	24.00	0.00
25yr3d	B2	24.0052	24.00	0.00
25yr3d	B2	24.2552	24.00	0.00
25yr3d	B2	24.5052	24.00	0.00
25yr3d	B2	24.7552	24.00	0.00
25yr3d	B2	25.0052	24.00	0.00
25yr3d	B2	25.2552	24.00	0.00
25yr3d	B2	25.5052	24.00	0.00
25yr3d	B2	25.7552	24.00	0.00
25yr3d	B2	26.0052	24.00	0.00
25yr3d	B2	26.2552	24.00	0.00
25yr3d	B2	26.5052	24.00	0.00
25yr3d	B2	26.7552	24.00	0.00
25yr3d	B2	27.0052	24.00	0.00
25yr3d	B2	27.2552	24.00	0.00
25yr3d	B2	27.5052	24.00	0.00
25yr3d	B2	27.7552	24.00	0.00
25yr3d	B2	28.0052	24.00	0.00
25yr3d	B2	28.2552	24.00	0.00
25yr3d	B2	28.5052	24.00	0.00
25yr3d	B2	28.7552	24.00	0.00
25yr3d	B2	29.0052	24.00	0.00
25yr3d	B2	29.2552	24.00	0.00
25yr3d	B2	29.5052	24.00	0.00
25yr3d	B2	29.7552	24.00	0.00
25yr3d	B2	30.0052	24.00	0.00
25yr3d	B2	30.2552	24.00	0.00
25yr3d	B2	30.5052	24.00	0.00
25yr3d	B2	30.7552	24.00	0.00
25yr3d	B2	31.0052	24.00	0.00
25yr3d	B2	31.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	31.5052	24.00	0.00
25yr3d	B2	31.7552	24.00	0.00
25yr3d	B2	32.0052	24.00	0.00
25yr3d	B2	32.2552	24.00	0.00
25yr3d	B2	32.5052	24.00	0.00
25yr3d	B2	32.7552	24.00	0.00
25yr3d	B2	33.0052	24.01	0.00
25yr3d	B2	33.2552	24.01	0.00
25yr3d	B2	33.5052	24.01	0.00
25yr3d	B2	33.7552	24.01	0.00
25yr3d	B2	34.0052	24.01	0.00
25yr3d	B2	34.2552	24.01	0.00
25yr3d	B2	34.5052	24.01	0.00
25yr3d	B2	34.7552	24.01	0.00
25yr3d	B2	35.0052	24.01	0.00
25yr3d	B2	35.2552	24.01	0.00
25yr3d	B2	35.5052	24.01	0.00
25yr3d	B2	35.7552	24.01	0.00
25yr3d	B2	36.0052	24.01	0.00
25yr3d	B2	36.2552	24.01	0.00
25yr3d	B2	36.5052	24.01	0.00
25yr3d	B2	36.7552	24.01	0.00
25yr3d	B2	37.0052	24.01	0.00
25yr3d	B2	37.2552	24.01	0.00
25yr3d	B2	37.5052	24.01	0.00
25yr3d	B2	37.7552	24.01	0.00
25yr3d	B2	38.0052	24.02	0.00
25yr3d	B2	38.2552	24.02	0.00
25yr3d	B2	38.5052	24.02	0.00
25yr3d	B2	38.7552	24.02	0.00
25yr3d	B2	39.0052	24.02	0.00
25yr3d	B2	39.2552	24.02	0.00
25yr3d	B2	39.5052	24.02	0.00
25yr3d	B2	39.7552	24.02	0.00
25yr3d	B2	40.0052	24.02	0.00
25yr3d	B2	40.2552	24.02	0.00
25yr3d	B2	40.5052	24.02	0.00
25yr3d	B2	40.7552	24.02	0.01
25yr3d	B2	41.0052	24.02	0.01
25yr3d	B2	41.2552	24.03	0.01
25yr3d	B2	41.5052	24.03	0.01
25yr3d	B2	41.7552	24.03	0.01

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	42.0052	24.03	0.01
25yr3d	B2	42.2552	24.03	0.01
25yr3d	B2	42.5052	24.03	0.01
25yr3d	B2	42.7552	24.03	0.01
25yr3d	B2	43.0052	24.03	0.01
25yr3d	B2	43.2552	24.03	0.01
25yr3d	B2	43.5052	24.03	0.01
25yr3d	B2	43.7552	24.04	0.01
25yr3d	B2	44.0052	24.04	0.01
25yr3d	B2	44.2552	24.04	0.01
25yr3d	B2	44.5052	24.04	0.01
25yr3d	B2	44.7552	24.04	0.01
25yr3d	B2	45.0052	24.04	0.02
25yr3d	B2	45.2552	24.04	0.02
25yr3d	B2	45.5052	24.04	0.02
25yr3d	B2	45.7552	24.05	0.02
25yr3d	B2	46.0052	24.05	0.02
25yr3d	B2	46.2552	24.05	0.02
25yr3d	B2	46.5052	24.05	0.02
25yr3d	B2	46.7552	24.05	0.02
25yr3d	B2	47.0052	24.05	0.02
25yr3d	B2	47.2552	24.05	0.02
25yr3d	B2	47.5052	24.06	0.03
25yr3d	B2	47.7552	24.06	0.03
25yr3d	B2	48.0052	24.06	0.03
25yr3d	B2	48.2552	24.06	0.03
25yr3d	B2	48.5052	24.06	0.03
25yr3d	B2	48.7552	24.06	0.03
25yr3d	B2	49.0052	24.07	0.03
25yr3d	B2	49.2552	24.07	0.04
25yr3d	B2	49.5052	24.07	0.04
25yr3d	B2	49.7552	24.07	0.04
25yr3d	B2	50.0052	24.07	0.04
25yr3d	B2	50.2552	24.07	0.04
25yr3d	B2	50.5052	24.08	0.05
25yr3d	B2	50.7552	24.08	0.05
25yr3d	B2	51.0052	24.08	0.05
25yr3d	B2	51.2552	24.08	0.05
25yr3d	B2	51.5052	24.08	0.05
25yr3d	B2	51.7552	24.09	0.06
25yr3d	B2	52.0052	24.09	0.06
25yr3d	B2	52.2552	24.09	0.06

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	52.5052	24.09	0.07
25yr3d	B2	52.7552	24.10	0.07
25yr3d	B2	53.0052	24.10	0.07
25yr3d	B2	53.2552	24.10	0.07
25yr3d	B2	53.5052	24.10	0.08
25yr3d	B2	53.7552	24.11	0.08
25yr3d	B2	54.0052	24.11	0.08
25yr3d	B2	54.2552	24.11	0.09
25yr3d	B2	54.5036	24.12	0.09
25yr3d	B2	54.7536	24.12	0.10
25yr3d	B2	55.0002	24.13	0.10
25yr3d	B2	55.2530	24.13	0.11
25yr3d	B2	55.5053	24.14	0.11
25yr3d	B2	55.7502	24.14	0.11
25yr3d	B2	56.0013	24.15	0.12
25yr3d	B2	56.2520	24.15	0.13
25yr3d	B2	56.5031	24.16	0.13
25yr3d	B2	56.7519	24.16	0.14
25yr3d	B2	57.0016	24.17	0.15
25yr3d	B2	57.2508	24.18	0.15
25yr3d	B2	57.5036	24.19	0.16
25yr3d	B2	57.7505	24.20	0.17
25yr3d	B2	58.0032	24.21	0.18
25yr3d	B2	58.2526	24.22	0.19
25yr3d	B2	58.5033	24.23	0.20
25yr3d	B2	58.7534	24.25	0.21
25yr3d	B2	59.0014	24.26	0.22
25yr3d	B2	59.2509	24.28	0.24
25yr3d	B2	59.5018	24.31	0.25
25yr3d	B2	59.7503	24.35	0.27
25yr3d	B2	60.0002	24.41	0.29
25yr3d	B2	60.2501	24.51	0.32
25yr3d	B2	60.5011	24.62	0.37
25yr3d	B2	60.7510	24.72	0.42
25yr3d	B2	61.0019	24.81	0.48
25yr3d	B2	61.2507	24.90	0.56
25yr3d	B2	61.5007	24.97	0.65
25yr3d	B2	61.7504	25.04	0.75
25yr3d	B2	62.0004	25.10	0.85
25yr3d	B2	62.2533	25.15	0.97
25yr3d	B2	62.5031	25.20	1.09
25yr3d	B2	62.7504	25.24	1.21

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	63.0028	25.28	1.35
25yr3d	B2	63.2555	25.32	1.49
25yr3d	B2	63.5061	25.35	1.63
25yr3d	B2	63.7556	25.39	1.78
25yr3d	B2	64.0051	25.42	1.93
25yr3d	B2	64.2527	25.45	2.09
25yr3d	B2	64.5019	25.48	2.25
25yr3d	B2	64.7519	25.51	2.42
25yr3d	B2	65.0019	25.53	2.59
25yr3d	B2	65.2552	25.56	2.76
25yr3d	B2	65.5011	25.58	2.94
25yr3d	B2	65.7502	25.60	3.12
25yr3d	B2	66.0002	25.62	3.30
25yr3d	B2	66.2544	25.65	3.49
25yr3d	B2	66.5002	25.66	3.67
25yr3d	B2	66.7519	25.68	3.87
25yr3d	B2	67.0019	25.70	4.06
25yr3d	B2	67.2519	25.72	4.26
25yr3d	B2	67.5019	25.74	4.45
25yr3d	B2	67.7519	25.76	4.65
25yr3d	B2	68.0019	25.77	4.86
25yr3d	B2	68.2519	25.79	5.06
25yr3d	B2	68.5019	25.80	5.27
25yr3d	B2	68.7519	25.82	5.48
25yr3d	B2	69.0019	25.83	5.69
25yr3d	B2	69.2519	25.84	5.90
25yr3d	B2	69.5019	25.86	6.11
25yr3d	B2	69.7519	25.87	6.33
25yr3d	B2	70.0019	25.88	6.54
25yr3d	B2	70.2519	25.89	6.76
25yr3d	B2	70.5019	25.90	6.98
25yr3d	B2	70.7519	25.91	7.19
25yr3d	B2	71.0019	25.92	7.41
25yr3d	B2	71.2519	25.93	7.63
25yr3d	B2	71.5019	25.93	7.85
25yr3d	B2	71.7519	25.94	8.08
25yr3d	B2	72.0019	25.95	8.30
25yr3d	B2	76.0019	25.97	11.81
25yr3d	B2	80.0019	25.97	15.11
25yr3d	B2	84.0019	25.96	18.34
25yr3d	B2	88.0019	25.95	21.65
25yr3d	B2	92.0019	25.93	25.03

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	96.0019	25.90	28.45
25yr3d	B2	100.0019	25.87	31.88
25yr3d	B2	104.0019	25.83	35.40
25yr3d	B2	108.0019	25.79	39.04
25yr3d	B2	112.0019	25.74	42.70
25yr3d	B2	116.0019	25.69	46.26
25yr3d	B2	120.0019	25.64	49.67
25yr3d	B2	124.0019	25.59	52.93
25yr3d	B2	128.0019	25.55	56.05
25yr3d	B2	132.0019	25.50	59.03
25yr3d	B2	136.0019	25.45	61.86
25yr3d	B2	140.0019	25.40	64.55
25yr3d	B2	144.0019	25.35	67.10
25yr3d	B2	148.0019	25.31	69.53
25yr3d	B2	152.0019	25.27	71.84
25yr3d	B2	156.0019	25.23	74.06
25yr3d	B2	160.0019	25.19	76.17
25yr3d	B2	164.0019	25.16	78.20
25yr3d	B2	168.0019	25.13	80.14
25yr3d	B2	172.0019	25.10	82.01
25yr3d	B2	176.0019	25.07	83.80
25yr3d	B2	180.0019	25.04	85.52
25yr3d	B2	184.0019	25.02	87.17
25yr3d	B2	188.0019	24.99	88.77
25yr3d	B2	192.0019	24.97	90.30
25yr3d	B2	196.0019	24.94	91.78
25yr3d	B2	200.0019	24.92	93.20
25yr3d	B2	204.0019	24.90	94.57
25yr3d	B2	208.0019	24.88	95.90
25yr3d	B2	212.0019	24.86	97.18
25yr3d	B2	216.0019	24.84	98.41
25yr3d	B2	220.0019	24.82	99.61
25yr3d	B2	224.0019	24.80	100.76
25yr3d	B2	228.0019	24.78	101.88
25yr3d	B2	232.0019	24.76	102.96
25yr3d	B2	236.0019	24.75	104.00
25yr3d	B2	240.0019	24.73	105.01
25yr3d	B2	244.0019	24.72	105.99
25yr3d	B2	248.0019	24.70	106.93
25yr3d	B2	252.0019	24.69	107.85
25yr3d	B2	256.0019	24.67	108.74
25yr3d	B2	260.0019	24.66	109.60

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	264.0019	24.64	110.43
25yr3d	B2	268.0019	24.63	111.24
25yr3d	B2	272.0019	24.62	112.02
25yr3d	B2	276.0019	24.61	112.78
25yr3d	B2	280.0019	24.59	113.52
25yr3d	B2	284.0019	24.58	114.23
25yr3d	B2	288.0019	24.57	114.93
25yr3d	B2	292.0019	24.56	115.60
25yr3d	B2	296.0019	24.55	116.26
25yr3d	B2	300.0019	24.54	116.89
25yr3d	B2	304.0019	24.53	117.51
25yr3d	B2	308.0019	24.52	118.11
25yr3d	B2	312.0019	24.51	118.69
25yr3d	B2	316.0019	24.50	119.25
25yr3d	B2	320.0019	24.49	119.80
25yr3d	B2	324.0019	24.48	120.34
25yr3d	B2	328.0019	24.47	120.86
25yr3d	B2	332.0019	24.46	121.36
25yr3d	B2	336.0019	24.45	121.85
25yr3d	B2	340.0019	24.44	122.32
25yr3d	B2	344.0019	24.44	122.79
25yr3d	B2	348.0019	24.43	123.24
25yr3d	B2	352.0019	24.42	123.67
25yr3d	B2	356.0019	24.41	124.10
25yr3d	B2	360.0019	24.40	124.51

IBIS LANDING (FKA COPPERHEAD)
WATER MANAGEMENT REPORT

PROJECT INFORMATION

This application is a request for a permit modification to the Ibis Landing Environmental Resource Permit (Permit Number 36-108291-P) for site plan revisions in the southeast corner of the project. Ibis Landing is a partially constructed 298.86-acre residential golf course project formerly known as Copperhead. The site is located along Beth Stacey Blvd, south of SW 23rd Street in Lehigh Acres, Lee County, Florida. The project is being developed by Lennar under the name Ibis Landing.

PROJECT BACKGROUND

On April 13, 2000, Permit Number 36-03765-P (Application Number 990415-2) was approved for construction and operation approval of a surface water management of a residential golf course development located on a 297.8-acre site known as Copperhead. The entire surface water management system, golf course and the first phase of residential development (approximately 100 lots) were constructed in 2001 and placed in service.

A new ERP (Permit Number 36-108291-P) was issued to reauthorize construction of the undeveloped residential areas in September 2023. The residential infrastructure for the western and northeastern areas of the project have been constructed over the last two years. Beth Stacy Blvd (Milwaukee Blvd) will be extended from north to south as part of the Ibis Landing project. This roadway will ultimately be owned by Lee County and will drain to the project's surface water management system.

SURFACE WATER MANAGEMENT SYSTEM

The existing onsite surface water management system consists of 9 primary surface water management basins which use the existing lakes and dry detention areas for treatment and storage. A 10th basin was proposed for the southern (offsite) portion of Beth Stacey Blvd that has not been constructed. The site is surrounded by Lehigh Acres Municipal Services Improvement District (LAMSID) Yellowtail Canal and Yellowtail Canal Extensions on the West, South and East sides. The project discharges to these canals which are part of the Orange River Watershed. The discharge rate is 30.1 cfm which is the allowable rate per LAMSID. No modifications to the previously permitted wetland preserves are proposed.

Stormwater calculations are only being provided for the basins that are being modified (Basins 2-5). No changes to the permitted control elevations are proposed. Per the permit, the eastern portion of the site (modified area) has a control elevation of 24.0' NGVD. The Ibis Landing Surface Water Management System provides the required water quality treatment and attenuation for the 25-year 3-day storm event. Water quality treatment volume is one inch plus an additional 50%. Water quality calculations are provided at the end of the calculations for the basins that discharge offsite (Basins 2).

The eastern portion of the site (Basins 1, 3, 4, 5 and 6) discharge to Basin 2, a 26.62 wetland which discharges to the east to the Yellowtail Canal. This permitted drainage pattern will be maintained. No modification to any weirs that discharge offsite are planned.

PROPOSED PROJECT MODIFICATIONS

This application proposes site plan revisions within Basins 3 and 4 in the southeast corner of the project. The existing lake in Basins 3 (Lake 3) is being reshaped which results in a increase in the lake size of 0.1 acres.

The existing lake in Basin 4 (Lake 4) is being reshaped which results in a increase in the lake size of 0.07 acres. Basins 3 and Basin 4 are also connected to help equalize the stages in these basins. The outfall location in Basin 4 is being maintained and will operate as previously permitted/constructed. The outfall location is Basin 3 is being modified to accommodate the lake reshaping. The bleeder in Basin 3 was sized to account for both Basin 3 and Basin 4 water quality bleed down. Minor revisions to Beth Stacey Blvd are also proposed which consist of providing closed drainage (curb and gutter) for the roadway along with the addition of a second 8' concrete sidewalk along the roadway (which is within Basin 5). The proposed Basin 10, which provided storage for runoff from the southern portion of Beth Stacey Blvd will be eliminated and the roadway will drain to Basins 4 and 5 which increase the sizes of these basins. Approximately 250' offsite roadway will drain to the proposed roadway swales. Compensating water quality storage will be provided for this area of the offsite roadway within Basin 5.

WATER QUALITY

All basins provide the required water quality treatment prior to connecting to the onsite wetland or before discharging offsite. Water quality treatment is provided in the interconnected lakes and dry detention ponds throughout the site. The required water quality volume of 150% of the greater of one inch or 2.5" over the impervious areas is provided. Compensating water quality treatment in the amount of 2.5 inches times the offsite impervious area is provided for the offsite portion of Beth Stacey Blve.

Basin #	Total Area (AC)	Control Elev. (NGVD)	Sub-basins Included	Treatment Provided By	Final Outfall WCS
1	49.33	24.00	B1L1 & B1L2	Wet Detention	WCS 2
2	32.44	24.00	N/A	Dry Detention	WCS 2
3	16.53	24.00	N/A	Wet Detention	WCS 2
4	18.34	24.00	N/A	Wet Detention	WCS 2
5	131.83	24.50	B5L5, B5L6, B5L7, B5L9, B5L10, B5L11	Wet Detention	WCS 2
6	18.20	24.00	N/A	Dry Detention	WCS 2
7	9.40	25.50	N/A	Wet Detention	WCS 7
8	6.11	25.50	N/A	Dry Detention	WCS 8
9	16.68	25.50	N/A	Dry Detention	WCS 8

Basin	CE	Proposed Basin Area	Permitted WQ V (ac-ft)	Proposed Construction WQ V (ac-ft)
B1 (P1-A, P1-B, P2-A, P2-B, P2-C & P2-D)	24.00	49.33	3.81	N/A
B2 (WA)	24.00	32.44	2.97	N/A
B3 (P3)	24.00	16.53	1.82	2.07
B4 (P4)	24.00	18.34	1.76	2.51
B5 (P5, P6, P7, P8-A, P8-B, P9, P10, & P11)	24.50	131.83	23.00	N/A
B6 (WC-A, WC-B, & WC-C)	24.00	18.20	1.75	N/A
B7 (P13)	25.50	9.40	1.18	N/A
B8 (WD)	25.50	6.11	0.93	N/A
B9 (P12)	25.50	16.68	0.38	N/A
	Totals	**298.86**	**37.60**	**4.58**

The proposed project provides 150% of the 1 inch over the project area. 298.86 x 1.5 inches = ***37.36 ac-ft***

The proposed project provides 2.5 inches times the percentage of imperviousness for the offsite improvements. 0.22 ac x 2.5 inches = ***0.05 ac-ft***

Total Required = ***37.41 ac-ft***
Total Provided = ***42.18 ac-ft***

WATER QUANTITY AND MODEL

The project is divided into 9 Basins with Basin 1 and Basin 5 having multiple interconnected lakes/sub-basins. The land use, CN, and stage storage was calculated by sub-basin. An Interconnected Pond Routing model (ICPR 4) was created to simulate the flood routing for the existing SWM system design. The weirs used in the model for Basins 1, 4, 5, 6, & 9 are modeled as they exist. The weir between Lake 3 and the downstream Wetland is proposed to be constructed. This weir will provide pre-treatment for areas that connect to Wetlands. The weirs, for Basins 2, 7, and 8, that discharge offsite will operate as previously permitted/constructed to meet the design parameters of water quality, allowable bleed down, and allowable discharge for SFWMD.

The design elevations are as follows and the flood routing results are included for comparison in the tables on the next page.

	CONDITION	
	Permitted (App#230103-37076)	Proposed (Current)
BASIN	WA	B2
5-YR	24.68	24.66
10-YR	24.86	24.84
25-YR	25.94	25.97
100-YR	27.42	27.43
DISCHARGE	10.98	11.09

	CONDITION	
	Permitted (App#230103-37076)	Proposed (Current)
BASIN	P3	B3
5-YR	25.43	25.78
10-YR	25.61	25.92
25-YR	26.81	26.86
100-YR	27.42	27.43
DISCHARGE	N/A (Internal)	N/A

BASIN	P4	B4
5-YR	25.64	25.79
10-YR	25.97	25.97
25-YR	26.68	26.73
100-YR	27.42	27.43
DISCHARGE	N/A (Internal)	N/A (Internal)

BASIN	P5-P11,P14-P15	B5
5-YR	25.54	25.54
10-YR	25.87	25.87
25-YR	27.32	27.32
100-YR	27.94	27.94
DISCHARGE	N/A (Internal)	N/A

The next page contains a land use comparison between the total permitted land use from Permit No. 36-108291-P application 230103-37076 and the proposed modification.

LAND USE COMPARISON

	Permitted Total	% of Proposed
Basin	B2	
Building	0.00	0.0%
Impervious	0.16	0.5%
Pervious	7.15	22.0%
Dry Detentiion	0.44	1.4%
Wetland	24.69	76.1%
Total	**32.44**	100.0%
Basin	B3	
Building	3.24	19.6%
Impervious	4.59	27.8%
Pervious	7.29	44.1%
Lake	1.41	8.5%
Total	**16.53**	100.0%
Basin	B4	
Building	2.37	13.3%
Impervious	3.74	21.0%
Pervious	10.33	58.0%
Lake	1.37	7.7%
Total	**17.81**	100.0%
Basin	B5	
Building	16.24	12.4%
Impervious	24.18	18.5%
Pervious	71.75	54.8%
Dry Detention	0.07	0.1%
Lake	18.64	14.2%
Total	**130.88**	100.0%
Basin	B10	
Building	0.04	2.7%
Impervious	0.12	8.1%
Pervious	1.11	75.0%
Dry Detention	0.21	14.2%
Total	**1.48**	100.0%

	Proposed Modification Total	% of Permitted
	B2	
	0.00	0.0%
	0.16	0.5%
	7.15	22.0%
	0.44	1.4%
	24.69	76.1%
	32.44	100.0%
	B3	
	3.05	18.5%
	4.71	28.5%
	7.35	44.5%
	1.42	8.6%
	16.53	100.0%
	B4	
	1.36	7.4%
	7.38	40.2%
	8.16	44.5%
	1.44	7.9%
	18.34	100.0%
	P5, P6, P7, P8-A, P8-B, P9, P10, & P11	
	16.25	12.3%
	24.73	18.8%
	72.14	54.7%
	0.07	0.1%
	18.64	14.1%
	131.83	100.0%

Basin 10 has been removed and added to Basins 4 and 5 as shown above.

	Permitted Total	% of Proposed
Impervious		
Building	21.89	11.0%
Impervious	32.79	16.5%
Lake	21.42	10.8%
Total	76.10	38.5%

Proposed Modification Total	% of Permitted
20.66	10.4%
36.98	18.6%
21.50	10.8%
79.14	39.8%

OFFSITE DISCHARGES

CS	Contributing Basins	Contributing Area (AC)	Allowable Discharge* (cfs)	Model Discharge (cfs)
WCS-2	B1, B2, B3, B4, B5, & B6	266.67	12.54	11.10

* Based on 30.1 SCM

The project lies within the Lehigh Acres Municipal Services Improvement District which has an allowable discharge limit of 30.1 CSM. The proposed surface water management system for the 266.67-acre basin has an allowable discharge rate of 12.54 cfs. The total discharge is 11.10 cfs which is less than allowed.

TOTAL SITE

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	26.04	8.7%	0.00	0.00	0.00	26.04
LAKE BANK @ CE + 1	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	2.28	0.8%	2.28	0.00	0.00	0.00
LAKE TOB	2.28	0.8%	2.28	0.00	0.00	0.00
DRY DETENTION	1.65	0.6%	1.65	0.00	0.00	0.00
PRESERVE	37.83	12.7%	37.83	0.00	0.00	0.00
ROW	32.59	10.9%	13.04	19.55	0.00	0.00
SF	63.92	21.4%	25.57	19.18	19.18	0.00
MF	25.26	8.5%	5.05	10.10	10.10	0.00
GOLF COURSE	96.36	32.2%	93.47	2.89	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	0.5%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	2.3%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**298.86**	**100%**	**185.12**	**55.00**	**32.56**	**26.18**

TOTAL PERVIOUS AREA	185.12	61.9%	
TOTAL IMPERVIOUS AREA	113.74	38.1%	*(INCLUDES WATER SURFACE AREA)*

TOTAL SITE

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 61.9% x 6.57 in.
S = 4.07 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 71

III. SURFACE STORAGE

Basin Area =	298.86	ac.			
Bldg. Coverage Area =	32.56	ac.			
Impervious =	55.00				
Lake Area (at Control)	26.18	ac.	***EL. =***	***24.00***	***(NGVD)***
Preserve Area (at control)	37.83	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less Preserved)=	147.29	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**26.18**	**0.00**	**0.00**	**0.00**	**26.18**	**0.00**
25.00	**28.32**	**0.00**	**0.00**	**0.00**	**28.32**	**27.25**
26.50	**30.60**	**0.00**	**0.00**	**0.00**	**30.60**	**71.44**
27.00	**32.88**	**7.57**	**29.46**	**27.50**	**97.41**	**103.44**
27.50	**32.93**	**15.13**	**58.92**	**49.50**	**156.48**	**166.92**
28.50	**32.93**	**18.92**	**132.57**	**55.00**	**239.41**	**364.86**

IV. WATER QUALITY DETENTION VOLUME

TOTAL SITE

Total Area =	298.86	ac.
Bldg. Coverage =	32.56	ac.
Water Surface Area =	26.18	ac.
Pervious Area =	185.12	ac.

First one (1) Inch of runoff over basin:

= 1" x 298.86 ac. x1'/12" = **24.91** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	240.12	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	55.00	ac.
% Impervious = Impervious Area / Site Area	22.9%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**13.01**	**ac-ft**
Required Water Quality Detention Volume	**24.91**	**ac-ft**
Additional 50% Water Quality Detention Volume	**12.45**	**ac-ft**
Total Required Water Quality Detention Volume	**37.36**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**37.36**	**ac-ft**

Between El. 24.00 and 25.34

WCS Stage-Storage Table

Stage (NGVD)	Site Storage Area ac	Site Storage Volume ac-ft
24.00	26.18	0.00
25.00	28.32	27.25
26.50	30.60	71.44
27.00	97.41	103.44
27.50	156.48	166.92
28.50	239.41	364.86

Water Quality Min. Weir Elevation = **25.34** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event:

= 55.0 CSM x WCS Area x 1 SM/640 ac.

= **25.68** **cfs**

METHOD OF DETERMINATION:		
Orange River Watershed (Approved ERP Appl # 060503-10)	55.0	CSM

Basin B2

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 1	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.00	0.0%	0.00	0.00	0.00	0.00
LAKE TOB	0.00	0.0%	0.00	0.00	0.00	0.00
DRY DETENTION	0.44	1.4%	0.44	0.00	0.00	0.00
PRESERVE	26.62	82.1%	26.62	0.00	0.00	0.00
ROW	0.00	0.0%	0.00	0.00	0.00	0.00
SF	0.00	0.0%	0.00	0.00	0.00	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	5.38	16.6%	5.22	0.16	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**32.44**	**100%**	**32.28**	**0.16**	**0.00**	**0.00**

TOTAL PERVIOUS AREA	32.28	99.5%
TOTAL IMPERVIOUS AREA	0.16	0.5%

Basin B2

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 99.5% x 6.57 in.
S = 6.53 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 60

III. SURFACE STORAGE

Basin Area =	32.44	ac.			
Bldg. Coverage Area =	0.00	ac.			
Dry Detention TOS (at CE + 1)	0.44	ac.	***EL. =***	***25.00***	***(NGVD)***
Dry Detention TOB (at CE + 2)	0.54	ac.	***EL. =***	***26.00***	***(NGVD)***
Golf Course	5.22	ac.			
Preserve Area (at control)	26.62	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area =	32.28	ac.			

Stage-Storage Table

Stage (NGVD)	Dry Detention ac	Preserve Area ac	Golf Course ac	Total Area ac	Total Volume ac-ft
24.00	**0.00**	**26.62**	**0.00**	**26.62**	**0.00**
25.00	**0.44**	**26.62**	**0.00**	**27.06**	**26.84**
26.50	**0.44**	**26.62**	**1.04**	**28.10**	**68.21**
27.00	**0.44**	**26.62**	**2.09**	**29.15**	**82.53**
27.50	**0.44**	**26.62**	**2.61**	**29.67**	**97.23**

IV. WATER QUALITY DETENTION VOLUME
The permitted 2.97 ac-ft of water quality was provided in the existing condition and no modification to the basin is proposed.

V. ALLOWABLE DISCHARGE

BASIN	**2**	
Contributing Flows		
Basin	1	49.33
Basin	2	32.44
Basin	3	16.53
Basin	4	18.34
Basin	5	131.83
Basin	6	18.20
Total Area =		266.68 ac.

Peak Allowable Discharge for 25 year - 3 Day Rainfall Event:

= 55.0 CSM x WCS Area x 1 SM/640 ac.

= **22.92** **cfs**

METHOD OF DETERMINATION:		
55 CSM allowed in Orange River Watershed & Approved ERP Appl # 994015-2	55.0	CSM

VI. WEIR DESIGN

Total Basin

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**266.68 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**11.11 Ac-FT**
½" Discharge Rate:	=		=	**5.72 CFS**
Water Quality Stage:	=		=	**24.56 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**22.22 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation)	=	**24.23 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**24.39 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)	=	**0.39 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q **=** 5.72 **CFS**

Weir Length (L)

H **=** height above weir

$L = Q/(3.13)(H^{1.5})$

L= 7.42 Length of weir **A 1.50 ft wide rectangular weir was used to meet the maximum discharge rate**

Basin B3L3

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.42	8.6%	0.00	0.00	0.00	1.42
LAKE BANK @ CE + 1	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.14	0.8%	0.14	0.00	0.00	0.00
LAKE TOB	0.14	0.8%	0.14	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	2.62	15.8%	1.05	1.57	0.00	0.00
SF	5.83	35.3%	2.33	1.75	1.75	0.00
MF	3.25	19.7%	0.65	1.30	1.30	0.00
GOLF COURSE	3.00	18.1%	2.91	0.09	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.53**	**100%**	**7.35**	**4.71**	**3.05**	**1.42**

TOTAL PERVIOUS AREA	7.35	44.5%	
TOTAL IMPERVIOUS AREA	9.18	55.5%	*(INCLUDES WATER SURFACE AREA)*

Basin B3L3

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 44.5% x 6.57 in.
S = 2.92 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 77

III. SURFACE STORAGE

Basin Area =	16.53	ac.			
Bldg. Coverage Area =	3.05	ac.			
Lake Area (at Control)	1.42	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.56	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.70	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.83	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	2.91	ac.			
Pervious Area (less bank & GC) =	4.03	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.42**	**0.00**	**0.00**	**0.00**	**1.42**	**0.00**
25.00	**1.56**	**0.00**	**0.00**	**0.00**	**1.56**	**1.49**
26.50	**1.70**	**0.00**	**0.00**	**0.00**	**1.70**	**3.93**
27.00	**1.83**	**0.58**	**0.81**	**2.36**	**5.58**	**5.75**
27.50	**1.83**	**1.16**	**1.61**	**4.24**	**8.85**	**9.35**
28.50	**1.83**	**1.46**	**3.63**	**4.71**	**11.63**	**19.59**

IV. WATER QUALITY DETENTION VOLUME

BASIN 3

Total Area =	16.53	ac.
Bldg. Coverage =	3.05	ac.
Water Surface Area =	1.42	ac.
Pervious Area =	7.35	ac.

First one (1) Inch of runoff over basin:

= 1" x 16.53 ac. x1'/12" = **1.38** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	12.06	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	4.71	ac.
% Impervious = Impervious Area / Site Area	39.0%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.23**	**ac-ft**
Required Water Quality Detention Volume	**1.38**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.69**	**ac-ft**
Total Required Water Quality Detention Volume	**2.07**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.07**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.42	0.00
25.00	1.56	1.49
26.50	1.70	3.93
27.00	1.83	4.81
27.50	1.83	5.73
28.50	1.83	7.56

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.42 ac + 1.44 ac (Lake 3 and Lake 4) = 2.86 ac, the WQ is provided at 25.60 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin (The required bleed down for B4L4 is accounted for in this basin)

Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**34.88 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**1.45 Ac-FT**
½" Discharge Rate:	=		=	**0.75 CFS**
Water Quality Stage:	=		=	**25.60 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)=		**3.12 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	(Treatment Vol. After 24 Hr. / Total Basin Storage) (+ Control Elevation) =		**24.07 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2 =		**24.84 Ft-NAVD**
Average Height During Bleed Down:	=	(Ave. Elev. During Bleed Down - Control Elev.)=		**0.84 Ft**

Size Bleeder:

Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q **=** 0.75 **CFS**

Weir Length (L)

H **=** height above weir

$L = Q/(3.13)(H^{1.5})$

L= 0.31 Length of weir **A 0.31 ft wide rectangular weir was used to meet the maximum bleeder rate**

Basin B4L4

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.44	7.9%	0.00	0.00	0.00	1.44
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	9.88	53.9%	3.95	5.93	0.00	0.00
SF	1.42	7.7%	0.57	0.43	0.43	0.00
MF	2.34	12.8%	0.47	0.94	0.94	0.00
GOLF COURSE	2.87	15.6%	2.78	0.09	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**18.34**	**100%**	**8.16**	**7.38**	**1.36**	**1.44**

TOTAL PERVIOUS AREA	8.16	44.5%	
TOTAL IMPERVIOUS AREA	10.18	55.5%	*(INCLUDES WATER SURFACE AREA)*

Basin B4L4

Wet Season Water Table (WSWT)	=	24.00	NGVD
Min. Centerline Road Elev.	=	26.50	NGVD
Average Developed Grade Elev.	=	27.50	NGVD
Min. Floor Elev.	=	28.50	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 27.50 - 24.00
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57 in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 44.5% x 6.57 in.
S = 2.92 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 77

III. SURFACE STORAGE

Basin Area =	18.34	ac.			
Bldg. Coverage Area =	1.36	ac.			
Lake Area (at Control)	1.44	ac.	***EL. =***	***24.00***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.57	ac.	***EL. =***	***25.00***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.70	ac.	***EL. =***	***27.00***	***(NGVD)***
Lake Bank Area (at TOB)	1.83	ac.	***EL. =***	***27.00***	***(NGVD)***
Golf Course	0.00	ac.	***EL. =***	***24.00***	***(NGVD)***
Pervious Area (less bank & GC) =	4.99	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.00	**1.44**	**0.00**	**0.00**	**0.00**	**1.44**	**0.00**
25.00	**1.57**	**0.00**	**0.00**	**0.00**	**1.57**	**1.51**
26.50	**1.70**	**0.00**	**0.00**	**0.00**	**1.70**	**3.96**
27.00	**1.83**	**0.56**	**1.00**	**3.69**	**7.07**	**6.15**
27.50	**1.83**	**1.11**	**2.00**	**6.64**	**11.58**	**10.82**
28.50	**1.83**	**1.39**	**4.49**	**7.38**	**15.09**	**24.15**

IV. WATER QUALITY DETENTION VOLUME

BASIN 4

Total Area =	18.34	ac.
Bldg. Coverage =	1.36	ac.
Water Surface Area =	1.44	ac.
Pervious Area =	8.16	ac.

First one (1) Inch of runoff over basin:

= 1" x 18.34 ac. x1'/12" = **1.53** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	15.54	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	7.38	ac.
% Impervious = Impervious Area / Site Area	47.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**1.67**	**ac-ft**
Required Water Quality Detention Volume	**1.67**	**ac-ft**
Additional 50% Water Quality Detention Volume	**0.84**	**ac-ft**
Total Required Water Quality Detention Volume	**2.51**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**2.51**	**ac-ft**

Between El. 24.00 and 0.00

Stage-Storage Table for Water Quality Calculations

Stage	Lake Storage Area	Site Storage Volume
(NGVD)	ac	ac-ft
24.00	1.44	0.00
25.00	1.57	1.51
26.50	1.70	3.96
27.00	1.83	4.84
27.50	1.83	5.76
28.50	1.83	7.59

Based on the designed weir the provided WQ is 2.07 ac-ft + 2.51 ac-ft (Basins 3 and 4) = 4.58 ac-ft. With an assumed vertical area of 1.16 ac + 1.44 ac (Lake 3 and Lake 4) = 2.60 ac, the WQ is provided at 25.76 ft.

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

The Existing weir, as approved in ERP 36-03765-P (App 990415-2) was utilized.

Basin B5L5

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.61	15.7%	0.00	0.00	0.00	2.61
LAKE BANK @ CE + 1	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.23	1.4%	0.23	0.00	0.00	0.00
LAKE TOB	0.23	1.4%	0.23	0.00	0.00	0.00
PRESERVE	0	0.0%	0.00	0.00	0.00	0.00
ROW	0.66	4.0%	0.26	0.40	0.00	0.00
SF	2	12.1%	0.80	0.60	0.60	0.00
MF	3.99	24.1%	0.80	1.60	1.60	0.00
GOLF COURSE	6.62	39.9%	6.42	0.20	0.00	0.00
REC/POOL	0	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
xxx	0	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**16.58**	**100%**	**8.98**	**2.79**	**2.20**	**2.61**

TOTAL PERVIOUS AREA 8.98 54.2%
TOTAL IMPERVIOUS AREA 7.60 45.8% *(INCLUDES WATER SURFACE AREA)*

Basin B5L5

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 54.2% x 6.57 in.
S = 3.56 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	16.58	ac.			
Bldg. Coverage Area =	2.20	ac.			
Lake Area (at Control)	2.61	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.84	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.08	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	3.31	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	6.62	ac.			
Pervious Area (less bank & GC) =	1.86	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.61**	**0.00**	**0.00**	**0.00**	**2.61**	**0.00**
25.50	**2.84**	**0.00**	**0.00**	**0.00**	**2.84**	**2.73**
27.00	**3.08**	**0.00**	**0.00**	**0.00**	**3.08**	**7.17**
27.50	**3.31**	**1.32**	**0.37**	**1.40**	**6.40**	**9.54**
28.00	**3.31**	**2.65**	**0.74**	**2.51**	**9.22**	**13.44**
29.00	**3.31**	**3.31**	**1.68**	**2.79**	**11.09**	**23.60**

Lake B5L6

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.23	9.6%	0.00	0.00	0.00	1.23
LAKE BANK @ CE + 1	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.16	1.2%	0.16	0.00	0.00	0.00
LAKE TOB	0.16	1.2%	0.16	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	1.40	10.9%	0.56	0.84	0.00	0.00
SF	5.62	43.7%	2.25	1.69	1.69	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	4.14	32.2%	4.01	0.12	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**12.86**	**100%**	**7.29**	**2.65**	**1.69**	**1.23**

TOTAL PERVIOUS AREA	7.29	56.7%	
TOTAL IMPERVIOUS AREA	5.57	43.3%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L6

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 56.7% x 6.57 in.
S = 3.72 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 73

III. SURFACE STORAGE

Basin Area =	12.86	ac.			
Bldg. Coverage Area =	1.69	ac.			
Lake Area (at Control)	1.23	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	0.16	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	0.16	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Bank Area (at TOB)	1.39	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.01	ac.			
Pervious Area (less bank & GC) =	2.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.23**	**0.00**	**0.00**	**0.00**	**1.23**	**0.00**
25.50	**1.31**	**0.00**	**0.00**	**0.00**	**1.31**	**1.27**
27.00	**1.39**	**0.00**	**0.00**	**0.00**	**1.39**	**3.29**
27.50	**1.39**	**0.80**	**0.56**	**1.33**	**4.08**	**4.66**
28.00	**1.39**	**1.61**	**1.12**	**2.39**	**6.50**	**7.30**
29.00	**1.39**	**2.01**	**2.53**	**2.65**	**8.57**	**14.84**

Lake B5L7

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	9.70	20.4%	0.00	0.00	0.00	9.70
LAKE BANK @ CE + 1	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.71	1.5%	0.71	0.00	0.00	0.00
LAKE TOB	0.71	1.5%	0.71	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	3.50	7.4%	1.40	2.10	0.00	0.00
SF	7.01	14.8%	2.80	2.10	2.10	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	18.16	38.2%	17.61	0.54	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	7.00	14.7%	1.40	2.80	2.80	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**47.51**	**100%**	**25.36**	**7.55**	**4.90**	**9.70**

TOTAL PERVIOUS AREA	25.36	53.4%	
TOTAL IMPERVIOUS AREA	22.15	46.6%	*(INCLUDES WATER SURFACE AREA)*

Lake B5L7

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.4% x 6.57 in.
S = 3.50 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	47.51	ac.			
Bldg. Coverage Area =	4.90	ac.			
Lake Area (at Control)	9.70	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	10.41	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	11.13	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	11.84	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	18.16	ac.			
Pervious Area (less bank & GC) =	5.60	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**9.70**	**0.00**	**0.00**	**0.00**	**9.70**	**0.00**
25.50	**10.41**	**0.00**	**0.00**	**0.00**	**10.41**	**10.06**
27.00	**11.13**	**0.00**	**0.00**	**0.00**	**11.13**	**26.21**
27.50	**11.84**	**3.63**	**1.12**	**3.77**	**20.37**	**34.09**
28.00	**11.84**	**7.26**	**2.24**	**6.79**	**28.14**	**46.21**
29.00	**11.84**	**9.08**	**5.04**	**7.55**	**33.51**	**77.04**

Lake B5L9

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	2.49	11.4%	0.00	0.00	0.00	2.49
LAKE BANK @ CE + 1	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.28	1.3%	0.28	0.00	0.00	0.00
LAKE TOB	0.28	1.3%	0.28	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.60	11.9%	1.04	1.56	0.00	0.00
SF	6.92	31.6%	2.77	2.08	2.08	0.00
MF	0.00	0.0%	0.00	0.00	0.00	0.00
GOLF COURSE	9.06	41.3%	8.79	0.27	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**21.92**	**100%**	**13.45**	**3.91**	**2.08**	**2.49**

TOTAL PERVIOUS AREA 13.45 61.3%
TOTAL IMPERVIOUS AREA 8.47 38.7% *(INCLUDES WATER SURFACE AREA)*

Lake B5L9

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 61.3% x 6.57 in.
S = 4.03 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 71

III. SURFACE STORAGE

Basin Area =	21.92	ac.			
Bldg. Coverage Area =	2.08	ac.			
Lake Area (at Control)	2.49	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	2.77	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	3.05	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	2.77	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	9.06	ac.			
Pervious Area (less bank & GC) =	3.81	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**2.49**	**0.00**	**0.00**	**0.00**	**2.49**	**0.00**
25.50	**2.77**	**0.00**	**0.00**	**0.00**	**2.77**	**2.63**
27.00	**3.05**	**0.00**	**0.00**	**0.00**	**3.05**	**7.00**
27.50	**3.05**	**1.81**	**0.76**	**1.95**	**7.58**	**9.66**
28.00	**3.05**	**3.63**	**1.52**	**3.52**	**11.72**	**14.49**
29.00	**3.05**	**4.53**	**3.43**	**3.91**	**14.92**	**27.81**

Lake B5L10

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.42	9.3%	0.00	0.00	0.00	1.42
LAKE BANK @ CE + 1	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.17	1.1%	0.17	0.00	0.00	0.00
LAKE TOB	0.17	1.1%	0.17	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	0.61	4.0%	0.24	0.37	0.00	0.00
SF	1.37	9.0%	0.55	0.41	0.41	0.00
MF	4.10	26.9%	0.82	1.64	1.64	0.00
GOLF COURSE	5.85	38.5%	5.68	0.18	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	1.37	9.0%	0.27	0.48	0.48	0.14
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**15.22**	**100%**	**8.06**	**3.07**	**2.53**	**1.56**

TOTAL PERVIOUS AREA 8.06 53.0%
TOTAL IMPERVIOUS AREA 7.16 47.0% *(INCLUDES WATER SURFACE AREA)*

Lake B5L10

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 53.0% x 6.57 in.
S = 3.48 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 74

III. SURFACE STORAGE

Basin Area =	15.22	ac.			
Bldg. Coverage Area =	2.53	ac.			
Lake Area (at Control)	1.56	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.72	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.89	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.92	ac.	***EL. =***	***26.50***	***(NGVD)***
Golf Course	5.68	ac.			
Pervious Area (less bank & GC) =	1.89	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.56**	**0.00**	**0.00**	**0.00**	**1.56**	**0.00**
25.50	**1.72**	**0.00**	**0.00**	**0.00**	**1.72**	**1.64**
27.00	**1.89**	**0.00**	**0.00**	**0.00**	**1.89**	**4.35**
27.50	**1.92**	**1.14**	**0.38**	**1.54**	**4.96**	**6.06**
28.00	**1.92**	**2.27**	**0.75**	**2.76**	**7.71**	**9.23**
29.00	**1.92**	**2.84**	**1.70**	**3.07**	**9.52**	**17.84**

Lake B5L11

LAND USE	TOTAL	PERCENTAGE	PERVIOUS AREA	IMPERVIOUS PAVEMENT	IMPERVIOUS ROOF	IMPERVIOUS WATER
	(ac)	(%)	(ac)	(ac)	(ac)	(ac)
LAKE @ CE	1.05	5.9%	0.00	0.00	0.00	1.05
LAKE BANK @ CE + 1	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE BANK @ CE + 2	0.13	0.7%	0.13	0.00	0.00	0.00
LAKE TOB	0.13	0.7%	0.13	0.00	0.00	0.00
PRESERVE	0.00	0.0%	0.00	0.00	0.00	0.00
ROW	2.94	16.6%	1.18	1.76	0.00	0.00
SF	6.61	37.3%	2.64	1.98	1.98	0.00
MF	2.20	12.4%	0.44	0.88	0.88	0.00
GOLF COURSE	4.55	25.6%	4.41	0.14	0.00	0.00
REC/POOL	0.00	0.0%	0.00	0.00	0.00	0.00
MAINTENANCE	0.00	0.0%	0.00	0.00	0.00	0.00
FILTER MARSH	0.00	0.0%	0.00	0.00	0.00	0.00
CLUBHOUSE	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
xxx	0.00	0.0%	0.00	0.00	0.00	0.00
TOTAL BASIN AREA	**17.74**	**100%**	**9.06**	**4.76**	**2.86**	**1.05**

TOTAL PERVIOUS AREA 9.06 51.1%
TOTAL IMPERVIOUS AREA 8.68 48.9% *(INCLUDES WATER SURFACE AREA)*

Lake B5L11

Wet Season Water Table (WSWT)	=	24.50	NGVD
Min. Centerline Road Elev.	=	27.00	NGVD
Average Developed Grade Elev.	=	28.00	NGVD
Min. Floor Elev.	=	29.00	NGVD

II. SOIL STORAGE

Depth to Water Table:
= (Average Developed Grade)-(Wet Season Water Table)
= 28.00 - 24.50
= 3.50 ft.
Available Soil Storage Depth from Vol. IV, Page C-III-3 = **6.57** **in.**

Available Soil Storage:
S = (% of Pervious Area) * (Soil Storage Depth)
S = 51.1% x 6.57 in.
S = 3.35 in.

Curve Number:
Cn = 1000/(S + 10)
Cn = 75

III. SURFACE STORAGE

Basin Area =	17.74	ac.			
Bldg. Coverage Area =	2.86	ac.			
Lake Area (at Control)	1.05	ac.	***EL. =***	***24.50***	***(NGVD)***
Lake Bank Area (at CE + 1)	1.18	ac.	***EL. =***	***25.50***	***(NGVD)***
Lake Bank Area (at CE + 2)	1.31	ac.	***EL. =***	***26.50***	***(NGVD)***
Lake Area (at TOB)	1.44	ac.	***EL. =***	***27.50***	***(NGVD)***
Golf Course	4.41	ac.			
Pervious Area (less bank & GC) =	4.26	ac.			

Stage-Storage Table

Stage	Lake Area	Golf Course	Pervious	Roadway	Total Area	Total Volume
(NGVD)	ac	ac	ac	ac	ac	ac-ft
24.50	**1.05**	**0.00**	**0.00**	**0.00**	**1.05**	**0.00**
25.50	**1.18**	**0.00**	**0.00**	**0.00**	**1.18**	**1.12**
27.00	**1.31**	**0.00**	**0.00**	**0.00**	**1.31**	**2.98**
27.50	**1.44**	**0.88**	**0.85**	**2.38**	**5.56**	**4.70**
28.00	**1.44**	**1.76**	**1.70**	**4.29**	**9.20**	**8.39**
29.00	**1.44**	**2.21**	**3.83**	**4.76**	**12.25**	**19.11**

IV. WATER QUALITY DETENTION VOLUME

BASIN 5

Total Area =	131.83	ac.
Bldg. Coverage =	16.25	ac.
Water Surface Area =	18.64	ac.
Pervious Area =	72.21	ac.

First one (1) Inch of runoff over basin:
= 1" x 131.83 ac. x1'/12" = **10.99** **ac-ft**

2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	96.94	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	24.73	ac.
% Impervious = Impervious Area / Site Area	25.5%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**6.02**	**ac-ft**
Required Water Quality Detention Volume	**10.99**	**ac-ft**
Additional 50% Water Quality Detention Volume	**5.49**	**ac-ft**

Beth Stacy Offsite Imperviousness
2.5 Inches Times the Percentage of Imperviousness:

Water Quality Treatment = Total Area - Bldg. Coverage - Water Surface Area	0.22	ac.
Impervious Area for W.Q. Treatment = Site Area for W.Q. - Pervious Area	0.22	ac.
% Impervious = Impervious Area / Site Area	100.0%	
Volume to be Treated = 2.5"x % Imperviousx(Sub-Basin Area-Water Surface Area)	**0.05**	**ac-ft**
Total Required Water Quality Detention Volume	**16.52**	**ac-ft**
Provided Water Quality Detention Volume Between El.	**16.52**	**ac-ft**

Between El. 24.00 and 25.35

Stage-Storage Table for Water Quality Calculations

Stage (NGVD)	Lake Storage Area ac	Lake Storage Volume ac-ft
24.50	18.64	0.00
25.50	20.24	19.44
27.00	21.85	51.01
27.50	22.95	62.20
28.00	22.95	73.68
29.00	22.95	96.63

23 ac-ft of total wq volume is provided

Water Quality Min. Weir Elevation = **25.35** **NGVD**
(From WCS Stage-Storage Table, Wet Storage)

V. ALLOWABLE DISCHARGE

This Basin flows into basin 2 before discharging to the outfall, so the discharge rate has been calculated for Basin 2 including this basin volume

VI. WEIR DESIGN

Total Basin
Design Control Structure to Dischrage 1/2" in 24 Hours.

Basin Area:	=		=	**131.83 Ac.**
½" Discharge Vol:	=	Basin * (1/2") * (1/12)	=	**5.49 Ac-FT**
½" Discharge Rate:	=		=	**2.83 CFS**
Water Quality Stage:	=		=	**25.67 Ft-NAVD**
Treatment Volume After 24 Hour Bleed Down:	=	(Required W.Q.) - (1/2" Discharge Volume)	=	**17.51 Ac-FT**
Lake Elevation After 24 Hour Bleed Down:	=	eatment Vol. After 24 Hr. / Total Basin Storag (+ Control Elevation)	=	**24.84 Ft-NAVD**
Average Elevation During Bleed Down:	=	(W.Q. + Elevation After 24 Hr.)/2	=	**25.26 Ft-NAVD**
Average Height During Bleed Down:	=	∖ve. Elev. During Bleed Down - Control Elev.	=	**0.76 Ft**

Size Bleeder:
Use Rectangular Weir

$Q = (3.13)(L)(H^{1.5})$

Q **=** 2.83 **CFS**

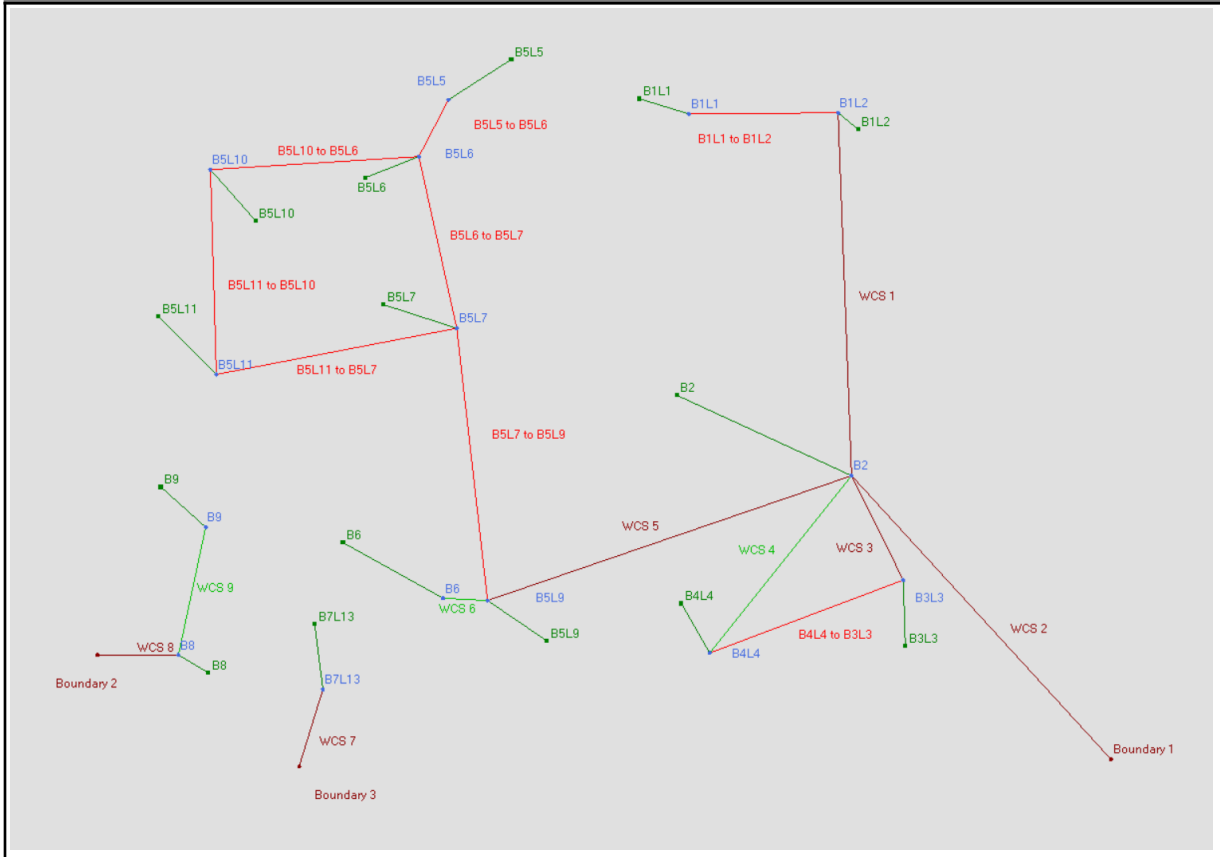
Weir Length (L)

H **=** height above weir

$L= Q/(3.13)(H^{1.5})$

L= 1.37 Length of weir **A 1.33 ft wide rectangular weir was used to meet the maximum bleeder rate**

Background Image: Background



Impervious: Scenario 1 [Set]

Land Cover Zone	% Impervious	% DCIA	% Direct	Ia/S [dec]	Ia Impervious [in]	Ia Pervious [in]
Impervious & Pervious	0.00	0.00	0.00	0.00	0.000	0.000

Curve Number: Scenario 1 [Set]

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 60	60.0
Impervious & Pervious	Calculated CN 63	63.0
Impervious & Pervious	Calculated CN 64	64.0
Impervious & Pervious	Calculated CN 70	70.0
Impervious & Pervious	Calculated CN 71	71.0
Impervious & Pervious	Calculated CN 72	72.0

Land Cover Zone	Soil Zone	Curve Number [dec]
Impervious & Pervious	Calculated CN 73	73.0
Impervious & Pervious	Calculated CN 74	74.0
Impervious & Pervious	Calculated CN 75	75.0
Impervious & Pervious	Calculated CN 76	76.0
Impervious & Pervious	Calculated CN 77	77.0

Manual Basin: B2

Scenario: Scenario1
Node: B2
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 60.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 32.4400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
32.4400	Impervious & Pervious	Calculated CN 60	

Comment:

Manual Basin: B3L3

Scenario: Scenario1
Node: B3L3
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5300 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5300	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B4L4

Scenario: Scenario1
Node: B4L4

Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 18.3400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
18.3400	Impervious & Pervious	Calculated CN 77	

Comment:

Manual Basin: B5L10

Scenario: Scenario1
Node: B5L10
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 15.2200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
15.2200	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L11

Scenario: Scenario1
Node: B5L11
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 17.7400 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
17.7400	Impervious & Pervious	Calculated CN 75	

Comment:

Manual Basin: B5L5

Scenario: Scenario1
Node: B5L5
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 16.5800 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
16.5800	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L6

Scenario: Scenario1
Node: B5L6
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 12.8600 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
12.8600	Impervious & Pervious	Calculated CN 73	

Comment:

Manual Basin: B5L7

Scenario: Scenario1
Node: B5L7
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 47.5100 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
47.5100	Impervious & Pervious	Calculated CN 74	

Comment:

Manual Basin: B5L9

Scenario: Scenario1
Node: B5L9
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 10.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 21.9200 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name
21.9200	Impervious & Pervious	Calculated CN 71	

Comment:

Node: B2

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	26.6200	1159567
25.00	27.0600	1178734
26.50	28.1000	1224036
27.00	29.1500	1269774
27.50	29.6700	1292425

Comment:

Node: B3L3

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft

Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.4200	61855
25.00	1.5600	67954
26.50	1.7000	74052
27.00	5.5800	243065
27.50	8.8500	385506
28.50	11.6300	506603

Comment:

Node: B4L4

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.00	1.4400	62726
25.00	1.5700	68389
26.50	1.7000	74052
27.00	7.0700	307969
27.50	11.5800	504425
28.50	15.0900	657320

Comment:

Node: B5L10

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.5600	67954
25.50	1.7200	74923
27.00	1.8900	82328
27.50	4.9600	216058
28.00	7.7100	335848
29.00	9.5200	414691

Comment:

Node: B5L11

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.0500	45738
25.50	1.1800	51401
27.00	1.3100	57064
27.50	5.5600	242194
28.00	9.2000	400752
29.00	12.2500	533610

Comment:

Node: B5L5

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.6100	113692
25.50	2.8400	123710
27.00	3.0800	134165
27.50	6.4000	278784
28.00	9.2200	401623
29.00	11.0900	483080

Comment:

Node: B5L6

Scenario: Scenario1
Type: Stage/Area

Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	1.2300	53579
25.50	1.3100	57064
27.00	1.3900	60548
27.50	4.0800	177725
28.00	6.5000	283140
29.00	8.5700	373309

Comment:

Node: B5L7

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	9.7000	422532
25.50	10.4100	453460
27.00	11.1300	484823
27.50	20.3700	887317
28.00	28.1400	1225778
29.00	33.5100	1459696

Comment:

Node: B5L9

Scenario: Scenario1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 24.50 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
24.50	2.4900	108464
25.50	2.7700	120661
27.00	3.0500	132858

Stage [ft]	Area [ac]	Area [ft2]
27.50	7.5800	330185
28.00	11.7200	510523
29.00	14.9200	649915

Comment:

Node: Boundary 1

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 23.00 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	23.00
0	0	0	60.0000	24.00
0	0	0	80.0000	25.00
0	0	0	100.0000	24.50
0	0	0	115.0000	23.30
0	0	0	336.0000	23.00

Comment:

Pipe Link: B4L4 to B3L3

Scenario: Scenario1
From Node: B3L3
To Node: B4L4
Link Count: 1
Flow Direction: Both
Damping: 0.0000 ft
Length: 731.00 ft
FHWA Code: 0
Entr Loss Coef: 1
Exit Loss Coef: 1
Bend Loss Coef: 0
Bend Location: 0.00 dec
Energy Switch: Energy

Upstream		Downstream	
Invert:	21.00 ft	Invert:	21.00 ft
Manning's N:	0.0130	Manning's N:	0.0130
Geometry: Circular		Geometry: Circular	
Max Depth:	3.00 ft	Max Depth:	3.00 ft
Bottom Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000
Top Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L10 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.70 ft	Invert:	20.60 ft
From Node:	B5L10	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	450.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L10		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.15 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L10	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	320.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L11 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	21.00 ft	Invert:	20.50 ft
From Node:	B5L11	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	430.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	1	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			

Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L5 to B5L6		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft
From Node:	B5L5	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L6	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	385.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L6 to B5L7		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.40 ft	Invert:	20.40 ft
From Node:	B5L6	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L7	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	220.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: B5L7 to B5L9		Upstream		Downstream	
Scenario:	Scenario1	Invert:	20.00 ft	Invert:	20.00 ft

From Node:	B5L7	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B5L9	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.50 ft	Max Depth:	2.50 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	585.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0	Top Clip			
Bend Loss Coef:	0	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Drop Structure Link: WCS 2		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B2	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	Boundary 1	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	65.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Sharp Crested Vertical	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	24.00 ft	Op Table:	
Control Elevation:	24.00 ft	Ref Node:	
Max Depth:	999.00 ft	Discharge Coefficients	
Max Width:	1.50 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600

Orifice Table:

Weir Comment:

Drop Structure Comment:

Drop Structure Link: WCS 3		Upstream Pipe		Downstream Pipe	
Scenario:	Scenario1	Invert:	19.00 ft	Invert:	19.00 ft
From Node:	B3L3	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	B2	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	237.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	1	Op Table:		Op Table:	
Exit Loss Coef:	1	Ref Node:		Ref Node:	
Bend Loss Coef:	0	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Sharp Crested Vertical	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	24.00 ft	Ref Node:	
Control Elevation:	24.00 ft	Discharge Coefficients	
Max Depth:	0.25 ft	Weir Default:	3.200
Max Width:	0.31 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component		Bottom Clip	
Weir:	2	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Horizontal		

Geometry Type:	Rectangular
Invert:	25.60 ft
Control Elevation:	25.60 ft
Max Depth:	3.00 ft
Max Width:	5.00 ft
Fillet:	0.00 ft

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	3.200
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Drop Structure Comment:

Weir Link: WCS 4

Scenario:	Scenario1
From Node:	B4L4
To Node:	B2
Link Count:	1
Flow Direction:	Both
Damping:	0.0000 ft
Weir Type:	Broad Crested Vertical
Geometry Type:	Rectangular
Invert:	26.00 ft
Control Elevation:	26.00 ft
Max Depth:	999.00 ft
Max Width:	30.00 ft
Fillet:	0.00 ft

Bottom Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Top Clip

Default:	0.00 ft
Op Table:	
Ref Node:	

Discharge Coefficients

Weir Default:	2.800
Weir Table:	
Orifice Default:	0.600
Orifice Table:	

Comment:

Simulation: 100yr3d

Scenario:	Scenario1
Run Date/Time:	11/26/2025 8:38:35 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	216.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1
Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~SFWMD-72
Rainfall Amount: 14.20 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 10yr 1d

Scenario: Scenario1
Run Date/Time: 11/26/2025 9:15:25 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1
Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
IA Recovery Time: 12.0000 hr
Ia/S: 0.20 dec

dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~FLMOD
		Rainfall Amount:	6.00 in
		Storm Duration:	24.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 25yr3d

Scenario: Scenario1
Run Date/Time: 11/26/2025 9:15:34 AM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
0	0	0	72.0000	240.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources		Lookup Tables	
Rainfall Folder:		Boundary Stage Set:	
		Extern Hydrograph Set:	
Unit Hydrograph Folder:		Curve Number Set:	Scenario 1
		Green-Ampt Set:	
		Vertical Layers Set:	
		Impervious Set:	Scenario 1

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft		
		Rainfall Name:	~SFWMD-72
		Rainfall Amount:	10.20 in
		Storm Duration:	72.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: 5yr1d

Scenario:	Scenario1
Run Date/Time:	11/26/2025 9:16:05 AM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.0500
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	10.0000
0	0	0	24.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Scenario 1
Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Scenario 1

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 12.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FLMOD
Rainfall Amount: 5.20 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	10yr 1d	B2	24.84	42.0421
Scenario1	10yr 1d	B3L3	25.92	13.5713
Scenario1	10yr 1d	B4L4	25.97	13.6057
Scenario1	10yr 1d	B5L10	25.86	13.0673
Scenario1	10yr 1d	B5L11	25.87	12.8036
Scenario1	10yr 1d	B5L5	25.73	15.8838
Scenario1	10yr 1d	B5L6	25.72	24.0671
Scenario1	10yr 1d	B5L7	25.71	24.1421
Scenario1	10yr 1d	B5L9	25.64	24.5088
Scenario1	25yr3d	B2	25.97	74.3888
Scenario1	25yr3d	B3L3	26.86	60.4576
Scenario1	25yr3d	B4L4	26.73	60.2311
Scenario1	25yr3d	B5L10	27.32	61.3670
Scenario1	25yr3d	B5L11	27.32	60.8877
Scenario1	25yr3d	B5L5	27.19	64.9738
Scenario1	25yr3d	B5L6	27.16	64.4155
Scenario1	25yr3d	B5L7	27.08	72.0555
Scenario1	25yr3d	B5L9	26.77	72.6888
Scenario1	5yr1d	B2	24.66	50.1984
Scenario1	5yr1d	B3L3	25.78	15.5401
Scenario1	5yr1d	B4L4	25.79	15.5401
Scenario1	5yr1d	B5L10	25.54	13.0176
Scenario1	5yr1d	B5L11	25.54	12.8223
Scenario1	5yr1d	B5L5	25.45	15.5867
Scenario1	5yr1d	B5L6	25.44	24.0901
Scenario1	5yr1d	B5L7	25.44	24.1651
Scenario1	5yr1d	B5L9	25.40	24.4401

Scenario	Sim	Node Name	Maximum Stage [ft]	Time to Maximum Stage [hrs]
Scenario1	100yr3d	B2	27.43	216.0068
Scenario1	100yr3d	B3L3	27.43	216.0068
Scenario1	100yr3d	B4L4	27.43	216.0068
Scenario1	100yr3d	B5L10	27.94	64.2225
Scenario1	100yr3d	B5L11	27.93	64.2225
Scenario1	100yr3d	B5L5	27.94	72.0985
Scenario1	100yr3d	B5L6	27.92	72.1068
Scenario1	100yr3d	B5L7	27.86	72.2151
Scenario1	100yr3d	B5L9	27.59	79.9485

Sim	Link Name	Maximum Flow Rate [cfs]	Time to Maximum Flow Rate [hrs]
25yr3d	WCS 2	11.10	109.5305

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	0.0000	24.00	0.00
25yr3d	B2	0.2525	24.00	0.00
25yr3d	B2	0.5010	24.00	0.00
25yr3d	B2	0.7552	24.00	0.00
25yr3d	B2	1.0052	24.00	0.00
25yr3d	B2	1.2552	24.00	0.00
25yr3d	B2	1.5052	24.00	0.00
25yr3d	B2	1.7552	24.00	0.00
25yr3d	B2	2.0052	24.00	0.00
25yr3d	B2	2.2552	24.00	0.00
25yr3d	B2	2.5052	24.00	0.00
25yr3d	B2	2.7552	24.00	0.00
25yr3d	B2	3.0052	24.00	0.00
25yr3d	B2	3.2552	24.00	0.00
25yr3d	B2	3.5052	24.00	0.00
25yr3d	B2	3.7552	24.00	0.00
25yr3d	B2	4.0052	24.00	0.00
25yr3d	B2	4.2552	24.00	0.00
25yr3d	B2	4.5052	24.00	0.00
25yr3d	B2	4.7552	24.00	0.00
25yr3d	B2	5.0052	24.00	0.00
25yr3d	B2	5.2552	24.00	0.00
25yr3d	B2	5.5052	24.00	0.00
25yr3d	B2	5.7552	24.00	0.00
25yr3d	B2	6.0052	24.00	0.00
25yr3d	B2	6.2552	24.00	0.00
25yr3d	B2	6.5052	24.00	0.00
25yr3d	B2	6.7552	24.00	0.00
25yr3d	B2	7.0052	24.00	0.00
25yr3d	B2	7.2552	24.00	0.00
25yr3d	B2	7.5052	24.00	0.00
25yr3d	B2	7.7552	24.00	0.00
25yr3d	B2	8.0052	24.00	0.00
25yr3d	B2	8.2552	24.00	0.00
25yr3d	B2	8.5052	24.00	0.00
25yr3d	B2	8.7552	24.00	0.00
25yr3d	B2	9.0052	24.00	0.00
25yr3d	B2	9.2552	24.00	0.00
25yr3d	B2	9.5052	24.00	0.00
25yr3d	B2	9.7552	24.00	0.00
25yr3d	B2	10.0052	24.00	0.00
25yr3d	B2	10.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	10.5052	24.00	0.00
25yr3d	B2	10.7552	24.00	0.00
25yr3d	B2	11.0052	24.00	0.00
25yr3d	B2	11.2552	24.00	0.00
25yr3d	B2	11.5052	24.00	0.00
25yr3d	B2	11.7552	24.00	0.00
25yr3d	B2	12.0052	24.00	0.00
25yr3d	B2	12.2552	24.00	0.00
25yr3d	B2	12.5052	24.00	0.00
25yr3d	B2	12.7552	24.00	0.00
25yr3d	B2	13.0052	24.00	0.00
25yr3d	B2	13.2552	24.00	0.00
25yr3d	B2	13.5052	24.00	0.00
25yr3d	B2	13.7552	24.00	0.00
25yr3d	B2	14.0052	24.00	0.00
25yr3d	B2	14.2552	24.00	0.00
25yr3d	B2	14.5052	24.00	0.00
25yr3d	B2	14.7552	24.00	0.00
25yr3d	B2	15.0052	24.00	0.00
25yr3d	B2	15.2552	24.00	0.00
25yr3d	B2	15.5052	24.00	0.00
25yr3d	B2	15.7552	24.00	0.00
25yr3d	B2	16.0052	24.00	0.00
25yr3d	B2	16.2552	24.00	0.00
25yr3d	B2	16.5052	24.00	0.00
25yr3d	B2	16.7552	24.00	0.00
25yr3d	B2	17.0052	24.00	0.00
25yr3d	B2	17.2552	24.00	0.00
25yr3d	B2	17.5052	24.00	0.00
25yr3d	B2	17.7552	24.00	0.00
25yr3d	B2	18.0052	24.00	0.00
25yr3d	B2	18.2552	24.00	0.00
25yr3d	B2	18.5052	24.00	0.00
25yr3d	B2	18.7552	24.00	0.00
25yr3d	B2	19.0052	24.00	0.00
25yr3d	B2	19.2552	24.00	0.00
25yr3d	B2	19.5052	24.00	0.00
25yr3d	B2	19.7552	24.00	0.00
25yr3d	B2	20.0052	24.00	0.00
25yr3d	B2	20.2552	24.00	0.00
25yr3d	B2	20.5052	24.00	0.00
25yr3d	B2	20.7552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	21.0052	24.00	0.00
25yr3d	B2	21.2552	24.00	0.00
25yr3d	B2	21.5052	24.00	0.00
25yr3d	B2	21.7552	24.00	0.00
25yr3d	B2	22.0052	24.00	0.00
25yr3d	B2	22.2552	24.00	0.00
25yr3d	B2	22.5052	24.00	0.00
25yr3d	B2	22.7552	24.00	0.00
25yr3d	B2	23.0052	24.00	0.00
25yr3d	B2	23.2552	24.00	0.00
25yr3d	B2	23.5052	24.00	0.00
25yr3d	B2	23.7552	24.00	0.00
25yr3d	B2	24.0052	24.00	0.00
25yr3d	B2	24.2552	24.00	0.00
25yr3d	B2	24.5052	24.00	0.00
25yr3d	B2	24.7552	24.00	0.00
25yr3d	B2	25.0052	24.00	0.00
25yr3d	B2	25.2552	24.00	0.00
25yr3d	B2	25.5052	24.00	0.00
25yr3d	B2	25.7552	24.00	0.00
25yr3d	B2	26.0052	24.00	0.00
25yr3d	B2	26.2552	24.00	0.00
25yr3d	B2	26.5052	24.00	0.00
25yr3d	B2	26.7552	24.00	0.00
25yr3d	B2	27.0052	24.00	0.00
25yr3d	B2	27.2552	24.00	0.00
25yr3d	B2	27.5052	24.00	0.00
25yr3d	B2	27.7552	24.00	0.00
25yr3d	B2	28.0052	24.00	0.00
25yr3d	B2	28.2552	24.00	0.00
25yr3d	B2	28.5052	24.00	0.00
25yr3d	B2	28.7552	24.00	0.00
25yr3d	B2	29.0052	24.00	0.00
25yr3d	B2	29.2552	24.00	0.00
25yr3d	B2	29.5052	24.00	0.00
25yr3d	B2	29.7552	24.00	0.00
25yr3d	B2	30.0052	24.00	0.00
25yr3d	B2	30.2552	24.00	0.00
25yr3d	B2	30.5052	24.00	0.00
25yr3d	B2	30.7552	24.00	0.00
25yr3d	B2	31.0052	24.00	0.00
25yr3d	B2	31.2552	24.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	31.5052	24.00	0.00
25yr3d	B2	31.7552	24.00	0.00
25yr3d	B2	32.0052	24.00	0.00
25yr3d	B2	32.2552	24.00	0.00
25yr3d	B2	32.5052	24.00	0.00
25yr3d	B2	32.7552	24.00	0.00
25yr3d	B2	33.0052	24.01	0.00
25yr3d	B2	33.2552	24.01	0.00
25yr3d	B2	33.5052	24.01	0.00
25yr3d	B2	33.7552	24.01	0.00
25yr3d	B2	34.0052	24.01	0.00
25yr3d	B2	34.2552	24.01	0.00
25yr3d	B2	34.5052	24.01	0.00
25yr3d	B2	34.7552	24.01	0.00
25yr3d	B2	35.0052	24.01	0.00
25yr3d	B2	35.2552	24.01	0.00
25yr3d	B2	35.5052	24.01	0.00
25yr3d	B2	35.7552	24.01	0.00
25yr3d	B2	36.0052	24.01	0.00
25yr3d	B2	36.2552	24.01	0.00
25yr3d	B2	36.5052	24.01	0.00
25yr3d	B2	36.7552	24.01	0.00
25yr3d	B2	37.0052	24.01	0.00
25yr3d	B2	37.2552	24.01	0.00
25yr3d	B2	37.5052	24.01	0.00
25yr3d	B2	37.7552	24.01	0.00
25yr3d	B2	38.0052	24.02	0.00
25yr3d	B2	38.2552	24.02	0.00
25yr3d	B2	38.5052	24.02	0.00
25yr3d	B2	38.7552	24.02	0.00
25yr3d	B2	39.0052	24.02	0.00
25yr3d	B2	39.2552	24.02	0.00
25yr3d	B2	39.5052	24.02	0.00
25yr3d	B2	39.7552	24.02	0.00
25yr3d	B2	40.0052	24.02	0.00
25yr3d	B2	40.2552	24.02	0.00
25yr3d	B2	40.5052	24.02	0.00
25yr3d	B2	40.7552	24.02	0.01
25yr3d	B2	41.0052	24.03	0.01
25yr3d	B2	41.2552	24.03	0.01
25yr3d	B2	41.5052	24.03	0.01
25yr3d	B2	41.7552	24.03	0.01

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	42.0052	24.03	0.01
25yr3d	B2	42.2552	24.03	0.01
25yr3d	B2	42.5052	24.03	0.01
25yr3d	B2	42.7552	24.03	0.01
25yr3d	B2	43.0052	24.03	0.01
25yr3d	B2	43.2552	24.03	0.01
25yr3d	B2	43.5052	24.04	0.01
25yr3d	B2	43.7552	24.04	0.01
25yr3d	B2	44.0052	24.04	0.01
25yr3d	B2	44.2552	24.04	0.01
25yr3d	B2	44.5052	24.04	0.01
25yr3d	B2	44.7552	24.04	0.01
25yr3d	B2	45.0052	24.04	0.02
25yr3d	B2	45.2552	24.04	0.02
25yr3d	B2	45.5052	24.04	0.02
25yr3d	B2	45.7552	24.05	0.02
25yr3d	B2	46.0052	24.05	0.02
25yr3d	B2	46.2552	24.05	0.02
25yr3d	B2	46.5052	24.05	0.02
25yr3d	B2	46.7552	24.05	0.02
25yr3d	B2	47.0052	24.05	0.02
25yr3d	B2	47.2552	24.05	0.02
25yr3d	B2	47.5052	24.06	0.03
25yr3d	B2	47.7552	24.06	0.03
25yr3d	B2	48.0052	24.06	0.03
25yr3d	B2	48.2552	24.06	0.03
25yr3d	B2	48.5052	24.06	0.03
25yr3d	B2	48.7552	24.06	0.03
25yr3d	B2	49.0052	24.07	0.03
25yr3d	B2	49.2552	24.07	0.04
25yr3d	B2	49.5052	24.07	0.04
25yr3d	B2	49.7552	24.07	0.04
25yr3d	B2	50.0052	24.07	0.04
25yr3d	B2	50.2552	24.07	0.04
25yr3d	B2	50.5052	24.08	0.05
25yr3d	B2	50.7552	24.08	0.05
25yr3d	B2	51.0052	24.08	0.05
25yr3d	B2	51.2552	24.08	0.05
25yr3d	B2	51.5052	24.09	0.06
25yr3d	B2	51.7552	24.09	0.06
25yr3d	B2	52.0052	24.09	0.06
25yr3d	B2	52.2552	24.09	0.06

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	52.5052	24.09	0.07
25yr3d	B2	52.7552	24.10	0.07
25yr3d	B2	53.0052	24.10	0.07
25yr3d	B2	53.2552	24.10	0.08
25yr3d	B2	53.5052	24.11	0.08
25yr3d	B2	53.7552	24.11	0.08
25yr3d	B2	54.0052	24.11	0.09
25yr3d	B2	54.2552	24.12	0.09
25yr3d	B2	54.5052	24.12	0.09
25yr3d	B2	54.7552	24.12	0.10
25yr3d	B2	55.0052	24.13	0.10
25yr3d	B2	55.2552	24.13	0.11
25yr3d	B2	55.5018	24.14	0.11
25yr3d	B2	55.7558	24.14	0.12
25yr3d	B2	56.0026	24.15	0.12
25yr3d	B2	56.2538	24.15	0.13
25yr3d	B2	56.5046	24.16	0.13
25yr3d	B2	56.7501	24.17	0.14
25yr3d	B2	57.0025	24.17	0.15
25yr3d	B2	57.2523	24.18	0.15
25yr3d	B2	57.5033	24.19	0.16
25yr3d	B2	57.7501	24.20	0.17
25yr3d	B2	58.0004	24.21	0.18
25yr3d	B2	58.2513	24.22	0.19
25yr3d	B2	58.5054	24.23	0.20
25yr3d	B2	58.7500	24.25	0.21
25yr3d	B2	59.0013	24.26	0.22
25yr3d	B2	59.2518	24.28	0.24
25yr3d	B2	59.5015	24.31	0.25
25yr3d	B2	59.7502	24.34	0.27
25yr3d	B2	60.0002	24.40	0.29
25yr3d	B2	60.2505	24.49	0.32
25yr3d	B2	60.5010	24.60	0.36
25yr3d	B2	60.7507	24.70	0.42
25yr3d	B2	61.0019	24.80	0.48
25yr3d	B2	61.2501	24.88	0.55
25yr3d	B2	61.5004	24.95	0.64
25yr3d	B2	61.7516	25.02	0.73
25yr3d	B2	62.0019	25.08	0.84
25yr3d	B2	62.2532	25.13	0.95
25yr3d	B2	62.5000	25.18	1.06
25yr3d	B2	62.7516	25.22	1.19

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	63.0011	25.26	1.32
25yr3d	B2	63.2556	25.30	1.46
25yr3d	B2	63.5040	25.34	1.60
25yr3d	B2	63.7529	25.37	1.74
25yr3d	B2	64.0034	25.41	1.90
25yr3d	B2	64.2538	25.44	2.05
25yr3d	B2	64.5055	25.47	2.21
25yr3d	B2	64.7530	25.50	2.38
25yr3d	B2	65.0005	25.52	2.54
25yr3d	B2	65.2505	25.55	2.72
25yr3d	B2	65.5063	25.57	2.90
25yr3d	B2	65.7522	25.60	3.07
25yr3d	B2	66.0055	25.62	3.26
25yr3d	B2	66.2555	25.64	3.44
25yr3d	B2	66.5055	25.66	3.63
25yr3d	B2	66.7555	25.68	3.82
25yr3d	B2	67.0055	25.70	4.01
25yr3d	B2	67.2555	25.72	4.21
25yr3d	B2	67.5055	25.73	4.41
25yr3d	B2	67.7555	25.75	4.61
25yr3d	B2	68.0055	25.77	4.81
25yr3d	B2	68.2555	25.78	5.01
25yr3d	B2	68.5055	25.80	5.22
25yr3d	B2	68.7555	25.81	5.42
25yr3d	B2	69.0055	25.82	5.63
25yr3d	B2	69.2555	25.84	5.84
25yr3d	B2	69.5055	25.85	6.06
25yr3d	B2	69.7555	25.86	6.27
25yr3d	B2	70.0055	25.87	6.48
25yr3d	B2	70.2555	25.88	6.70
25yr3d	B2	70.5055	25.89	6.92
25yr3d	B2	70.7555	25.90	7.13
25yr3d	B2	71.0055	25.91	7.35
25yr3d	B2	71.2555	25.92	7.57
25yr3d	B2	71.5055	25.92	7.79
25yr3d	B2	71.7555	25.93	8.01
25yr3d	B2	72.0055	25.94	8.23
25yr3d	B2	76.0055	25.96	11.71
25yr3d	B2	80.0055	25.96	14.99
25yr3d	B2	84.0055	25.96	18.19
25yr3d	B2	88.0055	25.94	21.49
25yr3d	B2	92.0055	25.92	24.85

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	96.0055	25.90	28.25
25yr3d	B2	100.0055	25.87	31.67
25yr3d	B2	104.0055	25.83	35.18
25yr3d	B2	108.0055	25.78	38.81
25yr3d	B2	112.0055	25.74	42.47
25yr3d	B2	116.0055	25.69	46.03
25yr3d	B2	120.0055	25.64	49.45
25yr3d	B2	124.0055	25.60	52.72
25yr3d	B2	128.0055	25.55	55.86
25yr3d	B2	132.0055	25.50	58.85
25yr3d	B2	136.0055	25.45	61.69
25yr3d	B2	140.0055	25.40	64.38
25yr3d	B2	144.0055	25.35	66.94
25yr3d	B2	148.0055	25.31	69.38
25yr3d	B2	152.0055	25.27	71.70
25yr3d	B2	156.0055	25.23	73.91
25yr3d	B2	160.0055	25.19	76.03
25yr3d	B2	164.0055	25.16	78.06
25yr3d	B2	168.0055	25.13	80.00
25yr3d	B2	172.0055	25.10	81.87
25yr3d	B2	176.0055	25.07	83.66
25yr3d	B2	180.0055	25.04	85.38
25yr3d	B2	184.0055	25.02	87.04
25yr3d	B2	188.0055	24.99	88.64
25yr3d	B2	192.0055	24.97	90.17
25yr3d	B2	196.0055	24.94	91.65
25yr3d	B2	200.0055	24.92	93.08
25yr3d	B2	204.0055	24.90	94.45
25yr3d	B2	208.0055	24.88	95.78
25yr3d	B2	212.0055	24.86	97.06
25yr3d	B2	216.0055	24.84	98.30
25yr3d	B2	220.0055	24.82	99.49
25yr3d	B2	224.0055	24.80	100.65
25yr3d	B2	228.0055	24.78	101.76
25yr3d	B2	232.0055	24.77	102.84
25yr3d	B2	236.0055	24.75	103.89
25yr3d	B2	240.0055	24.73	104.90
25yr3d	B2	244.0055	24.72	105.88
25yr3d	B2	248.0055	24.70	106.83
25yr3d	B2	252.0055	24.69	107.75
25yr3d	B2	256.0055	24.67	108.64
25yr3d	B2	260.0055	24.66	109.50

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Volume [ac_ft]
25yr3d	B2	264.0055	24.65	110.33
25yr3d	B2	268.0055	24.63	111.14
25yr3d	B2	272.0055	24.62	111.93
25yr3d	B2	276.0055	24.61	112.69
25yr3d	B2	280.0055	24.59	113.43
25yr3d	B2	284.0055	24.58	114.15
25yr3d	B2	288.0055	24.57	114.84
25yr3d	B2	292.0055	24.56	115.52
25yr3d	B2	296.0055	24.55	116.17
25yr3d	B2	300.0055	24.54	116.81
25yr3d	B2	304.0055	24.53	117.43
25yr3d	B2	308.0055	24.52	118.03
25yr3d	B2	312.0055	24.51	118.61
25yr3d	B2	316.0055	24.50	119.18
25yr3d	B2	320.0055	24.49	119.73
25yr3d	B2	324.0055	24.48	120.27
25yr3d	B2	328.0055	24.47	120.78
25yr3d	B2	332.0055	24.46	121.29
25yr3d	B2	336.0055	24.45	121.78
25yr3d	B2	340.0055	24.44	122.26
25yr3d	B2	344.0055	24.44	122.72
25yr3d	B2	348.0055	24.43	123.17
25yr3d	B2	352.0055	24.42	123.61
25yr3d	B2	356.0055	24.41	124.04
25yr3d	B2	360.0055	24.40	124.45

PART IV #25-86

Recommended Action

Engineering Review:

By: _____ 3/3/2026 **Approved** **X** **Denied** ___

District Field Review:

By: _____ **Approved** ___ **Denied** ___

District Manager: _____ **Date**: _____

Comments:

The referenced permit application, submitted by Atwell, LLC, proposes modifications to the existing LAMSID-owned Copperhead Canal (57-7-1) to facilitate the extension of Beth Stacey Boulevard. AIM Engineering & Surveying, Inc. has reviewed the submitted documents and recommends approval with the following stipulations:

1. Any disturbed canal right-of-way must be restored with sod and maintained until fully established. Seeding is not permitted.

2. The permittee shall submit signed and sealed record drawings within thirty (30) days of project completion. The drawings must include sufficient detail to demonstrate that the permitted facilities have been constructed in substantial compliance with LA-MSID–approved plans and design.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: PER #25-94 Riverhall 2nd Gated Canal Crossing

Meeting Date: March 16, 2026

Agenda Item # 2F

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve PER #25-94 Riverhall 2nd Gated Canal Crossing, with any listed stipulations.

What the action accomplishes: Allows RH Venture II, LLC to build a new gated entrance over Mackerel Canal (46-1-1) and provides an easement for them to connect to Karen Ave N.

Fiscal Impact: Permit Fees: $5,000.00 Capital Reimbursement Fee: $15,177.97

Staff comments: Approved with the stipulations listed in the permit detailing that all maintenance of the crossing is their responsibility.

Staff review and recommended approval:

[x] **MANAGER** [ ] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

LEHIGH ACRES - MUNICIPAL SERVICES IMPROVEMENT DISTRICT
PERMIT

THIS PERMIT is granted this 16th day of March, 2026, by Lehigh Acres Municipal Services Improvement District (hereinafter referred to as "DISTRICT"), 601 East County Lane, Lehigh Acres, Florida 33936, to RH Venture II, LLC, 7807 Baymeadows Road E., Suite 205, Jacksonville, FL 32256, hereinafter referred to as the "Permittee") as a non-exclusive permit for a secondary roadway crossing over Mackerel Canal (46-1-1) and provides a perpetual canal crossing easement over the canal right of way where the crossing will connect to Karen Ave. N., in Lehigh Acres, Florida.

W I T N E S S E T H:

1. In the event the DISTRICT is not the fee title owner of the real property (the "Property") to which this Permit is applicable, the Permittee agrees, prior to commencement, to obtain all necessary consents from the fee title owners of the Property.

2. This Permit does not constitute a waiver of the necessity by the Permittee to obtain such other necessary and appropriate permits required by other governmental bodies or agencies; therefore the Permittee, prior to commencement, is required to obtain any and all other applicable federal, state, and local permits required in connection with Permittee's use of the Property and agrees that at all times it will comply with the requirements of all federal, state and local laws, ordinances, rules and regulations applicable or pertaining to the use of the Property by the Permittee.

3. Permittee understands and agrees that the use of the Property pursuant to this Permit is subordinate to the rights and interests of the DISTRICT and to the extent applicable that of the fee title owner of the Property. Further, Permittee does hereby stipulate that the Permittee is not relying upon any representations whatsoever by the DISTRICT regarding the DISTRICT'S right, title, interest, or ownership as to the Property for which this Permit is applicable.

4. The Permittee, by accepting this permit, specifically agrees to allow authorized District Personnel, upon presentation of credentials or other documents as may be required by law, access to the premises, at reasonable times, where the permitted activity is located or conducted for the purpose of:
    a. Having access to and copying any records that must be kept under the conditions of the permit:
    b. Reviewing the facility, equipment, practices, or operations regulated or required under this permit; and
    c. Sampling or monitoring any substances or parameters at any location reasonably necessary to assure compliance with this permit or District rules.

5. The DISTRICT specifically reserves the right to maintain its facilities located over, under or upon the Property; to make improvements; add additional facilities; maintain, construct or alter its roads; maintain any facilities, devices, or improvements on the Property which aid in or are necessary to the DISTRICT'S operation; and the right to enter upon the Property at all times for such purposes. Permittee understands that in the exercise of such rights and interest, the DISTRICT may require Permittee, at no cost to DISTRICT, to relocate, alter, or remove the Permittee's facilities and equipment or other improvements made by Permittee pursuant to this Permit which interfere with or prevent the DISTRICT, in the DISTRICT'S opinion, from properly and faithfully constructing, improving and maintaining its facilities. If the Permittee should fail to relocate, alter, or remove the Permittee's facilities, equipment or other improvements made by the Permittee pursuant to this Permit within a reasonable time period following receipt of notice from the DISTRICT requesting same, then in that event the DISTRICT shall have the right to enter upon the lands and make such relocation, alterations or

removal of the Permittee's facilities, equipment/or other improvements at no cost to DISTRICT.

6. Permittee agrees that it will not use the Property in any manner which interferes with the DISTRICT'S use of the Property or causes a hazardous condition to exist.

7. The DISTRICT assumes no responsibility for the ownership, operation and/or maintenance of the Permittee's facilities as permitted herein.

8. The Permittee agrees to backfill any excavation it makes within the Property and to repair or replace (i) any below-grade authorized facilities and features located therein, including but not limited to: water, sewer, stormwater or irrigation facilities, structures or appurtenances and (ii) as to at or above-grade facilities and features, this requirement shall only be applicable to grasses and sod, sidewalks, and roadway improvements (the term roadway improvements shall include but not be limited to curbing, roadway base, sub grade and asphaltic or concrete surfaces) which are removed or damaged as a direct result of said excavation. Such repair or replacement shall substantially restore these features to their condition as it existed immediately prior to the event precipitating the excavation.

9. All below-grade Permittee installations, including water and sewer lines, whether sub aqueous or not, together with appurtenant facilities, shall be located underground and have a minimum ground cover of thirty inches (30") below original design profile grade, including District waterways and roadways.

10. All Permittee activity related to sub aqueous utility, water and/or sewer lines, including appurtenant facilities, shall be conducted in such fashion that the primary functional purpose of the canal or lake body facility, specifically flood control, within which the activity is taking place is not adversely affected. More specifically, sub aqueous construction shall conform to the criteria given in the Lehigh Acres Municipal Services Improvement District (Lehigh Acres – MSID) Administrative Policy and Procedures.

11. Permittee shall, at no expense to DISTRICT and within a reasonable time following notice, adjust the positions and elevations of its facilities as may be required in connection with future improvements to, or construction of, works of the DISTRICT.

12. Permittee does hereby indemnify and hold harmless the DISTRICT, its Board of Commissioners, officers, personnel, and it's Engineers against any claims, losses, damages (including consequential), expenses, or legal fees that might arise out or result from the implementation of the proposed project by the Permittee or that may be caused by a failure of DISTRICT facilities or property.

13. The Permittee shall be obligated at the time of submission of its Permit application, to provide a detail of any other existing improvements located over, under, upon or through the Property and the Permittee's accompanying plans and specifications shall clearly indicate how the Permittee will comply with required clearances between the requested permitted activity and existing improvements located over, under and upon the Property, including compliance with other permitted users duly adopted and published standards.

14. Permittee shall reimburse the DISTRICT for its legal, engineering and other expenses incurred as a result of the implementation of the proposed project.

15. If Permittee should violate any of the terms or conditions of the Permit and shall not correct or remedy same within ten (10) days of receiving written notice of said violation from the DISTRICT, then in that event, the DISTRICT may, at its option, revoke, cancel and terminate this Permit.

16. This Permit may not be assigned by the Permittee without the prior written approval of the DISTRICT.

17. Unless otherwise adjusted, modified or terminated as set forth hereinabove, including the Conditions attached hereto, this Permit shall continue in full force and effect so long as Permittee complies with the terms of this Permit.

18. In any litigation, including breach, enforcement or interpretation arising out of this Permit, the prevailing party to this Permit shall be entitled to recover reasonable attorney's fees, costs and expenses related to said action from the non-prevailing party.

19. That attached hereto are Supplemental Conditions to this Permit which are incorporated herein and made a part hereof.

LEHIGH ACRES – MUNICIPAL SERVICES IMPROVEMENT DISTRICT

BY: _____

NAME TYPED: David E. Lindsay

TITLE: District Manager

SUPPLEMENTAL CONDITIONS TO PERMIT NO. 25-94

A. GENERAL CONDITIONS

1. Notification shall be given by the Permittee to the District Engineer, AIM Engineering, at 239-332-4569 forty-eight (48) hours prior to commencement of the permitted activity. The District Engineer shall establish points of construction, if applicable, which will require inspection by the District Engineer prior to construction of the permitted activity. When the Permittee considers the permitted activity complete, the Permittee shall notify the District Engineer and schedule an on-site final inspection which shall be held in the presence of a representative of the Permittee and the District Engineer.

2. The permitted activity shall be implemented, constructed and/or installed in full accordance with the approved plans and specifications within six (6) full calendar years from the date of permit approval. Deviations from the approved plans or specifications shall be coordinated with and approved by the District Engineer prior to implementation of any such deviation.

3. When working in a District road right-of-way, not more than one-half (½) of the road or street shall be closed and traffic shall be controlled so as to provide for the safety and minimum hindrance to the public. All traffic control operations shall conform to the most current issue of the Florida Department of Transportation publication "Manual on Traffic Controls and safe Practices for Street and Highway Construction, Maintenance and Utility Operations."

4. The District Engineer shall be the final authority as to the quality and quantity of the material and works required to satisfy the terms and conditions of this Permit as they relate to District improvements and facilities.

5. Upon completion of the permitted activity and after its final review and acceptance by the District Engineer THE PERMITTEE SHALL DELIVER TO THE DISTRICT OFFICE ONE COMPLETE SET OF THE FINAL "RECORD DRAWINGS." FAILURE TO PROVIDE THE FINAL RECORD DRAWINGS MAY RESULT IN THE REVOCATION, CANCELLATION AND TERMINATION OF THIS PERMIT.

6. The above Conditions shall be continuing obligations of the Permittee and shall be complied with in the event of any subsequent maintenance, repair, replacement or modification of the permitted improvement.

Stipulations:

1). Permit #25-94 is approved according to the plans signed/sealed November 18th, 2025.

2). Please submit a copy of your approved Lee County Development Order prior to starting roadway extension.

3). Maintenance of the entire crossing, including the pipes, headwalls, road drainage ditches, rip-rap, pavement and guardrail are the responsibility of the Permittee. The crossing must be periodically inspected by qualified personnel and maintained in a safe operating condition.

4). Permittee will be responsible for all mowing, weed eating and trash removal around the entire canal crossing and it must be performed at least monthly.

5). Through access for LA-MSID mowing crews must be maintained at all times along the Mackerel Canal right of way.

6). An "As-Built" survey showing the completed improvements (including all drainage pipe inverts and structure elevations) must be submitted to Lehigh Acres - MSID within 30 days following completion of the improvements.

PART I

DATE SUBMITTED: 11-20-25

PERMIT #: 25-94

APPLICATION FOR A PERMIT

*Note: The review of permit applications will be conducted per the Lehigh Acres Municipal Services Improvement District's Administrative Policy and Procedures

AGENT NAME: Barraco and Associates, Inc. **PHONE #:** (239)461-3170

PROJECT NAME: River Hall Phase 2 South Crossing

TOTAL ACREAGE:

Property Address (STRAP associated with this address will be used as the primary STRAP for this project): 36-43-26-L4-100F1.0000					
Mailing Address:	7807 Baymeadows Road E. Suite 205				
City:	Jacksonville	**State:**	FL.	**Zip:**	32256

STRAP NUMBERS (*attach additional sheet if necessary*):

3	6	-	4	3	-	2	6	-	L	4	-	1	0	0	F	1	-	0	0	0	0		
		-			-			-			-						-						
		-			-			-			-						-						
		-			-			-			-						-						
		-			-			-			-						-						

Brief description of proposed project:

Application for: Addition of the second Southern connection across the LAMSID canal and connection to 75th Street W.

Is the project proposed to be developed in phases? ☒ **Yes** ☐ **No**

Estimated # of Phases:	2	
***Permit Fee:**	$ 5,000.00	**(to be verified by staff)**

* See Administrative Policy and Procedures Sections 4.15 – 4.17 for current Permit Fee Amount.

PART II

OWNER/APPLICANT/ DEVELOPER INFORMATION

A.

Name of Applicant	RH Venture II, LLC.						
Mailing Address:	**Street:**	7807 Baymeadows Road E. Suite 205					
City:	Jacksonville	**State:**	FL.		**Zip:**	32256	
Phone No.	**Area Code:**	(239)	**Number:**	461-3170	**Ext.**		
Fax No.	**Area Code:**		**Number:**				
E-mail Address:	gmiars@greenpointellc.com						

B. Relationship of applicant to property*:

☒ **Owner*** ☐ **Trustee*** ☐ **Option holder*** ☐ **Lessee***

☐ **Contract Purchaser*** ☐ **Other (indicate)*** _____

C.

Name of Developer:	Same as applicant					
Mailing Address:	**Street:**					
City:		**State:**		**Zip:**		
Phone No.	**Area Code:**	( )	**Number:**	-	**Ext.**	
Fax No.	**Area Code:**	( )	**Number:**	-		
E-mail Address:						

D.

Name of Authorized Agent(s):	Barraco and Associates, Inc. c/o Stephen Coleman					
Mailing Address:	**Street:**	2271 McGregor Blvd. Suite 100				
City:	Fort Myers	**State:**	FL.		**Zip:**	33901
Phone No.	**Area Code:**	(239)	**Number:**	461-3170	**Ext.**	
Fax No.	**Area Code:**	(239)	**Number:**	461-3169		
E-mail Address:	SteveC@Barraco.net; AshleyB@Barraco.net					

E.

Name of Property Owner:	Same as applicant					
Mailing Address:	**Street:**					
City:		**State:**		**Zip:**		
Phone No.	**Area Code:**	( )	**Number:**	-	**Ext.**	
Fax No.	**Area Code:**	( )	**Number:**	-		
E-mail Address:						

*A disclosure of the name of all persons or entities having an ownership interest in the property is required (attached).

PART III

DISCLOSURE OF OWNERSHIP INTEREST FORM FORM:

PRIMARY STRAP NUMBER: 36-43-26-L4-100F1-0000

1. If the property is owned in the fee simple by an **INDIVIDUAL**, tenancy by the entirety, tenancy in common, or joint tenancy, list all parties with an ownership interest as well as the percentage of such interest.

Name and Address	Percentage of Ownership

2. If the property is owned by a **CORPORATION**, list the officers and stockholders and the percentage of stock owned by each.

Name, Address, Office	Percentage of Stock
RH Venture II, LLC.	100%

3. If the property is in the name of a **TRUSTEE**, list the beneficiaries of the trust with percentages of interest.

Name and Address	Percentage of Interest

4. If the property is in the name of a **GENERAL PARTNERSHIP OR LIMITED PARTNERSHIP**, list the names of general and limited partners.

Name and Address	Percentage of Interest

5. If there is a **CONTACT FOR PURCHASE**, whether contingent on this application Or not, and whether a Corporation, Trustee, or Partnership, list the names of the contract purchasers below, including the officers, stockholders, beneficiaries, or partners.

Name and Address	Percentage of Interest

Date of Contract (mm/dd/yy): / /

6. If any contingency clause or contract terms involve additional parties, list all individuals or officers, if a corporation, partnership, or trust.

Name and Address	Percentage of Interest

For any changes of ownership or changes in contracts for purchase subsequent to the date of the application, but prior to the date of final certificate for compliance, a supplemental disclosure of interest must be filed.

The above is a full disclosure of all parties of interest in this application, to the best of my knowledge and belief.

SIGNATURE OF APPLICANT

Graydon Miars, Vice President, RH Venture II, LLC.
PRINTED OR TYPED NAME OF APPLICANT

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this 9 day of October (m), 2025, by Graydon Miars, Vice President who is personally know to me or has produced _____ as identification.

Notary Public State of Florida
Viktoriya Ososkalo
My Commission HH 606863
Expires 6/11/2027

Ososkalo
SIGNATURE OF NOTARY PUBLIC

Viktoriya Ososkalo
PRINTED NAME OF NOTARY PUBLIC

PART IV

EXPLANATION

All Permit Application sections must be completed and submitted.

Permit Application fees and submittal items (see Part VI) must be submitted with the application.

Any and all work involving the lands and/or facilities of the District shall not commence without a Board approved permit.

All set policies, procedures, technical requirements, and fee schedules can be found and reviewed in the 'Administrative Policy and Procedures Guide'. The Guide is available at the District Office located at 601 East County Lane, Lehigh Acres, FL or online at la-msid.com/.

PART V

PROFESSIONAL CONSULTANT FORM

ATTORNEY

Name:	Samantha Duran						
Mailing Address:	100 N Tampa St. Suite 2700						
City:	Tampa	**State:**	FL.			**Zip:**	33602
Business No.	**Area Code:**	(813)	**Number:**	225-4180		**Ext.**	
Other Contact:	Sduran@Foley.com						

ENGINEER

Name:	Carl A. Barraco, Jr., P.E.						
Mailing Address:	2271 McGregor Blvd. Suite 100						
City:	Fort Myers	**State:**	FL.			**Zip:**	33901
Business No.	**Area Code:**	(239)	**Number:**	461-3170		**Ext.**	
Other Contact:	Carljr@Barraco.net						

OTHER REPRESENTATIVE/PROFESSIONAL

Name:	Stephen Coleman, Senior Project Manager						
Mailing Address:	2271 McGregor Blvd. Suite 100						
City:	Fort Myers	**State:**	FL.			**Zip:**	33901
Business No.	**Area Code:**	(239)	**Number:**	461-3170		**Ext.**	
Other Contact:	SteveC@Barraco.net; AshleyB@Barraco.net						

PART VI

SUBMITTALS

NO. OF COPIES		SUBMITTAL DOCUMENT
SUB'D	REQ'D	
		ITEMS REQUIRED FOR INITIAL SUBMITTAL
2	2	**Construction/Development Plans** signed and sealed by a Licensed Florida Professional Engineer including: 1. Location map showing the location of the proposed project in relation to adjacent streets and LA-MSID facilities. 2. Details of proposed works that fully communicate the work and/or use desired (i.e. Existing Conditions Survey, Proposed Site Plan, Paving Grading & Drainage Plan, Cross Sections, Element Details, and Erosion Control Plan sheets). 3. Sufficient survey data to determine elevations, property and ROW lines, limits of existing and proposed conditions, and notation of utilized vertical datum (NAVD/NGVD).
N/A	2	**Stormwater Drainage Calculations** signed and sealed by a Licensed Florida Professional Engineer including, but not limited to, computer stormwater routing modeling data (ICPR4, Cascade, etc.) to verify compliance with the Design Criteria (input summary data below): **Water Quality Volume (Required):** _____(ACRE-FT) **Water Quality Volume (Provided):** _____(ACRE-FT) **Control Elevation:** _____(FT-NGVD) **Minimum Road/Parking Elevation:** _____(FT-NGVD) *(above the 10-year, 1-day design storm peak stage elevation)* **Minimum Perimeter Berm Elevation:** _____(FT-NGVD) *(above the 25-year, 3-day design storm peak stage elevation)* **Minimum Finished Floor Elevation:** _____(FT-NGVD) *(above the 100-year, 3-day design storm peak stage elevation and previously experienced high-water levels external to project)*
1	2	**Flash Drive / CD** containing plans in both AutoCAD (.dwg file drawn in "State Plane Coordinates") and PDF formats.
2	2	**Permit Application** (PARTS I & II)
2	2	**Disclosure of Owner Form (signed and notarized)** (PART III)
2	2	**Professional Consultant Form & Submittals** (PARTS V & VI)
N/A	2	**Copy of Approved FDEP Notice of Intent (NOI)** for all sites one (1) acre and greater in size.
N/A	2	**Copy of approved SWFMD permit**, for all sites required to get a permit, **or FDEP "10/2 Self-Certification"** approval.
2	1	**Check for appropriate Permit Fees** made out to LA-MSID.

Barraco and Associates, Inc.
www.barraco.net
Civil Engineers, Land Surveyors and Planners

TRANSMITTAL LETTER

TO:
Mr. Dave Lindsay
Lehigh Acres Municipal Services Improvement District
601 East County Lane
Lehigh Acres, FL. 33936
(239) 368-0044

FROM:
Ashley Butler

DATE: 11/20/2025

PROJECT NUMBER: 24285

PROJECT NAME:
River Hall Second South Crossing

DISTRIBUTION TO:

RE:
Site Plan Review Package

WE ARE SENDING THE FOLLOWING ITEMS VIA:

	STANDARD POSTAL MAIL	X	**COURIER SERVICE**		OTHER
	OVERNIGHT DELIVERY		CLIENT PICK-UP AT OUR OFFICE		

ITEM	COPIES	DATE	DESCRIPTION
1.	1	-	Check in the amount of $5,000 Permit Application Fee Ck. # 1943
2.	1	-	Check in the amount of $15,177.97 Landowners Agreement Fee Ck. 1944
3.	1	-	Landowners Agreement – Executed Original
4.	1	-	Developers Agreement – Executed Original
5.	2	-	Plan Set – Signed and Sealed Original
6.	2	-	LAMSID Application – 1 Original
7.	2	-	Canal Crossing Easement Agreement – Executed Original
8.	1	-	USB of submittal package including PDF & AutoCAD Files

THESE ARE TRANSMITTED AS CHECKED BELOW:

	AS REQUESTED		FOR BIDS DUE:		RESUBMIT ___ COPIES FOR APPROVAL
	FOR YOUR USE		APPROVED AS SUBMITTED		SUBMIT ___ COPIES FOR DISTRIBUTION
X	**FOR APPROVAL**		APPROVED AS NOTED		RETURN ____ CORRECTED PRINTS
	FOR REVIEW AND COMMENT		RETURNED FOR CORRECTIONS		SIGNATURE REQUEST

NOTES/COMMENTS: PLEASE CONTACT ME WITH ANY QUESTIONS REGARDING THIS INFORMATION AT **ASHLEYB@BARRACO.NET** OR BY CALLING THE PHONE NUMBER LISTED BELOW, THANK YOU.

DATE	INVOICE NO.	INVOICE AMOUNT	RETAINAGE	DEDUCTION	BALANCE
11-05-25	LAMSID	5000.00	.00		5000.00
CHECK DATE 11-05-25	CHECK NO. 1943	TOTAL 5000.00	.00		5000.00

620248-2

RIVER HALL INVESTMENT GROUP, LLC
JACKSONVILLE, FL 32256

PLEASE DETACH THIS PORTION AND RETAIN FOR YOUR RECORDS.

RIVER HALL INVESTMENT GROUP, LLC
7807 BAYMEADOWS ROAD, EAST
SUITE 205
JACKSONVILLE, FL 32256
(904) 996-2485

TRUIST 63-9138/2631 **1943**

November 5, 2025 1943 $******5,000.00

DATE CHECK NO. AMOUNT

Pay: **********Five thousand dollars and no cents

PAY TO THE ORDER OF
LEHIGH ACRES MUNICIPAL SRVCS
601A EAST COUNTY LANE
LEHIGH ACRES, FL 33936

RECEIVED
NOV 20 2025
LEHIGH ACRES
MUNICIPAL SERVICES
IMPROVEMENT DISTRICT

⑈001943⑈ ⑆263191387⑆0000240794519⑈

Permit # 25-94

DATE	INVOICE NO.	INVOICE AMOUNT	RETAINAGE	DEDUCTION	BALANCE
11-05-25	LANDOWNER	15177.97	.00		15177.97
CHECK DATE	11-05-25	CHECK NUMBER 1944	TOTAL 15177.97	.00		15177.97

0248-2

RIVER HALL INVESTMENT GROUP, LLC
JACKSONVILLE, FL 32256

PLEASE DETACH THIS PORTION AND RETAIN FOR YOUR RECORDS.

RIVER HALL INVESTMENT GROUP, LLC
7807 BAYMEADOWS ROAD, EAST
SUITE 205
JACKSONVILLE, FL 32256
(904) 996-2485

TRUIST 63-9138/2631 1944

November 5, 2025 (DATE) 1944 (CHECK NO.) $*****15,177.97 (AMOUNT)

Pay: *********Fifteen thousand one hundred seventy-seven dollars and 97 cents

PAY TO THE ORDER OF
LEHIGH ACRES MUNICIPAL SRVCS
601A EAST COUNTY LANE
LEHIGH ACRES, FL 33936

RECEIVED
NOV 20 2025
LEHIGH ACRES
MUNICIPAL SERVICES
IMPROVEMENT DISTRICT

Chris Rucd

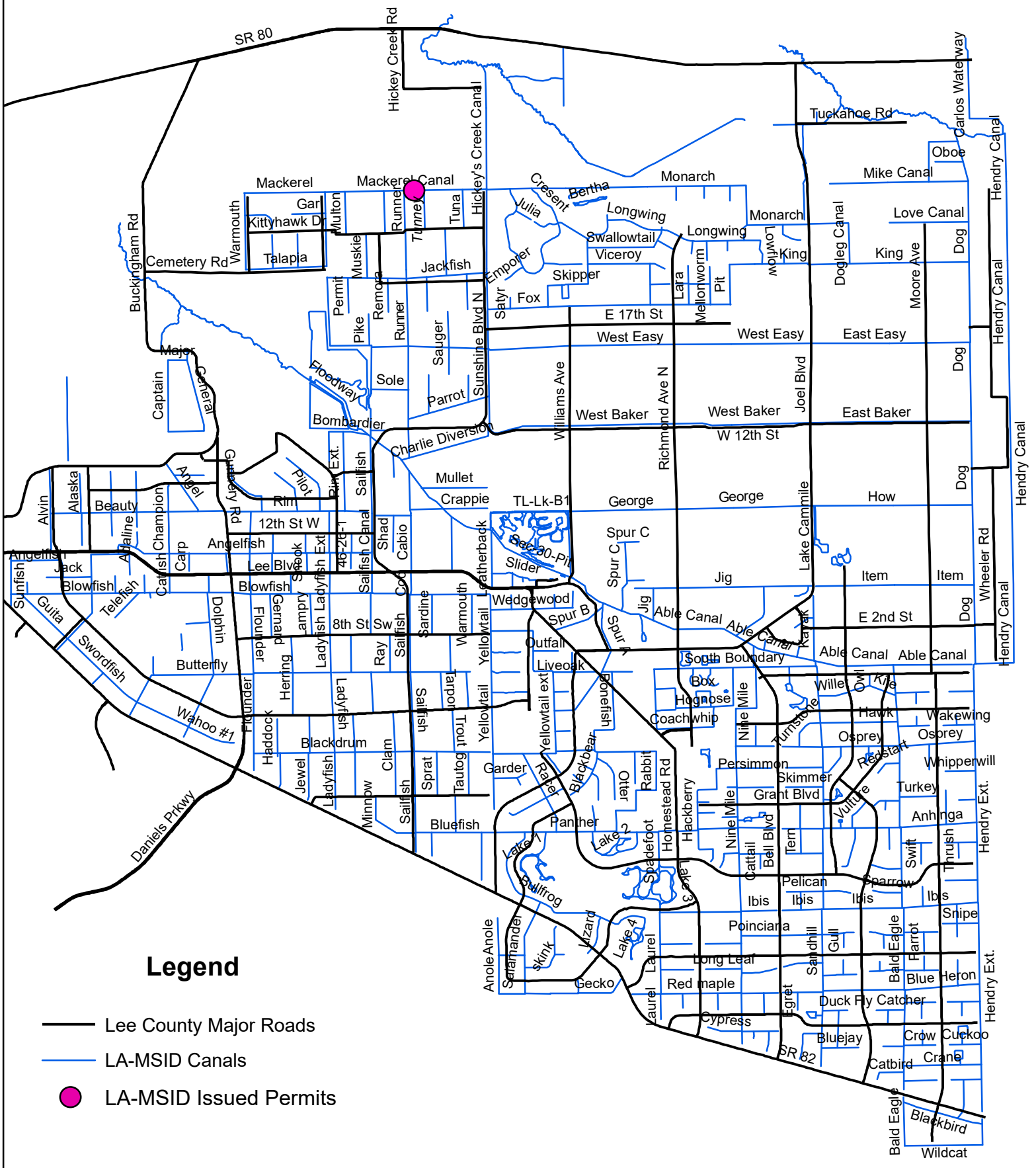
COPY

⑈001944⑈ ⑆263191387⑆0000240794519⑈

Capital Improvement Reimbursement Fee

Permit # 25-94

Permit # 2025-94



Legend

- Lee County Major Roads
- LA-MSID Canals
- LA-MSID Issued Permits

SITE DEVELOPMENT PLANS

FOR

RIVER HALL

SECONDARY ENTRANCE (L.A.M.S.I.D.)

PART OF SECTION 35, TOWNSHIP 43 SOUTH, RANGE 26 EAST
ALVA, FLORIDA

PROJECT DATA

SITE ADDRESS
RIGHT OF WAY
ALVA FL, 33920

PROJECT DATUM
STATE PLANE FLORIDA WEST ZONE (NAD 1983 (NSRS2007))
NORTH AMERICAN VERTICAL DATUM (NAVD) 1988

ZONING
HAWKS HAVEN RPD

RECORD PLAT
N/A

FLOOD ZONE
ACCORDING TO THE FLOOD INSURANCE RATE MAP NO. 12071C0316F, EFFECTIVE DATE: AUGUST 28, 2008, THE SUBJECT PARCEL IS LOCATED IN FLOOD ZONE "X"

STRAP NUMBERS
35-43-26-L4-080R1.00CE

PERMIT REQUIREMENTS

AGENCY	STATUS	NOTES
S. FLORIDA WATER MANAGEMENT DISTRICT	N/A	-
LEE COUNTY DEVELOPMENT ORDER	PENDING	DOS2025-00075
FLORIDA DEPARTMENT OF HEALTH	N/A	-
FLORIDA DEPT. OF ENVIRONMENTAL PROTECTION	N/A	-
F.D.O.T. CONNECTION PERMIT	N/A	-
F.D.O.T. DRAINAGE CONNECTION PERMIT	N/A	-
F.D.O.T. UTILITY PERMIT	N/A	-
ARMY CORPS OF ENGINEERS	N/A	-
F.D.E.P. NOTICE OF INTENT	N/A	-
		-

NOTE: CONTRACTOR MUST OBTAIN AND KEEP ON FILE A COPY OF ALL PERMITS REQUIRED PRIOR TO COMMENCEMENT OF ANY CONSTRUCTION ACTIVITY

DESIGN TEAM

PROJECT ENGINEER
CARL A. BARRACO, JR., P.E.

PROJECT MANAGEMENT
STEVE COLEMAN

DESIGN ENGINEER
VINNIE BARRACO, P.E.

PROJECT SURVEYOR
SCOTT A. WHEELER, PSM

LEAD DESIGN TECHNICIAN
JAN BILDZUKEWICZ

SITE PLANNING
-

DESIGN STAFF
BRYCE ROCHA

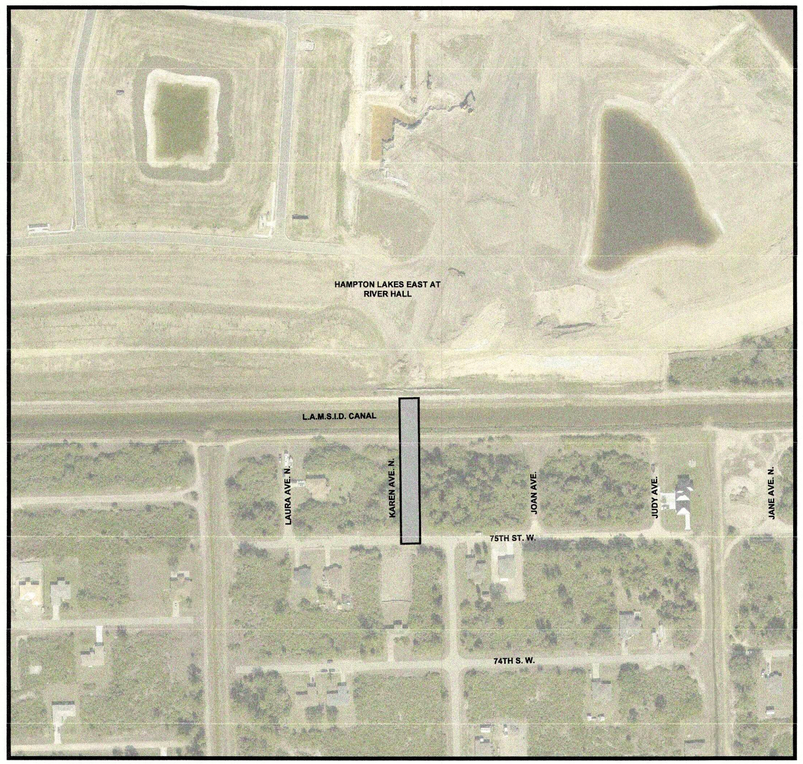
LAND PLANNING
-

QUALITY CONTROL
STEVE COLEMAN

RECORD DRAWINGS
PENDING

THESE PLANS MAY HAVE BEEN MODIFIED IN SIZE BY REPRODUCTION. THIS MUST BE CONSIDERED WHEN OBTAINING SCALED DATA.

ALL DIMENSIONS ARE IN FEET.



PROJECT LOCATION

N
W E
S

LOCATION MAP

0 200 400 800
SCALE IN FEET

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast. It's free. It's the law.
www.sunshine811.com

INDEX OF DRAWINGS

ENGINEER OF RECORDS SIGNATURE AND SEAL APPLY TO THE FOLLOWING PLAN SHEETS, EXCLUDING ANY SUPPLEMENTS:

SHEET	DESCRIPTION	XREF	DRAWING NAME
1.0	COVER SHEET AND LOCATION MAP	-	24285L01.DWG
2.0	STANDARD NOTES, LEGEND, AND ABBREVIATIONS	-	24285L02.DWG
3.0	AERIAL PHOTOGRAPH AND EXISTING CONDITIONS PLAN	B	24285L06.DWG
4.0	DETAILED SITE PLAN	A,B	24285L10.DWG
5.0	TYPICAL SECTIONS AND DETAILS	-	24285L18.DWG
6.0	EROSION CONTROL DETAILS	-	24285L50.DWG
7.0	STORMWATER POLLUTION PREVENTION PLAN	A,B	24285L52.DWG

CROSS-REFERENCED DRAWINGS:

XREF	DESCRIPTION	DRAWING NAME
A	BASE LINEWORK	24285L00.DWG
B	SURVEY LINEWORK	22951S00.DWG

PLAN STATUS

APPROVAL SUBMITTAL PLANS
2025-11-05

Barraco and Associates, Inc.
CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING
www.barraco.net
2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FORT MYERS | PANAMA CITY BEACH
FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6940

PREPARED FOR

GREENPOINTE COMMUNITIES, LLC
7807 BAYMEADOWS ROAD E
SUITE 205
JACKSONVILLE, FL 32256

PHONE (904) 562-1358
FAX (904) 996-2481

PROJECT DESCRIPTION

HAMPTON LAKES AT RIVER HALL
FORMERLY KNOWN AS SINGLE FAMILY RESIDENTIAL AT HAWKS HAVEN PHASE 1

SOUTH

PART OF SECTION 35
TOWNSHIP 43 SOUTH
RANGE 26 EAST
LEE COUNTY, FLORIDA

ENGINEER OF RECORD

CARL A. BARRACO, JR., P.E., FOR THE FIRM
FLORIDA P.E. NO. 81259 - CARLJR@BARRACO.NET

CARL A. BARRACO LICENSE No. 81259 STATE OF FLORIDA PROFESSIONAL ENGINEER
11-19-25

DRAWING NOT VALID WITHOUT SEAL, SIGNATURE AND DATE
© COPYRIGHT 2025, BARRACO AND ASSOCIATES, INC.
REPRODUCTION, CHANGES OR ASSIGNMENTS ARE PROHIBITED

FILE NAME	24285L01.DWG
LOCATION	J:\24285\DWG\LAMSID\
PLOT DATE	WED, 11-19-2025 - 2:27 PM
PLOT BY	BRYCE ROCHA

CROSS REFERENCED DRAWINGS

MASTER = BAI-COVER-1.DWG

PLAN REVISIONS

PLAN STATUS

APPROVAL SUBMITTAL PLANS
2025-11-05

COVER SHEET AND LOCATION MAP

PROJECT / FILE NO.	SHEET NUMBER
24285	1.0

STANDARD ABBREVIATIONS

NOTE: SEE FLORIDA DEPARTMENT OF TRANSPORTATION STANDARD PLANS ABBREVIATIONS FOR ADDITIONAL ABBREVIATIONS NOT LISTED BELOW.

AASHTO	AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS	CTR.	CENTER
		C.U.E.	COUNTY UTILITY EASEMENT
		CULV.	CULVERT
ABD.	ABANDONED	C.Y.	CUBIC YARD
AC.	ACRE	CYL.	CYLINDRICAL
A.D.A.	AMERICANS WITH DISABILITIES ACT	D.	DEGREE OF CURVATURE
ADJ.	ADJUST	DA.	DAY(S)
A.D.T.	AVERAGE DAILY TRAFFIC	(D)	DEED
A.E.	ACCESS EASEMENT	D.A.	DRAINAGE AREA
A.A.D.T.	ANNUAL AVERAGE DAILY TRAFFIC	D.B.	DEED BOOK
		D.B.I.	DITCH BOTTOM INLET
AGG.	AGGREGATE	DBL.	DOUBLE
A.I.C.P.	AMERICAN INSTITUTE OF CERTIFIED PLANNERS	D.C.S.	DEGREE OF CURVATURE (SPIRAL)
		D.D.C.V.	DOUBLE DETECTOR CHECK VALVE
ALT.	ALTERNATE	D.E.	DRAINAGE EASEMENT
ALUM.	ALUMINUM	DECEL.	DECELERATION
A.M.	12:00 MIDNIGHT UNTIL 11:59 NOON	DEMOBL.	DEMOBILIZATION
A.N.S.I.	AMERICAN NATIONAL STANDARDS INSTITUTE	DEN.	DENSITY
		DEPT.	DEPARTMENT
APPL.	APPLICATION	DET.	DETOUR
APPROX.	APPROXIMATELY	D.H.W.	DESIGN HIGH WATER
ASPH.	ASPHALT	DIA.	DIAMETER
ASSY.	ASSEMBLY	D.I.P.	DUCTILE IRON PIPE
A.S.T.M.	AMERICAN SOCIETY FOR TESTING MATERIALS	DIR.	DIRECTION
		DIST.	DISTANCE
AUX.	AUXILIARY	D.O.T.	DEPARTMENT OF TRANSPORTATION
AVE.	AVENUE	DR.	DRIVE
AVG.	AVERAGE	DRWY.	DRIVEWAY
B.E.	BURIED ELECTRIC	D.S.	DESIGN SPEED
B/L	BASE LINE	D.S.W.T	DRY SEASON WATER TABLE
BLVD.	BOULEVARD	DWG.	DRAWING
B.M.	BENCHMARK	E.	EAST
B.O.C.	BACK OF CURB	EA.	EACH
BOT.	BOTTOM	e.	RATE OF SUPER ELEVATION
B.P.	BORROW PIT	E.I.	ENGINEERING INTERN
BR.	BRIDGE	EL.orELEV.	ELEVATION
BRG.	BEARING	ELEC.	ELECTRIC
B.T.	BURIED TELEPHONE	ELLIP.	ELLIPTICAL
BTWN.	BETWEEN	EMBK.	EMBANKMENT
B/W	BARBED WIRE	ENCL.	ENCLOSURE
CALC.	CALCULATED	E.O.P.	EDGE OF PAVEMENT
(C)	CALCULATED OR CUT	E.O.W.	EDGE OF WATER
CAP.	CAPACITY	EQ.	EQUATION OR EQUAL
CATV.	CABLE TELEVISION	EQUIP.	EQUIPMENT
C.B.	CATCH BASIN	E.R.C.P.	ELLIPTICAL REINFORCED CONCRETE PIPE
CBC.	CONCRETE BOX CULVERT		
CBS.	CONCRETE BOX STRUCTURE	ESMT.	EASEMENT
C/C	CENTER TO CENTER	ETC.	ET CETERA (AND SO FORTH)
COMB.	COUNTY COMMISSIONERS MINUTES BOOK	E.W.	ENDWALL
		EXCAV.	EXCAVATION
C.E.	CONTROL ELEVATION	EX.	EXISTING
CEM.	CEMENT	EXP.	EXPANSION
C.F.	CUBIC FOOT	EXT.	EXTENSION
C.I.	CAST IRON	(F)	FILL
C.I.P.	CAST IRON PIPE	F. TO F.	FACE TO FACE
C/L	CENTER LINE	F.A.C.	FLORIDA ADMINISTRATIVE CODE
CONC.	CONCRETE	F.A.C.B.C.	FLORIDA ACCESSIBILITY CODE FOR BUILDING CONSTRUCTION
C.M.	CONCRETE MONUMENT		
C.M.P.	CORRUGATED METAL PIPE	F.B.C.	FLORIDA BUILDING CODE
CO.	COUNTY OR COMPANY	FD.	FOUND
C/O	CLEANOUT	F.D.C.	FIRE DEPARTMENT CONNECTION
COL.	COLUMN	FDN.	FOUNDATION
COM.	COMMERCIAL	F.D.O.T.	FLORIDA DEPARTMENT OF TRANSPORTATION
CONST.	CONSTRUCTION		
CONT.	CONTRACTOR	FED.	FEDERAL
COORD.	COORDINATE	FERT.	FERTILIZER
COR.	CORNER	F.E.S.	FLARED END SECTION
C.P.	CONTROL POINT	F.F.E.	FINISHED FLOOR ELEVATION
C.R.	CONTROL RADIUS	F.H.	FIRE HYDRANT
C.S.	CONTROL STRUCTURE		

FHWA.	FEDERAL HIGHWAY ADMINISTRATION	M.H.	MANHOLE
FIN.	FINISH	M.H.W.	MEAN HIGH WATER
FL.	FLORIDA	MI.	MILE
FLEX.	FLEXIBLE	MIN.	MINIMUM
F.M.	FORCEMAIN	MISC.	MISCELLANEOUS
F.O.C.	FACE OF CURB	M.L.W.	MEAN LOW WATER
F.P.S.	FEET PER SECOND	MM.	MILLIMETER
FRAC.	FRACTION	MOD.	MODIFY OR MODIFIED
FREQ.	FREQUENCY	MON.	MONUMENT
F.	FOOT OR FEET	M.O.T.	MAINTENANCE OF TRAFFIC
FURN.	FURNISH	M.P.H.	MILES PER HOUR
G.	GRAM	M.S.L.	MEAN SEA LEVEL
GAL.	GALLON	M.U.T.C.D.	MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES
GALV.	GALVANIZED		
GA.	GAUGE OR GAGE	N.	NORTH
GAR.	GARAGE	N/A	NOT APPLICABLE
GOVT.	GOVERNMENT	NE.	NORTHEAST
GM.	GROSS MILE	NGVD	NATIONAL GEODETIC VERTICAL DATUM OF 1929
GR.	GUARDRAIL		
GRND.	GROUND	NAVD	NORTH AMERICAN VERTICAL DATUM OF 1988
GV.	GATE VALVE		
H.	HEIGHT	N.G.S.	NATIONAL GEODETIC SURVEY
H.D.	HIGH DENSITY		
HDWL.	HEADWALL	N.H.W.	NORMAL HIGH WATER
HNDRL.	HANDRAIL	NM.	NET MILE
HORIZ.	HORIZONTAL	N.S.R.S.	NATIONAL SPATIAL REFERENCE SYSTEM
H.P.	HIGH PRESSURE OR HORSE POWER		
H.W.	HIGH WATER	N.T.S.	NOT TO SCALE
HWY.	HIGHWAY	NW.	NORTHWEST
HYD.	HYDRANT	O.D.	OUTSIDE DIAMETER
I.	EXTERNAL ANGLE (DELTA)	O.E.	OVERHEAD ELECTRIC
I.D.	INSIDE DIAMETER	OPT.	OPTIONAL
I.E.	INVERT ELEVATION	O.R.	OFFICIAL RECORD BOOK
IN.	INCH	OZ.	OUNCE
INC.	INCORPORATED	(P)	PLAT
INCL.	INCLUDED	PAVT.	PAVEMENT
I.P.	IRON PIPE	P.C.	POINT OF CURVATURE
I.R.	IRON ROD	P.E.	PROFESSIONAL ENGINEER
IRR.	IRRIGATION	PED.	PEDESTRIAN OR PEDESTAL
INSTL.	INSTALL OR INSTALLATION	PG.	PAGE
INST. NO.	INSTRUMENT NUMBER	P.G.L.	PROFILE GRADE LINE
INT.	INTERSECTION	P.I.E.	PRIVATE IRRIGATION EASEMENT
INV.	INVERT	P.I.V.	POST INDICATOR VALVE
J.B.	JUNCTION BOX	PH	MEASURE OF ACIDITY OR ALKALINITY
JCT.	JUNCTION		
JT.	JOINT	P.I.	POINT OF INTERSECTION
K.	RATE OF VERTICAL CURVATURE	PK.	PARKER-KALON
KM.	KILOMETER	PL.	PROPERTY LINE
KV.	KILOVOLT	P.M.	12:00 NOON UNTIL 11:59 MIDNIGHT
KVA.	KILOVOLT AMPERES	P.O.C.	POINT ON CURVE
L.	LENGTH, LENGTH OF CURVE	P.P.	POWER POLE
L.A.	LIMITED ACCESS	P.R.C.	POINT OF REVERSE CURVATURE
LAT.	LATERAL OR LATITUDE		
LB.	POUND OR LICENSED BUSINESS	P.R.M.	PERMANENT REFERENCE MONUMENT
L.C.R.	LEE COUNTY RECORDS		
L.C.U.	LEE COUNTY UTILITIES	PROP.	PROPERTY
L.C.U.E.	LEE COUNTY UTILITIES EASEMENT	P.U.E.	PUBLIC UTILITY EASEMENT
L=	ARC LENGTH	P.S.I.	POUNDS PER SQUARE INCH
LF.	LINEAR FOOT	P.S.M.	PROFESSIONAL SURVEYOR AND MAPPER
L.M.E.	LAKE MAINTENANCE EASEMENT		
L.M.T.	LAKE MAINTENANCE TRACT	P.V.C.	POLYVINYL CHLORIDE PIPE
LOC.	LOCATION	P.V.I.	POINT OF VERTICAL INTERSECTION
L.S.	LUMP SUM OR LIFT STATION	Q.	PEAK DISCHARGE OR FLOW VOLUME (CFS)
LT.	LEFT		
M.	MASS	QTY.	QUANTITY
(M)	MEASURED	R. or RAD.	RADIUS
MAINT.	MAINTENANCE	RNG.	RANGE
MAX.	MAXIMUM	REC.	RECORD
MED.	MEDIAN	RECT.	RECTANGULAR
M.E.S.	MITERED END SECTION	R.C.P.	REINFORCED CONCRETE PIPE
MG	MILLION GALLONS		

REINF.	REINFORCED OR REINFORCING		
REL.	RELOCATED		
REM.	REMOVED		
REQ.	REQUIRED		
RES.	RESIDENCE		
R.M.	REFERENCE MONUMENT		
R.P.M.	RAISED REFLECTIVE PAVEMENT MARKERS OR REVOLUTIONS PER MINUTE		
R.P.P.A.	REDUCED PRESSURE PRINCIPLE ASSEMBLY		
R.P.Z.D.	REDUCED PRESSURE BACKFLOW PREVENTION DEVICE		
R.R.	RAILROAD		
RT.	RIGHT		
R/W	RIGHT OF WAY		
S.	SOUTH		
SAN.	SANITARY		
SCH.	SCHEDULE		
SE.	SOUTHEAST		
SEC.	SECTION		
SED.	SEDIMENT		
SG.	SPECIFIC GRAVITY		
SH.	SHEET		
SHLDR.	SHOULDER		
S.H.W.T.	SEASONAL HIGH WATER TABLE		
SPEC.	SPECIFICATION		
S.F.	SQUARE FOOT		
S.P.I.	STANDARD PLAN INDEX		
S.R.	STATE ROAD		
ST.	STREET		
STA.	STATION		
STAB.	STABILITY OR STABILIZATION		
STD.	STANDARD		
STR.	STRUCTURE		
S.Y.	SQUARE YARD		
SW.	SOUTHWEST		
S/W	SIDEWALK		
T.	THICKNESS		
TAN.	TANGENT		
TWP.	TOWNSHIP		
T.B.M.	TEMPORARY BENCH MARK		
T.B.D.	TO BE DETERMINED		
T.B.R.	TO BE REMOVED		
TEL.	TELEPHONE		
TEMP.	TEMPORARY		
TN.	TON		
T.U.E.	TECHNOLOGY UTILITY ESMT.		
TYP.	TYPICAL		
USC&GS	UNITED STATES COAST AND GEODETIC SURVEY		
USGS	UNITED STATES GEOLOGICAL SURVEY		
UTIL.	UTILITIES		
VAR.	VARIABLE		
V.C.	VERTICAL CURVE		
VEH.	VEHICLE		
V.F.	VERTICAL FOOT		
VERT.	VERTICAL		
VOL.	VOLUME		
W.	WEST		
WIT.	WITNESS		
W.M.	WATERMAIN		
X.	COORDINATE DISTANCE(EAST-WEST)		
XING.	CROSSING		
Y.	COORDINATE DISTANCE (NORTH-SOUTH)		
YR.	YEAR		

GENERAL NOTES

NOTE: SEE ADDITIONAL NOTES ON SITE DEVELOPMENT PLANS AND DETAIL SHEETS WHICH ARE SPECIFIC TO THIS PROJECT. IN THE EVENT OF A CONFLICT, UTILIZE NOTES ON SITE DEVELOPMENT PLANS.

1. THESE PLANS HAVE BEEN PREPARED IN ACCORDANCE WITH AND ARE GOVERNED BY THE LATEST EDITIONS OF THE F.D.O.T. STANDARD PLANS AND SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION AND ALL LOCAL CODES. F.D.O.T. DESIGN STANDARD PLANS AND APPLICABLE DESIGN STANDARDS MODIFICATIONS ARE FOUND AT: https://www.fdot.gov/design/standardplans/default.shtm.
2. ALL APPROVALS REQUIRED IN CONJUNCTION WITH THESE NOTES MUST BE OBTAINED IN WRITING BY THE CONTRACTOR. ANY DEVIATIONS FROM THE PLANS AND / OR SPECIFICATIONS MUST BE APPROVED IN ADVANCE BY THE OWNER, ENGINEER, AND APPROPRIATE REGULATORY AGENCY.
3. CONTRACTOR SHALL ENSURE THAT ALL PERMITS ARE IN PLACE PRIOR TO CONSTRUCTION, THAT COPIES OF ALL PERMITS ARE RETAINED ON SITE AT ALL TIMES, AND SHALL ADHERE TO ALL PERMIT CONDITIONS. AT LEAST 72 HOURS PRIOR TO CONSTRUCTION, CONTRACTOR SHALL NOTIFY THE LOCAL MAINTENANCE ENGINEER'S OFFICE TO SECURE GENERAL USE PERMITS AND / OR OTHER PERMITS REQUIRED FOR WORK WITHIN F.D.O.T. RIGHT-OF-WAY.
4. ALL CONSTRUCTION SHALL COMPLY WITH THE LATEST VERSIONS OF THE AMERICANS WITH DISABILITIES ACT (A.D.A.) AND THE FLORIDA ACCESSIBILITY CODE FOR BUILDING CONSTRUCTION (F.A.C.B.C.). MAXIMUM SLOPE IN ACCESSIBLE PARKING SPACES AND LOADING ZONES SHALL NOT EXCEED 2.0% IN ALL DIRECTIONS. MAXIMUM RUNNING SLOPE SHALL NOT EXCEED 5% AND CROSS SLOPE SHALL NOT EXCEED 2% ON ALL SIDEWALKS AND CROSSWALKS. ALL PEDESTRIAN SURFACES SHALL BE ADA COMPLIANT.
5. SIDEWALKS ARE DESIGNED AND INTENDED FOR PEDESTRIAN TRAFFIC ONLY. ALL SIDEWALKS AND CURB CUTS SHALL BE IN ACCORDANCE WITH F.D.O.T. S.P.I. 522-002 AND SHALL INCLUDE DETECTABLE WARNING SURFACES. VALLEY GUTTER ADJACENT TO CURB CUTS SHALL DEVIATE FROM STANDARD DETAIL BY NOT EXCEEDING 12:1 SLOPES.
6. THE CONTRACTOR SHALL NOTIFY THE ENGINEER, OWNER'S REPRESENTATIVE AND APPROPRIATE AGENCY A MINIMUM OF 72 HOURS PRIOR TO ALL INSPECTIONS REQUIRED BY THE RESPECTIVE PERMIT.
7. THE CONTRACTOR SHALL NOTIFY THE OWNER AND CONTACT ALL UTILITY COMPANIES FOR LOCATIONS OF EXISTING UTILITIES A MINIMUM OF 72 HOURS PRIOR TO COMMENCING CONSTRUCTION. CONTRACTOR SHALL DETERMINE IF UTILITIES OTHER THAN THOSE SHOWN ON THESE PLANS EXIST. THE LOCATION OF EXISTING UTILITIES, PAVEMENT, VEGETATION, AND OTHER IMPROVEMENTS ARE APPROXIMATE ONLY. THE EXACT SIZES, ELEVATIONS, AND LOCATIONS SHALL BE VERIFIED BY THE CONTRACTOR IN THE FIELD PRIOR TO COMMENCING CONSTRUCTION OR ORDERING MATERIALS. CONTRACTOR SHALL NOTIFY ENGINEER / OWNER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.
8. ANY PUBLIC LAND CORNER OR BENCHMARK WITHIN THE LIMITS OF CONSTRUCTION IS TO BE PROTECTED AND, IF IN DANGER OF BEING DESTROYED, MUST BE PROPERLY REFERENCED BY THE CONTRACTOR.
9. EXISTING IMPROVEMENTS SHALL REMAIN UNLESS NOTED OTHERWISE AND SHALL BE RESTORED TO A CONDITION EQUIVALENT TO THAT WHICH EXISTED PRIOR TO COMMENCING CONSTRUCTION AT NO ADDITIONAL COST TO THE OWNER. EXISTING OFF-SITE DRAINAGE PATTERNS SHALL BE MAINTAINED DURING CONSTRUCTION.
10. THE CONTRACTOR SHALL OBTAIN ANY CLEARING, VEGETATION REMOVAL, OR RIGHT-OF-WAY PERMITS REQUIRED FOR THIS PROJECT. ALL DEBRIS SHALL BE REMOVED FROM SITE AND PROPERLY DISPOSED OF.
11. CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING TRAFFIC AND USAGE OF THE EXISTING STREETS ADJACENT TO THE PROJECT. ALL TRAFFIC MAINTENANCE CONTROL SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE FLORIDA MANUAL OF TRAFFIC CONTROL AND SAFE PRACTICES FOR STREET CONSTRUCTION, MAINTENANCE, AND UTILITY OPERATIONS. CONTRACTOR SHALL ASSURE THAT ALL TRAFFIC CONTROL DEVICES MEET ACCEPTABLE STANDARDS AS OUTLINED IN THE LATEST EDITION OF THE AMERICAN TRAFFIC SAFETY SERVICES ASSOCIATION (ATSSA's) "QUALITY STANDARDS FOR WORK ZONE TRAFFIC CONTROL DEVICES", AND SHALL IMMEDIATELY REPAIR, REPLACE, OR CLEAN DAMAGED, DEFACED, OR DIRTY DEVICES. TRAFFIC CONTROL OPERATION PROCEDURES SHALL BE SUBMITTED TO OWNER FOR APPROVAL PRIOR TO BEGINNING CONSTRUCTION. ANY IMPACTS TO TRAFFIC FLOW SHALL BE COORDINATED WITH AND APPROVED BY THE APPROPRIATE LOCAL AGENCY.
12. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS PRIOR TO COMMENCEMENT OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE IMMEDIATELY REPORTED TO THE ENGINEER AND OWNER'S REPRESENTATIVE.
13. THE LOCATIONS OF PROPOSED DRAINAGE STRUCTURES AND UTILITY LINES MAY BE FIELD ADJUSTED TO PRESERVE EXISTING VEGETATION AS APPROVED IN ADVANCE BY THE ENGINEER. UNDERGROUND CONTRACTOR SHALL MINIMIZE THE WORK AREA AND WIDTH OF TRENCHES TO AVOID DISTURBANCES OF NATURAL VEGETATION. EXERCISE CARE TO PROTECT THE ROOTS OF TREES TO REMAIN, WITHIN THE BRANCH SPREAD OF SUCH TREES, PERFORM ALL TRENCHING BY HAND. OPEN THE TRENCH ONLY WHEN UTILITIES CAN BE INSTALLED IMMEDIATELY. PRUNE INJURED ROOTS CLEANLY AND BACKFILL AS SOON AS POSSIBLE. SPOIL FROM TRENCHES SHALL BE PLACED ONLY ON PREVIOUSLY CLEARED AREAS OR AS DIRECTED BY THE OWNER. CONTRACTOR SHALL NOT REMOVE OR DISTURB ANY TREES AND/OR SHRUBS WITHOUT PRIOR APPROVAL BY OWNER.
14. INSTALLATION OF SUBSURFACE CONSTRUCTION, INCLUDING BUT NOT LIMITED TO WATER, SEWER AND IRRIGATION LINES, PUBLIC UTILITIES, AND STORM DRAINAGE IS REQUIRED PRIOR TO COMPACTION OF THE ROADWAY SUBGRADE.
15. SUBGRADE AND LIMEROCK BASE COURSES TO BE COMPACTED TO 98% MINIMUM OF THE MAXIMUM DRY DENSITY AS SPECIFIED BY AASHTO T-180 TESTING. CONTRACTOR SHALL PROVIDE THE ENGINEER SATISFACTORY DENSITY TESTS FOR SUBGRADE AND LIMEROCK PRIOR TO PAYMENT OR FINAL ACCEPTANCE. EXCESS ROAD BASE AND COMPACTED SOIL SHALL BE REMOVED FROM ALL LANDSCAPE AREAS PRIOR TO FINAL GRADING.
16. SWALE ELEVATIONS SHOWN ARE TO TOP OF SOD. CONTRACTOR TO ADJUST SWALE GRADING ACCORDINGLY. ALL UNPAVED AREAS SHALL BE GRADED TO DRAIN TO THE DRAINAGE SYSTEM TO PREVENT STANDING WATER. YARD DRAINS MAY BE ADDED IN GREEN AREAS AS NECESSARY AT THE DIRECTION OF THE LANDSCAPE ARCHITECT OR ENGINEER, AND SHALL HAVE PEDESTRIAN-SAFE GRATES MEETING THE REQUIREMENTS OF H-20 LOADING.
17. BERM ELEVATIONS SHOWN ARE THE MINIMUM REQUIRED. CONTRACTOR MAY EXCEED THE MINIMUM BERM ELEVATION BY UP TO 0.5 FEET PROVIDED ALL SLOPE CRITERIA IS MET. ANY CONSTRUCTION OF BERMS IN EXCESS OF THAT LIMIT MUST BE APPROVED BY THE ENGINEER PRIOR TO CONSTRUCTION.
18. THE CONTRACTOR SHALL NOTIFY THE ENGINEER WITHIN ONE WEEK OF COMPLETION OF DRAINAGE CONTROL STRUCTURES SO THAT ELEVATIONS MAY BE VERIFIED PRIOR TO COMPLETION OF CONSTRUCTION.
19. THE CONTRACTOR IS REQUIRED TO ADJUST ALL EXISTING AND PROPOSED VALVE BOXES, MANHOLE RIMS, GRATES AND OTHER IMPROVEMENTS AS REQUIRED TO MATCH THE FINAL GRADE.
20. THE CONTRACTOR SHALL COORDINATE WITH THE OWNER'S REPRESENTATIVE IN REGARDS TO THE LOCATION, SIZE AND QUANTITY OF ALL CONDUITS FOR UTILITIES, IRRIGATION, LIGHTING, AND OTHER IMPROVEMENTS PRIOR TO CONSTRUCTION OF THE ROADWAY SUBGRADE. CASING AND/OR CONDUIT SHALL HAVE A MINIMUM OF 30" COVER, EXTEND 5' BEYOND THE EDGE OF PAVEMENT, BACK OF CURB AND/OR SIDEWALK AT EACH END, AND ENDS SHALL BE SEALED AND DENOTED WITH MARKING TAPE
21. CONTRACTOR SHALL NOTIFY THE ENGINEER 24 HOURS PRIOR TO PLACING SOD ON AREAS THAT REQUIRE CRITICAL MINIMUM OR MAXIMUM ELEVATIONS IN ORDER FOR ENGINEER TO VERIFY SUCH ELEVATIONS. THE OBLIGATION TO ACHIEVE MINIMUM OR MAXIMUM ELEVATIONS REMAINS WITH THE CONTRACTOR.
22. AT MINIMUM, SOD A 12" STRIP ADJACENT TO ALL PAVEMENT PER F.D.O.T. REQUIREMENTS. SOD ALL UNPAVED DISTURBED AREAS WITHIN F.D.O.T. RIGHT OF WAY. ALL OTHER UNPAVED AREAS DISTURBED DURING CONSTRUCTION SHALL BE SEEDED, FERTILIZED AND MULCHED UNLESS NOTED OTHERWISE.
23. THE CONTRACTOR SHALL REMOVE ALL UNSUITABLE MATERIAL ENCOUNTERED FROM FILL AREAS PRIOR TO PLACEMENT OF FILL AND SUCH MATERIAL SHALL BE STOCKPILED OR REMOVED AS DIRECTED BY OWNER.
24. THE PROPERTY OWNER OR DESIGNATED AUTHORITY IS RESPONSIBLE FOR THE PERPETUAL MAINTENANCE OF ALL FEATURES OF THE SURFACE WATER MANAGEMENT SYSTEM AS SHOWN IN THESE DRAWINGS AND AS REQUIRED BY LOCAL, STATE, AND FEDERAL PERMITS. IT IS FURTHER RECOMMENDED THAT THE SURFACE WATER MANAGEMENT SYSTEM BE INSPECTED SEMI-ANNUALLY.
25. CONTRACTOR SHALL VERIFY DESIGN DETAILS WITH ENGINEER PRIOR TO ORDERING STRUCTURES. STRUCTURE TYPES LABELED ON THE PLANS FOR ITEMS SUCH AS JUNCTION BOXES AND INLETS MAY ONLY BE A REFERENCE TO THE TOP OF THE STRUCTURE. CERTAIN STRUCTURE TYPES REQUIRE A LARGER BOTTOM TO ACCOMMODATE LARGER PIPE SIZES AND/OR PIPE ANGLES. THE APPROPRIATE SIZE AND SHAPE OF THE BOTTOM OF THE STRUCTURE SHALL BE DETERMINED BY THE CONTRACTOR WITH CONSIDERATION GIVEN TO ADJACENT EXISTING OR PROPOSED UTILITIES. THE COST FOR THE ENTIRE STRUCTURE IS INCLUDED IN THE UNIT PRICE FOR THE PRIMARY STRUCTURE.
26. STORM DRAIN PIPE LENGTHS ARE APPROXIMATE AND HAVE BEEN MEASURED FROM INSIDE FACE OF STRUCTURE. SANITARY SEWER PIPE LENGTHS ARE APPROXIMATE AND ARE MEASURED FROM THE CENTER OF STRUCTURE.
27. CONTRACTOR SHALL MAINTAIN MINIMUM SEPARATION BETWEEN UTILITIES AS SHOWN ON "UTILITY LOCATION DETAIL" AND IN ACCORDANCE WITH APPLICABLE LOCAL UTILITY SERVICE PROVIDER REQUIREMENTS.
28. ALL COMPONENTS OF THE POTABLE WATER SYSTEM SHALL BE IN CONFORMANCE WITH AMERICAN NATIONAL STANDARDS INSTITUTE / NSF INTERNATIONAL STANDARD 61. ALL COMPONENTS, INSTALLATION AND TESTING OF THE POTABLE WATER SYSTEM SHALL BE IN ACCORDANCE WITH ALL A.W.W.A. APPLICABLE STANDARDS.
29. CONTRACTOR TO COORDINATE WITH ALL OTHER CONTRACTORS WORKING ON THE PROJECT SITE, THE OWNERS' REPRESENTATIVE, AND WITH ELECTRIC, TELEPHONE, AND CABLE COMPANIES TO ASSURE THAT NO SANITARY SEWER, WATERLINE, OR IRRIGATION SERVICES ARE INSTALLED UNDER PROPOSED TRANSFORMERS.
30. DEFLECTION TESTING IN ACCORDANCE WITH TECHNICAL SPECIFICATIONS FOR GRAVITY SEWER MAY NOT BE UNDERTAKEN UNTIL 30 DAYS AFTER FINAL BACKFILLING OF THE TRENCH IS COMPLETE. SEWER DEFLECTION MAY NOT EXCEED THE MAXIMUM PROVIDED IN THE TECHNICAL SPECIFICATIONS OR 5%, WHICHEVER IS LESS.
31. ALL GRAVITY SEWER MAINS SHALL BE TESTED WITH A MANDREL WITH A DIAMETER OF NOT LESS THAN 95% OF THE INSIDE PIPE DIAMETER. MANDREL TESTS MUST BE PERFORMED WITHOUT USE OF MECHANICAL PULLING DEVICES.
32. NO VEGETATION OR OTHER OBSTRUCTIONS SHALL BE PLACED SO AS TO BLOCK ACCESS OR LINE OF SIGHT TO FIRE HYDRANTS, F.D.C, P.I.V, ETC. IN ACCORDANCE WITH LOCAL FIRE JURISDICTION AND / OR AS SHOWN IN DETAILS.
33. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE FLORIDA FIRE CODE. SEPARATE PERMITS MAY BE REQUIRED FOR ANY DEDICATED FIRE LINE. UNDERGROUND FIRE LINES MUST MEET REQUIREMENT OF NFPA 24. CONTRACTOR TO VERIFY FIRE LINE SIZE WITH BUILDING PLANS PRIOR TO CONST.

SYMBOL LEGEND

NOTE: THE SYMBOLS REPRESENTED BELOW ARE STANDARDS UTILIZED BY THIS FIRM AND NOT ALL ARE REQUIRED FOR THE SUBJECT PROJECT.

DRAINAGE

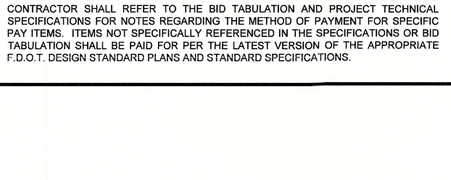
- VALLEY GUTTER INLET
- DITCH BOTTOM INLET
- DRAINAGE JUNCTION BOX
- MITERED END SECTION
- CONTROL STRUCTURE
- HEADWALL
- FLARED END SECTION
- TYPE 5 INLET (LEFT)
- TYPE 5 INLET (RIGHT)
- TYPE 6 INLET
- TYPE 9 INLET
- TYPE 10 INLET
- YARD DRAIN
- FLOW ARROW

UTILITIES

- 11.25° BEND
- 22.5° BEND
- 45° BEND
- 90° BEND
- TEE
- REDUCER
- AIR RELEASE VALVE
- WYE
- CROSS
- CAP OR PLUG
- BACTERIOLOGICAL SAMPLE POINT
- PERMANENT BLOW OFF ASSEMBLY
- TEMPORARY BLOW OFF ASSEMBLY
- 45° VERTICAL DEFLECTION
- GATE VALVE
- PLUG VALVE
- SANITARY SEWER MANHOLE
- SINGLE SANITARY SEWER SERVICE
- DOUBLE SANITARY SEWER SERVICE
- CLEANOUT
- PUMP STATION
- SINGLE IRRIGATION SERVICE
- DOUBLE IRRIGATION SERVICE
- SINGLE WATER SERVICE
- DOUBLE WATER SERVICE
- FIRE HYDRANT ASSEMBLY (PLAN VIEW)
- FIRE HYDRANT ASSEMBLY (PROFILE VIEW)
- GATE VALVE (PROFILE VIEW)
- HAND HOLE
- SPLICE BOX
- TRANSFORMER
- SWITCH CABINET

STANDARD CURB TYPES

SEE F.D.O.T. STANDARD PLAN INDEX 520-001 FOR ADDITIONAL DETAILS



TYPE "A" CURB
CURB TRANSITION
VALLEY GUTTER
TYPE "D" CURB
TYPE "E" CURB
TYPE "F" CURB

SIGNAGE AND MARKINGS

- SIGN (PLAN VIEW)
- CONSERVATION SIGN LOCATION
- HANDICAP SYMBOL
- M — MODEL HOMES, MODEL UNITS, MODEL DISPLAY CENTERS.
- LEFT ONLY
- RIGHT ONLY
- LEFT OR RIGHT
- STRAIGHT OR RIGHT
- STRAIGHT OR LEFT
- STRAIGHT, LEFT OR RIGHT
- STRAIGHT

IDENTIFICATION

- 1 — LOT NUMBER
- 1 — BUILDING ID NUMBER
- D1-1 — STORM STRUCTURE I.D.
- XXX — SANITARY STRUCTURE I.D.
- FH — FIRE HYDRANT LABEL
- SOIL BORING LOCATION
- 10 — SECTION LABEL
- NORTH ARROW
- 0, 30, 60 N.T.S. — BAR SCALE

EROSION CONTROL

NOTE: SEE EROSION CONTROL DETAILS FOR ADDITIONAL INFORMATION.

- INLET PROTECTION
- DOUBLE ROW SILT FENCE
- SINGLE ROW SILT FENCE
- FLOATING TURBIDITY BARRIER

SURVEYING

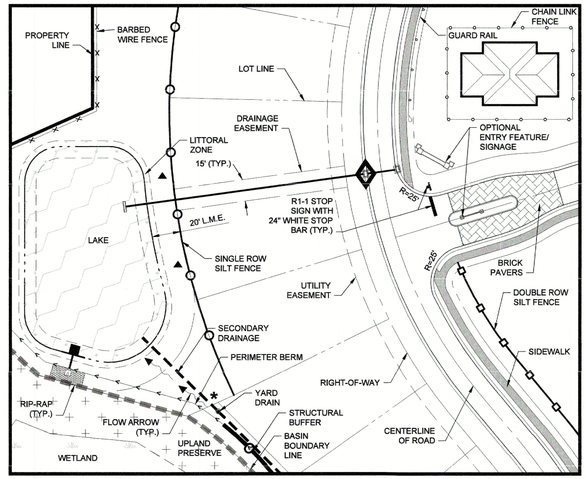
- FOUND NAIL AS NOTED
- SET 1/2" IRON ROD WITH CAP STAMPED LB6940
- SET 4"x4" CONC. MONUMENT STAMPED LB6940
- SET P.K. NAIL WITH DISC STAMPED LB6940
- FOUND CONC. MONUMENT AS NOTED
- FOUND IRON PIPE OR IRON ROD AS NOTED
- DRAINAGE INLET
- DRAINAGE MANHOLE
- MITERED END SECTION
- LIGHT POLE
- CONCRETE POWER POLE
- WOOD POWER POLE
- GUY ANCHOR
- GAS LINE MARKER
- TELEPHONE RISER
- WATER BOX
- ELECTRIC BOX
- SIGN
- FIRE HYDRANT
- WATER VALVE
- SANITARY MANHOLE
- SANITARY CLEAN OUT
- SANITARY SEWER VALVE
- BENCH MARK

QUANTITIES AND PAYMENT

CONTRACTOR SHALL REFER TO THE BID TABULATION AND PROJECT TECHNICAL SPECIFICATIONS FOR NOTES REGARDING THE METHOD OF PAYMENT FOR SPECIFIC PAY ITEMS. ITEMS NOT SPECIFICALLY REFERENCED IN THE SPECIFICATIONS OR BID TABULATION SHALL BE PAID FOR PER THE LATEST VERSION OF THE APPROPRIATE F.D.O.T. DESIGN STANDARD PLANS AND STANDARD SPECIFICATIONS.

LINETYPE AND HATCHING LEGEND

NOTE: THE DRAWING BELOW IS PROVIDED AS ONLY AN EXAMPLE OF STANDARD LINETYPES AND HATCHING UTILIZED BY THIS FIRM AND IS NOT REPRESENTATIVE OF THE SUBJECT PROJECT.



STANDARD STREET SIGNS

THE SIGN LEGEND BELOW IS PROVIDED AS A GRAPHICAL REFERENCE TO ONE OR MORE OF THE SIGNS REQUIRED FOR THIS PROJECT. THE LATEST EDITION OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (M.U.T.C.D.) SHOULD BE REFERENCED FOR SPECIFIC DETAILS AND LOCATION REQUIREMENTS.

- R1-1

RIVERHALL PROPERTY LINE

L.A.M.S.I.D. CANAL

EXISTING TOE OF SLOPE

EXISTING TOP OF BANK

EXISTING TOE OF SLOPE

EXISTING TOP OF BANK

EXISTING DIRT ROAD

EXISTING LIGHT POLE

EXISTING OM1-4

LOT 4

LOT 3

EXISTING LOT LINE (TYP.)

EXISTING 50' R/W

LOT 5

LOT 2

EXISTING GRAVEL ROAD

KAREN AVENUE NORTH

LOT 6

LOT 1

EXISTING EDGE OF PAVEMENT (TYP.)

EXISTING R1-1 SIGN

EXISTING OVER HEAD LINE

EX. 50' R/W

75TH STREET WEST

N

W E

S

0 10 20 40

SCALE IN FEET

TOPO PROVIDED BY BARRACO AND ASSOCIATES (2023-07-10)

Barraco and Associates, Inc.

CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING

www.barraco.net

2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FORT MYERS | PANAMA CITY BEACH

FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6940

PREPARED FOR

GREENPOINTE COMMUNITIES, LLC

7807 BAYMEADOWS ROAD E
SUITE 205
JACKSONVILLE, FL 32256

PHONE (904) 562-1358
FAX (904) 996-2481

PROJECT DESCRIPTION

HAMPTON LAKES AT RIVER HALL

FORMERLY KNOWN AS SINGLE FAMILY RESIDENTIAL AT HAWKS HAVEN PHASE 1

SOUTH

PART OF SECTION 35
TOWNSHIP 43 SOUTH
RANGE 26 EAST
LEE COUNTY, FLORIDA

ENGINEER OF RECORD

CARL A. BARRACO, JR., P.E., FOR THE FIRM
FLORIDA P.E. NO. 81259 - CARLJR@BARRACO.NET

NOV 19 2025

CARL A. BARRACO, JR. LICENSED No. 81259 STATE OF FLORIDA PROFESSIONAL ENGINEER

DRAWING NOT VALID WITHOUT SEAL, SIGNATURE AND DATE
© COPYRIGHT 2025, BARRACO AND ASSOCIATES, INC.
REPRODUCTION, CHANGES OR ASSIGNMENTS ARE PROHIBITED

FILE NAME	24285L06.DWG
LOCATION	J:\24285\DWG\LAMSID\
PLOT DATE	WED. 11-19-2025 - 2:27 PM
PLOT BY	BRYCE ROCHA

CROSS REFERENCED DRAWINGS

SURVEY = 22951S00.DWG

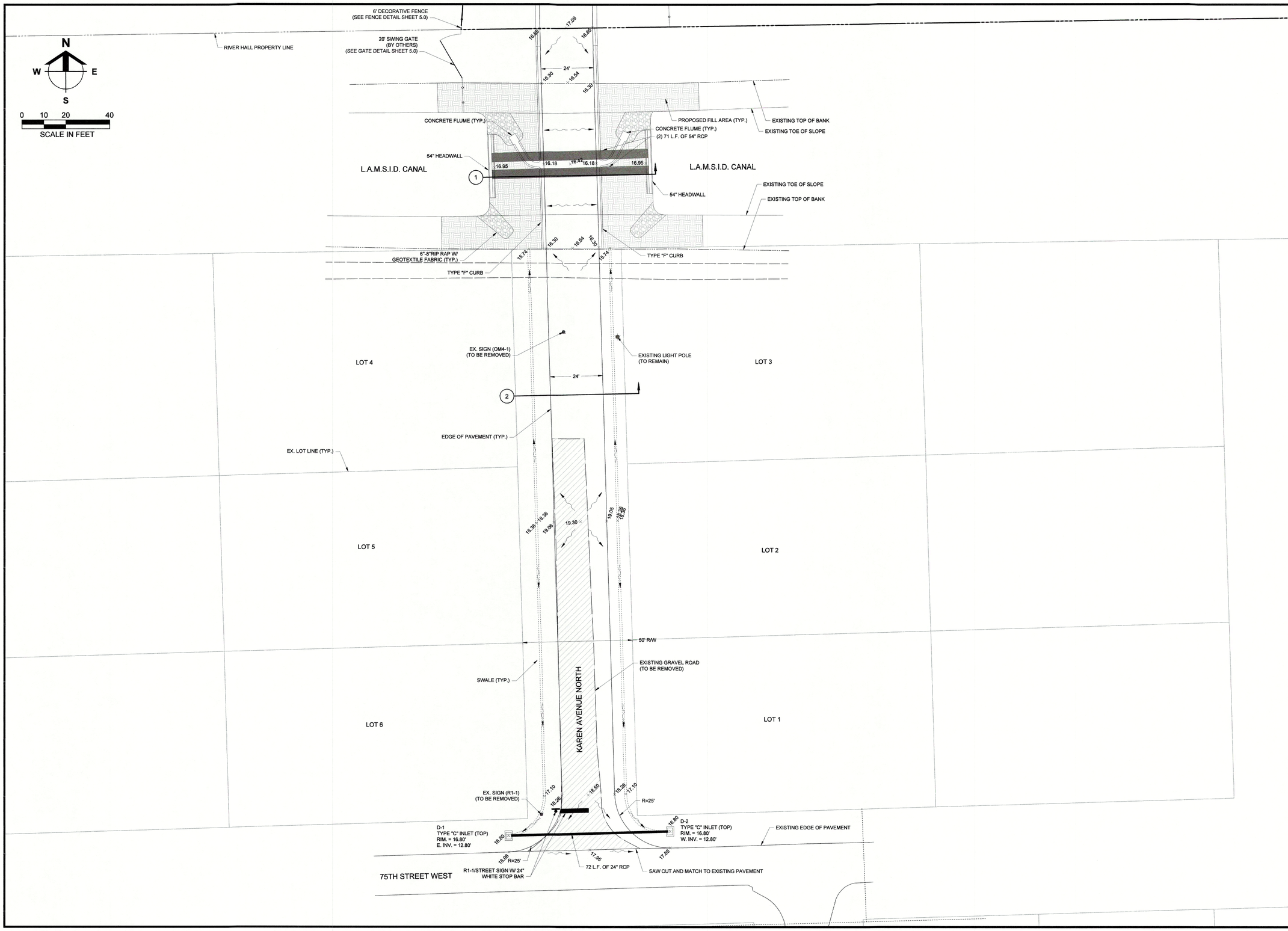
PLAN REVISIONS

PLAN STATUS

APPROVAL SUBMITTAL PLANS
2025-11-05

AERIAL PHOTOGRAPH AND EXISTING CONDITIONS PLAN

PROJECT / FILE NO.	SHEET NUMBER
24285	3.0



Barraco and Associates, Inc.

CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING

www.barraco.net

2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FORT MYERS | PANAMA CITY BEACH

FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6940

PREPARED FOR

GREENPOINTE COMMUNITIES, LLC

7807 BAYMEADOWS ROAD E
SUITE 205
JACKSONVILLE, FL 32256

PHONE (904) 562-1358
FAX (904) 996-2481

PROJECT DESCRIPTION

HAMPTON LAKES AT RIVER HALL

FORMERLY KNOWN AS SINGLE FAMILY RESIDENTIAL AT HAWKS HAVEN PHASE 1

SOUTH

PART OF SECTION 35
TOWNSHIP 43 SOUTH
RANGE 26 EAST
LEE COUNTY, FLORIDA

ENGINEER OF RECORD

CARL A. BARRACO, JR., P.E., FOR THE FIRM
FLORIDA P.E. NO. 81259 - CARLJR@BARRACO.NET

NOV 19 2025



DRAWING NOT VALID WITHOUT SEAL, SIGNATURE AND DATE.
© COPYRIGHT 2025, BARRACO AND ASSOCIATES, INC.
REPRODUCTION, CHANGES OR ASSIGNMENTS ARE PROHIBITED

FILE NAME	24285L10.DWG
LOCATION	J:\24285\DWG\LAMSID\
PLOT DATE	WED. 11-19-2025 - 2:27 PM
PLOT BY	BRYCE ROCHA

CROSS REFERENCED DRAWINGS

BASEPLAN = 24285L00.DWG
SURVEY = 22951S00.DWG

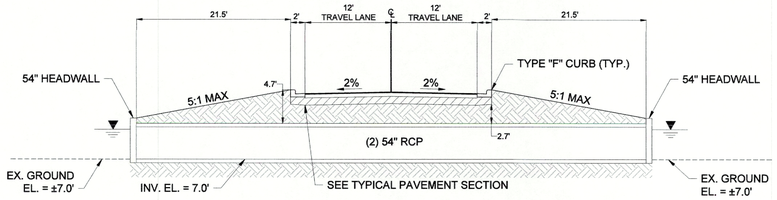
PLAN REVISIONS

PLAN STATUS

.

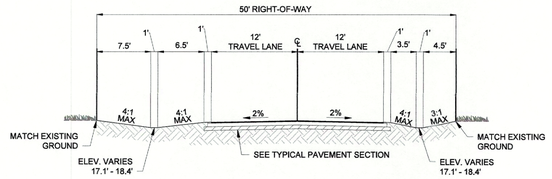
DETAILED SITE PLAN

PROJECT / FILE NO.	SHEET NUMBER
23741	4.0



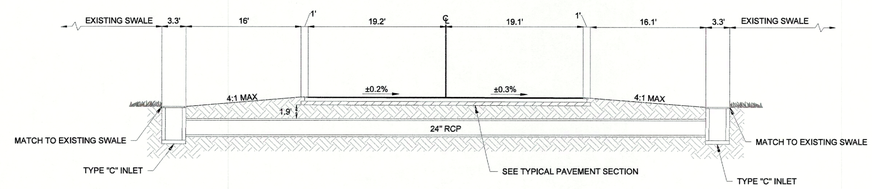
1 CANAL CROSSING

N.T.S.



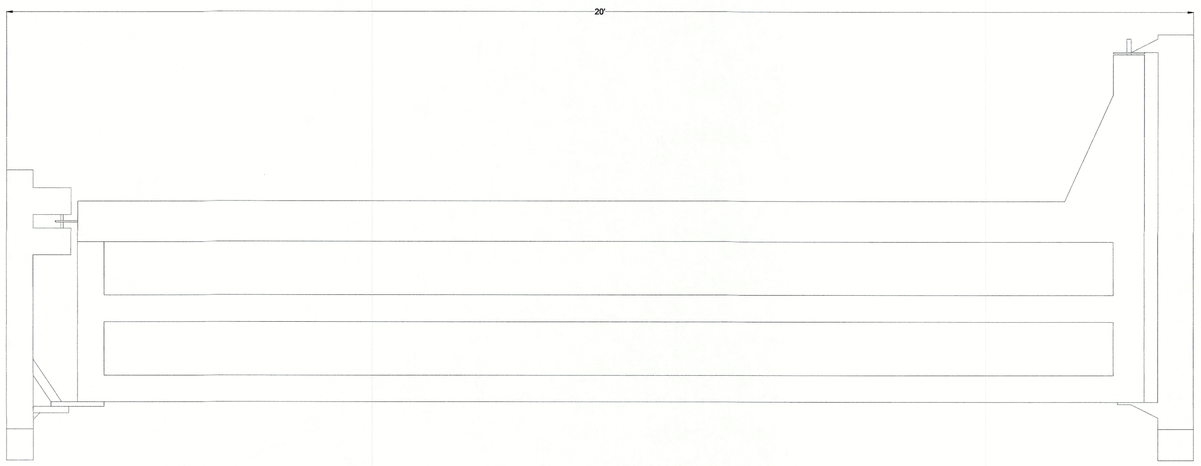
2 50' RIGHT-OF-WAY ROAD SECTION

N.T.S.



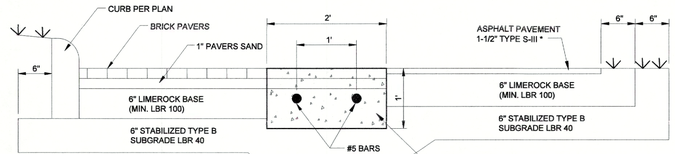
3 JOAN AVENUE AT CROSS DRAIN

N.T.S.



GATE DETAIL SCHEMATIC

N.T.S.



STRUCTURAL EQUIVALENCY CALCULATIONS:

STANDARD PAVEMENT SECTION	LAYER COEFFICIENT	THICKNESS	SUBTOTAL
TYPE S-III ASPHALT PAVEMENT	0.30 / INCH	1-1/2"	0.45
LIMEROCK BASE (LBR 100)	0.18 / INCH	8"	1.44
STABILIZED SUBGRADE (LBR 40)	0.08 / INCH	6"	0.96
TOTAL			2.85

PAVER BRICK SECTION	LAYER COEFFICIENT	THICKNESS	SUBTOTAL
ECONOCRETE (1100 PSI) *	0.44 / INCH	3 1/8"	1.38
LIMEROCK BASE	0.18 / INCH	8"	1.44
STABILIZED SUBGRADE (LBR 40)	0.08 / INCH	6"	0.96
TOTAL			3.78

* PAVER BRICKS ARE 8000 PSI. ECONOCRETE USED FOR COMPARISON ONLY.

2' WIDE CONCRETE RIBBON AT EACH END OF BRICK PAVERS OR AS SHOWN ON PLANS

COST OF BRICK PAVERS, LIMEROCK BASE AND SAND TO BE INCLUDED IN UNIT COST FOR BRICK PAVERS. COST OF SUBGRADE TO BE INCLUDED IN UNIT COST FOR STABILIZED SUBGRADE.

* TWO (2) 3/4" LIFTS OF TYPE S-III ASPHALTIC CONCRETE MAY BE PROVIDED AS AN ALTERNATE. THE FIRST LIFT SHALL BE PLACED PRIOR TO CERTIFICATE OF COMPLETION FOR SITE WORK. THE SECOND LIFT SHALL BE PLACED PRIOR TO RECEIVING THE CERTIFICATE OF OCCUPANCY FOR THE LAST UNIT IN EACH PHASE OF CONSTRUCTION.

ALL UNPAVED AREAS WITHIN THE RIGHT OF WAY SHALL RECEIVE SOD OR SEED & MULCH.

LIMEROCK BASE AND STABILIZED SUBGRADE SHALL BE COMPACTED TO A MINIMUM OF 98% OF MODIFIED PROCTOR (AASHTO T-180)

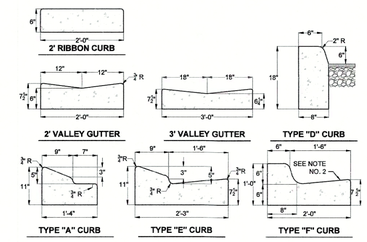
OPTIONAL BRICK PAVER SURFACE MAY BE UTILIZED FOR ALL OR PART OF THE PRIVATE ROADWAY AT THE DISCRETION OF THE OWNER.

TYPICAL PAVEMENT SECTION

N.T.S.

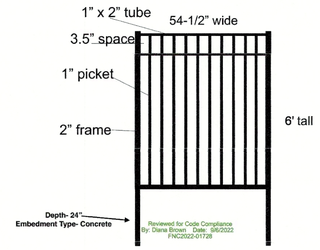
NOTES

1. SEE F.D.O.T. DESIGN STANDARDS INDEX 300 FOR ADDITIONAL DETAILS.
2. WHEN USED ON HIGH SIDE OF ROADWAYS, THE CROSS SLOPE OF THE GUTTER SHALL MATCH THE CROSS SLOPE OF THE ADJACENT PAVEMENT. THE THICKNESS OF THE LIP SHALL BE 6", UNLESS OTHERWISE SHOWN ON PLANS.
3. CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH OF 3,000 PSI. WITHIN 28 DAYS UNLESS NOTED OTHERWISE.



CONCRETE CURB

N.T.S.



FENCE DETAIL SCHEMATIC

THIS DRAWING CONTAINS FENCE AND GATE DETAILS ENGINEERED, DESIGNED AND PROVIDED BY OTHERS. THESE DETAILS WERE NOT DESIGNED BY BARRACO AND ASSOCIATES, INC.

Barraco and Associates, Inc.
CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING
www.barraco.net
2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FORT MYERS | PANAMA CITY BEACH
FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6940

PREPARED FOR

GREENPOINTE COMMUNITIES, LLC
7807 BAYMEADOWS ROAD E
SUITE 205
JACKSONVILLE, FL 32256

PHONE (904) 562-1358
FAX (904) 996-2481

PROJECT DESCRIPTION

HAMPTON LAKES AT RIVER HALL

FORMERLY KNOWN AS SINGLE FAMILY RESIDENTIAL AT HAWKS HAVEN PHASE 1

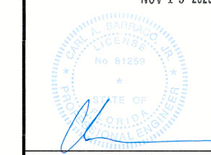
SOUTH

PART OF SECTION 35
TOWNSHIP 43 SOUTH
RANGE 26 EAST
LEE COUNTY, FLORIDA

ENGINEER OF RECORD

CARL A. BARRACO, JR., P.E., FOR THE FIRM
FLORIDA P.E. NO. 81259 - CARLJR@BARRACO.NET

NOV 19 2025



DRAWING NOT VALID WITHOUT SEAL, SIGNATURE AND DATE
© COPYRIGHT 2025, BARRACO AND ASSOCIATES, INC.
REPRODUCTION, CHANGES OR ASSIGNMENTS ARE PROHIBITED

FILE NAME	24285L18.DWG
LOCATION	J:\24285\DWG\LAMSID\
PLOT DATE	WED, 11-19-2025 - 2:28 PM
PLOT BY	BRYCE ROCHA

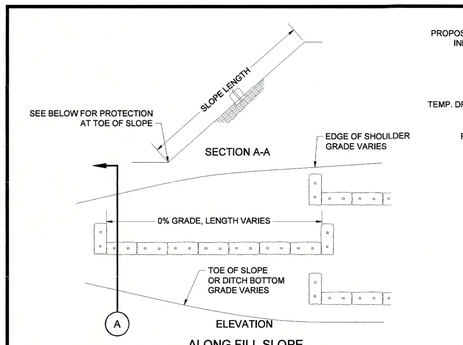
CROSS REFERENCED DRAWINGS

PLAN REVISIONS

PLAN STATUS

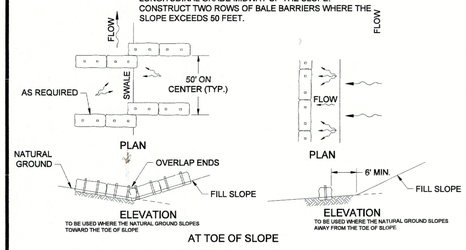
TYPICAL SECTIONS AND DETAILS

PROJECT / FILE NO.	SHEET NUMBER
24285	5.0



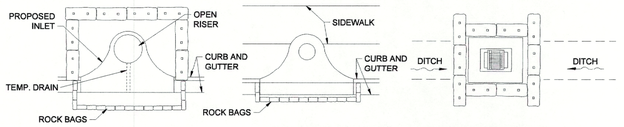
ALONG FILL SLOPE

NOTE:
WHERE THE SLOPE LENGTH EXCEEDS 25 FEET, CONSTRUCT ONE ROW OF BALE BARRIERS AT 0% LONGITUDINAL GRADE MIDWAY UP THE SLOPE. CONSTRUCT TWO ROWS OF BALE BARRIERS WHERE THE SLOPE EXCEEDS 50 FEET.

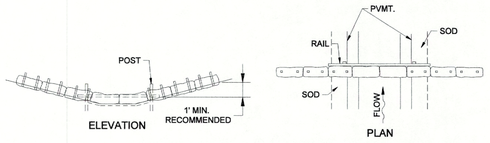


AT TOE OF SLOPE

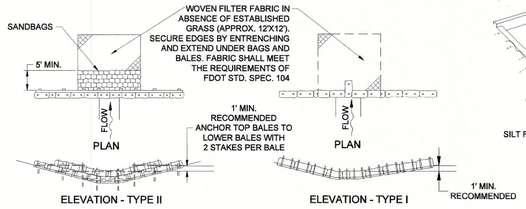
BARRIERS FOR FILL SLOPES



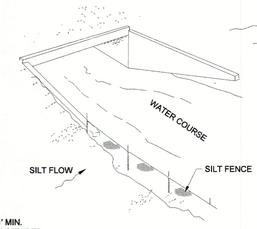
PROTECTION AROUND INLETS OR SIMILAR STRUCTURES

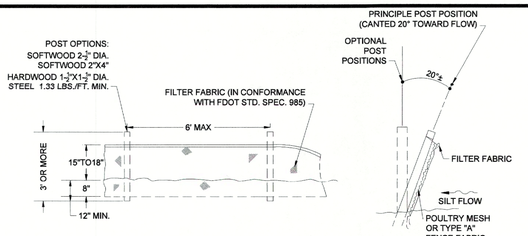


SYNTHETIC BALES OR BALE TYPE BARRIERS FOR PAVED DITCHES

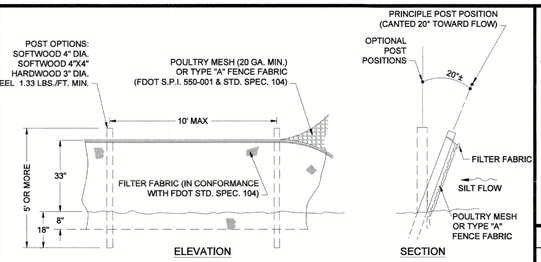


SYNTHETIC BALES OR BALE TYPE BARRIERS FOR UNPAVED DITCHES

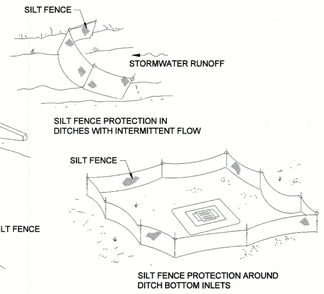




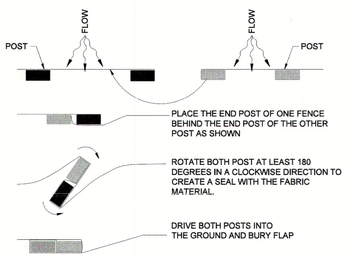
TYPE III SILT FENCE



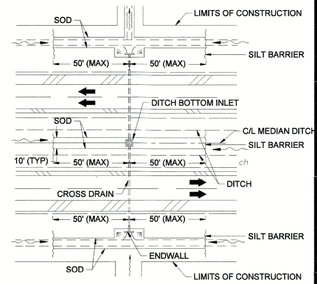
TYPE IV SILT FENCE



SILT FENCE APPLICATIONS



JOINING TWO SILT FENCES

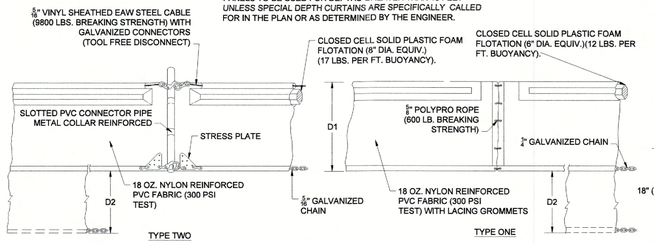


DITCH INSTALLATIONS AT DRAINAGE STRUCTURES

TEMPORARY EROSION AND SEDIMENT CONTROL DETAILS

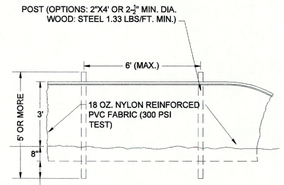
NOTE: NOT ALL NOTES, DETAILS, SYMBOLS OR OTHER STANDARDS SHOWN ON THIS SHEET MAY BE APPLICABLE TO THIS PROJECT

D1=5' STD. (SINGLE PANEL FOR DEPTHS 5' OR LESS)
D2=5' STD. (ADDITIONAL PANEL FOR DEPTHS GREATER THAN 5')
CURTAIN TO REACH BOTTOM UP TO DEPTHS OF 10 FEET. TWO (2) PANELS TO BE USED FOR DEPTHS GREATER THAN 10 FEET UNLESS SPECIAL DEPTH CURTAINS ARE SPECIFICALLY CALLED FOR IN THE PLAN OR AS DETERMINED BY THE ENGINEER.



NOTICE:
COMPONENTS OF TYPE ONE AND TWO MAY BE SIMILAR OR IDENTICAL TO PROPRIETARY DESIGNS. ANY INFRINGEMENT OF THE PROPRIETARY RIGHTS OF THE DESIGNER SHALL BE THE SOLE RESPONSIBILITY OF THE USER. SUBSTITUTIONS FOR TYPES ONE AND TWO SHALL BE AS APPROVED BY THE ENGINEER.

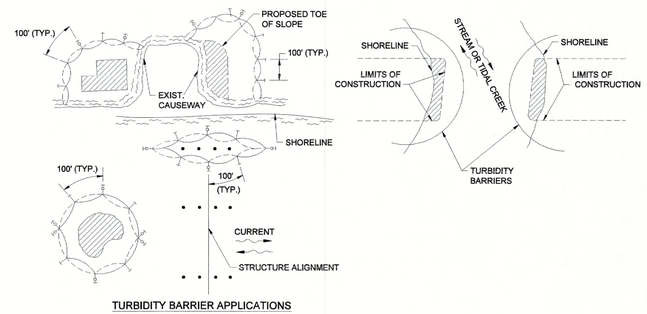
FLOATING TURBIDITY BARRIERS



STAKED TURBIDITY BARRIERS

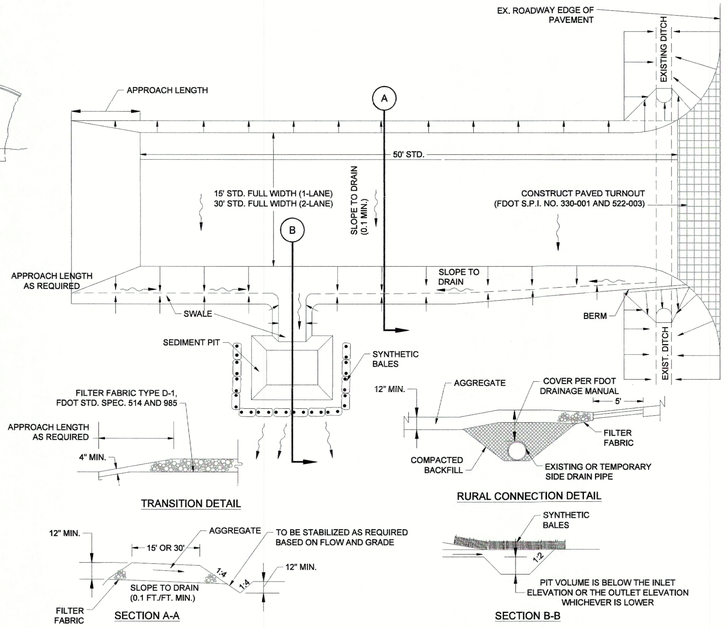
LEGEND

- PILE LOCATIONS
- DREDGE OR FILL AREA
- MOORING BUOY W/ANCHOR
- ANCHOR
- BARRIER MOVEMENT DUE TO CURRENT ACTION



TURBIDITY BARRIER APPLICATIONS

TURBIDITY BARRIERS



SOIL TRACKING PREVENTION DEVICE - TYPE "A"

EROSION CONTROL NOTES:

1. PRIOR TO COMMENCEMENT OF CONSTRUCTION ACTIVITY APPROPRIATE EROSION CONTROL DEVICES SHALL BE INSTALLED TO CONTROL AND REDUCE SOIL EROSION AND SEDIMENT TRANSPORT TO OFF-SITE AREAS. THE CONTRACTOR SHALL MAINTAIN THESE DEVICES THROUGHOUT THE DURATION OF CONSTRUCTION. ALL DEVICES SHALL REMAIN IN PLACE UNTIL PERMANENT EROSION CONTROL IS ESTABLISHED.
2. THE CONTRACTOR SHALL SUPPLEMENT THIS PLAN AS REQUIRED TO CONTROL AND REDUCE SOIL EROSION BASED ON THE CONTRACTORS MEANS, METHODS AND TECHNIQUE OF CONSTRUCTION. IT IS RECOMMENDED THAT THE CONTRACTOR COMPLY WITH THE LATEST EDITIONS OF THE F.D.O.T. STANDARD PLAN INDEX 570-001 (PREVENTION, CONTROL AND ABATEMENT OF EROSION AND WATER POLLUTION) AND THE STATE OF FLORIDA EROSION AND SEDIMENT CONTROL DESIGNER AND REVIEWER MANUAL (THE "E&SC MANUAL").
3. THE DETAILS ON THIS SHEET REPRESENT TYPICAL BEST MANAGEMENT PRACTICES FOR SOIL EROSION CONTROL. THEY MAY NOT SATISFY ALL REQUIREMENTS FOR COMPLIANCE WITH REGULATORY AGENCIES AND / OR SPECIFIC PERMIT CONDITIONS AND ALL MAY NOT APPLY TO THIS PROJECT.
4. THE CONTRACTOR AND/OR OWNER SHALL VERIFY THAT THE RESPONSIBLE ENTITY HAS OBTAINED A NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (N.P.D.E.S.) GENERAL PERMIT FROM THE UNITED STATES ENVIRONMENTAL PROTECTION AGENCY (E.P.A.) / F.D.E.P. AND LOCAL REGULATORY AGENCY AT LEAST 48 HOURS PRIOR TO THE START OF CONSTRUCTION. CONTRACTOR SHALL CREATE, IMPLEMENT AND MAINTAIN A STORMWATER POLLUTION PREVENTION PLAN AS REQUIRED BY THE N.P.D.E.S. GENERAL PERMIT.
5. AREAS LOCATED ADJACENT TO WETLAND / UPLAND PRESERVE AREAS SHALL BE STABILIZED WITH SOD (OR AS CALLED FOR ON DETAILED PLANS) IMMEDIATELY UPON ACHIEVING FINAL GRADE.
6. EROSION CONTROL DEVICES WILL BE INSTALLED ALONG THE BOUNDARY OF THE CONSERVATION AREAS, PRESERVES AND/OR WETLAND AREAS PRIOR TO CONSTRUCTION. THESE DEVICES SHALL BE SILT SCREENS AND, IF NECESSARY, SYNTHETIC BALES. THESE DEVICES SHALL REMAIN IN PLACE UNTIL CONSTRUCTION IS COMPLETED AND THE ADJACENT CONSTRUCTION ZONES ARE ESTABLISHED.
7. CONTRACTOR SHALL REMOVE ALL TEMPORARY EROSION CONTROL METHODS UPON COMPLETION OF SOIL STABILIZATION FOR THE PROJECT. AT THAT TIME THE RESPONSIBLE ENTITY SHALL FILE AN N.P.D.E.S. STORMWATER POLLUTION PREVENTION PLAN NOTICE OF TERMINATION WITH THE F.D.E.P.
8. DURING CONSTRUCTION, INLET OPENINGS, WHERE APPROPRIATE, SHALL BE COVERED WITH FILTER FABRIC (MIRAFI 140N OR APPROVED EQUAL) TO PREVENT DEBRIS FROM FALLING INTO THE INLET.
9. CONTRACTOR SHALL UTILIZE APPROPRIATE METHODS TO CONTROL WIND EROSION AND DUST. ALL AREAS SHALL BE STABILIZED AS NECESSARY TO CONTROL EROSION WITHIN 7 DAYS OF FINAL GRADING.
10. FOR ADDITIONAL EROSION CONTROL DETAILS NOT SHOWN ON THIS PLAN, SEE LATEST EDITION OF THE STATE OF FLORIDA EROSION AND SEDIMENT CONTROL DESIGNER AND REVIEWER MANUAL.

CONTRACTOR RESPONSIBILITIES:

1. PREPARE AND SIGN A NOTICE OF INTENT FORM (N.O.I.) AND SUBMIT TO THE REGULATORY AGENCY ALONG WITH ANY REQUIRED FEES AND ATTACHMENTS, OR ASSURE THAT AN N.O.I. HAS BEEN FILED AS REQUIRED.
2. IMPLEMENT THE STABILIZATION, EROSION CONTROL AND OTHER REQUIREMENTS OF THE S.W.P.P.P / EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN.
3. PROVIDE QUALIFIED INSPECTORS, AND DOCUMENTATION OF QUALIFICATIONS, FOR THE CONTROLS IMPLEMENTED AT THE JOB SITE.
4. CONDUCT ALL NECESSARY INSPECTIONS AT THE REQUIRED INTERVALS AND PREPARE AND RETAIN WRITTEN DOCUMENTATION OF THOSE INSPECTIONS AND ALL OTHER WRITTEN DOCUMENTATION REQUIRED BY THE GENERAL PERMIT.
5. KEEP A COPY OF THE S.W.P.P.P. / EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN, ALL N.O.I.'S, PERMIT CERTIFICATES; PERMIT LANGUAGE; SPILL PREVENTION, COUNTERMEASURES, AND CLEANUP ("S.P.C.C.") PLAN; INSPECTION RECORDS; AND OTHER REQUIRED RECORDS ON THE JOB SITE AND POST IN A PROMINENT PLACE NEAR THE JOB SITE ENTRANCE THOSE DOCUMENTS REQUIRED TO BE POSTED UNDER THE TERMS OF THE GENERAL PERMIT.
6. CONTRACTOR SHALL PROVIDE MONTHLY TRAINING SESSIONS FOR ALL ENTITIES AND SUBCONTRACTORS INVOLVED WITH INSTALLING, APPLYING, PERFORMING, MAINTAINING AND INSPECTION OF THE S.W.P.P.P. / EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN.
7. UPDATE AND MAKE CHANGES TO THE S.W.P.P.P. / EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN AND SUPPORTING DOCUMENTS (SUCH AS THE B.M.P.'S AND / OR S.P.C.C.) AS NEEDED AND WITH THE APPROVAL OF THE OPERATOR AND THE OPERATOR'S ENGINEER.
8. PREPARE AND SIGN A NOTICE OF TERMINATION (N.O.T.) FORM WHEN SITE WORK CONSTRUCTION IS COMPLETED AND STABILIZATION IS ACHIEVED. TRANSFER THE S.W.P.P.P. / EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN DOCUMENTS, ALONG WITH ALL N.O.I.'S, PERMIT CERTIFICATES, N.O.T.'S, AND WRITTEN RECORDS REQUIRED BY THE GENERAL PERMIT TO THE OPERATOR FOR ARCHIVING IN BOTH PAPER AND OPTICALLY-SCANNED FORMAT ON A CD.

Barraco and Associates, Inc.
CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING
www.barraco.net
2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FORT MYERS | PANAMA CITY BEACH
FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6940

PREPARED FOR

GREENPOINTE COMMUNITIES, LLC
7807 BAYMEADOWS ROAD E
SUITE 205
JACKSONVILLE, FL 32256

PHONE (904) 562-1358
FAX (904) 996-2481

PROJECT DESCRIPTION

HAMPTON LAKES AT RIVER HALL
FORMERLY KNOWN AS SINGLE FAMILY RESIDENTIAL AT HAWKS HAVEN PHASE 1

SOUTH

PART OF SECTION 35
TOWNSHIP 43 SOUTH
RANGE 26 EAST
LEE COUNTY, FLORIDA

ENGINEER OF RECORD

CARL A. BARRACO, JR., P.E., FOR THE FIRM
FLORIDA P.E. NO. 81259 - CARLJR@BARRACO.NET

NOV 19 2025



DRAWING NOT VALID WITHOUT SEAL, SIGNATURE AND DATE
© COPYRIGHT 2025, BARRACO AND ASSOCIATES, INC.
REPRODUCTION, CHANGES OR ASSIGNMENTS ARE PROHIBITED

FILE NAME	24285L50.DWG
LOCATION	J:\24285\DWG\LAMSID\
PLOT DATE	WED. 11-19-2025 - 2:28 PM
PLOT BY	BRYCE ROCHA

CROSS REFERENCED DRAWINGS

MASTER = BAI-EROSION-1-3D.DWG

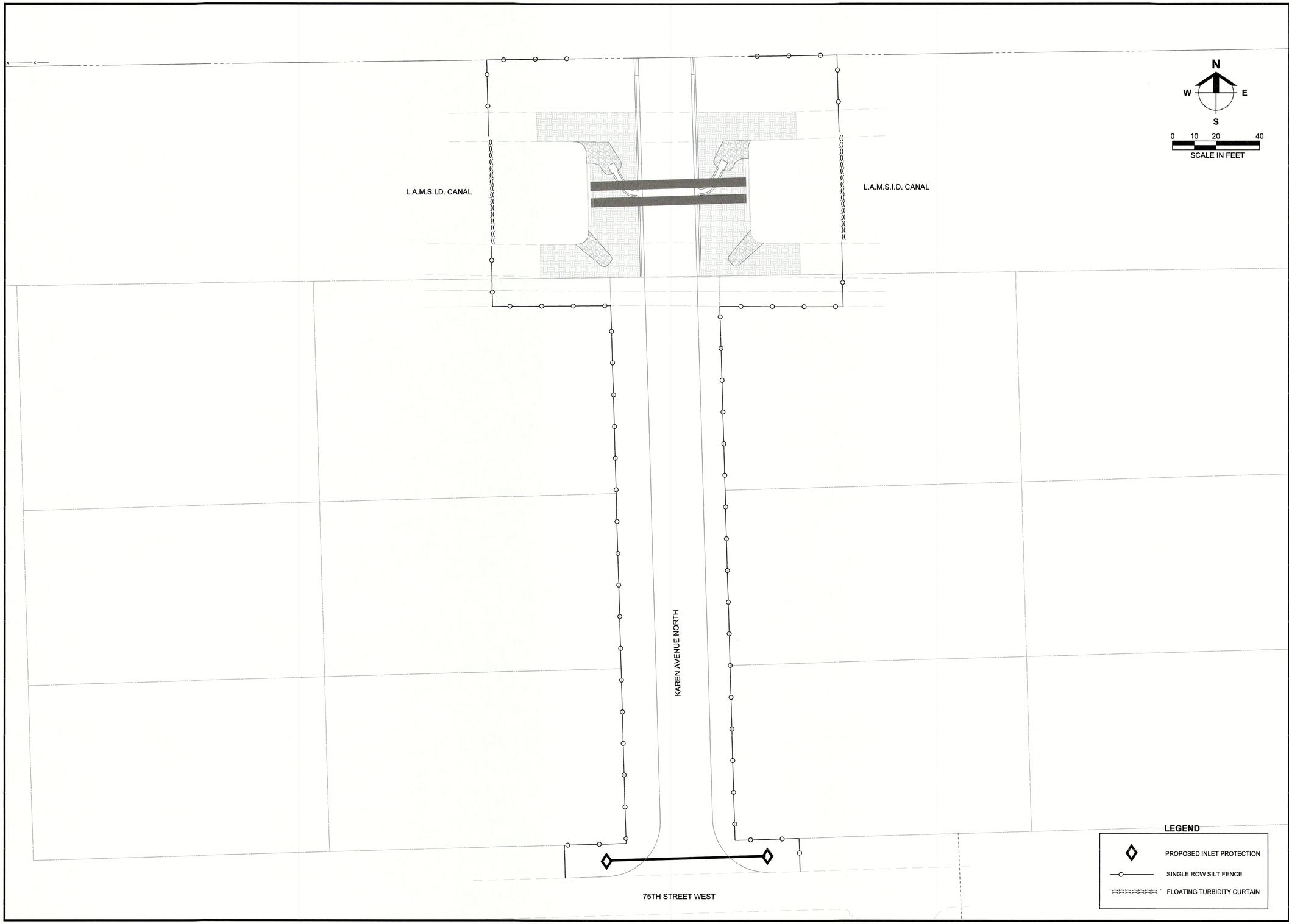
PLAN REVISIONS

PLAN STATUS

APPROVAL SUBMITTAL PLANS
2025-11-05

EROSION CONTROL DETAILS

PROJECT / FILE NO.	SHEET NUMBER
24285	6.0



Barraco and Associates, Inc.
CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING

www.barraco.net

2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FORT MYERS | PANAMA CITY BEACH

FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6940

PREPARED FOR

GREENPOINTE COMMUNITIES, LLC

7807 BAYMEADOWS ROAD E
SUITE 205
JACKSONVILLE, FL 32256

PHONE (904) 562-1358
FAX (904) 996-2481

PROJECT DESCRIPTION

HAMPTON LAKES AT RIVER HALL

FORMERLY KNOWN AS SINGLE FAMILY RESIDENTIAL AT HAWKS HAVEN PHASE 1

SOUTH

PART OF SECTION 35
TOWNSHIP 43 SOUTH
RANGE 26 EAST
LEE COUNTY, FLORIDA

ENGINEER OF RECORD

CARL A. BARRACO, JR., P.E., FOR THE FIRM
FLORIDA P.E. NO. 81259 - CARL.JR@BARRACO.NET

NOV 19 2025

CARL A. BARRACO JR. LICENSE No. 81259 STATE OF FLORIDA PROFESSIONAL ENGINEER

DRAWING NOT VALID WITHOUT SEAL, SIGNATURE AND DATE
© COPYRIGHT 2025, BARRACO AND ASSOCIATES, INC.
REPRODUCTION, CHANGES OR ASSIGNMENTS ARE PROHIBITED

FILE NAME	24285L52.DWG
LOCATION	J:\24285\DWG\LAMSID\
PLOT DATE	WED, 11-19-2025 - 2:28 PM
PLOT BY	BRYCE ROCHA

CROSS REFERENCED DRAWINGS

BASEPLAN = 24285L00.DWG

PLAN REVISIONS

PLAN STATUS

APPROVAL SUBMITTAL PLANS
2025-11-05

STORMWATER POLLUTION PREVENTION PLAN

PROJECT / FILE NO.	SHEET NUMBER
24285	**7.0**

LANDOWNER'S AGREEMENT
BETWEEN
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
AND
RH VENTURE II, LLC

THIS LANDOWNER'S AGREEMENT ("Agreement") is entered into as of the _____ day of _____, by and between **LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT,** 601 East County Lane, Lehigh Acres. Florida 33936 (hereinafter referred to as "the District") and **RH VENTURE II, LLC,** a Florida limited liability company, 7807 Baymeadows Road East, Suite 205, Jacksonville, FL 32256, and its successors and assigns (hereinafter referred to as the "Landowner").

WITNESSETH:

WHEREAS, the Board of Commissioners (hereafter referred to as the "Board") of Lehigh Acres Municipal Services Improvement District (hereafter referred to as the "District") is authorized and empowered by Chapters 189, 197, and 298, F.S., Chapter 2015-202 and 2017-216 Laws of Florida (hereafter referred to as the "Special Acts"), to exercise the General and Special Powers authorized by the Special Acts; and

WHEREAS, the District is authorized by the Special Acts to acquire, operate and maintain water management facilities and other public improvements, which includes a certain canal right-of-way identified in the attached Exhibit "A" (the "District Right-of-Way"); and

WHEREAS, the Landowner owns certain real property adjacent to the District Right-of-Way identified as the entirety of the Hamptons Lakes East Subdivision of River Hall and more particularly described and identified in a map attached hereto as Exhibit "B" (the "Property"), which Property the Landowner intends to develop for residential and other purposes; and

WHEREAS, the Landowner is petitioning the District to allow a "road crossing" to 75th Street W from the Property and over a District owned canal at a location more particularly described and identified in a map attached hereto as Exhibit "C" (the "Canal Crossing"), for the purposes of providing a secondary ingress/egress access for emergency vehicles, general residential traffic and school bus traffic exclusively from the Property; and

WHEREAS, the District is willing to provide the Landowner, its successors and assigns the right to construct an access road at the Canal Crossing provided the Landowner satisfies certain terms and conditions; and

WHEREAS, the Parties wish to reduce to writing their understanding and agreements pertaining to construction of the aforementioned Canal Crossing improvements so that: (1) the Canal Crossing improvements are constructed in strict conformity with District engineering standards; and (2) adequate compensation may be provided to the District by the Landowner for the benefits provided to the Property from the use of the District's existing drainage infrastructure; and

NOW THEREFORE, in consideration of the premises and mutual covenants expressed herein, and for other good and valuable consideration, receipt of which is hereby acknowledged, the Parties agree as follows:

ARTICLE I. Ratification of Recitals. The parties to this Agreement do hereby ratify and acknowledge that the recitals as set forth hereinabove are true and correct to the best of their knowledge and belief and are incorporated herein by this reference.

ARTICLE II. Landowner's Obligations.

Section 2.01 The Landowner shall pay a Capital Improvement Reimbursement Fee based on a per-acre contribution that is calculated based upon the associated total acreage within the Landowner's Property as identified in Exhibit B. The Landowner's Capital Improvement Reimbursement shall be payable to the District to compensate the District for the District's existing capital improvements and water management system throughout the District (which the District has financed and maintained) which the Landowner will be benefiting from, utilizing and connecting to as by virtue of the construction of the Road Crossing over the District's canal system. The Landowner's proportional share of the Capital Improvement Reimbursement Fee has been calculated based upon: (1) the value of the District's audited total capital assets divided by the total number of assessable acres within the District, which yielded a Total Infrastructure Impact Cost of $60.47 per acre, (2) multiplied by the Landowner's Property's acreage (251 acres) as delineated and defined in Exhibit B, resulting in a total Capital Improvement Reimbursement Fee of $15,177.97. Said Capital Improvement Reimbursement Fee shall be payable by the Landowner to the District upon execution of this Agreement.

Section 2.02 Each time the Landowner's Property acreage increases beyond the acreage indicated in Exhibit "B", the Landowner shall be obligated to notify the District in writing of any such expansion within 60 days of the land expansion for purposes of paying the District an additional Capital Improvement Reimbursement Fee. In such an event, the Landowner shall be required to pay the District an additional Capital Improvement Reimbursement Fee for each expansion within 120 days of said expansion. The additional Capital Improvement Reimbursement Fee shall be calculated by multiplying the Landowner's newly added expanded acreage by the District's per acre total capital asset costs ($59.69) multiplied by a cost-of-living increase of 2% per year for each year following the effective date of this Agreement.

Section 2.03 The Landowner agrees, at no cost to the District, to provide the

District with any easements necessary within Landowner's Property to connect the Canal Crossing to the District's facilities, in recordable form, for construction, operation and maintenance of all District facilities located proximate to the Canal Crossing.

Section 2.04 The Landowner must apply for a LA-MSID permit. The permit will consider construction method, time of completion and maintenance issues. The landowner shall not commence construction of the Canal Crossing overon the District's right of way until such time as the District Engineer has reviewed and approved the permit which will include the plans and proposed improvements. The District shall not unreasonable withhold approval of the permit. In the event the District does not approve the permit required in this section, the Capital Improvement Reimbursement Fee outlined in section 2.01 shall be returned to the Landowner. The Landowner and District shall enter into a separate agreement that reimburses the District for the professional fees and expenses (including but not limited to the District's engineering and legal) incurred by the District associated with the Landowner's project. The Landowner's obligation to reimburse the District its professional fees shall continue for any additional professional services necessitated by any expansions as described in Section 2.02, above.

Section 2.05 Landowner shall, in general, be responsible for all costs associated with the construction activities for the proposed access road at the Canal Crossing. In addition, Landowner shall be responsible for all costs associated with operation and maintenance of the subject improvements including, but not necessarily limited to, periodic inspection of said improvements and any future removal and/or replacement of the subject improvements.

Section 2.06 Default Remedies

(a) In the event the Landowner should fail to timely comply with the terms, obligations and provisions of this Agreement, then, in that event the District may, if it so elects, terminate this Agreement following the provision of sixty (60) calendar days advanced written notice of such intent to terminate which notice shall reflect the nature of the basis for termination. During said sixty (60)-day calendar period, the Landowner shall have the right to cure the item or items that are the basis for the District's issuance of the notice of termination.

(b) In the event the Landowner should fail to pay any Capital Improvement Reimbursement Fee in accordance with Article II of this Agreement, the District shall be entitled, in addition to all other available legal remedies, to the following cumulative and non-exclusive remedies and/or damages:

(i) To impose and collect simple interest at the rate of 18% per annum on the unpaid balance of any outstanding Capital Improvement Reimbursement Fee with said interest to commence to accrue on the day immediately following the due date (the date of execution of this Agreement as provided for herein).

(ii) Following provision of 60 calendar days advance written notice to the Landowner, to: (A) terminate this Agreement and Remove said Canal Crossing Improvement, or (B) initiate such legal action as it deems necessary and appropriate to compel payment of the unpaid Capital Improvement Reimbursement Fee, plus accrued interest and/or compliance with the terms and provisions of this Agreement. Any unpaid Capital Improvement Reimbursement Fee, plus accrued interest, shall survive the termination of this Agreement and may be reduced to a judgment.

ARTICLE III. District's Obligations and Rights.

Section 3.01. Following the receipt of the Landowner's Capital Improvement Reimbursement Fee, the District shall timely process and review any design and engineering plans for the Canal Crossing that are submitted by the Landowner for the District's review and approval. The District, its engineers and its agents shall not unreasonably withhold approval the Canal Crossing designs submitted by the Landowner. Once the Canal Crossing is approved, the District shall cooperate with the Landowner in providing verification to Lee County that the Developer's obligation to provide secondary access has been satisfied.

Section 3.02 The District shall further have the right, but not the obligation, to perform, at the District's cost, periodic inspections of the subject improvements.

Section 3.03 In addition, the District shall have the right to complete emergency repairs of the subject improvements if deemed necessary as a matter of public health, safety, and welfare. In such case, the District shall be entitled to reimbursement from the Landowner of any costs incurred by the District in performing any such emergency repairs.

Section 3.04 Upon full execution of this Agreement, issuance of a District permit (after review and approval of plans) and the payment of the Landowner's Capital Improvement Reimbursement Fee to the District, a license over and across the Canal Crossing in favor of Landowner will be deemed to exist for the purposes expressed herein.

ARTICLE IV. Additional Terms.

Section 4.01 ATTORNEY'S FEES. It is hereby understood and agreed that in the event any lawsuit in the judicial system, federal or state, is brought to enforce compliance with this Agreement or interpret same, or if any administrative proceeding is brought for the same purposes, the prevailing party to said action shall recover all attorney's fees and costs, including appellate fees and costs.

Section 4.02 AMENDMENTS. This Agreement may be amended or modified at any time and in all respects by an instrument in writing executed by all the parties to this Agreement.

Section 4.03 SUCCESSORS. All of the terms and provisions of this Agreement shall be binding upon and inure to the benefit of and be enforceable by the parties, their heirs, executors, administrators, successors and assigns.

Section 4.04 ASSUMPTION OF COSTS. Landowner agrees to enter into a separate agreement with the District to pay all professional and legal fees and costs of the District in connection with the review and execution of this Agreement as described in Section 2.04, including without limitation, the costs associated with the District Engineer's review and approval of the Landowner's plans for connection to the District's infrastructure, and the fees and costs of the District's legal counsel.

Section 4.05 SEVERABILITY. If any provision of this Agreement shall be held or deemed to be or shall, in fact, be illegal, inoperative or unenforceable in any context, the same shall not affect any other provisions herein or render any other provision invalid, inoperative or unenforceable to any extent whatsoever. Further, this Agreement shall be construed and enforced as though said provision had not been contained herein and the Agreement shall be given full force and effect to the extent reasonably practicable.

Section 4.06 DISCLAIMER OF THIRD-PARTY BENEFICIARIES. With the exception of any rights herein expressly conferred, nothing expressed or mentioned in or to be implied from this Agreement is intended or shall be construed to give any person or entity other than the parties hereto, any legal or equitable right, remedy or claim under or with respect to this Agreement since this Agreement is intended to be for the sole and exclusive benefit of the parties hereto.

Section 4.07 NOTICES. All notices or other communications required or desired to be given or made under this Agreement shall be in writing and be either: (A) personally delivered, (B) e-mailed or (C) sent by certified mail, return receipt requested or registered mail with postage prepaid. All notices or other written communications shall be addressed as follows:

As to District:

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
601 East County Lane
Lehigh Acres, FL 33936
Attention: District Manager
E-mail: dlindsay@la-msid.com

As to Landowner:

RH Venture II
c/o GreenPointe Communities, LLC
7807 Baymeadows Road East, Suite 205
Jacksonville, FL 32256
Attention: Grady Miars
E-mail: gmiars@greenpointellc.com

If either party changes its mailing address, e-mail address or its designated recipient for notices, such change shall be communicated in writing to the other party within thirty (30) days of the change.

All notices personally delivered shall be deemed given or made upon actual receipt by the party, its agent or employee, to whom delivered; all notices sent electronically shall be deemed given on the date sent; and all notices sent by Certified or Registered Mail shall be deemed delivered on the earlier of (i) actual receipt by the party, its agent or employee or (ii) five (5) business days after deposit in U.S. Mail in accordance with the foregoing.

Section 4.08 CONFLICTS. Any prior agreements between the parties, in conflict with the provisions contained herein are, to the extent of any such conflict, hereby superseded and repealed by this Agreement.

Section 4.09 COUNTERPARTS. This Agreement may be executed in several counterparts, all or any of which shall be regarded for all purposes as one original and which together shall constitute but one and the same instrument.

Section 4.10 EFFECTIVE DATE. This Agreement shall be effective as of the last date it has been executed by all parties.

Section 4.11 GOVERNING LAW. It is the intention of the parties that the laws of the State of Florida shall govern the validity of this Agreement, the construction of its terms, and the interpretation of the rights and duties of the parties.

Section 4.12 VENUE. It is the intention of the parties that in the event any litigation should arise between the parties to this Agreement, then in that event the venue of said litigation shall be in Lee County, Florida.

Section 4.13 RECORDING. This Agreement shall not be recorded within the Public Records of Lee County, Florida.

Section 4.14 HOLD HARMLESS. Landowner hereby agrees to indemnify and hold harmless the District and its officers, supervisors, employees and agents from and against any and all liability, loss, cost, expense or damage they may incur as a

result of claims, demands, suits, costs, or judgments against them due to or arising out of this Agreement.

Section 4.15 ASSIGNABILITY. This Agreement may be assigned by Landowner to any subsequent owner of the Property, whereupon Landowner will be relieved of all obligations hereunder. The maintenance obligations for the Canal Crossing contained in this Agreement may be assigned to the River Hall Community Development District ("RHCDD") or to such other entity as may be approved in writing by the District. In the event of an assignment of interest from the Landowner to a successor entity, including but not limited to the RHCDD, such assigned entity shall be fully responsible for any additional Capital Improvement Reimbursement Fee due and owing to the District for any acreage expansion beyond the acreage indicated in Exhibit "B." The calculation of such a fee shall be as provided in Section 2.02, herein. Regardless of whether the Landowner assigns its interest in this Agreement to a public or private entity, the Capital Improvement Reimbursement Fee shall be deemed a user fee, and not a tax or assessment for benefit.

IN WITNESS WHEREOF, the parties have hereunto set their hands and seals on the date hereinafter set forth.

Executed by the District this _____ day of _____, 202____.

Lehigh Acres Municipal Services Improvement District

By: _____

Printed Name: Katy Hoover

Title: Chair

_____
Witness Signature

David E. Lindsay
Typed/Printed Witness Name

_____
Witness Signature

Wade Weatherford

Typed/Printed Witness Name

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this 22nd day of July, 2025, by Katy Hoover, Chair, of the LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT, an independent special district of the State of Florida, on behalf of said Lehigh Acres Municipal Services Improvement District, who is personally known to me.

[NOTARY SEAL]

CHRISTINA VALDEZ
Notary Public - State of Florida
Commission # HH 259641
My Comm. Expires Aug 27, 2026
Bonded through National Notary Assn.

NOTARY PUBLIC
My Commission Number: HH 259641
My Commission Expires: Aug 27, 2026

Executed by the Landowner this 22nd, day of July, 2025.

RH VENTURE II, LLC

Paola Sanchez
Witness Signature

Paola Sanchez
Typed/Printed Witness Name

By:
Printed Name: Graydon Miars
Title: Vice President

Witness Signature

Typed/Printed Witness Name

STATE OF FLORIDA

COUNTY OF Duval

The foregoing instrument was acknowledged before me this 9 day of October, 2025, by Graydon Miars, the Vice President, of RH Venture II, LLC., a Florida Limited Liability Company on behalf of the said corporation, who is personally known to me or produced _____ as identification.

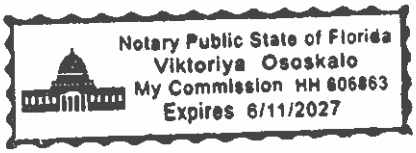
Vososkalo

NOTARY PUBLIC

My Commission Number: HH606863

My Commission Expires: 6/11/27

[NOTARY SEAL]





Barraco and Associates, Inc.

CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING

www.barraco.net

2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FORT MYERS | PANAMA CITY BEACH

FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6940

PREPARED FOR

GREENPOINTE COMMUNITIES, LLC

7807 BAYMEADOWS ROAD E
SUITE 205
JACKSONVILLE, FL 32256

PHONE (904) 562-1358
FAX (904) 996-2481

PROJECT DESCRIPTION

RIVER HALL

FORMERLY KNOWN AS
HAWKS HAVEN

PART OF SECTIONS 27,34,35 AND 36
TOWNSHIP 43 SOUTH
RANGE 26 EAST
LEE COUNTY, FLORIDA

THIS PLAN IS PRELIMINARY AND INTENDED FOR CONCEPTUAL PLANNING PURPOSES ONLY.

SITE LAYOUT AND LAND USE INTENSITIES OR DENSITIES MAY CHANGE SIGNIFICANTLY BASED UPON SURVEY, ENGINEERING, ENVIRONMENTAL AND / OR REGULATORY CONSTRAINTS AND / OR OPPORTUNITIES.

DRAWING NOT VALID WITHOUT SEAL, SIGNATURE AND DATE
© COPYRIGHT 2025, BARRACO AND ASSOCIATES, INC.
REPRODUCTION, CHANGES OR ASSIGNMENTS ARE PROHIBITED

FILE NAME	21988 HL EAST PH1-2 BOUNDARY EXH_A
LOCATION	J:\21988\DWG\EXHIBITS\
PLOT DATE	WED. 4-30-2025 - 11:08 AM
PLOT BY	BRYCE ROCHA

CROSS REFERENCED DRAWINGS

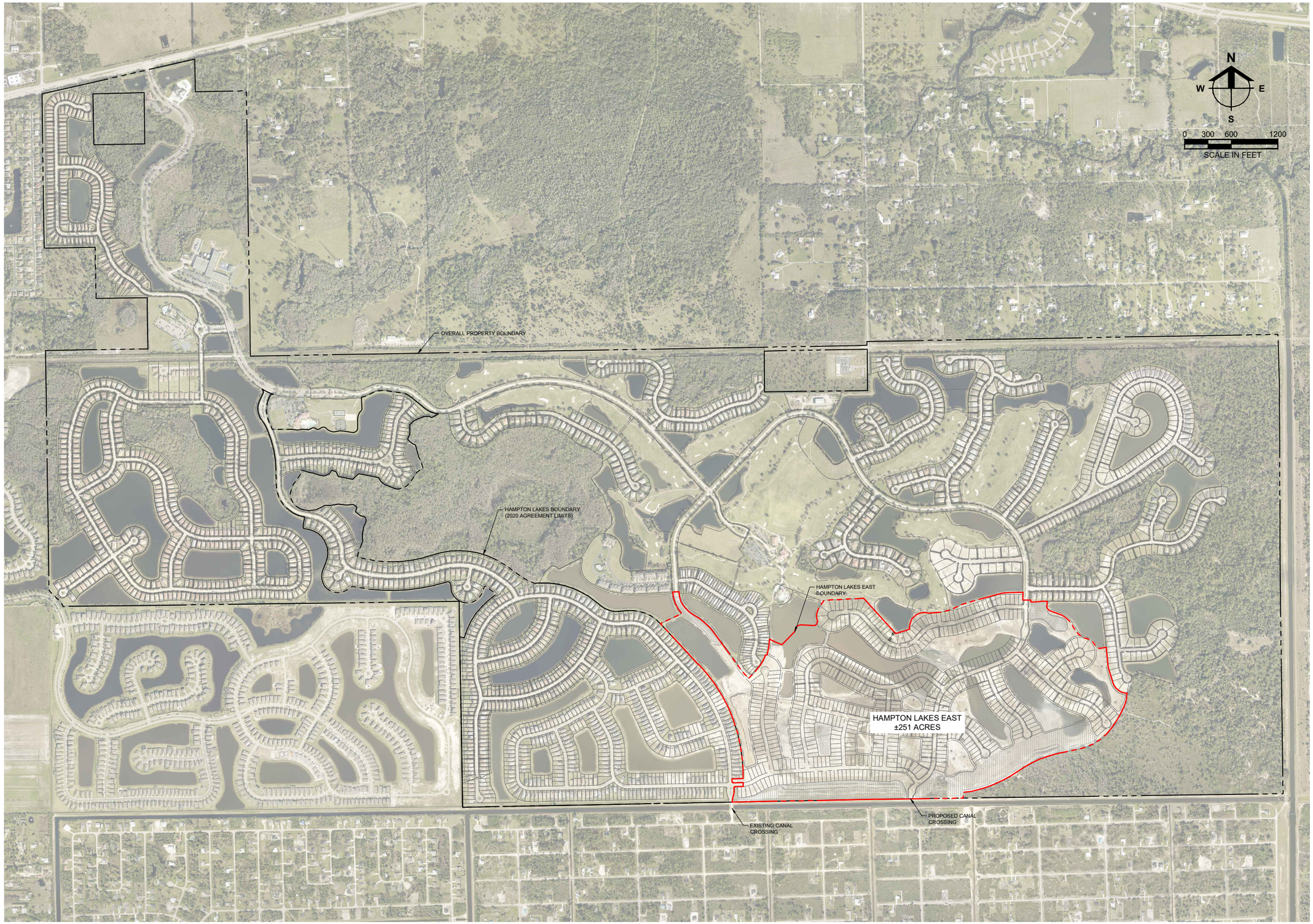
PLAN REVISIONS

PLAN STATUS

EXHIBIT

EXHIBIT "A"

PROJECT / FILE NO.	SHEET NUMBER
21988	**EXH**



Barraco and Associates, Inc.

CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING

www.barraco.net

2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FORT MYERS | PANAMA CITY BEACH

FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6940

PREPARED FOR

GREENPOINTE COMMUNITIES, LLC

7807 BAYMEADOWS ROAD E
SUITE 205
JACKSONVILLE, FL 32256

PHONE (904) 562-1358
FAX (904) 996-2481

PROJECT DESCRIPTION

RIVER HALL

FORMERLY KNOWN AS
HAWKS HAVEN

PART OF SECTIONS 27,34,35 AND 36
TOWNSHIP 43 SOUTH
RANGE 26 EAST
LEE COUNTY, FLORIDA

THIS PLAN IS PRELIMINARY AND INTENDED FOR CONCEPTUAL PLANNING PURPOSES ONLY.

SITE LAYOUT AND LAND USE INTENSITIES OR DENSITIES MAY CHANGE SIGNIFICANTLY BASED UPON SURVEY, ENGINEERING, ENVIRONMENTAL AND / OR REGULATORY CONSTRAINTS AND / OR OPPORTUNITIES.

DRAWING NOT VALID WITHOUT SEAL, SIGNATURE AND DATE
© COPYRIGHT 2025, BARRACO AND ASSOCIATES, INC.
REPRODUCTION, CHANGES OR ASSIGNMENTS ARE PROHIBITED

FILE NAME	21988 HAMPTONLAKESEASTBOUNDARY_
LOCATION	J:\21988\DWG\EXHIBITS\
PLOT DATE	WED. 4-30-2025 - 11:40 AM
PLOT BY	BRYCE ROCHA

CROSS REFERENCED DRAWINGS

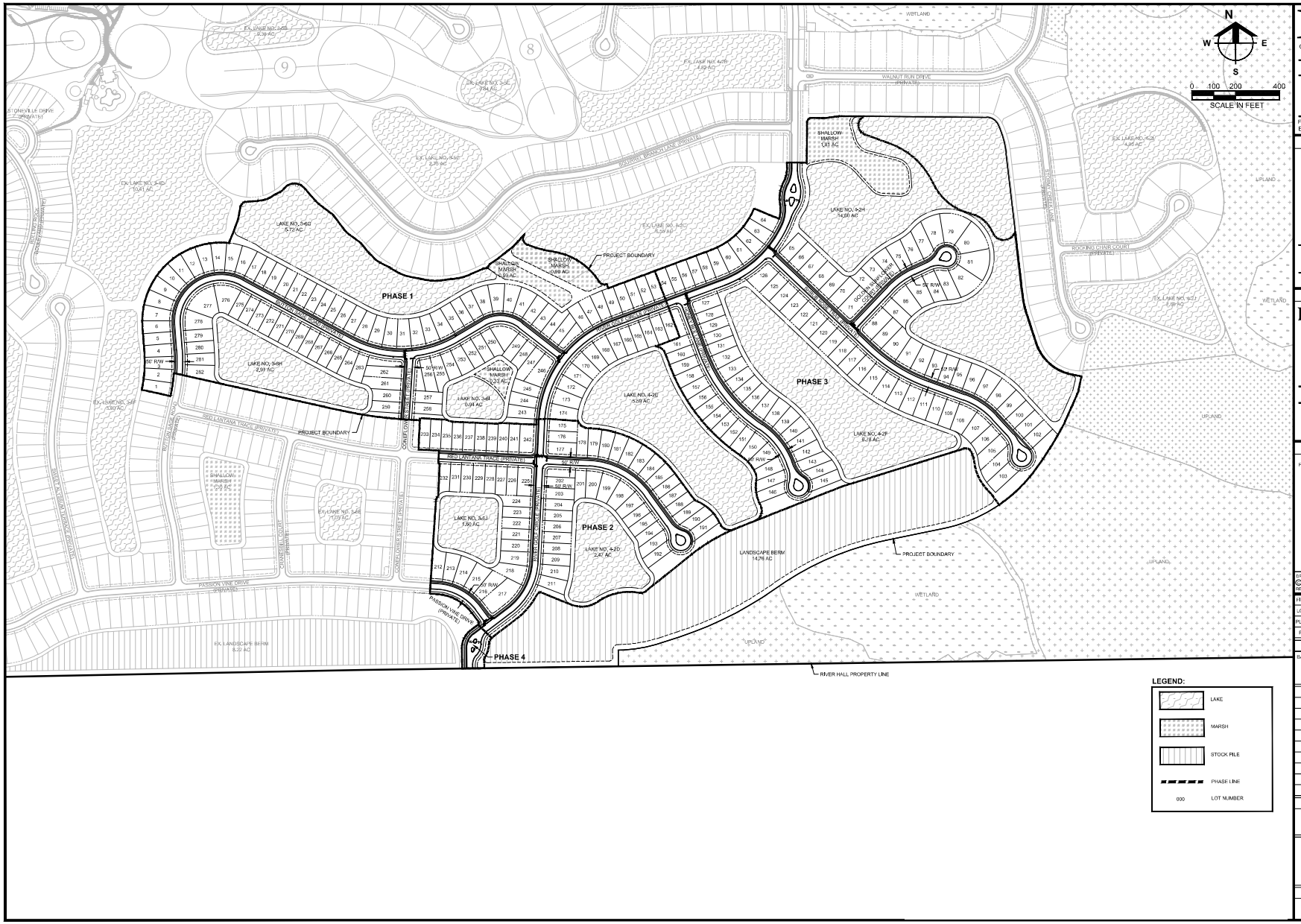
PLAN REVISIONS

PLAN STATUS

EXHIBIT

HAMPTON LAKES EAST BOUNDARY EXHIBIT

PROJECT / FILE NO.	SHEET NUMBER
21988	EXH



Barraco and Associates, Inc.
CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING
www.barraco.net
2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FORT MYERS | PANAMA CITY BEACH
FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6940

PREPARED FOR

GREENPOINTE COMMUNITIES, LLC
7807 BAYMEADOWS ROAD E
SUITE 205
JACKSONVILLE, FL 32256

PHONE (904) 562-1358
FAX (904) 996-2481

PROJECT DESCRIPTION

HAMPTON LAKES AT RIVER HALL

EAST PHASE II

PART OF SECTION 35
TOWNSHIP 43 SOUTH
RANGE 26 EAST
LEE COUNTY, FLORIDA

ENGINEER OF RECORD

CARL A. BARRACO, JR., P.E., FOR THE FIRM
FLORIDA P.E. NO. 81269 - CARL.R@BARRACO.NET

DRAWING NOT VALID WITHOUT SEAL, SIGNATURE AND DATE
©COPYRIGHT 2025, BARRACO AND ASSOCIATES, INC.
REPRODUCTION, CHANGES OR ASSIGNMENTS ARE PROHIBITED

FILE NAME	24285A10.DWG
LOCATION	S:\DESIGN DATA\24285\DWG\DO\
PLOT DATE	TUE, 4-29-2025 - 3:02 PM
PLOT BY	JAN RILEY/JUKEWICZ

CROSS REFERENCED DRAWINGS

BASEPLAN = 24285A00.DWG

PLAN REVISIONS

PLAN STATUS

APPROVAL SUBMITTAL PLANS
2025-04-29

EXHIBIT "C"

PROJECT / FILE NO.	SHEET NUMBER
	EX

Consideration: $10.00
Documentary transfer taxes $0.70

Prepared by and when recorded mail to:

Samanatha Duran
Foley & Lardner, LLP
100 N Tampa Street, Suite 2700
Tampa, Florida 33602

Project: Hampton Lakes at River Hall East Canal Crossing

PERPETUAL CANAL CROSSING EASEMENT AGREEMENT

THIS PERPETUAL CANAL CROSSING EASEMENT AGREEMENT ("**Agreement**") is made as of _____, 2025, by and between **LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT**, an Independent Special District of the State of Florida, formerly East County Water Control District ("**Grantor**"), whose address for notice purposes is 601 East County Lane, Lehigh Acres, Florida 33936, and **RH VENTURE II, LLC**, a Florida limited liability company ("**Grantee**"), whose address for notice purposes is 7807 Baymeadows Road East, Suite 205, Jacksonville, Florida 32256, with reference to the following facts:

A. Grantor owns certain real property situated in unincorporated Lee County, Florida (the "**County**"), more particularly described on **Exhibit "A"** attached hereto ("**Easement Area**").

B. Grantee is the owner and developer of certain real property situated in unincorporated Lee County, Florida, which is adjacent to the Easement Area and being developed as a community known as River Hall ("**Grantee's Property**").

C. Grantee has requested, and Grantor is willing to grant to Grantee, a non-exclusive easement to (i) install, construct, maintain, and repair a paved roadway crossing over, across, and through the Easement Area for pedestrian and vehicular ingress to and egress from Grantee's Property, and (ii) install, construct, maintain, and repair an associated drainage conveyance over, across, under, and through the Easement Area, all subject to and in accordance with the terms and conditions hereof.

NOW, THEREFORE, for Ten Dollars ($10.00) and other good and valuable consideration, the receipt and adequacy of which are hereby acknowledged, the parties agree as follows:

1. Grant of Easement. Grantor hereby grants to Grantee, its successors and assigns, a perpetual, non-exclusive easement ("**Easement**") for (a) the installation, construction, maintenance, repair, and replacement of a paved roadway crossing over, across and through the Easement Area (collectively, the "**Crossing Improvements**") to provide pedestrian and vehicular ingress to and egress from Grantee's Property, and (b) the installation, construction, maintenance, repair, replacement, and removal of an associated drainage conveyance, including, without limitation, the installation of pipes, structures, and appurtenant facilities, over, across, under, and through the Easement Area (collectively, the "**Drainage Improvements**", and together with the Crossing Improvements, the "**Improvements**"). The Easement shall include the associated rights of pedestrian and vehicular ingress and egress for Grantee and its employees, contractors, subcontractors, agents, and representatives (collectively,

"**Grantee's Permitted Parties**") as may be necessary to exercise Grantee's rights hereunder in accordance with the applicable permits and approvals from all applicable governmental agencies. Grantee shall have the rights to remove or trim any roots, trees, and other vegetation, and to remove structures or other obstructions, including fencing, within the Easement Area in order to properly install, construct, maintain, repair, replace, and remove the Improvements.

2. Grantor's Representations and Warranties. Grantor warrants that subject to any existing public roadway or utility easements, Grantor is in lawful possession of the Easement Area free and clear of all liens and encumbrances, except those recorded in the public records of the County (the "**Public Records**"), and has the right and power to convey the Easement and enter into this Agreement.

3. Maintenance and Repair. Grantee shall own all rights, title, and interest in the Improvements, and Grantee, at its sole cost, agrees to maintain the Improvements in accordance with the applicable permits and approvals from all applicable governmental agencies.

4. Grantee's Work in Easement Area. Any work conducted by or on behalf of Grantee in connection with the exercise of Grantee's rights hereunder shall be conducted subject to the following: (a) Grantee shall provide Grantor with the plans approved by the applicable governmental agencies for the work to be performed; and (b) Grantee shall utilize the Easement Area in a manner which does not unreasonably interfere with Grantor's use of Grantor's property surrounding the Easement Area.

5. Damage to Easement Area; Indemnification. If Grantee, or any of Grantee's Permitted Parties, causes any damage to the Easement Area or surrounding property owned by Grantor, or any improvements located thereon, now or in the future, as a result of exercising Grantee's rights hereunder, then Grantee shall promptly repair the damaged portion of the Easement Area, surrounding land owned by Grantor and/or improvements located thereon, as applicable, to the condition that existed immediately prior to such damage. Grantee shall indemnify, defend, and hold Grantor harmless from and against any and all claims, actions, suits, liability, damages, penalties, fines, costs and expenses, whether personal injury, property damage, liens or otherwise, suffered or incurred in connection with the negligence or willful misconduct of Grantee or Grantee's Permitted Parties. The foregoing obligations of Grantee shall survive any termination of this Agreement.

6. Covenants Running With the Land; Binding Effect. The Easement and terms of this Agreement shall be appurtenant and a benefit to Grantee's Property, and a burden against the Easement Area, constituting covenants running with the land, binding upon and inuring to the benefit of the parties' respective successors and assigns. Grantee shall have the right to assign its rights, obligations, and liabilities under this Agreement to a homeowner's association, community development district, or other governmental or quasi-governmental entity without the written consent of Grantor, and upon execution of such assignment, Grantee shall be released from all obligations and liabilities under this Agreement.

7. General Provisions. Upon full and complete execution of this Agreement, Grantee shall record this Agreement in the Public Records at Grantee's expense. This Agreement is for the benefit of the parties hereto only, and their successors and assigns as to the Easement Area and Grantee's Property. The Easement shall not constitute dedications to the public or any governmental agency. No waiver of any provision hereof, or of any obligation or default of any other party hereto, shall be implied or deemed effective against the party entitled to the benefit of such provision unless set forth in writing. This Agreement, together with and all exhibits attached hereto which are incorporated herein by this reference, contains the entire agreement between Grantor and Grantee with regard to the Easement, and

shall not be amended or modified except in writing executed by both parties hereto with the same formality as contained herein. If any provision of this Agreement is found to be invalid, illegal, or unenforceable, such invalidity, illegality, or unenforceability shall not affect any other provision hereof, and this Agreement shall be construed as if such invalid, illegal, or unenforceable provision had never been contained herein unless such unenforceable provision results in a frustration of the purpose of this Agreement or the failure of consideration. This Agreement may be executed in two or more separate counterparts, each of which shall be deemed an original, and all of which when taken together, shall constitute one and the same instrument. Any notices to be delivered pursuant to this Agreement shall be delivered to the parties at the addresses set forth in the preamble hereof. Notices shall be effective if delivered by hand, overnight courier service, or U.S. Mail. If a dispute arises under this Agreement, the prevailing party in such action shall be entitled, in addition to all other relief granted or awarded by the court, to a judgment for its reasonable attorneys' and paralegals' fees and costs incurred by reason of such action and all costs of mediation, arbitration, suit at both the trial and appellate levels, and any bankruptcy action. This Agreement shall be governed in accordance with Florida law. Venue for any dispute arising under this Agreement shall lie exclusively in the courts located in the County. This provision shall survive any termination of this Agreement.

[Signatures begin on following page.]

[Grantor's signature page to Perpetual Canal Crossing Easement Agreement]

IN WITNESS WHEREOF, the parties hereto have executed this Agreement as of the day and year set forth above.

WITNESSES:

_____
Print Name: _____
Address: _____

_____
Print Name: _____
Address: _____

GRANTOR:

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT, an Independent Special District of the State of Florida

By: _____
Print Name: _____
Title: _____

STATE OF FLORIDA

COUNTY OF _____

The foregoing instrument was acknowledged before me [_] by physical presence or [_] on line notarization this _____ day of _____, 2025, by _____, as _____ of Lehigh Acres Municipal Services Improvement District, an Independent Special District of the State of Florida, on behalf of such entity, who [ ] is personally known to me or [ ] has produced _____ as identification.

_____
Signature of Notary Public

(Seal)

_____
(Print, Type or Stamp Name)

My Commission Expires:

[Signatures continue on following page.]

[Grantee's signature page to Perpetual Canal Crossing Easement Agreement]

WITNESSES:

Print Name: Ellen Johnson
Address: 19850 Southern Hills Blvd

Print Name: Kyra Rivera
Address: 19850 Southern Hills Blvd

GRANTEE:

RH VENTURE II, LLC,
a Florida limited liability company

By: _____
Graydon E. Miars, Vice President

STATE OF FLORIDA

COUNTY OF Hernando

The foregoing instrument was acknowledged before me [X] by physical presence or [_] online notarization this 31st day of October, 2025, by Graydon E. Miars, as Vice President of RH VENTURE II, LLC, a Florida limited liability company, on behalf of the company. He [X] is personally known to me or [ ] has produced _____ as identification.

ELLEN JOHNSON
State of Florida - Notary Public
Commission # HH 367096
My Commission Expires 05/28/2027

(Seal)

Signature of Notary Public

Ellen Johnson
(Print, Type or Stamp Name)

My Commission Expires: 5/28/27

[Signatures continue on following page.]

DESCRIPTION

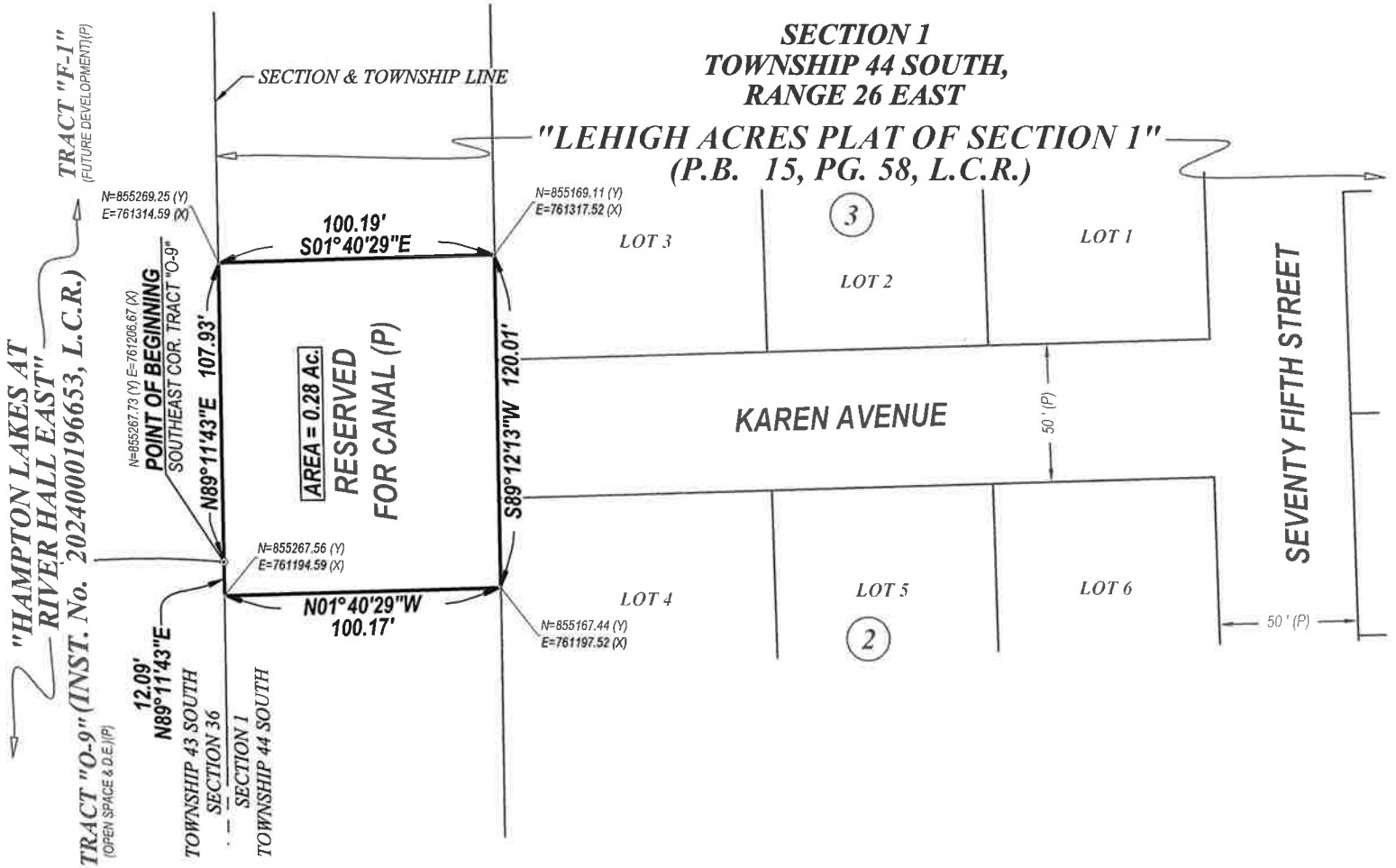
Parcel in
Section 1, Township 44 South, Range 26 East
Lee County, Florida

A tract or parcel of land being portion of an area Reserved for Canal of the record plat of "LEHIGH ACRES PLAT OF SECTION 1" recorded in Plat Book 15, Page 58, Lee County Records, lying in Section 1, Township 44 South, Range 26 East, Lee County, Florida, said tract or parcel of land being more particularly described as follows

BEGINNING at the Southeast corner of TRACT "O-9" of the record plat of "HAMPTON LAKES AT RIVER HALL EAST", as recorded in Instrument No. 2024000196653, Lee County Records run N89°11'43"E along the North line of the Northwest Quarter (NW 1/4) said Section 1 for 107.93 feet; thence run S01°40'29"E for 100.19 feet to an intersection with the South line of said area Reserved for Canal; thence run S89°12'13"W along said South line for 120.01 feet; thence run N01°40'29"W for 100.17 feet to an intersection with said North line; thence run N89°11'43"E along said North line for 12.09 feet to the POINT OF BEGINNING. Containing 0.28 acres, more or less.

Bearings hereinabove mentioned are State Plane for the Florida West Zone (NAD1983)(NSRS 2011) and are based on the North line of the Northwest Quarter (NW 1/4) said Section 1 to bear N89°11'43"E.

Scott A. Wheeler (For The Firm)
Professional Surveyor and Mapper
Florida Certificate No. 5949



NOTES:

1. ALL DISTANCES SHOWN ARE IN FEET AND DECIMALS THEREOF. UNLESS OTHERWISE NOTED DISTANCES ARE ALSO (U.S. SURVEY FEET) GROUND AND CAN BE MULTIPLIED BY 0.9999525 TO OBTAIN GRID DISTANCES.
2. D.E. - DENOTES DRAINAGE EASEMENT.
3. INST. No.- DENOTES INSTRUMENT NUMBER, LEE COUNTY PUBLIC RECORDS.
4. O.R. - DENOTES OFFICIAL RECORD BOOK, LEE COUNTY PUBLIC RECORDS.
5. (P) - DENOTES PLAT.
6. P.B. - DENOTES PLAT BOOK.
7. PG. - DENOTES PAGE.
8. BEARINGS AND COORDINATES SHOWN ARE STATE PLANE FLORIDA WEST ZONE (NAD1983)(NSRS 2011) AND ARE BASED ON NORTH LINE OF THE NORTHWEST QUARTER (NW 1/4) OF SECTION 1 TO BEAR N89°11'43"E.
9. DESCRIPTION IS ATTACHED.

(NSRS2011)
NAD 1983
FLORIDA PLANE
COORDINATE SYSTEM
(WEST ZONE)

0 30 60
SCALE IN FEET

THIS IS NOT A SURVEY

SCOTT A. WHEELER (FOR THE FIRM - LB-6940)
PROFESSIONAL SURVEYOR AND MAPPER
FLORIDA CERTIFICATE NO. 5949

DATE SIGNED:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED OR DIGITAL SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

Barraco and Associates, Inc.
CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING
www.barraco.net
2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FORT MYERS | PANAMA CITY BEACH
FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6940

PREPARED FOR

GREENPOINTE COMMUNITIES, LLC
7807 BAYMEADOWS ROAD E
SUITE 205
JACKSONVILLE, FL 32256

PHONE (904) 562-1358
FAX (904) 996-2461

PROJECT DESCRIPTION

A TRACT OR PARCEL OF LAND LYING IN THE PLAT OF

LEHIGH ACRES PLAT OF SECTION 1

PLAT BOOK 15, PAGE 58, LEE COUNTY RECORDS

A SUBDIVISION LYING IN SECTION 1, TOWNSHIP 44 SOUTH, RANGE 26 EAST, LEE COUNTY, FLORIDA

PROJECT SURVEYOR

SCOTT A. WHEELER LICENSE NUMBER 5949 STATE OF FLORIDA PROFESSIONAL SURVEYOR AND MAPPER

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED OR DIGITAL SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

FILE NAME	24285402.DWG
LAYOUT	2
LOCATION	J:\24285\DWG\SURVEY\INDEX\SKETCH
PLOT DATE	THU 9-18-2025 - 4:35 PM
PLOT BY	PETER OLSEN
DRAWING DATA	
SURVEY DATE	09-18-2025
DRAWN BY	P. OLSEN
CHECKED BY	SAW
SCALE	1"=60'
FIELD BOOK	
PLAN REVISIONS	
STRAP NUMBERS	

SKETCH TO ACCOMPANY DESCRIPTION

PROJECT / FILE NO	SHEET NUMBER
24285 1-44-26	2 OF 2

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: PER #26-16 Comcast Bore along S. Boundary Canal (47-33-5)

Meeting Date: March 16, 2026

Agenda Item # 2G

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve PER #26-16 Comcast Bore along S. Boundary Canal (47-33-5), with any listed stipulations.

What the action accomplishes: Allows Comcast to install a fiber cable along the south side of S. Boundary Canal between Ringneck Canal and the east side of Richmond Ave S. (Approximately 1,800 LF)

Fiscal Impact: Permit Fees: $27,600.00

Staff comments: Approved according to the revised plans submitted February 9, 2026.

Staff review and recommended approval:

[x] **MANAGER** [ ] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

LEHIGH ACRES - MUNICIPAL SERVICES IMPROVEMENT DISTRICT
PERMIT

THIS PERMIT is granted this 16th day of March, 2026, by Lehigh Acres Municipal Services Improvement District (hereinafter referred to as "DISTRICT"), 601 East County Lane, Lehigh Acres, Florida 33936, to Comcast Cable Communications, 5205 Fruitville Road, Sarasota, FL 34232, hereinafter referred to as the "Permittee") as a non-exclusive permit for a proposed bore along the south side of S. Boundary Canal (47-33-5), between Ringneck Canal (57-4-2) and the east side of Richmond Ave S., in Lehigh Acres, FL.

W I T N E S S E T H:

1. In the event the DISTRICT is not the fee title owner of the real property (the "Property") to which this Permit is applicable, the Permittee agrees, prior to commencement, to obtain all necessary consents from the fee title owners of the Property.

2. This Permit does not constitute a waiver of the necessity by the Permittee to obtain such other necessary and appropriate permits required by other governmental bodies or agencies; therefore the Permittee, prior to commencement, is required to obtain any and all other applicable federal, state, and local permits required in connection with Permittee's use of the Property and agrees that at all times it will comply with the requirements of all federal, state and local laws, ordinances, rules and regulations applicable or pertaining to the use of the Property by the Permittee.

3. Permittee understands and agrees that the use of the Property pursuant to this Permit is subordinate to the rights and interests of the DISTRICT and to the extent applicable that of the fee title owner of the Property. Further, Permittee does hereby stipulate that the Permittee is not relying upon any representations whatsoever by the DISTRICT regarding the DISTRICT'S right, title, interest, or ownership as to the Property for which this Permit is applicable.

4. The Permittee, by accepting this permit, specifically agrees to allow authorized District Personnel, upon presentation of credentials or other documents as may be required by law, access to the premises, at reasonable times, where the permitted activity is located or conducted for the purpose of:
    a. Having access to and copying any records that must be kept under the conditions of the permit:
    b. Reviewing the facility, equipment, practices, or operations regulated or required under this permit; and
    c. Sampling or monitoring any substances or parameters at any location reasonably necessary to assure compliance with this permit or District rules.

5. The DISTRICT specifically reserves the right to maintain its facilities located over, under or upon the Property; to make improvements; add additional facilities; maintain, construct or alter its roads; maintain any facilities, devices, or improvements on the Property which aid in or are necessary to the DISTRICT'S operation; and the right to enter upon the Property at all times for such purposes. Permittee understands that in the exercise of such rights and interest, the DISTRICT may require Permittee, at no cost to DISTRICT, to relocate, alter, or remove the Permittee's facilities and equipment or other improvements made by Permittee pursuant to this Permit which interfere with or prevent the DISTRICT, in the DISTRICT'S opinion, from properly and faithfully constructing, improving and maintaining its facilities. If the Permittee should fail to relocate, alter, or remove the Permittee's facilities, equipment or other improvements made by the Permittee pursuant to this Permit within a reasonable time period following receipt of notice from the DISTRICT requesting same, then in that event the DISTRICT shall have the right to enter upon the lands and make such relocation, alterations or removal of the Permittee's facilities, equipment/or other improvements at no cost to DISTRICT.

6. Permittee agrees that it will not use the Property in any manner which interferes with the DISTRICT'S

use of the Property or causes a hazardous condition to exist.

7. The DISTRICT assumes no responsibility for the ownership, operation and/or maintenance of the Permittee's facilities as permitted herein.

8. The Permittee agrees to backfill any excavation it makes within the Property and to repair or replace (i) any below-grade authorized facilities and features located therein, including but not limited to: water, sewer, stormwater or irrigation facilities, structures or appurtenances and (ii) as to at or above-grade facilities and features, this requirement shall only be applicable to grasses and sod, sidewalks, and roadway improvements (the term roadway improvements shall include but not be limited to curbing, roadway base, sub grade and asphaltic or concrete surfaces) which are removed or damaged as a direct result of said excavation. Such repair or replacement shall substantially restore these features to their condition as it existed immediately prior to the event precipitating the excavation.

9. All below-grade Permittee installations, including water and sewer lines, whether sub aqueous or not, together with appurtenant facilities, shall be located underground and have a minimum ground cover of thirty inches (30") below original design profile grade, including District waterways and roadways.

10. All Permittee activity related to sub aqueous utility, water and/or sewer lines, including appurtenant facilities, shall be conducted in such fashion that the primary functional purpose of the canal or lake body facility, specifically flood control, within which the activity is taking place is not adversely affected. More specifically, sub aqueous construction shall conform to the criteria given in the Lehigh Acres Municipal Services Improvement District (Lehigh Acres – MSID) Administrative Policy and Procedures.

11. Permittee shall, at no expense to DISTRICT and within a reasonable time following notice, adjust the positions and elevations of its facilities as may be required in connection with future improvements to, or construction of, works of the DISTRICT.

12. Permittee does hereby indemnify and hold harmless the DISTRICT, its Board of Commissioners, officers, personnel, and it's Engineers against any claims, losses, damages (including consequential), expenses, or legal fees that might arise out or result from the implementation of the proposed project by the Permittee or that may be caused by a failure of DISTRICT facilities or property.

13. The Permittee shall be obligated at the time of submission of its Permit application, to provide a detail of any other existing improvements located over, under, upon or through the Property and the Permittee's accompanying plans and specifications shall clearly indicate how the Permittee will comply with required clearances between the requested permitted activity and existing improvements located over, under and upon the Property, including compliance with other permitted users duly adopted and published standards.

14. Permittee shall reimburse the DISTRICT for its legal, engineering and other expenses incurred as a result of the implementation of the proposed project.

15. If Permittee should violate any of the terms or conditions of the Permit and shall not correct or remedy same within ten (10) days of receiving written notice of said violation from the DISTRICT, then in that event, the DISTRICT may, at its option, revoke, cancel and terminate this Permit.

16. This Permit may not be assigned by the Permittee without the prior written approval of the DISTRICT.

17. Unless otherwise adjusted, modified or terminated as set forth hereinabove, including the Conditions attached hereto, this Permit shall continue in full force and effect so long as Permittee complies with the

terms of this Permit.

18. In any litigation, including breach, enforcement or interpretation arising out of this Permit, the prevailing party to this Permit shall be entitled to recover reasonable attorney's fees, costs and expenses related to said action from the non-prevailing party.

19. That attached hereto are Supplemental Conditions to this Permit which are incorporated herein and made a part hereof.

LEHIGH ACRES – MUNICIPAL SERVICES IMPROVEMENT DISTRICT

BY: _____

NAME TYPED: David E. Lindsay

TITLE: District Manager

SUPPLEMENTAL CONDITIONS TO PERMIT NO. 26-16

A. GENERAL CONDITIONS

1. Notification shall be given by the Permittee to the District Engineer, AIM Engineering, at 239-332-4569 forty-eight (48) hours prior to commencement of the permitted activity. The District Engineer shall establish points of construction, if applicable, which will require inspection by the District Engineer prior to construction of the permitted activity. When the Permittee considers the permitted activity complete, the Permittee shall notify the District Engineer and schedule an on-site final inspection which shall be held in the presence of a representative of the Permittee and the District Engineer.

2. The permitted activity shall be implemented, constructed and/or installed in full accordance with the approved plans and specifications within six (6) full calendar years from the date of permit approval. Deviations from the approved plans or specifications shall be coordinated with and approved by the District Engineer prior to implementation of any such deviation.

3. When working in a District road right-of-way, not more than one-half (½) of the road or street shall be closed and traffic shall be controlled so as to provide for the safety and minimum hindrance to the public. All traffic control operations shall conform to the most current issue of the Florida Department of Transportation publication "Manual on Traffic Controls and safe Practices for Street and Highway Construction, Maintenance and Utility Operations."

4. The District Engineer shall be the final authority as to the quality and quantity of the material and works required to satisfy the terms and conditions of this Permit as they relate to District improvements and facilities.

5. Upon completion of the permitted activity and after its final review and acceptance by the District Engineer THE PERMITTEE SHALL DELIVER TO THE DISTRICT OFFICE ONE COMPLETE SET OF THE FINAL "RECORD DRAWINGS." FAILURE TO PROVIDE THE FINAL RECORD DRAWINGS MAY RESULT IN THE REVOCATION, CANCELLATION AND TERMINATION OF THIS PERMIT.

6. The above Conditions shall be continuing obligations of the Permittee and shall be complied with in the event of any subsequent maintenance, repair, replacement or modification of the permitted improvement.

Stipulations:

1). Permit #26-16 is approved according to the revised plans submitted February 9th, 2026.

2). The proposed bore must maintain a minimum clearance of 36" below the bottom elevation of the storm sewer pipes connecting into S. Boundary Canal for the entire length of the canal right of way.

3). Permanent utility markers, (at least 4' high) with the utility companies contact information clearly labeled on it, must be installed at each canal right of way line over top the proposed line and near the location of each intersecting storm pipe.

4). All underground vaults, pull boxes, cable cabinets, etc. must be located outside of the canal right of way and not interfere with the District's canal maintenance activities unless shown and approved on the submitted permit plans within the first 5' of the canal right of way.

5). All canal R/W that is disturbed must be restored with sod and maintained until well established (no seeding will be allowed).

6). Boring contractor is not allowed to pump water out of the LAMSID canals for their boring operations unless they apply for and receive a water use permit from LAMSID.

7). The Permittee will be responsible for any repairs, alterations, or relocation of the utility if required for repairs or maintenance of the canal in the future.

8). Provide LAMSID with a copy of your survey record drawings, showing where your utilities are located within the canal R/W and at what elevation they are located, within 30 days following completion of the improvements.

9). Lee County DOT must review and approve the utility work that is outside of the canal right of way along Richmond Ave. S.

PART I

DATE SUBMITTED: 02 - 09 - 26

PERMIT #: 26 - 16

APPLICATION FOR A PERMIT

*Note: The review of permit applications will be conducted per the Lehigh Acres Municipal Services Improvement District's Administrative Policy and Procedures

AGENT NAME: Joanie George **PHONE #:** (904)769-9064

PROJECT NAME: JB0001857949 - L084B - FLLAD00H0AC add FLLAD00G0AC add FLLAD00J0AC

TOTAL ACREAGE: 0.041

Property Address (STRAP associated with this address will be used as the primary STRAP for this project): 101 RICHMOND AVE S					
Mailing Address:	NA				
City:	LEHIGH ACRES	**State:**	FL	**Zip:**	33936

STRAP NUMBERS (*attach additional sheet if necessary*):

0	4	-	4	5	-	2	7	-	L	1	-	0	2	0	0	6	-	0	0	1	0
		-			-			-			-						-				
		-			-			-			-						-				
		-			-			-			-						-				
		-			-			-			-						-				

Brief description of proposed project:

Application for: Comcast proposes to bore 1786' alongside South Boundary Canal Starting north of 101 RICHMOND AVE S going west crossing Richmond AVE S continuing west and place 4" HDPE conduit with 24ct FOC

Is the project proposed to be developed in phases? ☐ Yes ☒ No

Estimated # of Phases:	1	
***Permit Fee:**	$ 27,600.00	**(to be verified by staff)**

* See Administrative Policy and Procedures Sections 4.15 – 4.17 for current Permit Fee Amount.

PART II

OWNER/APPLICANT/ DEVELOPER INFORMATION

A.

Name of Applicant	Comcast Cable Communications-Ted Wheless						
Mailing Address:	Street:	5205 Fruitville Rd					
City:	Sarasota	State:	FL			Zip:	34232
Phone No.	Area Code:	941	Number:	400-3386		Ext.	
Fax No.	Area Code:		Number:	-			
E-mail Address:	ted_wheless@comcast.com						

B. Relationship of applicant to property*:

☐ Owner* ☐ Trustee* ☐ Option holder* ☐ Lessee*

☐ Contract Purchaser* ☒ Other (indicate)* UTILITY

C.

Name of Developer:							
Mailing Address:	Street:						
City:		State:				Zip:	
Phone No.	Area Code:	( )	Number:	-		Ext.	
Fax No.	Area Code:	( )	Number:	-			
E-mail Address:							

D.

Name of Authorized Agent(s):	Joanie George						
Mailing Address:	Street:	13277 NE 235th CT					
City:	Raiford	State:	FL			Zip:	32083
Phone No.	Area Code:	(904)	Number:	769-9064		Ext.	
Fax No.	Area Code:	( )	Number:	-			
E-mail Address:	jgeorge@truenetcommunications.com						

E.

Name of Property Owner:							
Mailing Address:	Street:						
City:		State:				Zip:	
Phone No.	Area Code:	( )	Number:	-		Ext.	
Fax No.	Area Code:	( )	Number:	-			
E-mail Address:							

*A disclosure of the name of all persons or entities having an ownership interest in the property is required (attached).

PART III

DISCLOSURE OF OWNERSHIP INTEREST FORM FORM:

PRIMARY STRAP NUMBER: ☐☐ - ☐☐ - ☐☐ - ☐☐ - ☐☐☐☐☐ - ☐☐☐☐

1. If the property is owned in the fee simple by an **INDIVIDUAL**, tenancy by the entirety, tenancy in common, or joint tenancy, list all parties with an ownership interest as well as the percentage of such interest.

Name and Address	Percentage of Ownership

2. If the property is owned by a **CORPORATION**, list the officers and stockholders and the percentage of stock owned by each.

Name, Address, Office	Percentage of Stock

3. If the property is in the name of a **TRUSTEE**, list the beneficiaries of the trust with percentages of interest.

Name and Address	Percentage of Interest

4. If the property is in the name of a **GENERAL PARTNERSHIP OR LIMITED PARTNERSHIP**, list the names of general and limited partners.

Name and Address	Percentage of Interest

5. If there is a **CONTACT FOR PURCHASE**, whether contingent on this application Or not, and whether a Corporation, Trustee, or Partnership, list the names of the contract purchasers below, including the officers, stockholders, beneficiaries, or partners.

Name and Address	Percentage of Interest

Date of Contract (mm/dd/yy): / /

6. If any contingency clause or contract terms involve additional parties, list all individuals or officers, if a corporation, partnership, or trust.

Name and Address	Percentage of Interest

For any changes of ownership of changes in contracts for purchase subsequent to the date of the application, but prior to the date of final certificate for compliance, a supplemental disclosure of interest must be filed.

The above is a full disclosure of all parties of interest in this application, to the best of my knowledge and belief.

Joanne George
SIGNATURE OF APPLICANT

Joanne George
PRINTED OR TYPED NAME OF APPLICANT

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this 27 day of Jan (m), 2026(y), by Joanne George who is personally know to me or has produced _____ as identification.

Notary Public State of Florida
Ashley Woods
My Commission HH 337625
Expires 12/1/2026

Ashley Woods
SIGNATURE OF NOTARY PUBLIC

Ashley Woods
PRINTED NAME OF NOTARY PUBLIC

PART IV

EXPLANATION

All Permit Application sections must be completed and submitted.

Permit Application fees and submittal items (see Part VI) must be submitted with the application.

Any and all work involving the lands and/or facilities of the District shall not commence without a Board approved permit.

All set policies, procedures, technical requirements, and fee schedules can be found and reviewed in the 'Administrative Policy and Procedures Guide'. The Guide is available at the District Office located at 601 East County Lane, Lehigh Acres, FL or online at la-msid.com/.

PART V

PROFESSIONAL CONSULTANT FORM

ATTORNEY

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

ENGINEER

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

OTHER REPRESENTATIVE/PROFESSIONAL

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

PART VI

SUBMITTALS

NO. OF COPIES		SUBMITTAL DOCUMENT
SUB'D	REQ'D	
		ITEMS REQUIRED FOR INITIAL SUBMITTAL
2	2	**Construction/Development Plans** signed and sealed by a Licensed Florida Professional Engineer including: 1. Location map showing the location of the proposed project in relation to adjacent streets and LA-MSID facilities. 2. Details of proposed works that fully communicate the work and/or use desired (i.e. Existing Conditions Survey, Proposed Site Plan, Paving Grading & Drainage Plan, Cross Sections, Element Details, and Erosion Control Plan sheets). 3. Sufficient survey data to determine elevations, property and ROW lines, limits of existing and proposed conditions, and notation of utilized vertical datum (NAVD/NGVD).
N/A	2	**Stormwater Drainage Calculations** signed and sealed by a Licensed Florida Professional Engineer including, but not limited to, computer stormwater routing modeling data (ICPR4, Cascade, etc.) to verify compliance with the Design Criteria (input summary data below): **Water Quality Volume (Required):** _____(ACRE-FT) **Water Quality Volume (Provided):** _____(ACRE-FT) **Control Elevation:** _____(FT-NGVD) **Minimum Road/Parking Elevation:** _____(FT-NGVD) *(above the 10-year, 1-day design storm peak stage elevation)* **Minimum Perimeter Berm Elevation:** _____(FT-NGVD) *(above the 25-year, 3-day design storm peak stage elevation)* **Minimum Finished Floor Elevation:** _____(FT-NGVD) *(above the 100-year, 3-day design storm peak stage elevation and previously experienced high-water levels external to project)*
2	2	**Flash Drive / CD** containing plans in both AutoCAD (.dwg file drawn in "State Plane Coordinates") and PDF formats.
2	2	**Permit Application** (PARTS I & II)
2	2	**Disclosure of Owner Form (signed and notarized)** (PART III)
2	2	**Professional Consultant Form & Submittals** (PARTS V & VI)
N/A	2	**Copy of Approved FDEP Notice of Intent (NOI)** for all sites one (1) acre and greater in size.
N/A	2	**Copy of approved SWFMD permit**, for all sites required to get a permit, **or FDEP "10/2 Self-Certification"** approval.
1	1	**Check for appropriate Permit Fees** made out to LA-MSID.

TrueNet Communications Corp

060830

~~LEHIGH ACRES MUNICIPAL SERVICE - IMPROVEMENT DISTRICT~~

DATE	INVOICE NO.	DESCRIPTION	INVOICE AMOUNT	DEDUCTION	BALANCE
1/13/2026	01132026	JB0001857949 - 160 S HOMESTEAD RD		27,600.00	27,600.00

CHECK DATE		CHECK NUMBER		TOTALS			

PLEASE DETACH THIS PORTION AND RETAIN FOR YOUR RECORDS 9321

DOCUMENT IS PRINTED ON CHEMICALLY REACTIVE PAPER - THE BACK OF THIS DOCUMENT INCLUDES A TAMPER EVIDENT CHEMICAL WASH WARNING BOX

060830

TrueNet Communications Corp.
90 FORT WADE RD.
SUITE 230
PONTE VEDRA, FL 32081
(904) 777-9052
APINV@TrueNetCommunications.com

Bank of America
11-35/1210 GL

Pay: **********twenty-seven thousand six hundred and 00/100

DATE	CHECK NO.	AMOUNT
January 15, 2026	60830	$*****27,600.00

PAY TO THE ORDER OF

LEHIGH ACRES MUNICIPAL SERVICE - IMPROVEMENT DISTRICT
601 EAST COUNTY LANE
LEHIGH ACRES FL 33936

~~VOID AFTER 90 DAYS~~

AUTHORIZED SIGNATURE

HEAT SENSITIVE COLOR FADES WITH HEAT

ORIGINAL DOCUMENT HAS RED KEYHOLE ICON THAT DISAPPEARS WITH HEAT

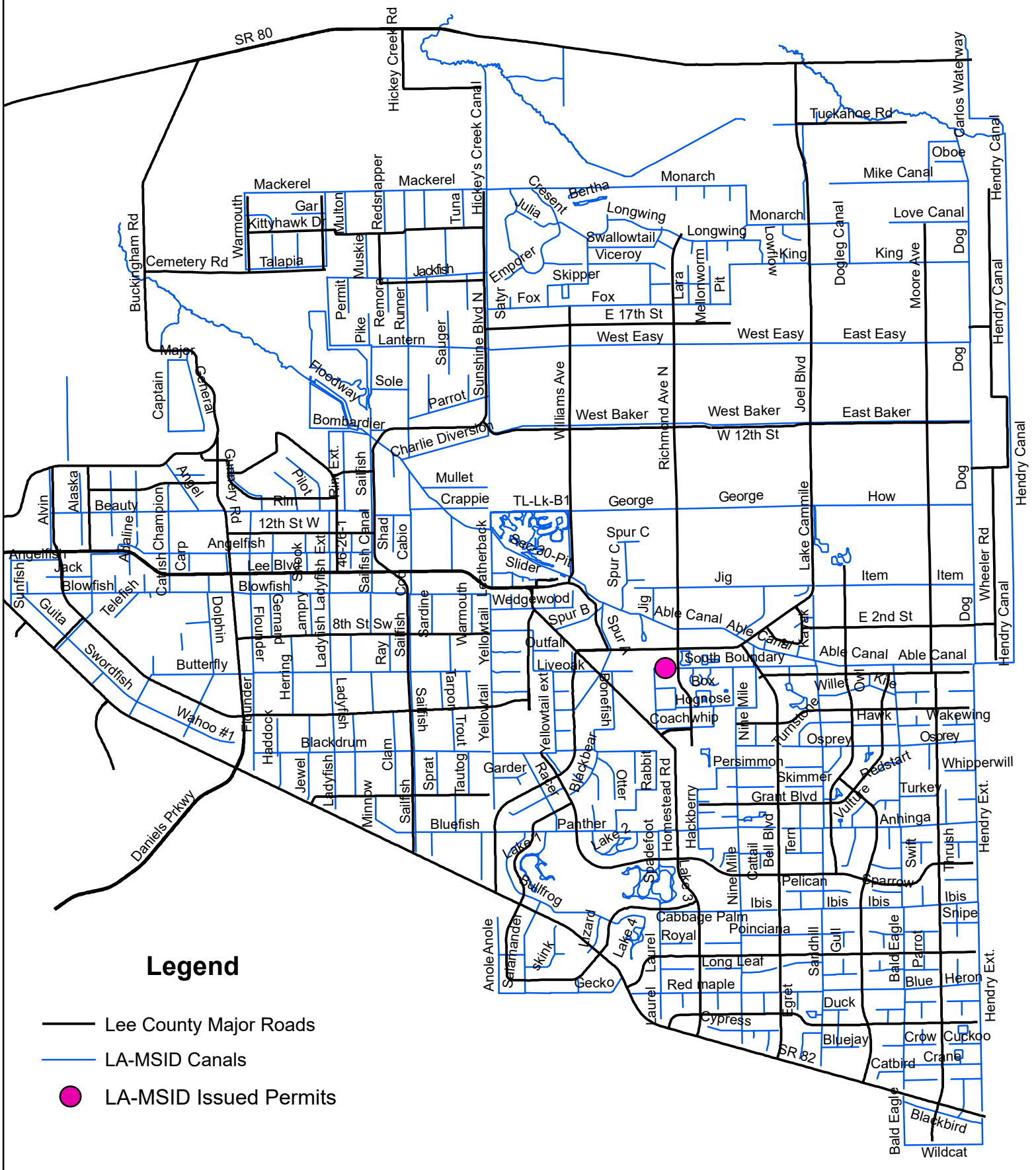
⑈060830⑈ ⑆121000358⑆ 001233592930⑈

RECEIVED
FEB 09 2026
LEHIGH ACRES
MUNICIPAL SERVICES
IMPROVEMENT DISTRICT

COPY

Permit #26-16

Permit # 2026-16

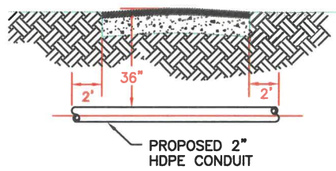


Legend

- Lee County Major Roads
- LA-MSID Canals
- LA-MSID Issued Permits

160 S HOMESTEAD RD
LA-MSID PERMIT
NOTES & TYPICALS

Length in R/W = 1790' Total
Page 2 = 604'
Page 3 = 767'
Page 4 = 417
+3.5' at each end
= 1783 + 7
= 1790 LF



TYPICAL DRIVEWAY DIRECTIONAL BORE
N.T.S.

Permit Fee:
$750.00 Base Fee
+ $.15 x 1,790 LF
= $750 + $268.50.00
= $27,600.00

AERIAL & U/G NOTES

ALL PROPOSED CONSTRUCTION WILL BE PER FDOT SPECIFICATIONS AS WELL AS ALL NESC SAFETY CODES.

ALL PROPOSED CATV WILL BE WITHIN THE FDOT RIGHT-OF-WAY.

ALL AERIAL CANAL RIGHT-OF-WAY CROSSINGS MUST BE A MINIMUM HEIGHT OF 40' ABOVE THE TOP OF THE CANAL BANK.

ALL PROPOSED CANAL RIGHT-OF-WAY BORES MUST BE A MINIMUM HEIGHT OF 10' (120") BELOW THE BOTTOM OF THE CANAL OR PIPE INVERT.

ALL PROPOSED UNDERGROUND CATV WILL BE BURIED A MINIMUM OF 66" DEEP, EXCEPT BORES 48" DEEP.

ALL PROPOSED UNDERGROUND CATV ROAD BORES WILL USE FDOT APPROVED DIRECTIONAL BORE MACHINE, AS WELL AS FDOT APPROVED CONDUIT.

ALL PROPOSED CATV ROAD BORES WILL EXTEND A MINIMUM OF 8' BEYOND THE EDGE OF PAVEMENT.

ALL PROPOSED CATV DRIVEWAY BORES WILL EXTEND A MINIMUM OF 4' BEYOND THE EDGE OF PAVEMENT.

ALL PROPOSED CATV WILL BE .625" OR .875" DIAMETER, INSULATED COAXIAL CABLE OR 1.0" DIAMETER, INSULATED FIBER OPTIC CABLE, AND WILL BE LASHED TO .250" STEEL STRAND CABLE USING .125" STD. GALV. LASHING WIRE.

ALL PROPOSED AERIAL CATV CROSSINGS (BOTH ROADWAYS AND DRIVEWAYS) WILL HAVE MINIMUM MID-SPAN HEIGHT OF 18'-0".

ALL PARALLEL PROPOSED AERIAL CATV WILL HAVE MINIMUM MID-SPAN HEIGHT OF 16'-6" AS PER NESC SPECIFICATIONS.

LOCATES WILL BE REQUIRED IN ALL PROPOSED UNDERGROUND AREAS AT LEAST 48 HRS. PRIOR TO CONSTRUCTION.

NOTIFICATIONS TO ALL UTILITIES INVOLVED WILL BE MADE PRIOR TO CONSTRUCTION.

GENERAL NOTES

MAINTENANCE OF TRAFFIC TO BE SUPERVISED BY A CERTIFIED PERSON.

TEST RESULTS ARE REQUIRED UPON REQUEST.

THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT A MINIMUM OF TWO BUSINESS DAYS PRIOR TO ANY LANE CLOSURES OR BEGINNING ANY CONSTRUCTION WITHIN THE FDOT RIGHT-OF-WAY.

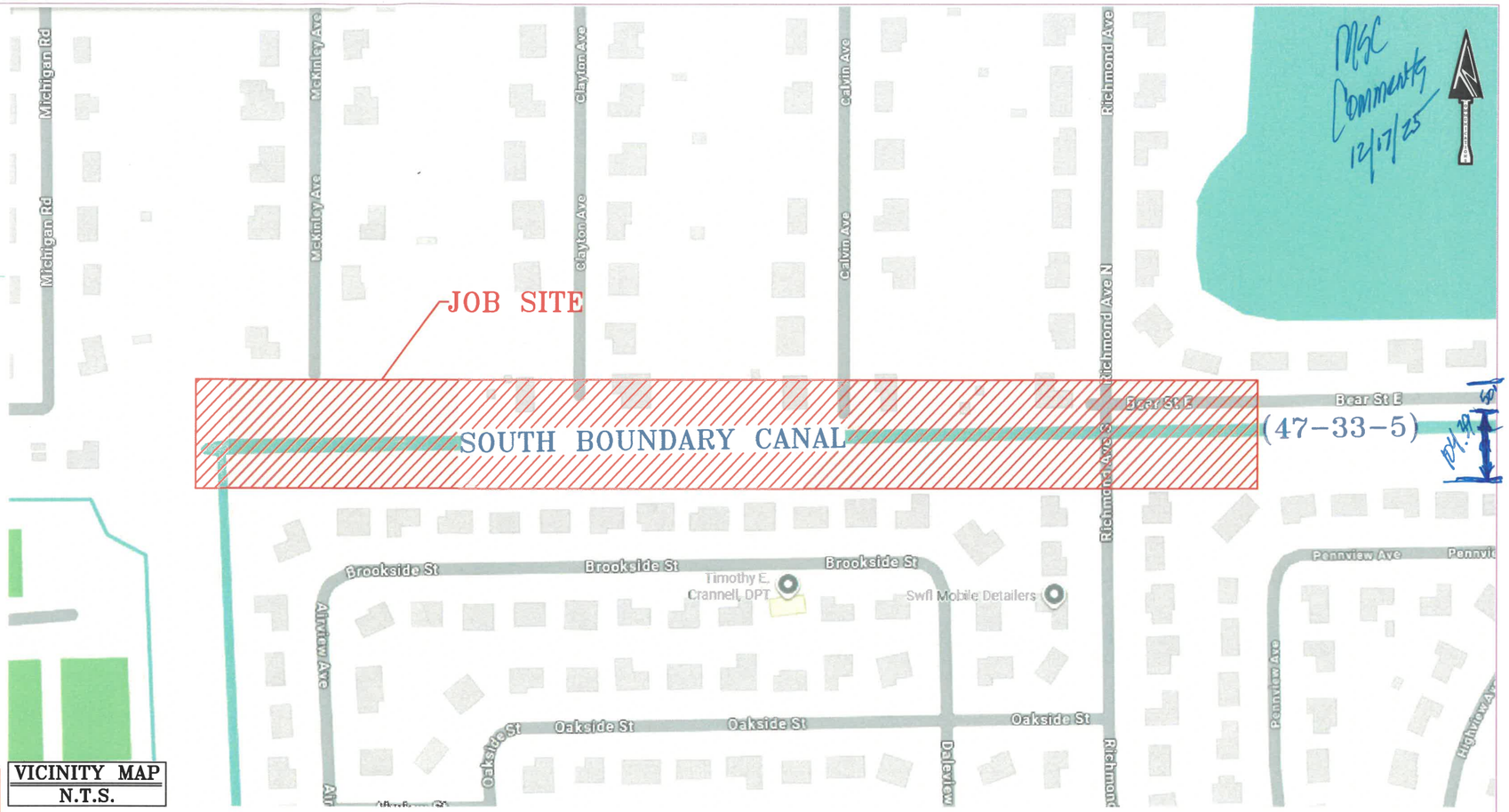
ALL WORK PERFORMED WITHIN THE FDOT RIGHT-OF-WAY SHALL BE IN ACCORDANCE WITH FDOT DESIGN STANDARDS, SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION AND THE UTILITY ACCOMODATION MANUAL, LATEST EDITIONS.

ENGINEER'S CERTIFICATION TOGETHER WITH AS-BUILTS, IF REQUIRED TO BE SUBMITTED WITHIN 30 DAYS OF COMPLETION.

ALL CONCRETE TO BE REMOVED SHALL BE SAW CUT AT THE NEAREST GOOD JOINT. GUTTER SHALL BE SAW CUT BETWEEN ASPHALT AND GUTTER BEFORE REMOVAL.

IT WILL BE THE RESPONSIBILITY OF THE PERMITTEE TO REPAIR ANY DAMAGE TO FDOT FACILITIES CAUSED BY THE CONSTRUCTION OF THE PROJECT.

ALL CONCRETE SHALL BE 3,000 PSI. APPROVED FDOT MIX DESIGN.



VICINITY MAP
N.T.S.

LEGEND

	BORE
	SWALE/DITCH
	GUARD RAIL
GAS	GAS
PWR	POWER
	U/G CATV
STLT	STREET LIGHT
SD	STORM DRAIN
SWR	SEWER
TELE	TELEPHONE
	TRAFFIC SIGNAL
	WATER
	MISC U/G UTILITY
	PROPOSED U/G CATV
	PROPOSED AERIAL FIBER
	PROPOSED AERIAL CATV
℄	CENTERLINE
E/P	EDGE OF PAVEMENT
B/C	BACK OF CURB
R/W	RIGHT OF WAY

	POWER POLE
	POWER TRANSFORMER POLE
	JOINT USE TRANSFORMER POLE
	PHONE POLE
	CATV POLE
S	STEEL POLE
C	CONCRETE POLE
	JOINT USE POLE
R	RISER
	ANCHOR
	OVERHEAD GUY
V	CATV FIBER VAULT
TV	TRAFFIC SIGNAL VAULT
	TRAFFIC SIGNAL POLE
	TRAFFIC SIGNAL BOX
	CROSS SECTION REFERENCE
XX'	FTG BTWN REFERENCE POINTS
	REFERNCE POINT
	STORM DRAIN

PROJECT TOTALS	
PROP. BURIED CATV:	1785'
PROP. OH CATV:	0'
TOTAL PROP. CATV:	1785' (0.34MI.)

MOT NOTES

TRAFFIC CONTROL SHALL COMPLY WITH FEDERAL MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) AND F.D.O.T. DESIGN STANDARD #NO. 600.

160 S HOMESTEAD RD - LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 1 OF 7
	DATE: 12/12/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

BINDING SPACE

160 S HOMESTEAD RD
LA-MSID PERMIT
PLAN VIEW

0 25' 50'

BINDING SPACE

Proposed Permanent Utility Marker (At Least 4' High) w/ Utility Company Contact Information clearly Marked on it.

CANAL R/W

NORTH R/W OF CANAL

SOUTH BOUNDARY CANAL
47-33-5
33-44-27-00-00002.0010
EAST 880 FEET OF WEST
2570 FEET OF SOUTH 104.39
FEET OF SEC 33,
TWP 44S, RNG 27E

CANAL

EOW ELEV = 18.7'

TOE

TOB

INV = 18.5'

104'

61'

43'

PROJECT START
PROPOSED U/G CATV
FACILITIES TIE INTO
EXISTING CATV FACILITIES
1728' WEST OF C/L OF
RICHMOND RD S

DIRECTIONAL BORE
PROPOSED U/G CATV
FACILITIES @ 84"
MINIMUM DEPTH

SOUTH LINE OF SEC 33
TWP 44S, RNG 27E

245'

180'

179'

28

TC

GRAIN LINK FENCE

LOT 20, BLK 1
P.B. 18, PG 156
100 AIRVIEW AVE
(OCCUPIED)

LOT 19, BLK 1
P.B. 18, PG 156
129 BROOKSIDE ST
(OCCUPIED)

LOT 18, BLK 1
P.B. 18, PG 156
131 BROOKSIDE ST
(OCCUPIED)

LOT 17, BLK 1
P.B. 18, PG 156
133 BROOKSIDE ST
(OCCUPIED)

LOT 16, BLK 1
P.B. 18, PG 156
135 BROOKSIDE ST
(OCCUPIED)

LOT 15, BLK 1
P.B. 18, PG 156
137 BROOKSIDE ST
(OCCUPIED)

LOT 14,
P.B. 18,
139 BROO
(OCCU

PROPOSED PERMANENT
UTILITY MARKER (AT LEAST 4'
HIGH) W/ UTILITY COMPANY
CONTACT INFORMATION CLEARLY
MARKED ON IT

BROOKSIDE ST

MATCH TO SHEET 3

160 S HOMESTEAD RD – LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

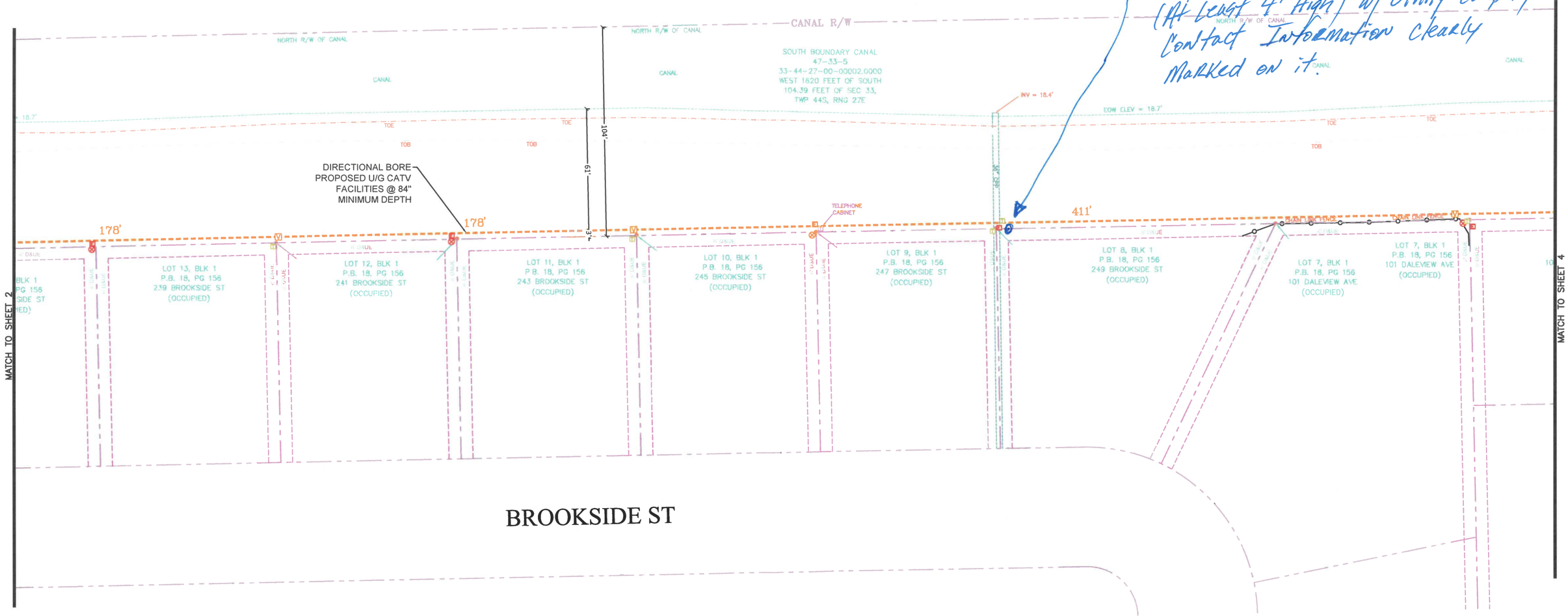
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 2 OF 7
	DATE: 12/12/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
PLAN VIEW

0 25' 50'

BINDING SPACE



160 S HOMESTEAD RD – LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

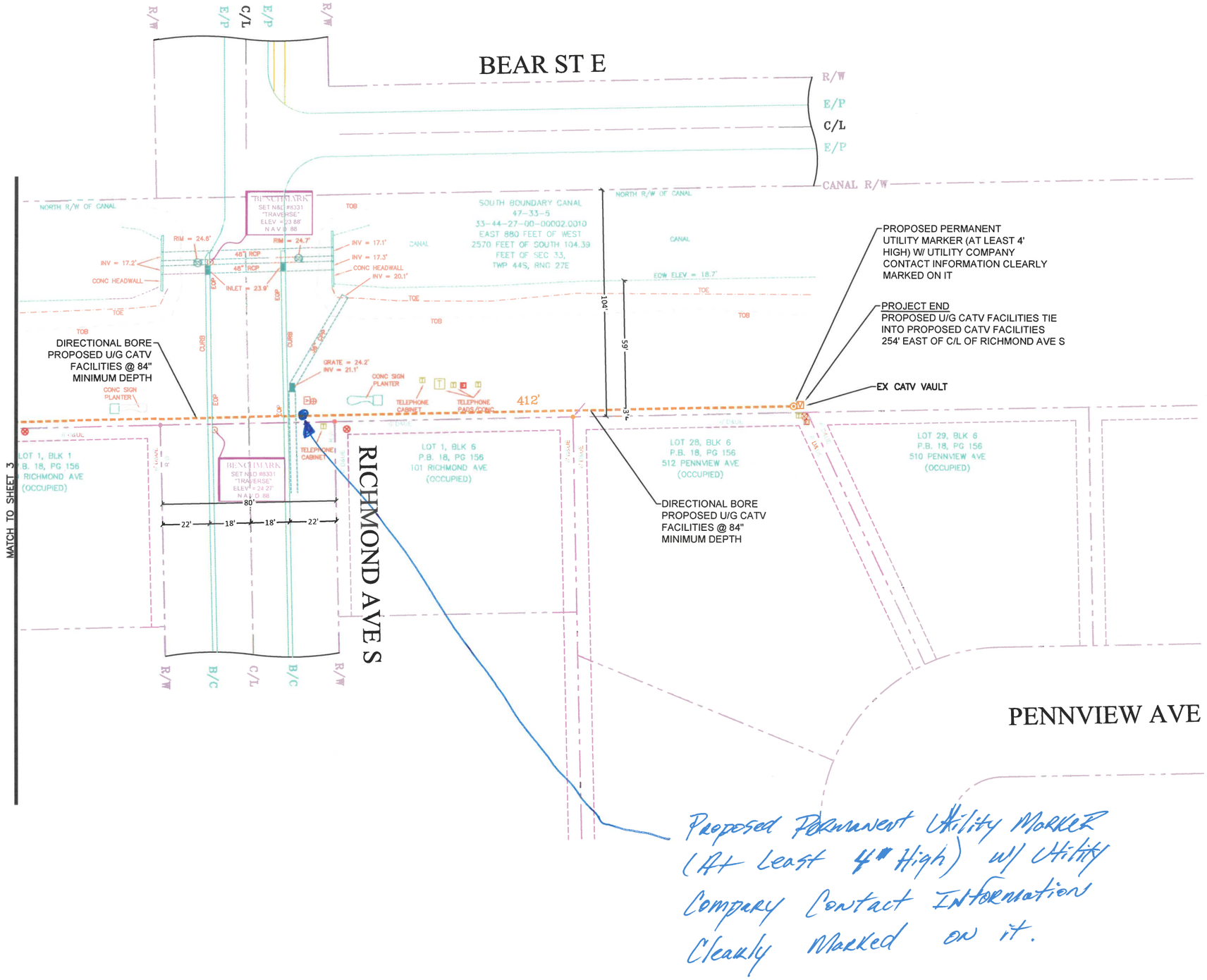
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 3 OF 7
	DATE: 12/12/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
PLAN VIEW

0 25' 50'

BINDING SPACE



Proposed Permanent Utility Marker (At Least 4" High) w/ Utility Company Contact Information Clearly Marked on it.

160 S HOMESTEAD RD – LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 4 OF 7
	DATE: 12/12/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
PROFILE VIEW

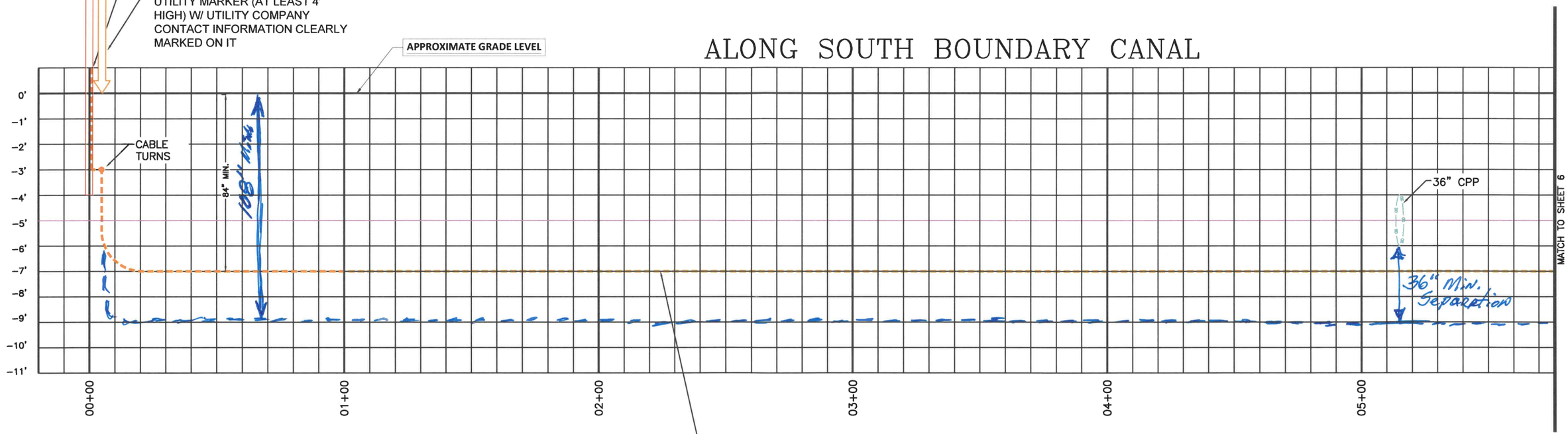
BINDING SPACE

PROJECT START
PROPOSED U/G CATV FACILITIES TIE INTO EXISTING CATV FACILITIES 1728' WEST OF C/L OF RICHMOND RD S

PROPOSED PERMANENT UTILITY MARKER (AT LEAST 4' HIGH) W/ UTILITY COMPANY CONTACT INFORMATION CLEARLY MARKED ON IT

APPROXIMATE GRADE LEVEL

ALONG SOUTH BOUNDARY CANAL



HORIZONTAL SCALE 1"=40'
VERTICAL SCALE 1"=4'

CONSTRUCTION NOTE:
ALL CANAL RIGHT OF WAY THAT IS DISTURBED MUST BE RESTORED WITH SOD AND MAINTAINED UNTIL WELL ESTABLISHED. NO SEEDING WILL BE ALLOWED.

DIRECTIONAL BORE PROPOSED U/G CATV FACILITIES @ ~~84"~~ 108" MINIMUM DEPTH

160 S HOMESTEAD RD – LAMSID

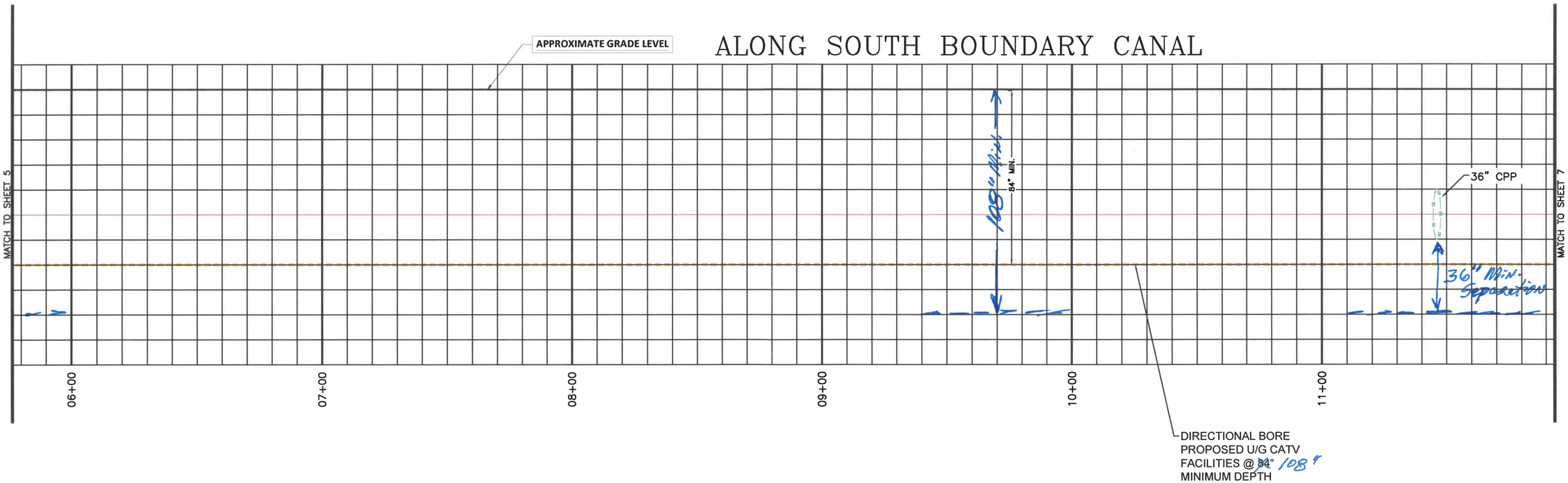
COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 5 OF 7
	DATE: 12/12/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
PROFILE VIEW

BINDING SPACE



160 S HOMESTEAD RD – LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

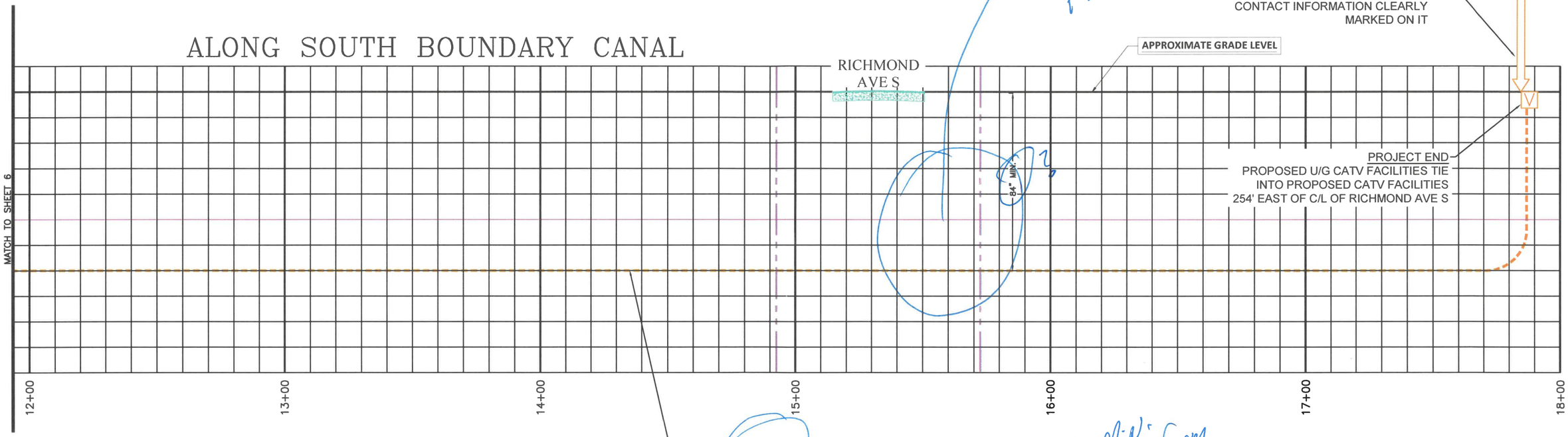
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 6 OF 7
	DATE: 12/12/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
PROFILE VIEW

BINDING SPACE

ALONG SOUTH BOUNDARY CANAL



Need to show Existing Storm Sewer Pipe

May need to be deeper depending on where the bottom of the existing Storm Sewer is located.

Need 36" Min. Separation from the existing Pipe

Survey Needs to show size and depth of the pipe along Richmond Ave. S.

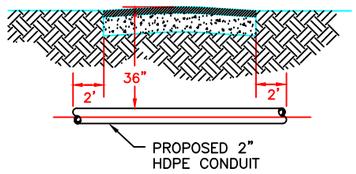
160 S HOMESTEAD RD – LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 7 OF 7
	DATE: 12/12/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
NOTES & TYPICALS



TYPICAL DRIVEWAY
DIRECTIONAL BORE
N.T.S.

AERIAL & U/G NOTES

ALL PROPOSED CONSTRUCTION WILL BE PER FDOT SPECIFICATIONS AS WELL AS ALL NESC SAFETY CODES.

ALL PROPOSED CATV WILL BE WITHIN THE FDOT RIGHT-OF-WAY.

ALL AERIAL CANAL RIGHT-OF-WAY CROSSINGS MUST BE A MINIMUM HEIGHT OF 40' ABOVE THE TOP OF THE CANAL BANK.

ALL PROPOSED CANAL RIGHT-OF-WAY BORES MUST BE A MINIMUM HEIGHT OF 10' (120") BELOW THE BOTTOM OF THE CANAL OR PIPE INVERT.

ALL PROPOSED UNDERGROUND CATV WILL BE BURIED A MINIMUM OF 66" DEEP, EXCEPT BORES 48" DEEP.

ALL PROPOSED UNDERGROUND CATV ROAD BORES WILL USE FDOT APPROVED DIRECTIONAL BORE MACHINE, AS WELL AS FDOT APPROVED CONDUIT.

ALL PROPOSED CATV ROAD BORES WILL EXTEND A MINIMUM OF 8' BEYOND THE EDGE OF PAVEMENT.

ALL PROPOSED CATV DRIVEWAY BORES WILL EXTEND A MINIMUM OF 4' BEYOND THE EDGE OF PAVEMENT.

ALL PROPOSED CATV WILL BE .625" OR .875" DIAMETER, INSULATED COAXIAL CABLE OR 1.0" DIAMETER, INSULATED FIBER OPTIC CABLE, AND WILL BE LASHED TO .250" STEEL STRAND CABLE USING .125" STD. GALV. LASHING WIRE.

ALL PROPOSED AERIAL CATV CROSSINGS (BOTH ROADWAYS AND DRIVEWAYS) WILL HAVE MINIMUM MID-SPAN HEIGHT OF 18'-0".

ALL PARALLEL PROPOSED AERIAL CATV WILL HAVE MINIMUM MID-SPAN HEIGHT OF 16'-6" AS PER NESC SPECIFICATIONS.

LOCATES WILL BE REQUIRED IN ALL PROPOSED UNDERGROUND AREAS AT LEAST 48 HRS. PRIOR TO CONSTRUCTION.

NOTIFICATIONS TO ALL UTILITIES INVOLVED WILL BE MADE PRIOR TO CONSTRUCTION.

GENERAL NOTES

MAINTENANCE OF TRAFFIC TO BE SUPERVISED BY A CERTIFIED PERSON.

TEST RESULTS ARE REQUIRED UPON REQUEST.

THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT A MINIMUM OF TWO BUSINESS DAYS PRIOR TO ANY LANE CLOSURES OR BEGINNING ANY CONSTRUCTION WITHIN THE FDOT RIGHT-OF-WAY.

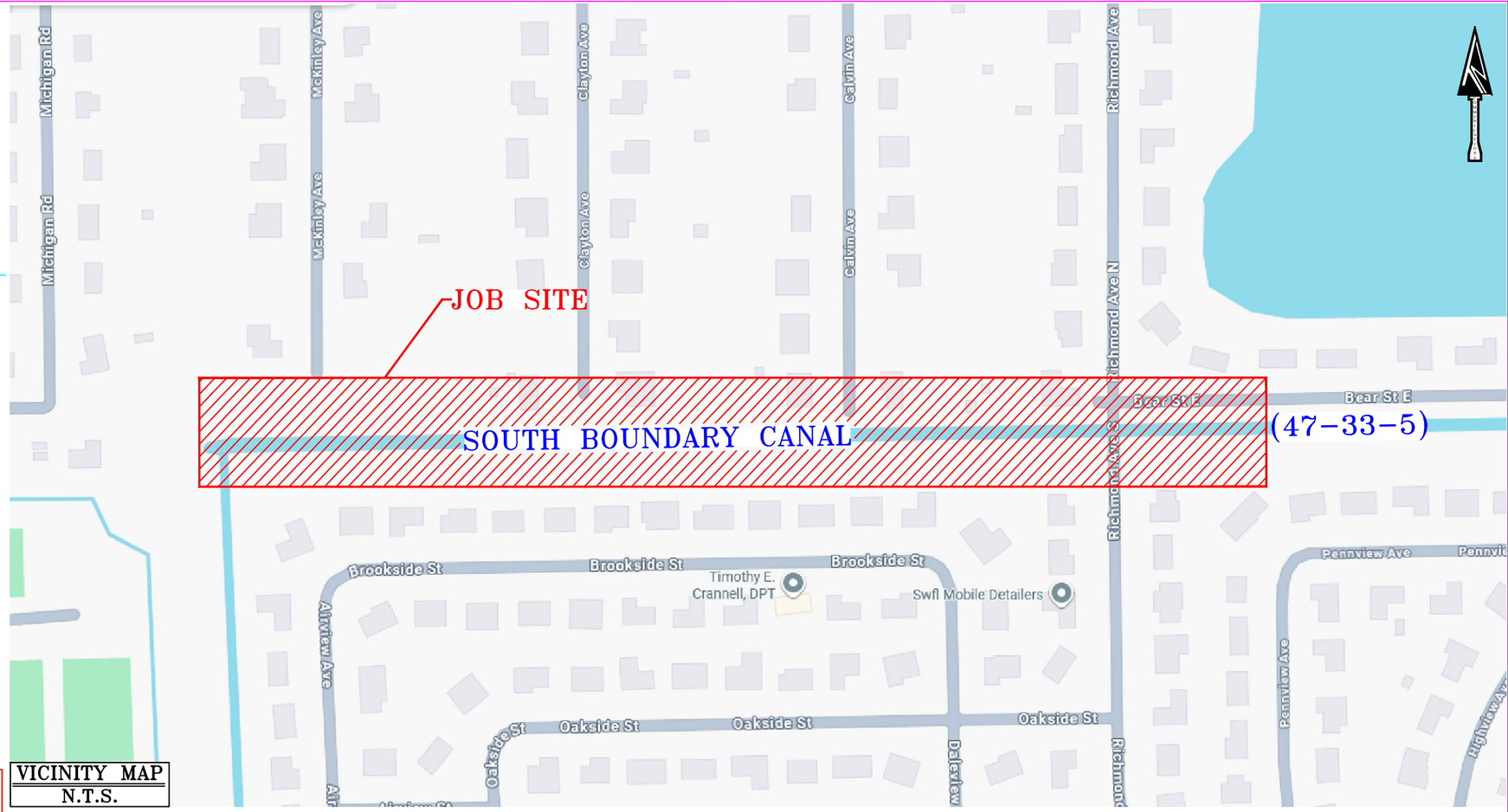
ALL WORK PERFORMED WITHIN THE FDOT RIGHT-OF-WAY SHALL BE IN ACCORDANCE WITH FDOT DESIGN STANDARDS, SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION AND THE UTILITY ACCOMODATION MANUAL, LATEST EDITIONS.

ENGINEER'S CERTIFICATION TOGETHER WITH AS-BUILTS, IF REQUIRED TO BE SUBMITTED WITHIN 30 DAYS OF COMPLETION.

ALL CONCRETE TO BE REMOVED SHALL BE SAW CUT AT THE NEAREST GOOD JOINT. GUTTER SHALL BE SAW CUT BETWEEN ASPHALT AND GUTTER BEFORE REMOVAL.

IT WILL BE THE RESPONSIBILITY OF THE PERMITTEE TO REPAIR ANY DAMAGE TO FDOT FACILITIES CAUSED BY THE CONSTRUCTION OF THE PROJECT.

ALL CONCRETE SHALL BE 3,000 PSI. APPROVED FDOT MIX DESIGN.



VICINITY MAP
N.T.S.

LEGEND

BORE	POWER POLE
SWALE/DITCH	POWER TRANSFORMER POLE
GUARD RAIL	JOINT USE TRANSFORMER POLE
GAS	PHONE POLE
POWER	CATV POLE
U/G CATV	STEEL POLE
STREET LIGHT	CONCRETE POLE
STORM DRAIN	JOINT USE POLE
SEWER	RISER
TELEPHONE	ANCHOR
TRAFFIC SIGNAL	OVERHEAD GUY
WATER	CATV FIBER VAULT
MISC U/G UTILITY	TRAFFIC SIGNAL VAULT
PROPOSED U/G CATV	TRAFFIC SIGNAL POLE
PROPOSED AERIAL FIBER	TRAFFIC SIGNAL BOX
PROPOSED AERIAL CATV	CROSS SECTION REFERENCE
CENTERLINE	XX' FTG BTWN REFERENCE POINTS
E/P EDGE OF PAVEMENT	REFERNCE POINT
B/C BACK OF CURB	STORM DRAIN
R/W RIGHT OF WAY	

PROJECT TOTALS

PROP. BURIED CATV:	1785'
PROP. OH CATV:	0'
TOTAL PROP. CATV:	1785' (0.34MI.)

MOT NOTES

TRAFFIC CONTROL SHALL COMPLY WITH FEDERAL MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) AND F.D.O.T. DESIGN STANDARD #NO. 600.

160 S HOMESTEAD RD – LAMSID

BINDING SPACE

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

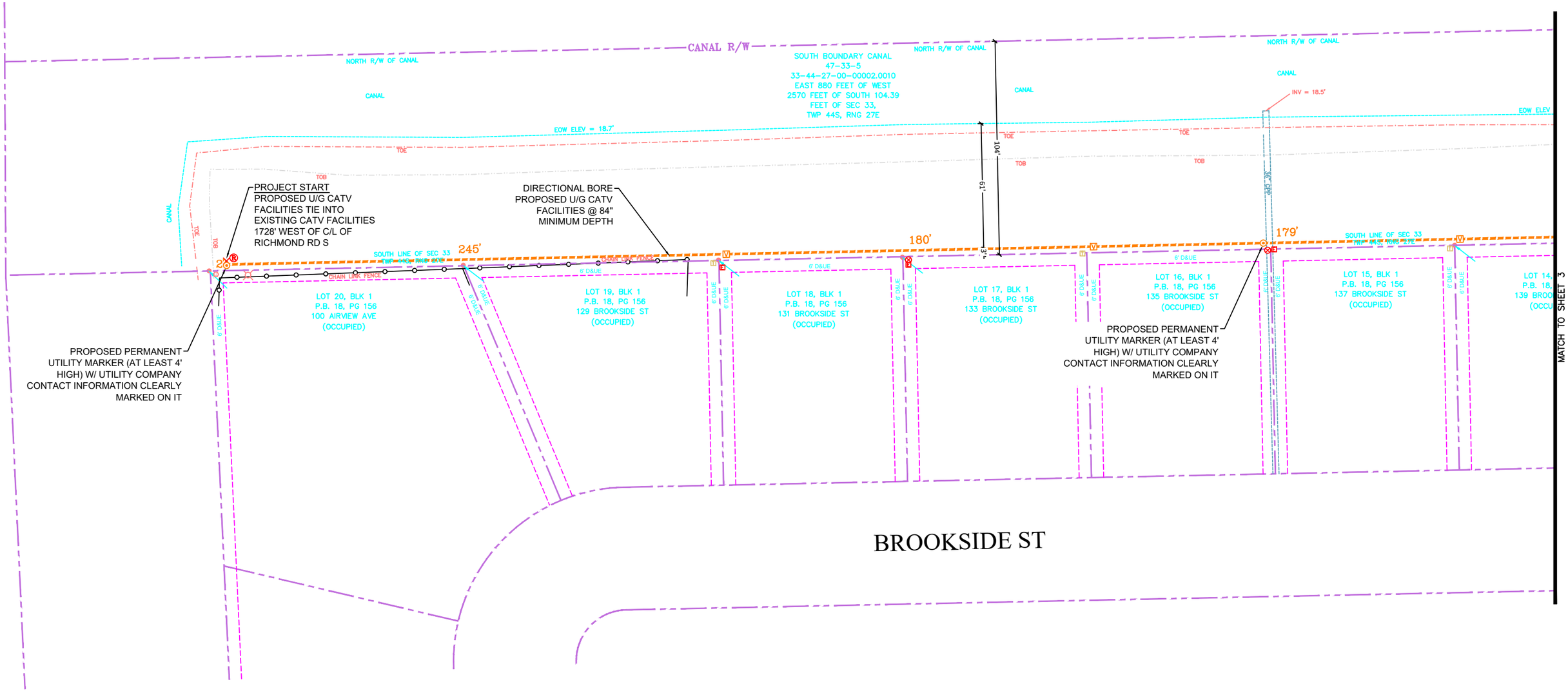
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 1 OF 7
	DATE: 12/18/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
PLAN VIEW

0 25' 50'

BINDING SPACE



160 S HOMESTEAD RD – LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

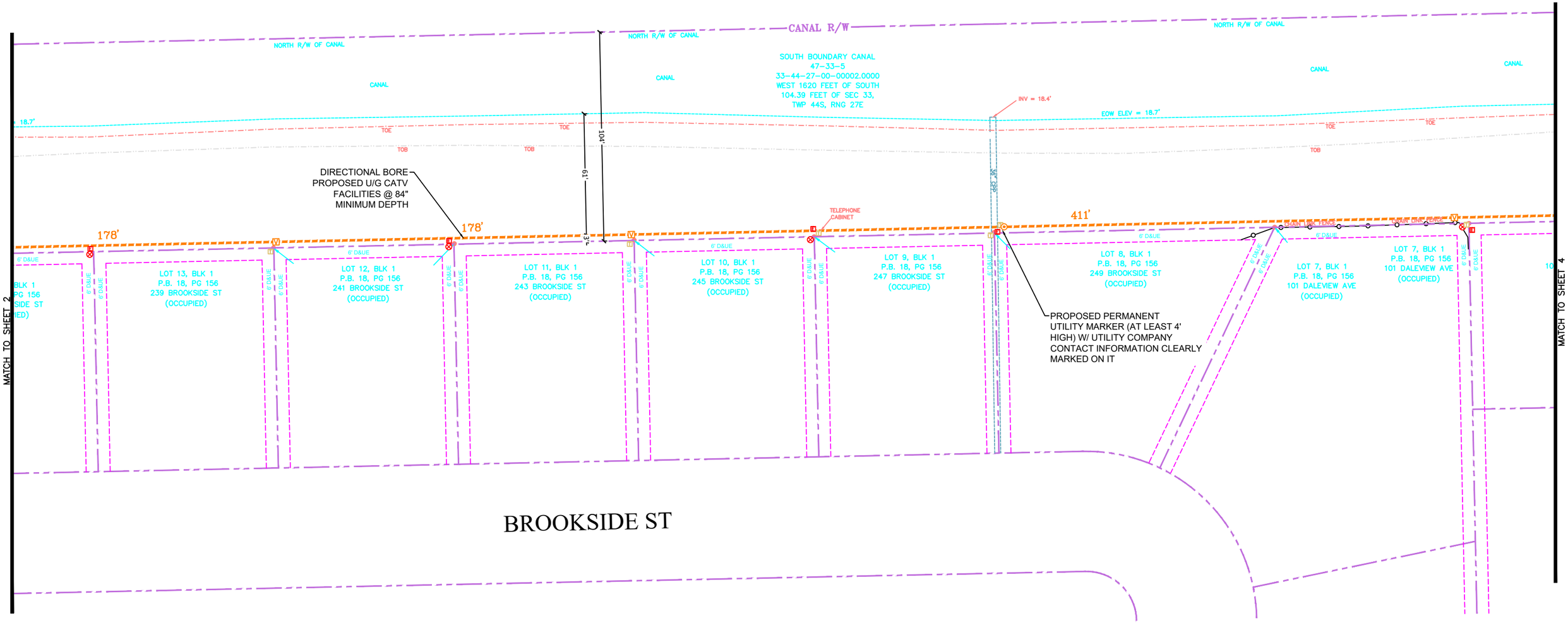
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 2 OF 7
	DATE: 12/18/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA–MSID PERMIT
PLAN VIEW

0 25’ 50’

BINDING SPACE



160 S HOMESTEAD RD – LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT–OF–WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 3 OF 7
	DATE: 12/18/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
PLAN VIEW

0 25' 50'

BINDING SPACE

MATCH TO SHEET 3

BEAR ST E

R/W E/P C/L E/P R/W

CANAL R/W

NORTH R/W OF CANAL

BENCHMARK
SET N&D #8331
"TRAVERSE"
ELEV. = 23.88'
N.A.V.D. 88

RIM = 24.6'
INV = 17.2'
CONC HEADWALL
48" RCP
INLET = 23.9'
EOP
TOE
TOB
CURB

RIM = 24.7'
INV = 17.1'
INV = 17.3'
CONC HEADWALL
INV = 20.1'
48" RCP

SOUTH BOUNDARY CANAL
47-33-5
33-44-27-00-00002.0010
EAST 880 FEET OF WEST
2570 FEET OF SOUTH 104.39
FEET OF SEC 33,
TWP 44S, RNG 27E

CANAL

EOW ELEV = 18.7'

104'

59'

13'

36" CPP

GRATE = 24.2'
INV = 21.1'

CONC SIGN PLANTER

TELEPHONE CABINET

TELEPHONE PADS/CONC

412'

6' D&UE

R/W

DIRECTIONAL BORE
PROPOSED U/G CATV
FACILITIES @ 84"
MINIMUM DEPTH

PROPOSED PERMANENT
UTILITY MARKER (AT LEAST 4'
HIGH) W/ UTILITY COMPANY
CONTACT INFORMATION CLEARLY
MARKED ON IT

PROJECT END
PROPOSED U/G CATV FACILITIES TIE
INTO PROPOSED CATV FACILITIES
254' EAST OF C/L OF RICHMOND AVE S

EX CATV VAULT

LOT 1, BLK 1
P.B. 18, PG 156
RICHMOND AVE
(OCCUPIED)

LOT 1, BLK 6
P.B. 18, PG 156
101 RICHMOND AVE
(OCCUPIED)

LOT 28, BLK 6
P.B. 18, PG 156
512 PENNVIEW AVE
(OCCUPIED)

LOT 29, BLK 6
P.B. 18, PG 156
510 PENNVIEW AVE
(OCCUPIED)

RICHMOND AVE S

BENCHMARK
SET N&D #8331
"TRAVERSE"
ELEV = 24.27'
N.A.V.D. 88

80'
22' 18' 18' 22'

R/W B/C C/L B/C R/W

PENNVIEW AVE

160 S HOMESTEAD RD – LAMSID

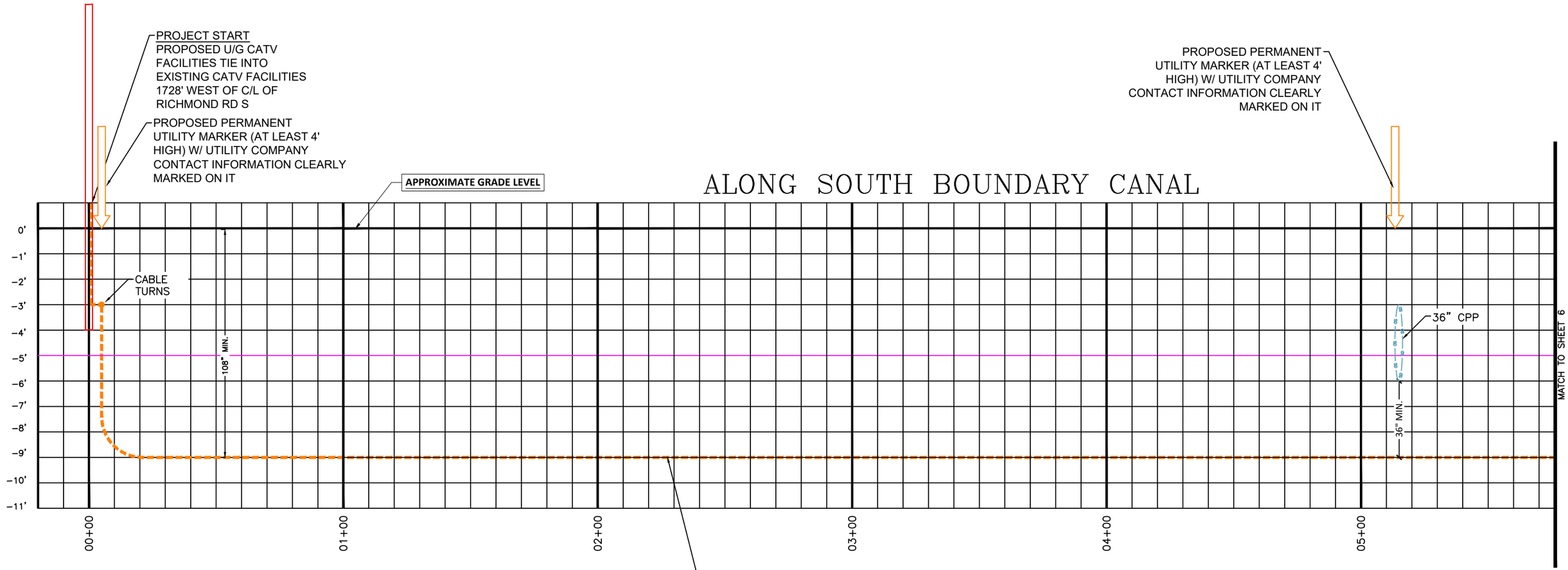
COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 4 OF 7
	DATE: 12/18/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
PROFILE VIEW

BINDING SPACE



HORIZONTAL SCALE 1"=40'
VERTICAL SCALE 1"=4'

CONSTRUCTION NOTE:
ALL CANAL RIGHT OF WAY THAT IS DISTURBED MUST BE RESTORED WITH SOD AND MAINTAINED UNTIL WELL ESTABLISHED. NO SEEDING WILL BE ALLOWED.

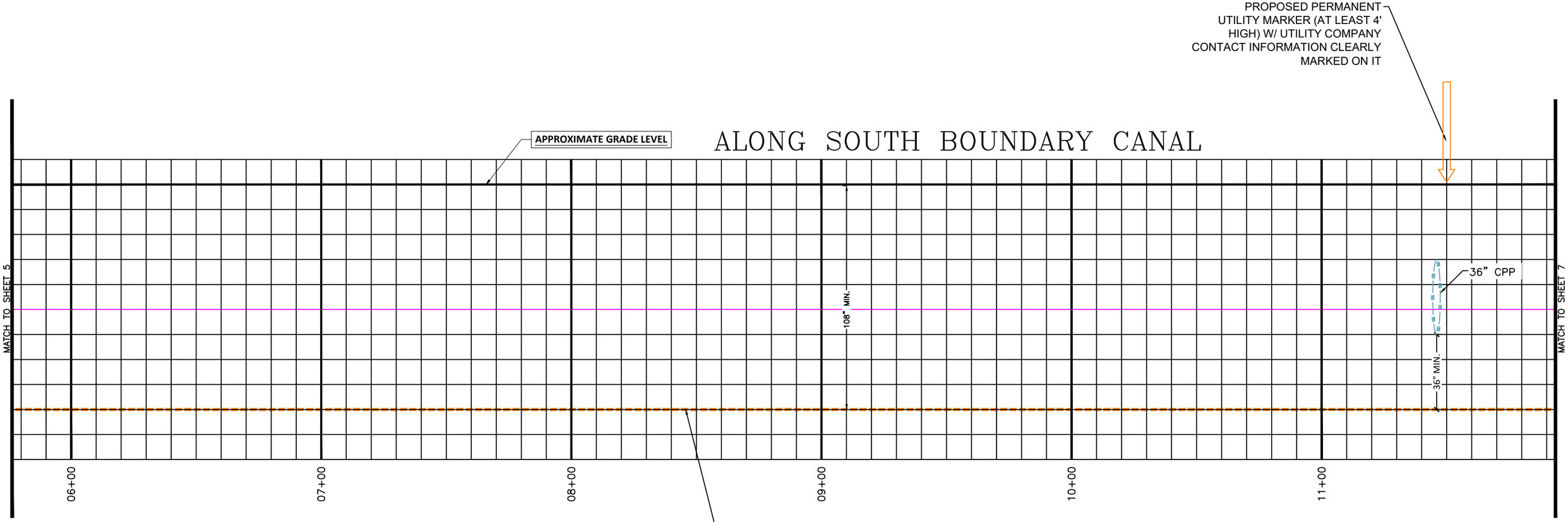
160 S HOMESTEAD RD – LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 5 OF 7
	DATE: 12/18/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
PROFILE VIEW



HORIZONTAL SCALE 1"=40'
VERTICAL SCALE 1"=4'

CONSTRUCTION NOTE:
ALL CANAL RIGHT OF WAY THAT IS DISTURBED MUST BE RESTORED WITH SOD AND MAINTAINED UNTIL WELL ESTABLISHED. NO SEEDING WILL BE ALLOWED.

BINDING SPACE

160 S HOMESTEAD RD – LAMSID

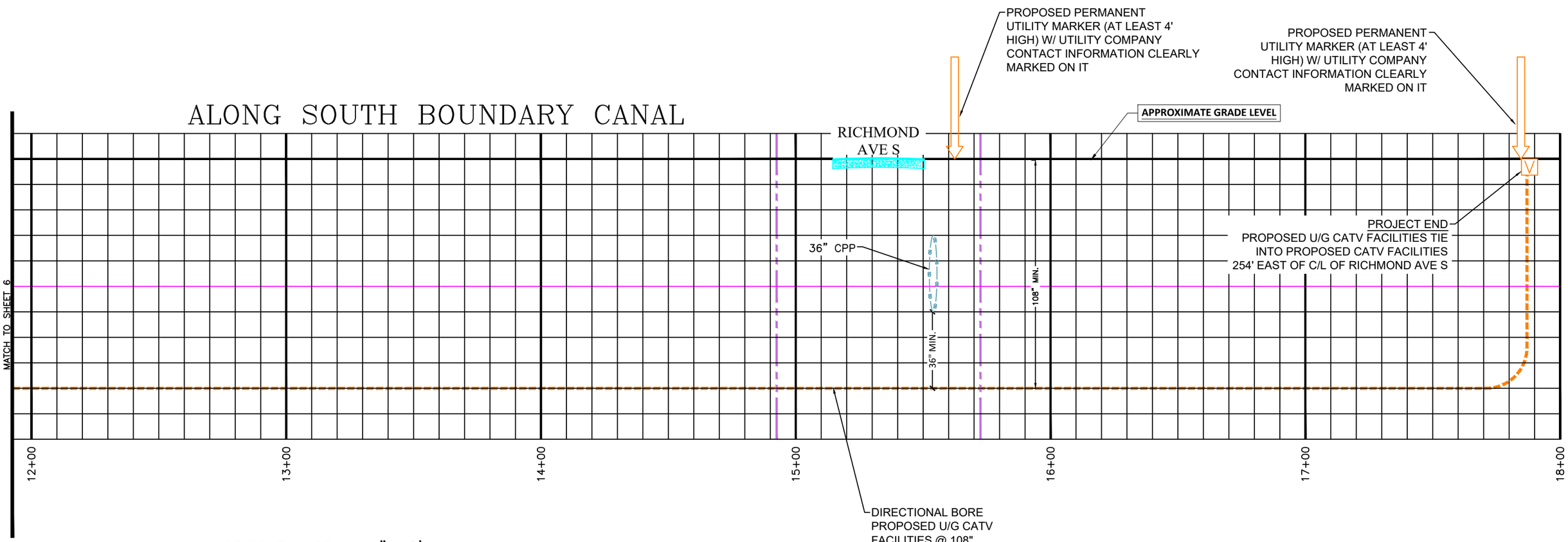
COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 6 OF 7
	DATE: 12/18/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

160 S HOMESTEAD RD
LA-MSID PERMIT
PROFILE VIEW

BINDING SPACE



HORIZONTAL SCALE 1"=40'
VERTICAL SCALE 1"=4'

CONSTRUCTION NOTE:
ALL CANAL RIGHT OF WAY THAT IS DISTURBED MUST BE RESTORED WITH SOD AND MAINTAINED UNTIL WELL ESTABLISHED. NO SEEDING WILL BE ALLOWED.

160 S HOMESTEAD RD – LAMSID

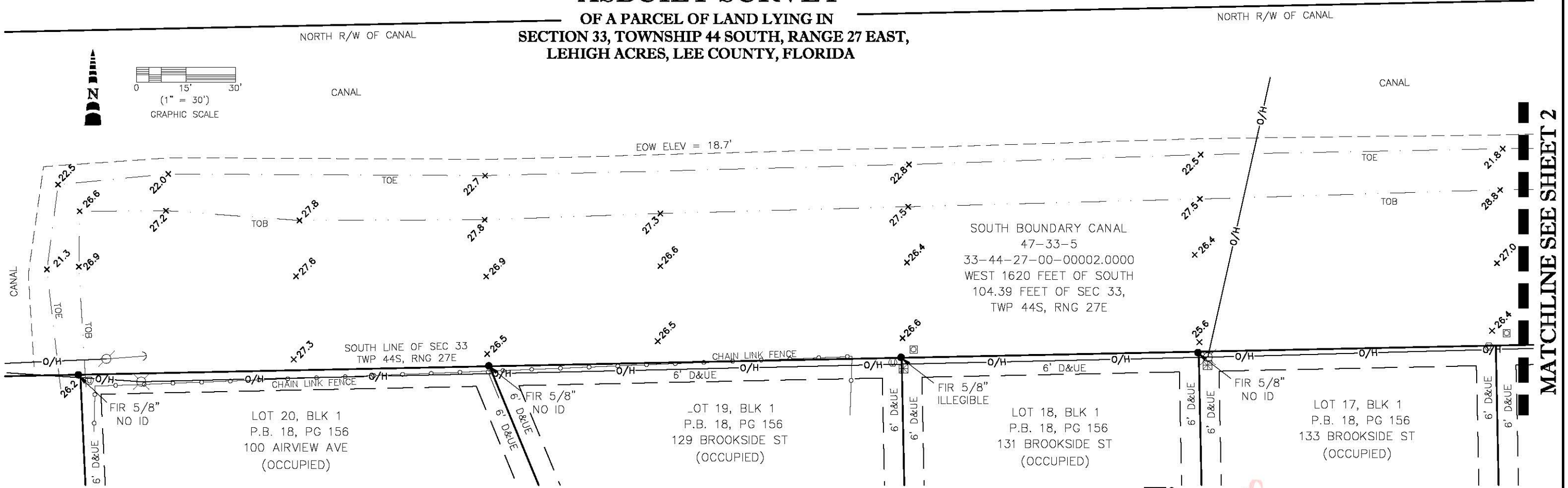
COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED U/G CATV FACILITIES ON SOUTH BOUNDARY CANAL AT RICHMOND AVE S.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 7 OF 7
	DATE: 12/18/2025
CIFA: N/A	**MAP:** N/A
JB#: JB0001857949	**DRAFTED BY:** J. COLLINS

ASBUILT SURVEY

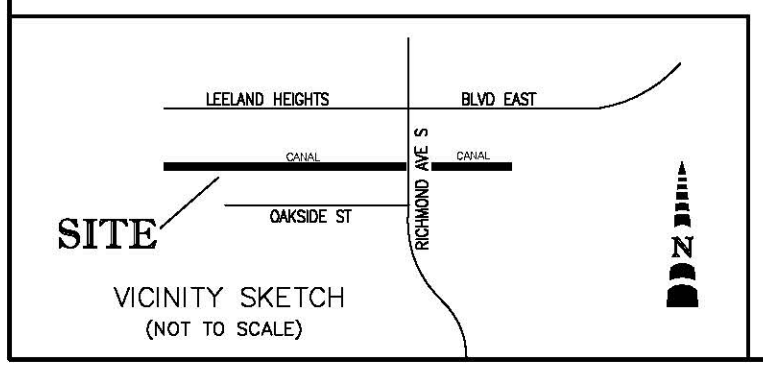
OF A PARCEL OF LAND LYING IN
SECTION 33, TOWNSHIP 44 SOUTH, RANGE 27 EAST,
LEHIGH ACRES, LEE COUNTY, FLORIDA



LEGEND:

FIR = FOUND IRON ROD
N&D = NAIL AND DISC
ELEV = ELEVATION
P.B. = PLAT BOOK
PG = PAGE
TRAV = TRAVERSE
CONC = CONCRETE
D&UE = DRAINAGE & UTILITY EASEMENT
RCP = REINFORCED CONC PIPE
CCP = CORRUGATED PLASTIC PIPE
EOP = EDGE OF PAVEMENT
R/W = RIGHT OF WAY
TOB = TOP OF BANK
TOE = TOE OF SLOPE
INV = INVERT
SEC = SECTION
TWP = TOWNSHIP
RNG = RANGE
EOW = EDGE OF WATER

= ELECTRIC SERVICE
= BURIED VAULT
= CABLE TELEVISION BOX
= CONCRETE POWER POLE
= TELEPHONE SERVICE BOX
= WOOD UTILITY POLE
= FIBER OPTIC CABLE MARKER
+28.1' = SPOT ELEVATION
= CATCH BASIN
—O/H— = OVERHEAD UTILITY LINES
= STORM SEWER CURB INLET
= DRAINAGE MANHOLE
= FREE-STANDING ELECTRIC METER
= GUY WIRE ANCHOR
N.A.V.D. 88 = NORTH AMERICAN VERTICAL DATUM 1988



VICINITY SKETCH
(NOT TO SCALE)

NOTES:

"SURVEY BASED ON THE PLAT OF LEHIGH ACRES UNIT 1 AS RECORDED IN PLAT BOOK 18, PAGE 156 AND PLAT OF LEHIGH ACRES UNIT 2 AS RECORDED IN PLAT BOOK 18, PAGE 157, OF THE PUBLIC RECORDS OF LEE COUNTY, FLORIDA AND EXISTING MONUMENTATION.

THIS PLAT PREPARED AS AN ASBUILT SURVEY FOR THE PURPOSE OF LOCATING THE FEATURES WITHIN AND ADJACENT TO THE CANAL LOCATED NORTH OF THE SOUTH LINE OF SEC 33, TWP 44S, RNG 27E, ALSO EAST AND WEST OF RICHMOND AVE SOUTH BETWEEN LEELAND HEIGHTS BLVD E AND OAKSIDE ST, LEHIGH ACRES.

THE ELEVATIONS SHOWN HEREON ARE NORTH AMERICAN VERTICAL DATUM OF 1988 (N.A.V.D. 88) IN FEET AND DECIMALS THEREOF AND BASED UPON CONTINUOUSLY OPERATING FLORIDA PERMANENT REFERENCE NETWORK (FPRN) STATIONS MAINTAINED BY THE FLORIDA DEPARTMENT OF TRANSPORTATION.

PARCEL LIES WITHIN INSURANCE RATE MAP 125124 0475F, PANEL NOT PRINTED, THEREFORE HAS NO BASE FLOOD ELEVATION.

THE F.E.M.A. FLOOD ZONE INFORMATION INDICATED HEREON IS BASED ON MAPS SUPPLIED BY THE FEDERAL GOVERNMENT. THIS FLOOD INFORMATION MUST BE VERIFIED WITH ALL PERMITTING REGULATORY ENTITIES PRIOR TO COMMENCING ANY WORK OR APPLICATION DEPENDENT ON SAID FLOOD INFORMATION.

UNDERGROUND IMPROVEMENTS, UTILITIES AND/OR FOUNDATIONS WERE NOT LOCATED UNLESS OTHERWISE NOTED.

SUBJECT TO EASEMENTS, RESTRICTIONS, RESERVATIONS AND RIGHTS-OF-WAY (RECORDED AND UNRECORDED, WRITTEN AND UNWRITTEN).

DATE OF LAST FIELD WORK: 11-25-2025.

TIMOTHY LEE MANN
LICENSE NUMBER
LS 5838
STATE OF FLORIDA
PROFESSIONAL SURVEYOR & MAPPER

Timothy L Mann
Digitally signed by Timothy L Mann
Date: 2025.12.03 11:10:44 -05'00'

BY: TIMOTHY LEE MANN
REGISTERED SURVEYOR AND MAPPER
FLORIDA CERTIFICATE NO. LS# 5838

THIS SURVEY HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY TIMOTHY LEE MANN USING A DIGITAL SIGNATURE AND DATE. PRINT COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

THIS SURVEY IS ONLY FOR THE LANDS AS DESCRIBED. IT IS NOT A CERTIFICATE OF TITLE, ZONING, EASEMENTS OR FREEDOM OF ENCUMBRANCES.

THIS SURVEY WAS PREPARED WITHOUT BENEFIT OF AN ABSTRACT OF TITLE AND ALL MATTERS OF TITLE SHOULD BE REFERRED TO AN ATTORNEY AT LAW.

RICHMOND AVE SOUTH, LEHIGH ACRES

TITLE: ASBUILT SURVEY

KEYSTONE
A CLS COMPANY
www.keystoneconsultants.net

10970 S. CLEVELAND AVE.
SUITE #605
FORT MYERS, FLORIDA 33907
PHONE: (239) 275-8575
FAX: (239) 275-8457

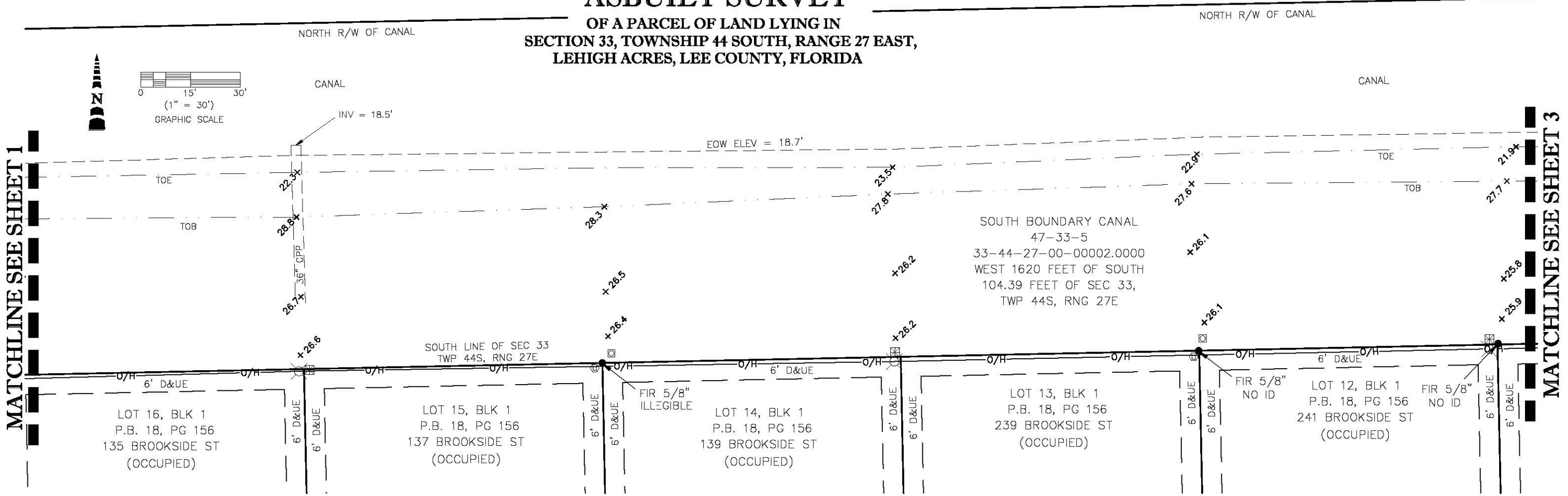
LB #8331

FILE NAME:	FIELD BOOK/PAGE:	PROJECT NO.:	SHEET:
15736SR.DWG	SEE FILE	15736	1 OF 4

SURVEY DATE:	DRAWN BY:	SCALE:	CHECKED BY:	FILE NO. (S-T-R)
11/19/25	BUD	1"= 30'	TLM	33-44-27

ASBUILT SURVEY

OF A PARCEL OF LAND LYING IN
SECTION 33, TOWNSHIP 44 SOUTH, RANGE 27 EAST,
LEHIGH ACRES, LEE COUNTY, FLORIDA



LEGEND:

FIR = FOUND IRON ROD
N&D = NAIL AND DISC
ELEV = ELEVATION
P.B. = PLAT BOOK
PG = PAGE
TRAV = TRAVERSE
CONC = CONCRETE
D&UE = DRAINAGE & UTILITY EASEMENT
RCP = REINFORCED CONC PIPE
CCP = CORRUGATED PLASTIC PIPE
EOP = EDGE OF PAVEMENT
R/W = RIGHT OF WAY
TOB = TOP OF BANK
TOE = TOE OF SLOPE
INV = INVERT
SEC = SECTION
TWP = TOWNSHIP
RNG = RANGE
EOW = EDGE OF WATER

= ELECTRIC SERVICE
= BURIED VAULT
= CABLE TELEVISION BOX
= CONCRETE POWER POLE
= TELEPHONE SERVICE BOX
= WOOD UTILITY POLE
= FIBER OPTIC CABLE MARKER
+28.7 = SPOT ELEVATION
= CATCH BASIN
—O/H— = OVERHEAD UTILITY LINES
= STORM SEWER CURB INLET
= DRAINAGE MANHOLE
= FREE-STANDING ELECTRIC METER
= GUY WIRE ANCHOR
N.A.V.D. 88 = NORTH AMERICAN VERTICAL DATUM 1988

NOTES:

SURVEY BASED ON THE PLAT OF LEHIGH ACRES UNIT 1 AS RECORDED IN PLAT BOOK 18, PAGE 156 AND PLAT OF LEHIGH ACRES UNIT 2 AS RECORDED IN PLAT BOOK 18, PAGE 157, OF THE PUBLIC RECORDS OF LEE COUNTY, FLORIDA AND EXISTING MONUMENTATION.

THIS PLAT PREPARED AS AN ASBUILT SURVEY FOR THE PURPOSE OF LOCATING THE FEATURES WITHIN AND ADJACENT TO THE CANAL LOCATED NORTH OF THE SOUTH LINE OF SEC 33, TWP 44S, RNG 27E, ALSO EAST AND WEST OF RICHMOND AVE SOUTH BETWEEN LEELAND HEIGHTS BLVD E AND OAKSIDE ST, LEHIGH ACRES.

THE ELEVATIONS SHOWN HEREON ARE NORTH AMERICAN VERTICAL DATUM OF 1988 (N.A.V.D. 88) IN FEET AND DECIMALS THEREOF AND BASED UPON CONTINUOUSLY OPERATING FLORIDA PERMANENT REFERENCE NETWORK (FPRN) STATIONS MAINTAINED BY THE FLORIDA DEPARTMENT OF TRANSPORTATION.

PARCEL LIES WITHIN INSURANCE RATE MAP 125124 0475F, PANEL NOT PRINTED, THEREFORE HAS NO BASE FLOOD ELEVATION.

THE F.E.M.A. FLOOD ZONE INFORMATION INDICATED HEREON IS BASED ON MAPS SUPPLIED BY THE FEDERAL GOVERNMENT. THIS FLOOD INFORMATION MUST BE VERIFIED WITH ALL PERMITTING REGULATORY ENTITIES PRIOR TO COMMENCING ANY WORK OR APPLICATION DEPENDENT ON SAID FLOOD INFORMATION.

UNDERGROUND IMPROVEMENTS, UTILITIES AND/OR FOUNDATIONS WERE NOT LOCATED UNLESS OTHERWISE NOTED.

SUBJECT TO EASEMENTS, RESTRICTIONS, RESERVATIONS AND RIGHTS-OF-WAY (RECORDED AND UNRECORDED, WRITTEN AND UNWRITTEN).

DATE OF LAST FIELD WORK: 11-25-2025.

RICHMOND AVE SOUTH, LEHIGH ACRES

TITLE: ASBUILT SURVEY

KEYSTONE
A CLS COMPANY
www.keystoneconsultants.net

10970 S. CLEVELAND AVE.
SUITE #605
FORT MYERS, FLORIDA 33907
PHONE: (239) 275-8575
FAX: (239) 275-8457

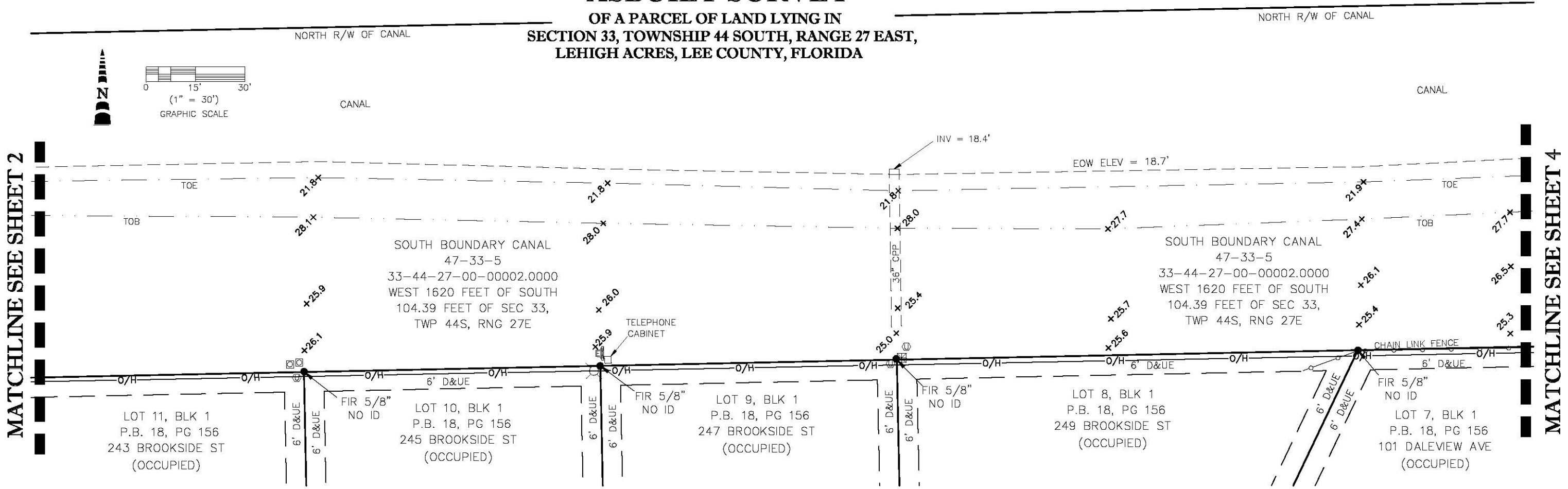
LB #8331

FILE NAME:	FIELD BOOK/PAGE:	PROJECT NO.:	SHEET:
15736SR.DWG	SEE FILE	15736	2 OF 4

SURVEY DATE:	DRAWN BY:	SCALE:	CHECKED BY:	FILE NO. (S-T-R)
11/19/25	BUD	1"= 30'	TLM	33-44-27

ASBUILT SURVEY

OF A PARCEL OF LAND LYING IN
SECTION 33, TOWNSHIP 44 SOUTH, RANGE 27 EAST,
LEHIGH ACRES, LEE COUNTY, FLORIDA



LEGEND:

FIR = FOUND IRON ROD
N&D = NAIL AND DISC
ELEV = ELEVATION
P.B. = PLAT BOOK
PG = PAGE
TRAV = TRAVERSE
CONC = CONCRETE
D&UE = DRAINAGE & UTILITY EASEMENT
RCP = REINFORCED CONC PIPE
CCP = CORRUGATED PLASTIC PIPE
EOP = EDGE OF PAVEMENT
R/W = RIGHT OF WAY
TOB = TOP OF BANK
TOE = TOE OF SLOPE
INV = INVERT
SEC = SECTION
TWP = TOWNSHIP
RNG = RANGE
EOW = EDGE OF WATER

= ELECTRIC SERVICE
= BURIED VAULT
= CABLE TELEVISION BOX
= CONCRETE POWER POLE
= TELEPHONE SERVICE BOX
= WOOD UTILITY POLE
= FIBER OPTIC CABLE MARKER
+28.7 = SPOT ELEVATION
= CATCH BASIN
—O/H— = OVERHEAD UTILITY LINES
= STORM SEWER CURB INLET
= DRAINAGE MANHOLE
= FREE-STANDING ELECTRIC METER
= GUY WIRE ANCHOR
N.A.V.D. 88 = NORTH AMERICAN VERTICAL DATUM 1988

NOTES:

SURVEY BASED ON THE PLAT OF LEHIGH ACRES UNIT 1 AS RECORDED IN PLAT BOOK 18, PAGE 156 AND PLAT OF LEHIGH ACRES UNIT 2 AS RECORDED IN PLAT BOOK 18, PAGE 157, OF THE PUBLIC RECORDS OF LEE COUNTY, FLORIDA AND EXISTING MONUMENTATION.

THIS PLAT PREPARED AS AN ASBUILT SURVEY FOR THE PURPOSE OF LOCATING THE FEATURES WITHIN AND ADJACENT TO THE CANAL LOCATED NORTH OF THE SOUTH LINE OF SEC 33, TWP 44S, RNG 27E, ALSO EAST AND WEST OF RICHMOND AVE SOUTH BETWEEN LEELAND HEIGHTS BLVD E AND OAKSIDE ST, LEHIGH ACRES.

THE ELEVATIONS SHOWN HEREON ARE NORTH AMERICAN VERTICAL DATUM OF 1988 (N.A.V.D. 88) IN FEET AND DECIMALS THEREOF AND BASED UPON CONTINUOUSLY OPERATING FLORIDA PERMANENT REFERENCE NETWORK (FPRN) STATIONS MAINTAINED BY THE FLORIDA DEPARTMENT OF TRANSPORTATION.

PARCEL LIES WITHIN INSURANCE RATE MAP 125124 0475F, PANEL NOT PRINTED, THEREFORE HAS NO BASE FLOOD ELEVATION.

THE F.E.M.A. FLOOD ZONE INFORMATION INDICATED HEREON IS BASED ON MAPS SUPPLIED BY THE FEDERAL GOVERNMENT. THIS FLOOD INFORMATION MUST BE VERIFIED WITH ALL PERMITTING REGULATORY ENTITIES PRIOR TO COMMENCING ANY WORK OR APPLICATION DEPENDENT ON SAID FLOOD INFORMATION.

UNDERGROUND IMPROVEMENTS, UTILITIES AND/OR FOUNDATIONS WERE NOT LOCATED UNLESS OTHERWISE NOTED.

SUBJECT TO EASEMENTS, RESTRICTIONS, RESERVATIONS AND RIGHTS-OF-WAY (RECORDED AND UNRECORDED, WRITTEN AND UNWRITTEN).

DATE OF LAST FIELD WORK: 11-25-2025.

RICHMOND AVE SOUTH, LEHIGH ACRES

TITLE: ASBUILT SURVEY

KEYSTONE
A CLS COMPANY
www.keystoneconsultants.net

10970 S. CLEVELAND AVE.
SUITE #605
FORT MYERS, FLORIDA 33907
PHONE: (239) 275-8575
FAX: (239) 275-8457

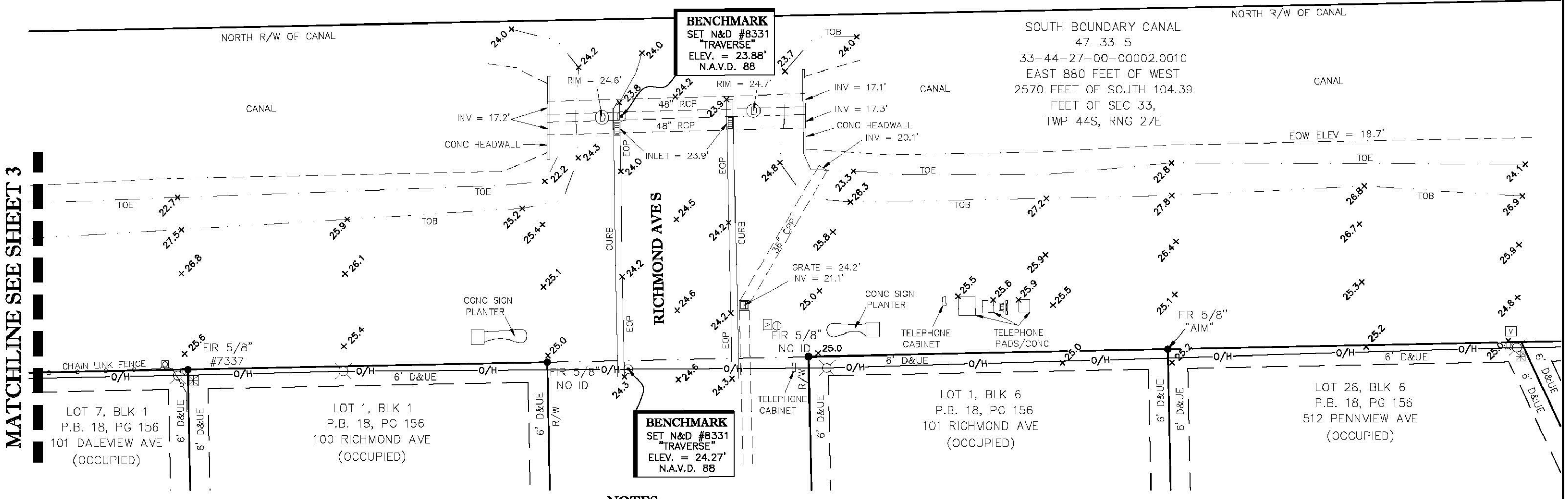
LB #8331

FILE NAME:	FIELD BOOK/PAGE:	PROJECT NO.:	SHEET:
15736SR.DWG	SEE FILE	15736	3 OF 4

SURVEY DATE:	DRAWN BY:	SCALE:	CHECKED BY:	FILE NO. (S-T-R)
11/19/25	BUD	1"= 30'	TLM	33-44-27

ASBUILT SURVEY

OF A PARCEL OF LAND LYING IN
SECTION 33, TOWNSHIP 44 SOUTH, RANGE 27 EAST,
LEHIGH ACRES, LEE COUNTY, FLORIDA



LEGEND:

FIR = FOUND IRON ROD
N&D = NAIL AND DISC
ELEV = ELEVATION
P.B. = PLAT BOOK
PG = PAGE
TRAV = TRAVERSE
CONC = CONCRETE
D&UE = DRAINAGE & UTILITY EASEMENT
RCP = REINFORCED CONC PIPE
CCP = CORRUGATED PLASTIC PIPE
EOP = EDGE OF PAVEMENT
R/W = RIGHT OF WAY
TOB = TOP OF BANK
TOE = TOE OF SLOPE
INV = INVERT
SEC = SECTION
TWP = TOWNSHIP
RNG = RANGE
EOW = EDGE OF WATER

= ELECTRIC SERVICE
= BURIED VAULT
= CABLE TELEVISION BOX
= CONCRETE POWER POLE
= TELEPHONE SERVICE BOX
= WOOD UTILITY POLE
= FIBER OPTIC CABLE MARKER
+28.7' = SPOT ELEVATION
= CATCH BASIN
—O/H— = OVERHEAD UTILITY LINES
= STORM SEWER CURB INLET
= DRAINAGE MANHOLE
= FREE-STANDING ELECTRIC METER
= GUY WIRE ANCHOR
N.A.V.D. 88 = NORTH AMERICAN VERTICAL DATUM 1988

NOTES:

SURVEY BASED ON THE PLAT OF LEHIGH ACRES UNIT 1 AS RECORDED IN PLAT BOOK 18, PAGE 156 AND PLAT OF LEHIGH ACRES UNIT 2 AS RECORDED IN PLAT BOOK 18, PAGE 157, OF THE PUBLIC RECORDS OF LEE COUNTY, FLORIDA AND EXISTING MONUMENTATION.

THIS PLAT PREPARED AS AN ASBUILT SURVEY FOR THE PURPOSE OF LOCATING THE FEATURES WITHIN AND ADJACENT TO THE CANAL LOCATED NORTH OF THE SOUTH LINE OF SEC 33, TWP 44S, RNG 27E, ALSO EAST AND WEST OF RICHMOND AVE SOUTH BETWEEN LEELAND HEIGHTS BLVD E AND OAKSIDE ST, LEHIGH ACRES.

THE ELEVATIONS SHOWN HEREON ARE NORTH AMERICAN VERTICAL DATUM OF 1988 (N.A.V.D. 88) IN FEET AND DECIMALS THEREOF AND BASED UPON CONTINUOUSLY OPERATING FLORIDA PERMANENT REFERENCE NETWORK (FPRN) STATIONS MAINTAINED BY THE FLORIDA DEPARTMENT OF TRANSPORTATION.

PARCEL LIES WITHIN INSURANCE RATE MAP 125124 0475F, PANEL NOT PRINTED, THEREFORE HAS NO BASE FLOOD ELEVATION.

THE F.E.M.A. FLOOD ZONE INFORMATION INDICATED HEREON IS BASED ON MAPS SUPPLIED BY THE FEDERAL GOVERNMENT. THIS FLOOD INFORMATION MUST BE VERIFIED WITH ALL PERMITTING REGULATORY ENTITIES PRIOR TO COMMENCING ANY WORK OR APPLICATION DEPENDENT ON SAID FLOOD INFORMATION.

UNDERGROUND IMPROVEMENTS, UTILITIES AND/OR FOUNDATIONS WERE NOT LOCATED UNLESS OTHERWISE NOTED.

SUBJECT TO EASEMENTS, RESTRICTIONS, RESERVATIONS AND RIGHTS-OF-WAY (RECORDED AND UNRECORDED, WRITTEN AND UNWRITTEN).

DATE OF LAST FIELD WORK: 11-25-2025.

N

0 15' 30'
(1" = 30')
GRAPHIC SCALE

RICHMOND AVE SOUTH, LEHIGH ACRES

TITLE: ASBUILT SURVEY

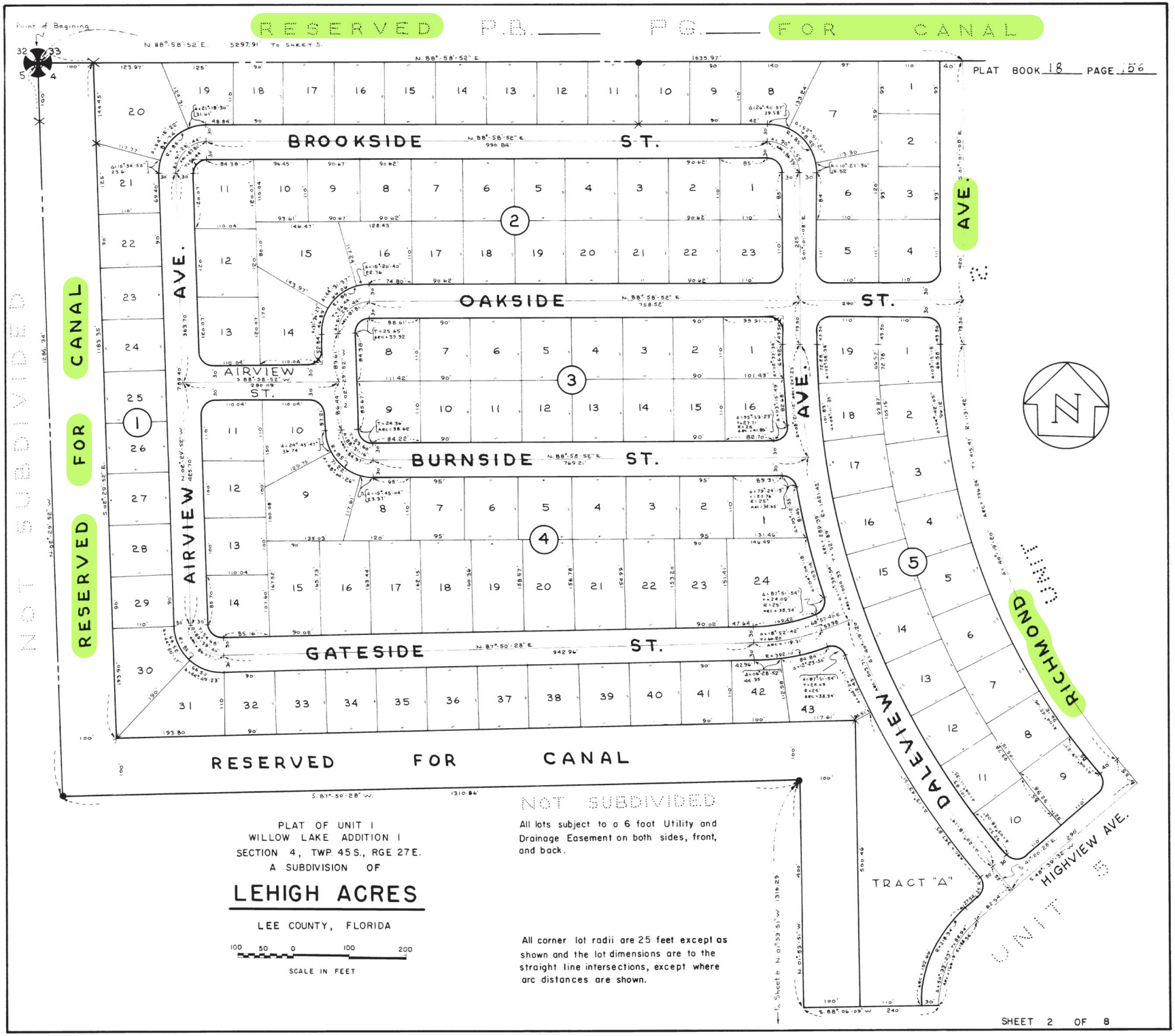
KEYSTONE
A CLS COMPANY
www.keystoneconsultants.net

10970 S. CLEVELAND AVE.
SUITE #605
FORT MYERS, FLORIDA 33907
PHONE: (239) 275-8575
FAX: (239) 275-8457

LB #8331

FILE NAME:	FIELD BOOK/PAGE:	PROJECT NO.:	SHEET:
15736SR.DWG	SEE FILE	15736	4 OF 4

SURVEY DATE:	DRAWN BY:	SCALE:	CHECKED BY:	FILE NO. (S-T-R)
11/19/25	BUD	1"= 30'	TLM	33-44-27


RESERVED P.B.____ PG.____ FOR CANAL
PLAT BOOK 18 PAGE 156
Point of Beginning
N 88°-58'-52" E. 5297.91' TO SHEET 3.
BROOKSIDE ST.
OAKSIDE ST.
AIRVIEW ST.
BURNSIDE ST.
GATESIDE ST.
AIRVIEW AVE.
DALEVIEW AVE.
RICHMOND AVE.
HIGHVIEW AVE.
RESERVED FOR CANAL
RESERVED FOR CANAL
NOT SUBDIVIDED
TRACT "A"
1
2
3
4
5
N
PLAT OF UNIT 1
WILLOW LAKE ADDITION 1
SECTION 4, TWP. 45 S., RGE 27 E.
A SUBDIVISION OF
LEHIGH ACRES
LEE COUNTY, FLORIDA
100 50 0 100 200
SCALE IN FEET
All lots subject to a 6 foot Utility and Drainage Easement on both sides, front, and back.
All corner lot radii are 25 feet except as shown and the lot dimensions are to the straight line intersections, except where arc distances are shown.
SHEET 2 OF 8

PLAT BOOK 18, PAGE 155

WILLOW LAKE ADDITION 1

A SUBDIVISION OF

LEHIGH ACRES

LEE COUNTY, FLORIDA

LEHIGH ACRES DEVELOPMENT CORPORATION

DEVELOPERS

DESCRIPTION

ALL THAT PART OF SECTION 4, TOWNSHIP 45 SOUTH, RANGE 27 EAST, LEE COUNTY, FLORIDA; DESCRIBED AS FOLLOWS: BEGINNING AT THE NORTHWEST CORNER OF SAID SECTION 4; THENCE N.88°-58'-52"E. ALONG THE NORTH LINE THEREOF, A DISTANCE OF 5297.91 FEET, TO THE NORTHEAST CORNER THEREOF; THENCE S.00°-02'-50"E. ALONG THE EAST LINE OF SAID SECTION, A DISTANCE OF 2497.96 FEET, TO THE EAST 1/4 CORNER THEREOF; THENCE S. 87°-50'-32"W. ALONG THE EAST AND WEST CENTER LINE OF SAID SECTION, 3892.32 FEET, TO THE SOUTHEAST CORNER OF THE SOUTHWEST QUARTER OF THE NORTHWEST QUARTER (S.W. 1/4 OF THE N.W. 1/4); THENCE N. 01°-53'-51"W. ALONG THE EAST LINE OF SAID 1/4 1/4, A DISTANCE OF 1316.29 FEET, TO THE NORTHEAST CORNER OF SAID 1/4 1/4; THENCE S.87°-50'-28"W. ALONG THE NORTH LINE OF SAID 1/4 1/4, A DISTANCE OF 1310.86 FEET, TO THE NORTHWEST CORNER THEROF; THENCE N 02°-29'-52"W. ALONG THE WEST LINE OF SAID SECTION 4, A DISTANCE OF 1285.34 FEET, TO THE NORTHWEST CORNER THEREOF, BEING THE POINT OF BEGINNING. BEARINGS USED AGREE WITH OTHER PLATTING IN LEHIGH ACRES, AND USED FOR DESCRIPTION PURPOSES AND CALCULATIONS.

KNOW ALL MEN BY THESE PRESENTS, THAT LEHIGH ACRES DEVELOPMENT CORPORATION, A CORPORATION UNDER THE LAWS OF FLORIDA, BEING THE OWNER OF THE LAND DESCRIBED IN THE CAPTION HEREON HAS CAUSED THIS PLAT OF WILLOW LAKE ADDITION 1 LEHIGH ACRES TO BE MADE AND DO HEREBY DEDICATE TO THE PERPETUAL USE OF THE PUBLIC STREETS, ROADS, DRIVES AND/OR LANES SHOWN HEREON. ALL CANAL RIGHTS OF WAY ARE HEREBY DEDICATED TO THE EAST COUNTY WATER CONTROL DISTRICT.

IN WITNESS WHEREOF GERALD H. GOULD AND ARTHUR KESSLER RESPECTIVELY PRESIDENT AND ASST. SEC. OF LEHIGH ACRES DEVELOPMENT CORPORATION, BY AND WITH AUTHORITY OF IT'S BOARD OF DIRECTORS HAVE EXECUTED THESE PRESENTS FOR AND ON BEHALF OF SAID CORPORATION AND CAUSED IT'S CORPORATE SEAL TO BE AFFIXED THIS 12th DAY OF May A.D. 196

LEHIGH ACRES DEVELOPMENT CORPORATION

BY Gerald H. Gould
President

ATTEST Arthur Kessler
Asst. Sec.

STATE OF FLORIDA
COUNTY OF LEE

I HEREBY CERTIFY THAT ON THIS DAY, BEFORE ME AN OFFICER DULY AUTHORIZED TO TAKE ACKNOWLEDGEMENTS, PERSONALLY APPEARED GERALD H. GOULD AND ARTHUR KESSLER RESPECTIVELY PRESIDENT AND ASST. SEC. OF LEHIGH ACRES DEVELOPMENT CORPORATION TO ME KNOWN TO BE THE PERSONS DESCRIBED IN AND WHO EXECUTED THE FOREGOING DEDICATION AS PRESIDENT AND ASST SEC. RESPECTIVELY OF LEHIGH ACRES DEVELOPMENT CORPORATION FOR THE PURPOSES THEREIN SET FORTH.

WITNESS MY HAND AND OFFICIAL SEAL AT FORT MYERS, SAID COUNTY AND STATE THIS 12th DAY OF May A.D. 1964

Lar Liebler
NOTARY PUBLIC STATE OF FLORIDA AT LARGE

My Commission Expires

ALL BEARINGS ARE FROM THE TRUE MERIDIAN BASED ON OBSERVATION OF POLARIS AT S.W. CORNER OF SECTION 30, TOWNSHIP 44, RANGE 27, LEE COUNTY, FLORIDA

APPROVALS

THIS PLAT APPROVED THIS 13 DAY OF MAY A.D. 1964 IN OPEN MEETING OF THE BOARD OF COUNTY COMMISSIONERS OF LEE COUNTY, FLORIDA.

APPROVED
Chairman
By D. B. Starnes
Clerk

Frank K. Parsons
County Attorney

County Engineer

I HEREBY CERTIFY THAT THIS PLAT OF WILLOW LAKE ADDITION 1 LEHIGH ACRES, HAS BEEN EXAMINED BY ME AND FROM MY EXAMINATION I FIND THAT SAID PLAT COMPLIES IN FORM WITH THE REQUIREMENTS OF SURVEY LAWS OF FLORIDA.

I FURTHER CERTIFY THAT SAID PLAT WAS FILED FOR RECORD AT FORT MYERS THIS 18 DAY OF May A.D. 1964, AND DULY RECORDED IN PLAT BOOK 18 ON PAGE 155-162 OF PUBLIC RECORDS OF LEE COUNTY, FLORIDA.

D. B. Starnes
Clerk Of The Circuit Court in And For Lee County

CERTIFICATE OF SURVEY

I HEREBY CERTIFY THAT THIS PLAT IS A CORRECT REPRESENTATION OF A SURVEY MADE UNDER MY DIRECTION AND THAT PERMANENT REFERENCE MONUMENTS HAVE BEEN PLACED AS SHOWN.

R. V. Jamme
FLORIDA REGISTERED LAND SURVEYOR No. 1198

THE OWNER DOES HEREBY DEDICATE TO THE PERPETUAL USE OF THE EAST COUNTY WATER CONTROL DISTRICT THE CANAL RIGHTS OF WAY SHOWN HEREON AND ALL PRESENT AND FUTURE WORKS BUILT OR CONSTRUCTED WITHIN THESE RIGHTS OF WAY SHALL BE OWNED AND MAINTAINED BY SAID EAST COUNTY WATER CONTROL DISTRICT.

⊙ P.R.M., 2 MONUMENTS, BOTTOM 4"x 4"x 18" WITH 3/8" STEEL ROD, TOP 4"x 4"x 18" CONC.
• MONUMENT 4"x 4"x 24" CONCRETE x WITNESS MONUMENT 4"x 4"x 24" CONCRETE.

PART IV #26-16

Recommended Action

Engineering Review:

By: _____ 2/10/2026 **Approved** _X_ **Denied** __

District Field Review:

By: _____ **Approved** ___ **Denied** ___

District Manager: _____ **Date**: _____

Comments:

The referenced permit application for a boring alongside Sound Boundary Canal (47-33-5) starting at 101 Richmond Ave S, in Lehigh Acres, submitted by Comcast, has been reviewed and is recommended for approval with the following stipulations:

1. Permanent above-ground cable markers must be placed at the canal right-of-way (R/W) line on each side of the crossing.

2. Any disturbed canal R/W must be restored with sod and maintained until fully established. Seeding will not be permitted.

3. Within 30 days of completing the improvements, provide LA-MSID with a copy of the survey record drawings indicating the location and elevation of utilities within the canal R/W.

4. The permittee will be responsible for any necessary repairs, alterations, or relocation of the utility if required for future maintenance or repairs of the canal.

5. Lee County DOT must review and approve any utility work proposed within the County Road R/W.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: PER #26-17 Comcast Bore under Dog Canal (47-12-2)

Meeting Date: March 16, 2026

Agenda Item # 2H

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve PER #26-17 Comcast Bore under Dog Canal (47-12-2), with any listed stipulations.

What the action accomplishes: Allows Comcast to install a fiber cable under Dog Canal (47-12-2) at NW 16th Ter and Hines Ave

Fiscal Impact: Permit Fees: $2,100.00

Staff comments: Approved according to the revised plans submitted February 9, 2026.

Staff review and recommended approval:

[x] **MANAGER** [ ] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

LEHIGH ACRES - MUNICIPAL SERVICES IMPROVEMENT DISTRICT
PERMIT

THIS PERMIT is granted this 16th day of March, 2026, by Lehigh Acres Municipal Services Improvement District (hereinafter referred to as "DISTRICT"), 601 East County Lane, Lehigh Acres, Florida 33936, to Comcast Cable Communications, 12600 Westlinks Drive, Fort Myers, FL 33913, hereinafter referred to as the "Permittee") as a non-exclusive permit for a proposed bore under Dog Canal (47-12-2), at NW 16th Terrace and Hines Ave., in Lehigh Acres, FL.

W I T N E S S E T H:

1. In the event the DISTRICT is not the fee title owner of the real property (the "Property") to which this Permit is applicable, the Permittee agrees, prior to commencement, to obtain all necessary consents from the fee title owners of the Property.

2. This Permit does not constitute a waiver of the necessity by the Permittee to obtain such other necessary and appropriate permits required by other governmental bodies or agencies; therefore the Permittee, prior to commencement, is required to obtain any and all other applicable federal, state, and local permits required in connection with Permittee's use of the Property and agrees that at all times it will comply with the requirements of all federal, state and local laws, ordinances, rules and regulations applicable or pertaining to the use of the Property by the Permittee.

3. Permittee understands and agrees that the use of the Property pursuant to this Permit is subordinate to the rights and interests of the DISTRICT and to the extent applicable that of the fee title owner of the Property. Further, Permittee does hereby stipulate that the Permittee is not relying upon any representations whatsoever by the DISTRICT regarding the DISTRICT'S right, title, interest, or ownership as to the Property for which this Permit is applicable.

4. The Permittee, by accepting this permit, specifically agrees to allow authorized District Personnel, upon presentation of credentials or other documents as may be required by law, access to the premises, at reasonable times, where the permitted activity is located or conducted for the purpose of:
    a. Having access to and copying any records that must be kept under the conditions of the permit:
    b. Reviewing the facility, equipment, practices, or operations regulated or required under this permit; and
    c. Sampling or monitoring any substances or parameters at any location reasonably necessary to assure compliance with this permit or District rules.

5. The DISTRICT specifically reserves the right to maintain its facilities located over, under or upon the Property; to make improvements; add additional facilities; maintain, construct or alter its roads; maintain any facilities, devices, or improvements on the Property which aid in or are necessary to the DISTRICT'S operation; and the right to enter upon the Property at all times for such purposes. Permittee understands that in the exercise of such rights and interest, the DISTRICT may require Permittee, at no cost to DISTRICT, to relocate, alter, or remove the Permittee's facilities and equipment or other improvements made by Permittee pursuant to this Permit which interfere with or prevent the DISTRICT, in the DISTRICT'S opinion, from properly and faithfully constructing, improving and maintaining its facilities. If the Permittee should fail to relocate, alter, or remove the Permittee's facilities, equipment or other improvements made by the Permittee pursuant to this Permit within a reasonable time period following receipt of notice from the DISTRICT requesting same, then in that event the DISTRICT shall have the right to enter upon the lands and make such relocation, alterations or removal of the Permittee's facilities, equipment/or other improvements at no cost to DISTRICT.

6. Permittee agrees that it will not use the Property in any manner which interferes with the DISTRICT'S

use of the Property or causes a hazardous condition to exist.

7. The DISTRICT assumes no responsibility for the ownership, operation and/or maintenance of the Permittee's facilities as permitted herein.

8. The Permittee agrees to backfill any excavation it makes within the Property and to repair or replace (i) any below-grade authorized facilities and features located therein, including but not limited to: water, sewer, stormwater or irrigation facilities, structures or appurtenances and (ii) as to at or above-grade facilities and features, this requirement shall only be applicable to grasses and sod, sidewalks, and roadway improvements (the term roadway improvements shall include but not be limited to curbing, roadway base, sub grade and asphaltic or concrete surfaces) which are removed or damaged as a direct result of said excavation. Such repair or replacement shall substantially restore these features to their condition as it existed immediately prior to the event precipitating the excavation.

9. All below-grade Permittee installations, including water and sewer lines, whether sub aqueous or not, together with appurtenant facilities, shall be located underground and have a minimum ground cover of thirty inches (30") below original design profile grade, including District waterways and roadways.

10. All Permittee activity related to sub aqueous utility, water and/or sewer lines, including appurtenant facilities, shall be conducted in such fashion that the primary functional purpose of the canal or lake body facility, specifically flood control, within which the activity is taking place is not adversely affected. More specifically, sub aqueous construction shall conform to the criteria given in the Lehigh Acres Municipal Services Improvement District (Lehigh Acres – MSID) Administrative Policy and Procedures.

11. Permittee shall, at no expense to DISTRICT and within a reasonable time following notice, adjust the positions and elevations of its facilities as may be required in connection with future improvements to, or construction of, works of the DISTRICT.

12. Permittee does hereby indemnify and hold harmless the DISTRICT, its Board of Commissioners, officers, personnel, and it's Engineers against any claims, losses, damages (including consequential), expenses, or legal fees that might arise out or result from the implementation of the proposed project by the Permittee or that may be caused by a failure of DISTRICT facilities or property.

13. The Permittee shall be obligated at the time of submission of its Permit application, to provide a detail of any other existing improvements located over, under, upon or through the Property and the Permittee's accompanying plans and specifications shall clearly indicate how the Permittee will comply with required clearances between the requested permitted activity and existing improvements located over, under and upon the Property, including compliance with other permitted users duly adopted and published standards.

14. Permittee shall reimburse the DISTRICT for its legal, engineering and other expenses incurred as a result of the implementation of the proposed project.

15. If Permittee should violate any of the terms or conditions of the Permit and shall not correct or remedy same within ten (10) days of receiving written notice of said violation from the DISTRICT, then in that event, the DISTRICT may, at its option, revoke, cancel and terminate this Permit.

16. This Permit may not be assigned by the Permittee without the prior written approval of the DISTRICT.

17. Unless otherwise adjusted, modified or terminated as set forth hereinabove, including the Conditions attached hereto, this Permit shall continue in full force and effect so long as Permittee complies with the

terms of this Permit.

18. In any litigation, including breach, enforcement or interpretation arising out of this Permit, the prevailing party to this Permit shall be entitled to recover reasonable attorney's fees, costs and expenses related to said action from the non-prevailing party.

19. That attached hereto are Supplemental Conditions to this Permit which are incorporated herein and made a part hereof.

LEHIGH ACRES – MUNICIPAL SERVICES IMPROVEMENT DISTRICT

BY: _____

NAME TYPED: David E. Lindsay

TITLE: District Manager

SUPPLEMENTAL CONDITIONS TO PERMIT NO. 26-17

A. GENERAL CONDITIONS

1. Notification shall be given by the Permittee to the District Engineer, AIM Engineering, at 239-332-4569 forty-eight (48) hours prior to commencement of the permitted activity. The District Engineer shall establish points of construction, if applicable, which will require inspection by the District Engineer prior to construction of the permitted activity. When the Permittee considers the permitted activity complete, the Permittee shall notify the District Engineer and schedule an on-site final inspection which shall be held in the presence of a representative of the Permittee and the District Engineer.

2. The permitted activity shall be implemented, constructed and/or installed in full accordance with the approved plans and specifications within six (6) full calendar years from the date of permit approval. Deviations from the approved plans or specifications shall be coordinated with and approved by the District Engineer prior to implementation of any such deviation.

3. When working in a District road right-of-way, not more than one-half (½) of the road or street shall be closed and traffic shall be controlled so as to provide for the safety and minimum hindrance to the public. All traffic control operations shall conform to the most current issue of the Florida Department of Transportation publication "Manual on Traffic Controls and safe Practices for Street and Highway Construction, Maintenance and Utility Operations."

4. The District Engineer shall be the final authority as to the quality and quantity of the material and works required to satisfy the terms and conditions of this Permit as they relate to District improvements and facilities.

5. Upon completion of the permitted activity and after its final review and acceptance by the District Engineer THE PERMITTEE SHALL DELIVER TO THE DISTRICT OFFICE ONE COMPLETE SET OF THE FINAL "RECORD DRAWINGS." FAILURE TO PROVIDE THE FINAL RECORD DRAWINGS MAY RESULT IN THE REVOCATION, CANCELLATION AND TERMINATION OF THIS PERMIT.

6. The above Conditions shall be continuing obligations of the Permittee and shall be complied with in the event of any subsequent maintenance, repair, replacement or modification of the permitted improvement.

Stipulations:

1). Permit #26-17 is approved according to the plans submitted February 9th, 2026.

2). The proposed bore must maintain a minimum clearance of 10’ below the bottom elevation of the canal or the lowest pipe invert, whichever is lower, for the entire width of the canal right of way.

3). Permanent utility markers, (at least 4’ high) with the utility companies contact information clearly labeled on it, must be installed at each canal right of way line over top the proposed line.

4). All underground vaults, pull boxes, cable cabinets, etc. must be located outside of the canal right of way and not interfere with the District’s canal maintenance activities.

5). All canal R/W that is disturbed must be restored with sod and maintained until well established (no seeding will be allowed).

6). Boring contractor is not allowed to pump water out of the LAMSID canals for their boring operations unless they apply for and receive a water use permit from LAMSID.

7). The Permittee will be responsible for any repairs, alterations, or relocation of the utility if required for repairs or maintenance of the canal in the future.

8). Provide LAMSID with a copy of your survey record drawings, showing where your utilities are located within the canal R/W and at what elevation they are located, within 30 days following completion of the improvements.

9). Lee County DOT must review and approve the utility work that is outside of the canal right of way along Hines Ave. and Hendry County DOT must review and approve the utility work in Hendry County along NW 16th Terrace.

PART I

DATE SUBMITTED: 02 - 09 - 26

PERMIT #: 26 - 17

APPLICATION FOR A PERMIT

*Note: The review of permit applications will be conducted per the Lehigh Acres Municipal Services Improvement District's Administrative Policy and Procedures

AGENT NAME: Joanie George **PHONE #:** (904)769-9064

PROJECT NAME: JB0002402546-7897 16TH TER

TOTAL ACREAGE: 0.002

Property Address (STRAP associated with this address will be used as the primary STRAP for this project): 1617 HINES AVE, LEHIGH ACRES

Mailing Address:	NA				
City:	LEHIGH ACRES	**State:**	FL	**Zip:**	33972

STRAP NUMBERS (*attach additional sheet if necessary*):

12-44-27-L3-03011-0210

Brief description of proposed project:

Application for: Comcast proposes to bore 90' in LAMSID ROW crossing Dog Canal Placing 4" HDPE conduit with 24ct FOC

Is the project proposed to be developed in phases? ☐ Yes ☒ No

Estimated # of Phases:	
***Permit Fee:**	$ 2,100.00 **(to be verified by staff)**

* See Administrative Policy and Procedures Sections 4.15 – 4.17 for current Permit Fee Amount.

PART II

OWNER/APPLICANT/ DEVELOPER INFORMATION

A.

Name of Applicant	Comcast Cable Communications – Marc Louis						
Mailing Address:	Street:	12600 WESTLINKS DR #4					
City:	Ft. Myers	State:	FL		Zip:	33913	
Phone No.	Area Code:	239	Number:	986-7043	Ext.		
Fax No.	Area Code:		Number:	-			
E-mail Address:							

B. Relationship of applicant to property*:

☐ Owner* ☐ Trustee* ☐ Option holder* ☐ Lessee*

☐ Contract Purchaser* ☒ Other (indicate)* UTILITY

C.

Name of Developer:						
Mailing Address:	Street:					
City:		State:			Zip:	
Phone No.	Area Code:	( )	Number:	-	Ext.	
Fax No.	Area Code:	( )	Number:	-		
E-mail Address:						

D.

Name of Authorized Agent(s):	Ashley Woods					
Mailing Address:	Street:	13277 NE 235th CT				
City: Raiford		State:	FL		Zip:	32083
Phone No.	Area Code:	(904)	Number:	769-9064	Ext.	
Fax No.	Area Code:	( )	Number:	-		
E-mail Address:						

E.

Name of Property Owner:						
Mailing Address:	Street:					
City:		State:			Zip:	
Phone No.	Area Code:	( )	Number:	-	Ext.	
Fax No.	Area Code:	( )	Number:	-		
E-mail Address:						

*A disclosure of the name of all persons or entities having an ownership interest in the property is required (attached).

PART III

DISCLOSURE OF OWNERSHIP INTEREST FORM FORM:

PRIMARY STRAP NUMBER: | | | - | | | - | | | - | | | - | | | | | - | | | | |

1. If the property is owned in the fee simple by an **INDIVIDUAL**, tenancy by the entirety, tenancy in common, or joint tenancy, list all parties with an ownership interest as well as the percentage of such interest.

Name and Address	Percentage of Ownership

2. If the property is owned by a **CORPORATION**, list the officers and stockholders and the percentage of stock owned by each.

Name, Address, Office	Percentage of Stock

3. If the property is in the name of a **TRUSTEE**, list the beneficiaries of the trust with percentages of interest.

Name and Address	Percentage of Interest

4. If the property is in the name of a **GENERAL PARTNERSHIP OR LIMITED PARTNERSHIP**, list the names of general and limited partners.

Name and Address	Percentage of Interest

5. If there is a **CONTACT FOR PURCHASE**, whether contingent on this application Or not, and whether a Corporation, Trustee, or Partnership, list the names of the contract purchasers below, including the officers, stockholders, beneficiaries, or partners.

Name and Address	Percentage of Interest

Date of Contract (mm/dd/yy): / /

6. If any contingency clause or contract terms involve additional parties, list all individuals or officers, if a corporation, partnership, or trust.

Name and Address	Percentage of Interest

For any changes of ownership of changes in contracts for purchase subsequent to the date of the application, but prior to the date of final certificate for compliance, a supplemental disclosure of interest must be filed.

The above is a full disclosure of all parties of interest in this application, to the best of my knowledge and belief.

Joanie George
SIGNATURE OF APPLICANT

Joanie George
PRINTED OR TYPED NAME OF APPLICANT

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this 20 day of Jan (m), 2026(y), by Joanie George who is personally know to me or has produced _____ as identification.

Notary Public State of Florida
Ashley Woods
My Commission HH 337625
Expires 12/1/2026

Ashley Woods
SIGNATURE OF NOTARY PUBLIC

Ashley Woods
PRINTED NAME OF NOTARY PUBLIC

PART IV

EXPLANATION

All Permit Application sections must be completed and submitted.

Permit Application fees and submittal items (see Part VI) must be submitted with the application.

Any and all work involving the lands and/or facilities of the District shall not commence without a Board approved permit.

All set policies, procedures, technical requirements, and fee schedules can be found and reviewed in the 'Administrative Policy and Procedures Guide'. The Guide is available at the District Office located at 601 East County Lane, Lehigh Acres, FL or online at la-msid.com/.

PART VI

SUBMITTALS

NO. OF COPIES		SUBMITTAL DOCUMENT
SUB'D	REQ'D	
		ITEMS REQUIRED FOR INITIAL SUBMITTAL
	2	**Construction/Development Plans** signed and sealed by a Licensed Florida Professional Engineer including: 1. Location map showing the location of the proposed project in relation to adjacent streets and LA-MSID facilities. 2. Details of proposed works that fully communicate the work and/or use desired (i.e. Existing Conditions Survey, Proposed Site Plan, Paving Grading & Drainage Plan, Cross Sections, Element Details, and Erosion Control Plan sheets). 3. Sufficient survey data to determine elevations, property and ROW lines, limits of existing and proposed conditions, and notation of utilized vertical datum (NAVD/NGVD).
	2	**Stormwater Drainage Calculations** signed and sealed by a Licensed Florida Professional Engineer including, but not limited to, computer stormwater routing modeling data (ICPR4, Cascade, etc.) to verify compliance with the Design Criteria (input summary data below): **Water Quality Volume (Required):** _____(ACRE-FT) **Water Quality Volume (Provided):** _____(ACRE-FT) **Control Elevation:** _____(FT-NGVD) **Minimum Road/Parking Elevation:** _____(FT-NGVD) *(above the 10-year, 1-day design storm peak stage elevation)* **Minimum Perimeter Berm Elevation:** _____(FT-NGVD) *(above the 25-year, 3-day design storm peak stage elevation)* **Minimum Finished Floor Elevation:** _____(FT-NGVD) *(above the 100-year, 3-day design storm peak stage elevation and previously experienced high-water levels external to project)*
	2	**Flash Drive / CD** containing plans in both AutoCAD (.dwg file drawn in "State Plane Coordinates") and PDF formats.
	2	**Permit Application** (PARTS I & II)
	2	**Disclosure of Owner Form (signed and notarized)** (PART III)
	2	**Professional Consultant Form & Submittals** (PARTS V & VI)
	2	**Copy of Approved FDEP Notice of Intent (NOI)** for all sites one (1) acre and greater in size.
	2	**Copy of approved SWFMD permit**, for all sites required to get a permit, **or FDEP "10/2 Self-Certification"** approval.
	1	**Check for appropriate Permit Fees** made out to LA-MSID.

PART V

PROFESSIONAL CONSULTANT FORM

ATTORNEY

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:		-	Ext.	
Other Contact:							

ENGINEER

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:		-	Ext.	
Other Contact:							

OTHER REPRESENTATIVE/PROFESSIONAL

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:		-	Ext.	
Other Contact:							

TrueNet Communications Corp

060850

~~LEHIGH ACRES MUNICIPAL SERVICE - IMPROVEMENT DISTRICT~~

DATE	INVOICE NO.	DESCRIPTION	INVOICE AMOUNT	DEDUCTION	BALANCE
1/20/2026	01212026	JB0002402546 - 7897 16TH TERR	1,200.00		1,200.00
CHECK DATE		CHECK NUMBER		TOTALS			

RECEIVED
FEB 09 2026
LEHIGH ACRES
MUNICIPAL SERVICES
IMPROVEMENT DISTRICT

PLEASE DETACH THIS PORTION AND RETAIN FOR YOUR RECORDS 9321

DOCUMENT IS PRINTED ON CHEMICALLY REACTIVE PAPER - THE BACK OF THIS DOCUMENT INCLUDES A TAMPER EVIDENT CHEMICAL WASH WARNING BOX

THE BACK OF THIS DOCUMENT INCLUDES MICROPRINTED ENDORSEMENT LINES

ORIGINAL DOCUMENT HAS RED KEYHOLE ICON THAT DISAPPEARS WITH HEAT

060850

TrueNet Communications Corp.
90 FORT WADE RD.
SUITE 230
PONTE VEDRA, FL 32081
(904) 777-9052
APINV@TrueNetCommunications.com

Bank of America
11-35/1210 GL

Pay: **********one thousand two hundred and 00/100

DATE	CHECK NO.	AMOUNT
January 22, 2026	60850	$*****1,200.00

PAY TO THE ORDER OF

LEHIGH ACRES MUNICIPAL SERVICE - IMPROVEMENT DISTRICT
601 EAST COUNTY LANE
LEHIGH ACRES FL 33936

VOID AFTER 90 DAYS

HEAT SENSITIVE COLOR FADES WITH HEAT

AUTHORIZED SIGNATURE

⑈060850⑈ ⑆121000358⑆ 001233592930⑈

#26-17

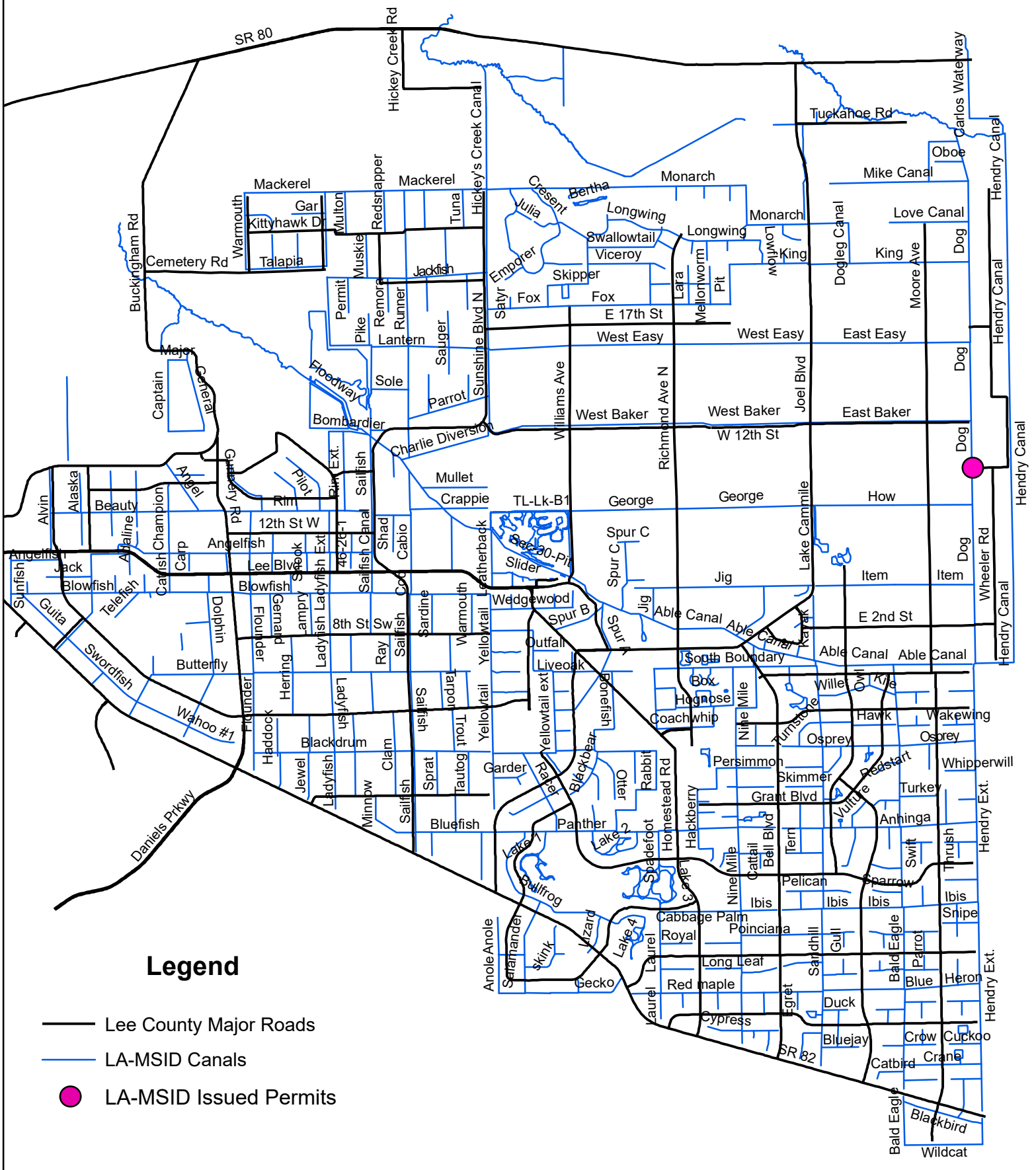
TrueNet Communications Corp

060897

LEHIGH ACRES MUNICIPAL SERVICE - IMPROVEMENT DISTRICT

DATE	INVOICE NO.	DESCRIPTION	INVOICE AMOUNT	DEDUCTION	BALANCE
2/2/2026	02102026	JB0002402546 - 7897 16TH TERR		900.00	900.00

Permit # 2026-17



Legend

- Lee County Major Roads
- LA-MSID Canals
- LA-MSID Issued Permits

7897 16TH TER
LA-MSID PERMIT
NOTES & TYPICALS

BINDING SPACE

LEE CO. AERIAL & U/G NOTES

ALL PROPOSED CONSTRUCTION WILL BE PER DOT SPECIFICATIONS AS WELL AS ALL NESC SAFETY CODES.

ALL PROPOSED CATV WILL BE WITHIN THE DOT RIGHT-OF-WAY.

ALL AERIAL CANAL RIGHT-OF-WAY CROSSINGS MUST BE A MINIUM HEIGHT OF 40' ABOVE THE TOP OF THE CANAL BANK

ALL PROPOSED CANAL RIGHT-OF-WAY BORES MUST BE A MINIMUM HEIGHT OF 10' (120") BELOW THE BOTTOM OF THE CANAL OR PIPE INVERT

ALL PROPOSED UNDERGROUND CATV WILL BE BURIES A MINIMUM OF 66" DEEP, EXCEPT BORES 48" DEEP

ALL PROPOSED UNDERGROUND CATV ROAD BORES WILL USE FDOT APPROVED DIRECTIONAL BORE MACHINE, AS WELL AS FDOT APPROVED CONDUIT

ALL PROPOSED CATV ROAD BORES WILL EXTEND A MINIMUM OF 8' BEYOND THE EDGE OF PAVEMENT

ALL PROPOSED CATV DRIVEWAY BORES WILL EXTEND A MINIMUM OF 4' BEYOND THE EDGE OF PAVEMENT

ALL PROPOSED CATV WILL BE .625" OR .875" DIAMETER, INSULATED COAXIAL CABLE OR 1.0" DIAMETER, INSULATED FIBER OPTIC CABLE, AND WILL BE LASHED TO .250" STEEL STRAND CABLE USING .125" STD. GALV. LASHING WIRE

ALL PROPOSED AERIAL CATV CROSSINGS (BOTH ROADWAYS AND DRIVEWAYS) WILL HAVE MINIMUM MID-SPAN HEIGHT OF 18'-0".

ALL PAEALLEL PROPOSED AERIAL CATV WILL HAVE MINIMUM MID-SPAN HEIGHT OF 16'-6" AS PER NESC SPECIFICATIONS.

LOCATES WILL BE REQUIRED IN ALL PROPOSED UNDERGROUND AREAS AT LEAST 48 HRS. PRIOR TO CONSTRUCTION.

NOTIFICATIONS TO ALL UTILITIES INVOLVED WILL BE MADE PRIOR TO CONSTRUCTION.

GENERAL NOTES

MAINTENANCE OF TRAFFIC TO BE SUPERVISED BY A CERTIFIED PERSON.

TEST RESULTS ARE REQUIRED UPON REQUEST.

THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT A MINIMUM OF TWO BUSINESS DAYS PRIOR TO ANY LANE CLOSURES OR BEGINNING ANY CONSTRUCTION WITHIN THE DOT RIGHT-OF-WAY.

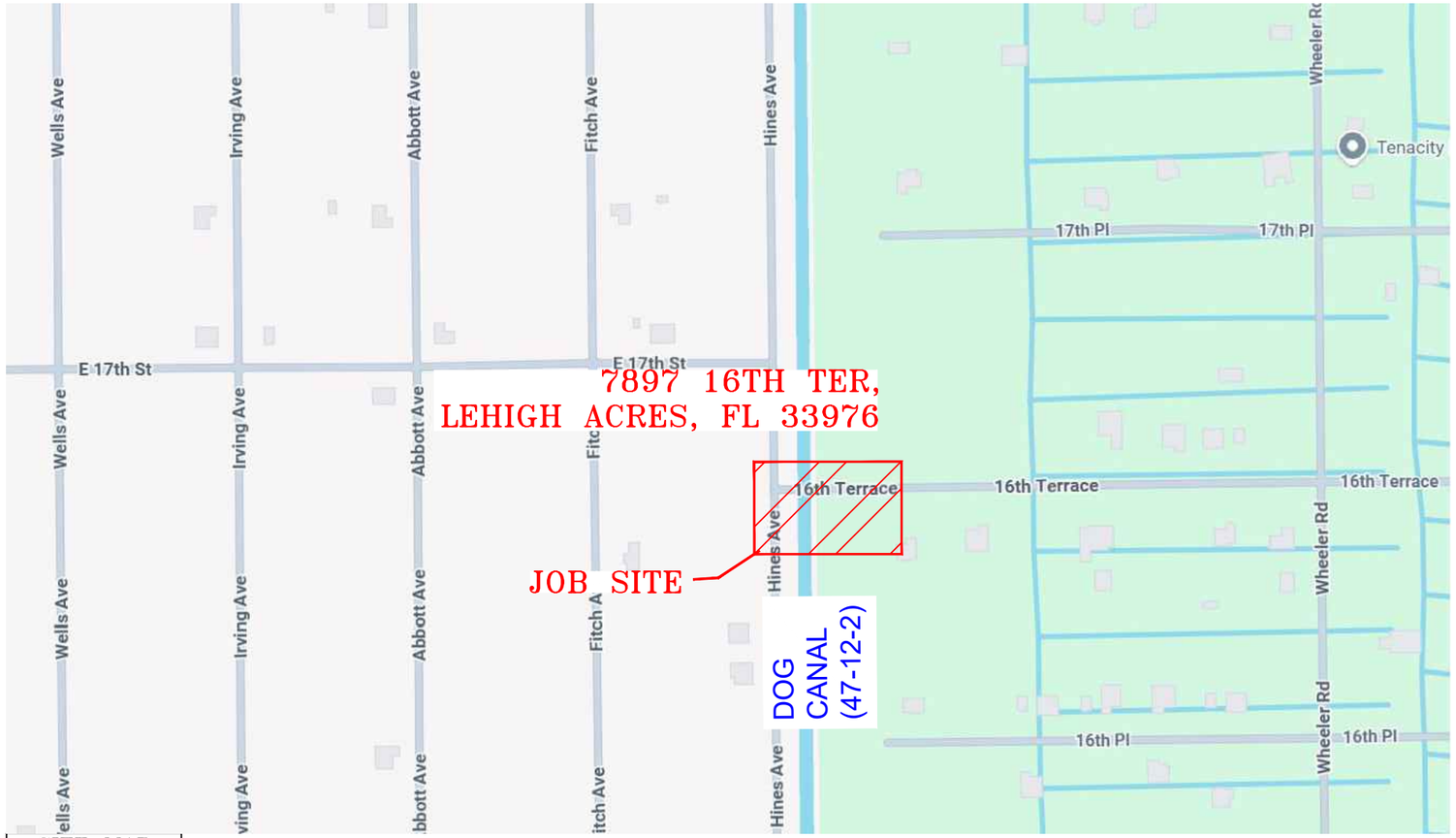
ALL WORK PERFORMED WITHIN THE DOT RIGHT-OF-WAY SHALL BE IN ACCORDANCE WITH DOT DESIGN STANDARDS, SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION AND THE UTILITY ACCOMODATION MANUAL, LATEST EDITIONS.

ENGINEER'S CERTIFICATION TOGETHER WITH AS-BUILTS, IF REQUIRED TO BE SUBMITTED WITHIN 30 DAYS OF COMPLETION.

ALL CONCRETE TO BE REMOVED SHALL BE SAW CUT AT THE NEAREST GOOD JOINT. GUTTER SHALL BE SAW CUT BETWEEN ASPHALT AND GUTTER BEFORE REMOVAL.

IT WILL BE THE RESPONSIBILITY OF THE PERMITTEE TO REPAIR ANY DAMAGE TO DOT FACILITIES CAUSED BY THE CONSTRUCTION OF THE PROJECT.

ALL CONCRETE SHALL BE 3,000 PSI. APPROVED DOT MIX DESIGN.

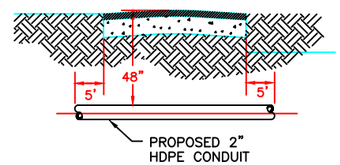


SITE MAP
SCALE: NONE

LEGEND

	BORE
	SWALE/DITCH
	GUARD RAIL
GAS	GAS
PWR	POWER
	U/G CATV
STLT	STREET LIGHT
SD	STORM DRAIN
SWR	SEWER
TELE	TELEPHONE
	TRAFFIC SIGNAL
W	WATER
	MISC U/G UTILITY
	PROPOSED U/G CATV
	PROPOSED AERIAL FIBER
	PROPOSED AERIAL CATV
℄	CENTERLINE
E/P	EDGE OF PAVEMENT
B/C	BACK OF CURB
R/W	RIGHT OF WAY

	POWER POLE
	POWER TRANSFORMER POLE
	JOINT USE TRANSFORMER POLE
	PHONE POLE
	CATV POLE
S	STEEL POLE
C	CONCRETE POLE
	JOINT USE POLE
R	RISER
	ANCHOR
	OVERHEAD GUY
V	CATV FIBER VAULT
V	TRAFFIC SIGNAL VAULT
	TRAFFIC SIGNAL POLE
	TRAFFIC SIGNAL BOX
A	CROSS SECTION REFERENCE
XX'	FTG BTWN REFERENCE POINTS
	REFERNCE POINT
	STORM DRAIN



TYPICAL DRIVEWAY DIRECTIONAL BORE
N.T.S.

PROJECT TOTALS

PROP. BURIED CATV:	155'
PROP. OH CATV:	0'
TOTAL PROP. CATV:	155' (0.03MI.)

MOT NOTES

TRAFFIC CONTROL SHALL COMPLY WITH FEDERAL MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) AND F.D.O.T. DESIGN STANDARD #NO. 600.

JB0002402546-7897 16TH TER-LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

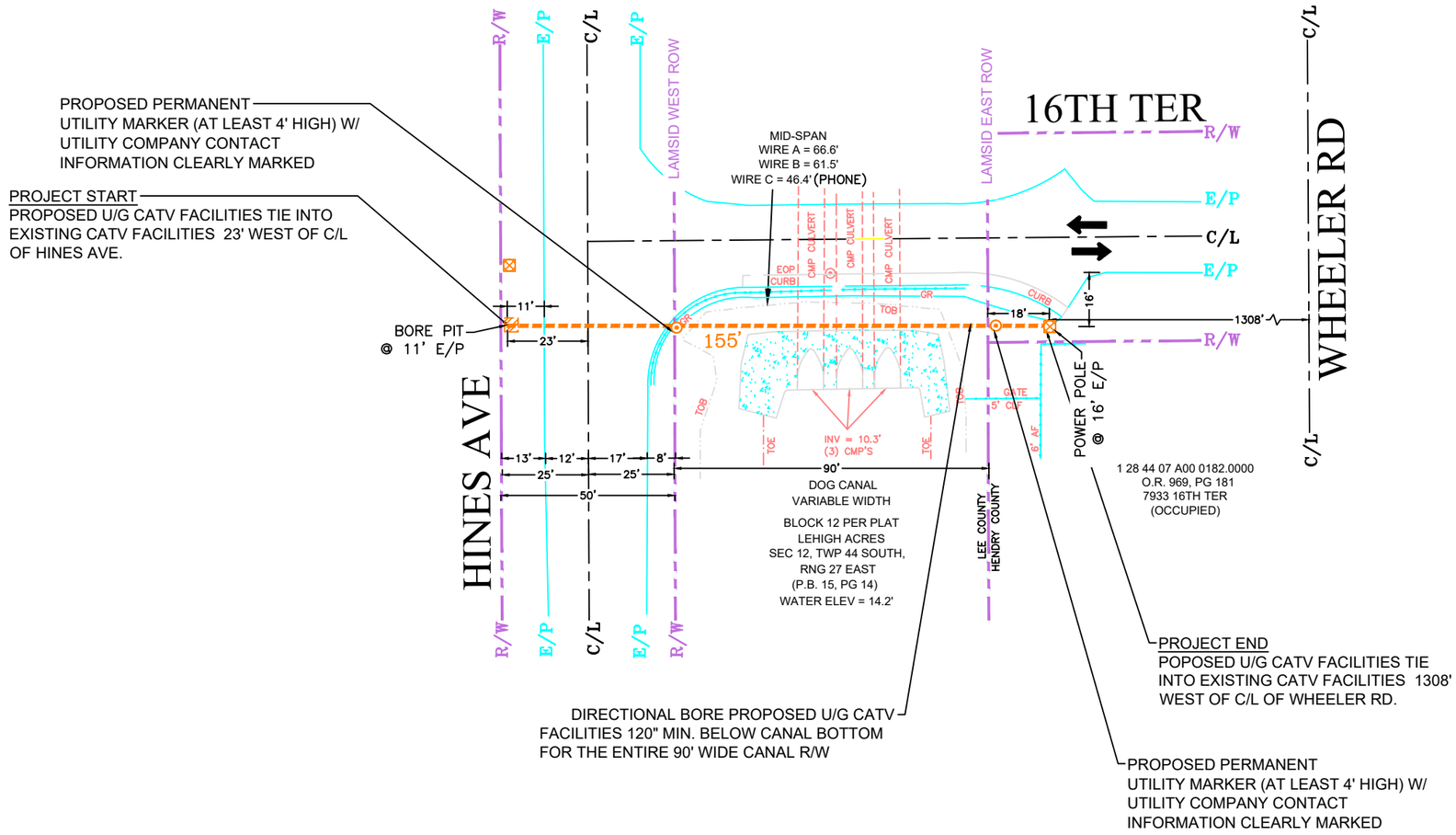
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED UG CATV FACILITIES ON 16TH TER.	COUNTY: LEE
	STATE: FLORIDA
	SHEET NO.: 1 OF 4
	DATE: 01/16/2026
CIFA: N/A	MAP: 26.661062, -81.565910
JB#: JB0002402546	DRAFTED BY: A. PERDOMO

7897 16TH TER
LA-MSID PERMIT
PLAN VIEW

0 20' 40'

BINDING SPACE



CONSTRUCTION NOTE:
ALL BORE WORK WILL BE HORIZONTAL DIRECTIONAL DRILL, NOT UNGUIDED BORING.

JB0002402546-7897 16TH TER-LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

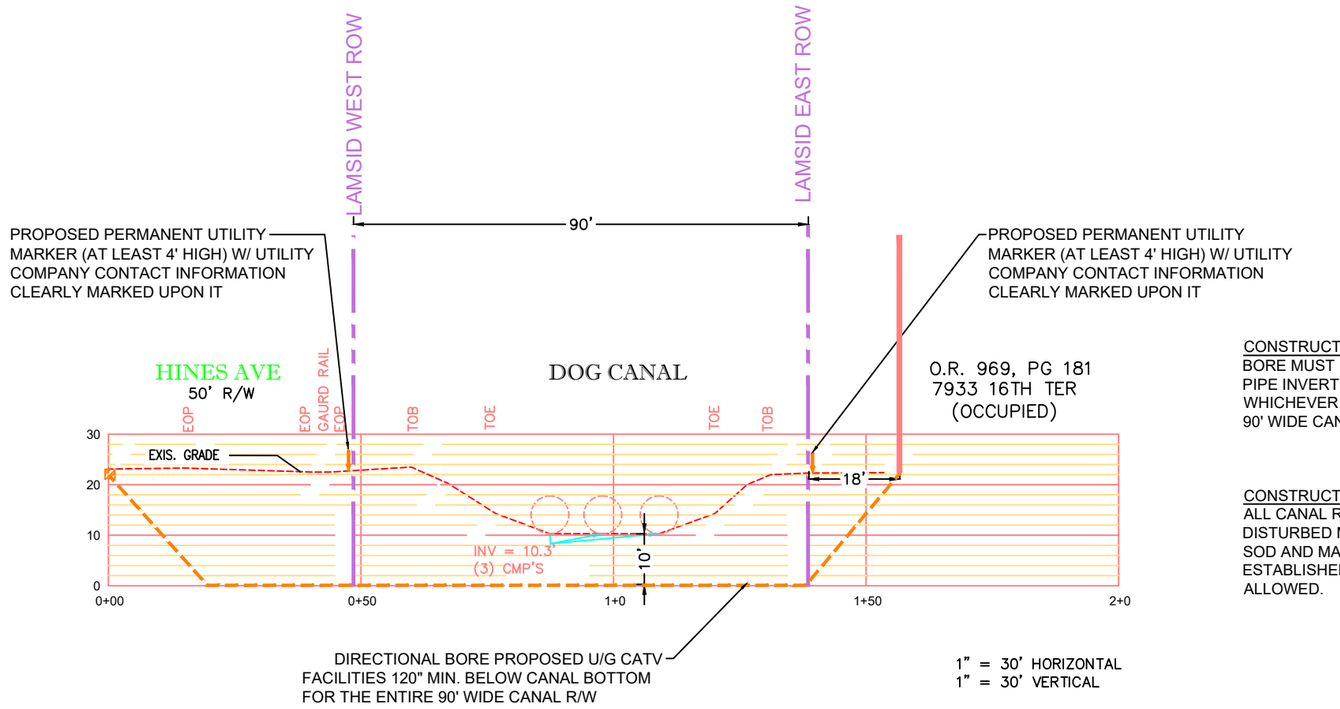
LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

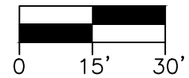
RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED UG CATV FACILITIES ON 16TH TER.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 2 OF 4
	DATE: 01/16/2026
CIFA: N/A	**MAP:** 26.661062, -81.565910
JB#: JB0002402546	**DRAFTED BY:** A. PERDOMO

7897 16TH TER
LA-MSID PERMIT
CROSS SECTION

BINDING SPACE





CONSTRUCTION NOTE:
BORE MUST BE 10' BELOW THE LOWEST PIPE INVERT OR THE CANAL BOTTOM, WHICHEVER IS LOWER, FOR THE ENTIRE 90' WIDE CANAL RIGHT OF WAY.

CONSTRUCTION NOTE:
ALL CANAL RIGHT OF WAY THAT IS DISTURBED MUST BE RESTORED WITH SOD AND MAINTAINED UNTIL WELL ESTABLISHED. NO SEEDING WILL BE ALLOWED.

JB0002402546-7897 16TH TER-LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED UG CATV FACILITIES ON 16TH TER.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 3 OF 4
	DATE: 01/16/2026
CIFA: N/A	**MAP:** 26.661062, -81.565910
JB#: JB0002402546	**DRAFTED BY:** A. PERDOMO

7897 16TH TER
LA-MSID PERMIT
SITE PHOTOS

BINDING SPACE



ROUTE LOOKING EAST



END POINT LOOKING SOUTH

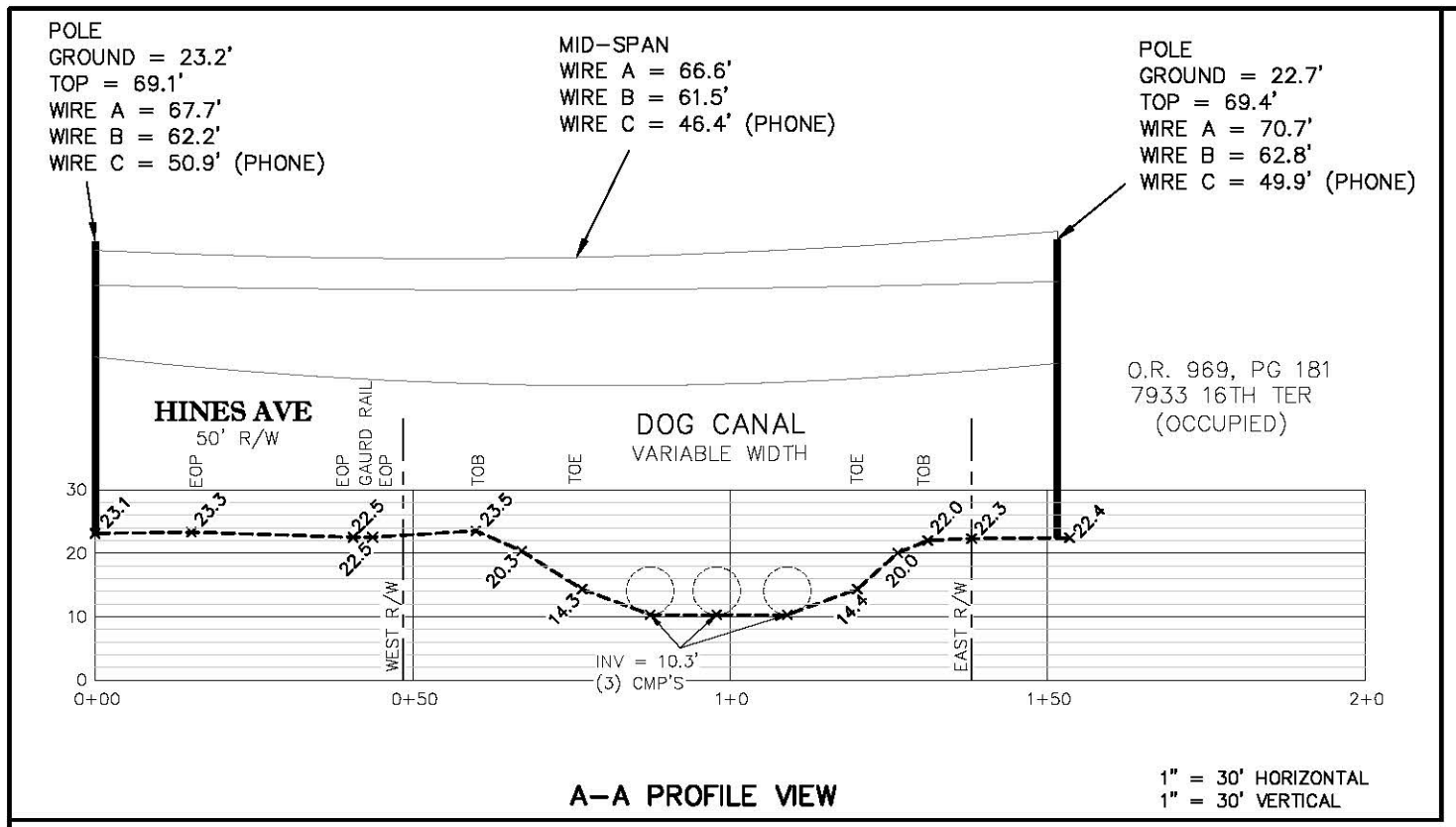
JB0002402546-7897 16TH TER-LAMSID

COMCAST
PROPRIETARY AND CONFIDENTIAL
12600 WESTLINKS DR., SUITE 4
FORT MYERS, FL 33913

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

RIGHT-OF-WAY PERMIT

SUBJECT: PROPOSED UG CATV FACILITIES ON 16TH TER.	**COUNTY:** LEE
	STATE: FLORIDA
	SHEET NO.: 4 OF 4
	DATE: 01/16/2026
CIFA: N/A	**MAP:** 26.661062, -81.565910
JB#: JB0002402546	**DRAFTED BY:** A. PERDOMO

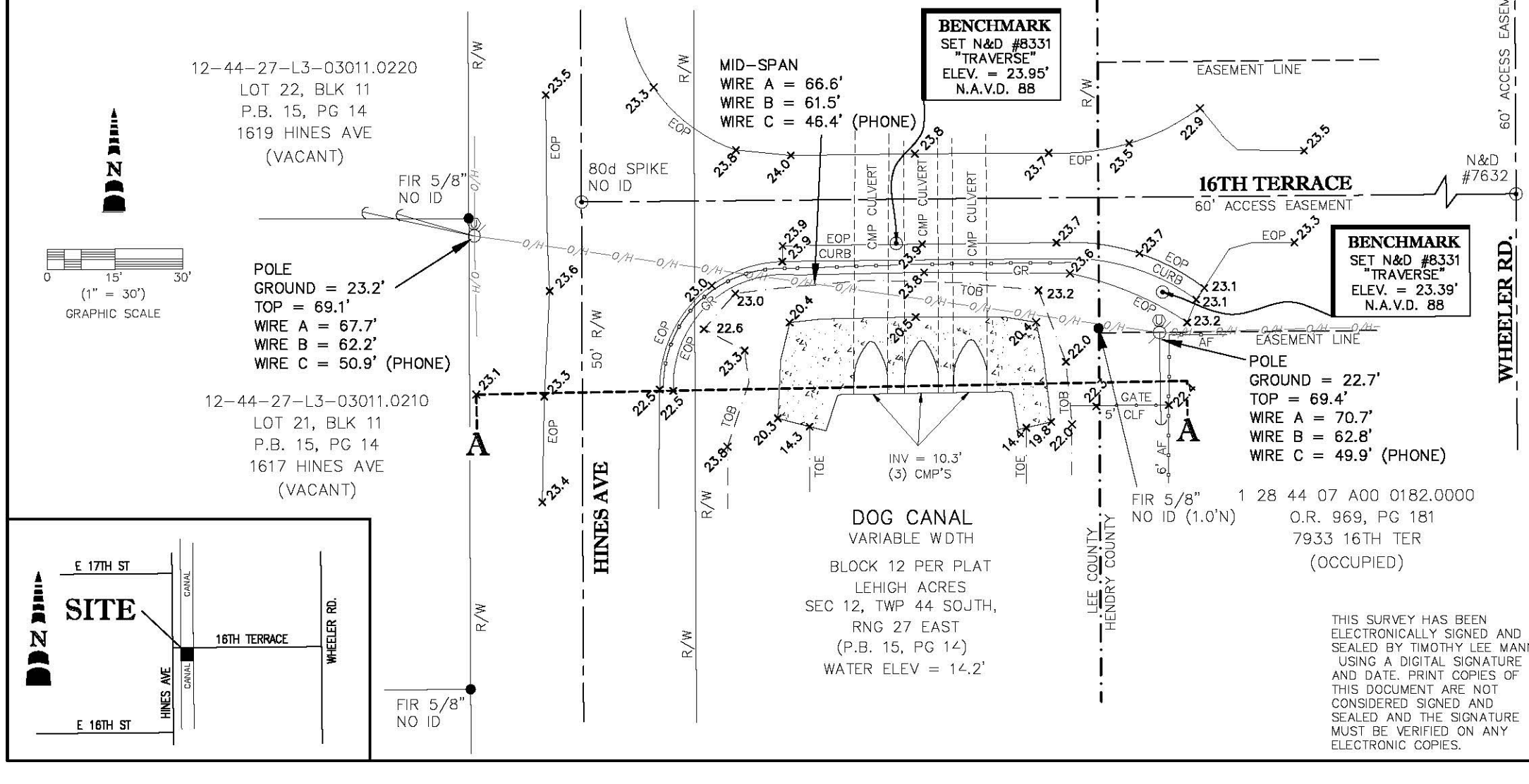


A–A PROFILE VIEW

1" = 30' HORIZONTAL
1" = 30' VERTICAL

LEGEND:

FIR = FOUND IRON ROD
N&D = NAIL AND DISC
ELEV = ELEVATION
TRAV = TRAVERSE
CONC = CONCRETE
D&UE = DRAINAGE & UTILITY EASEMENT
CMP = CORRUGATED METAL PIPE
EOP = EDGE OF PAVEMENT
R/W = RIGHT OF WAY
GR = GUARD RAIL
CLF = CHAIN LINK FENCE
AF = ALUMINUM FENCE
TOB = TOP OF BANK
TOE = TOE OF SLOPE
INV = INVERT
N.A.V.D. 88 = NORTH AMERICAN VERTICAL DATUM 1988
= TELEPHONE SERVICE BOX
= WOOD UTILITY POLE
= GUY WIRE ANCHOR
+28.7' = SPOT ELEVATION
—O/H— = OVERHEAD UTILITY LINES
= CONCRETE MAT/SPILLWAY



ASBUILT SURVEY

OF A PARCEL OF LAND LYING IN SECTION 12, TOWNSHIP 44 SOUTH, RANGE 27 EAST, LEHIGH ACRES, LEE COUNTY, FLORIDA

NOTES:

SURVEY BASED ON THE PLAT OF LEHIGH ACRES SECTION 12, TWP 44S, RNG 27E AS RECORDED IN PLAT BOOK 15, PAGE 14 OF THE PUBLIC RECORDS OF LEE COUNTY, FLORIDA, PROPERTY DESCRIPTIONS ON FILE IN THE PUBLIC RECORDS OF HENDRY COUNTY, FLORIDA AND EXISTING MONUMENTATION.

THIS PLAT PREPARED AS AN ASBUILT SURVEY FOR THE PURPOSE OF LOCATING THE FEATURES WITHIN AND ADJACENT TO THE CANAL CROSSING EAST OF HINES AVE AND SOUTH OF 16TH TERRACE, LEHIGH ACRES.

THE ELEVATIONS SHOWN HEREON ARE NORTH AMERICAN VERTICAL DATUM OF 1988 (N.A.V.D. 88) IN FEET AND DECIMALS THEREOF AND BASED UPON CONTINUOUSLY OPERATING FLORIDA PERMANENT REFERENCE NETWORK (FPRN) STATIONS MAINTAINED BY THE FLORIDA DEPARTMENT OF TRANSPORTATION.

PARCEL LIES WITHIN ZONE AE (ELEV 23.6' N.A.V.D. 88) AND A FLOODWAY INSURANCE RATE MAP 12071C 0377F, HAVING AN EFFECTIVE DATE OF 11/22/25.

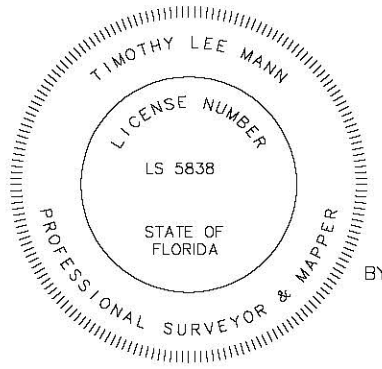
THE F.E.M.A. FLOOD ZONE INFORMATION INDICATED HEREON IS BASED ON MAPS SUPPLIED BY THE FEDERAL GOVERNMENT. THIS FLOOD INFORMATION MUST BE VERIFIED WITH ALL PERMITTING REGULATORY ENTITIES PRIOR TO COMMENCING ANY WORK OR APPLICATION DEPENDENT ON SAID FLOOD INFORMATION.

UNDERGROUND IMPROVEMENTS, UTILITIES AND/OR FOUNDATIONS WERE NOT LOCATED UNLESS OTHERWISE NOTED.

WETLANDS, IF ANY, WERE NOT LOCATED.

SUBJECT TO EASEMENTS, RESTRICTIONS, RESERVATIONS AND RIGHTS-OF-WAY (RECORDED AND UNRECORDED, WRITTEN AND UNWRITTEN).

DATE OF LAST FIELD WORK: 12-17-2025.



BY: TIMOTHY LEE MANN
PROFESSIONAL SURVEYOR AND MAPPER
FLORIDA CERTIFICATE NO. LS 5838

Timothy L Mann
Digitally signed by Timothy L Mann
Date: 2026.01.06 10:10:41 -05'00'

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

THIS SURVEY IS ONLY FOR THE LANDS AS DESCRIBED. IT IS NOT A CERTIFICATE OF TITLE, ZONING, EASEMENTS OR FREEDOM OF ENCUMBRANCES.

THIS SURVEY WAS PREPARED WITHOUT BENEFIT OF AN ABSTRACT OF TITLE AND ALL MATTERS OF TITLE SHOULD BE REFERRED TO AN ATTORNEY AT LAW.

THIS SURVEY HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY TIMOTHY LEE MANN USING A DIGITAL SIGNATURE AND DATE. PRINT COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

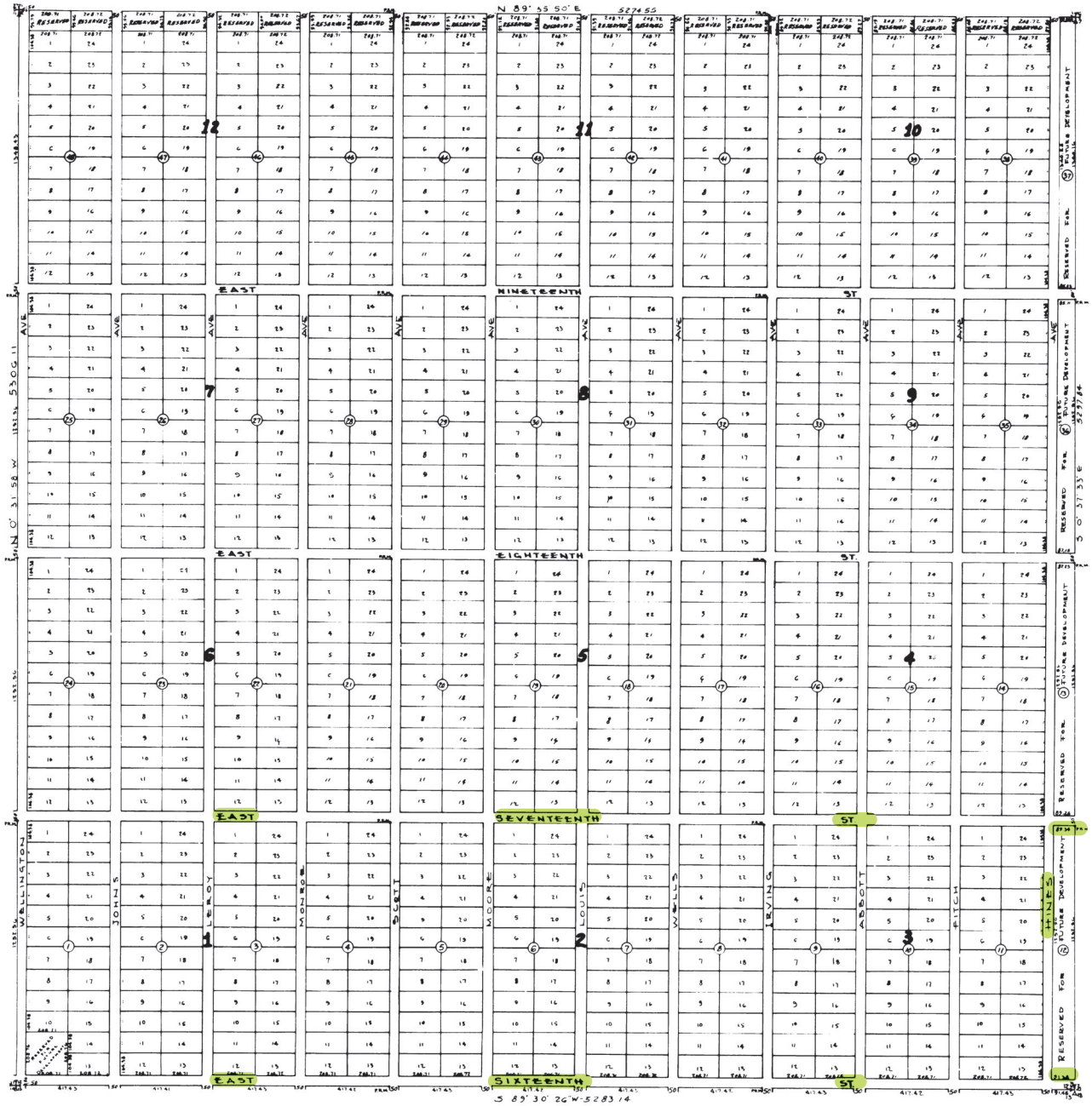
HINES AVE, LEHIGH ACRES

TITLE: ASBUILT SURVEY

KEYSTONE
A CLS COMPANY
www.keystoneconsultants.net
10970 S. CLEVELAND AVE.
SUITE #605
FORT MYERS, FLORIDA 33907
PHONE: (239) 275-8575
FAX: (239) 275-8457
LB #8331

FILE NAME:	FIELD BOOK/PAGE:	PROJECT NO.:	SHEET:
15747SR.DWG	SEE FILE	15747	1 OF 1

SURVEY DATE:	DRAWN BY:	SCALE:	CHECKED BY:	FILE NO. (S-T-R)
1/06/25	BUD	1"= 30'	TLM	12-44-27



PLAT OF SECTION 12

TWP 44 S RGE 27 E

A SUBDIVISION OF

"LEHIGH ACRES"

LEE COUNTY, FLA.

LEE COUNTY LAND & TITLE CO.
DEVELOPERS

SCALE 0 100 200 300 600

UNIT BOUNDARY

FEB. 16, 1956

CERTIFICATE OF SURVEY

I HEREBY CERTIFY that the plat shown hereon is a correct representation of the land as platted and that Permanent Reference Monuments have been placed as shown.

Earl J. Royer Jr. Fla.
Florida Reg. Land Surveyor # 675
Engineer # 1680

NOTE
ALL LOTS SUBJECT TO A 10 UTILITY EASEMENT ON ALL LOT LINES.
INTERIOR PRM'S = 4"x4"x24" CONCRETE
SEE COR PRM'S = 6"x6"x36" CONCRETE
ALL LOTS ARE RECTANGULAR, EXCEPT WHERE NOTED

NOTE
DIMENSIONS, BEARINGS, LOT AND LAND LINES ON DRAWING OF LATEST DATE SHALL GOVERN.

Petition to Vacate
Petition No:96-03-150.04R
Description:a 12ft wide public utility easement;
See CCMB
Date of Approval:06/19/96
Resolution No:96-06-39
CCMB:1996R Page:394
Recorded on:6/27/96 OR Book:2720 Page:0295-0298

PLAT OF

LEHIGH ACRES

LEE COUNTY, FLORIDA

LEE COUNTY LAND AND TITLE COMPANY

DEVELOPERS

ACKNOWLEDGEMENTS

KNOW ALL MEN BY THESE PRESENTS, THAT LEE COUNTY LAND AND TITLE COMPANY, A CORPORATION UNDER THE LAWS OF FLORIDA, BEING THE OWNER OF THE LAND DESCRIBED IN THE CAPTION HEREON HAS CAUSED THIS PLAT OF LEHIGH ACRES TO BE MADE AND DO HEREBY DEDICATE TO THE PERPETUAL USE OF THE PUBLIC STREETS, ROADS, DRIVES AND/OR LANES SHOWN HEREON AND ALL CANAL RIGHT OF WAYS SHALL BE DEDICATED TO THE EAST COUNTY WATER CONTROL DISTRICT.

IN WITNESS WHEREOF GERALD H. GOULD AND EDWARD SHAPIRO, RESPECTIVELY PRESIDENT AND SECRETARY OF LEE COUNTY LAND AND TITLE COMPANY, BY AND WITH AUTHORITY OF IT'S BOARD OF DIRECTORS HAVE EXECUTED THESE PRESENTS FOR AND ON BEHALF OF SAID CORPORATION AND CAUSED IT'S CORPORATE SEAL TO BE AFFIXED THIS 10th DAY OF April A.D. 1961

LEE COUNTY LAND AND TITLE CO.

BY Gerald H. Gould
President

ATTEST Edward Shapiro
Secretary

STATE OF FLORIDA
COUNTY OF LEE

I HEREBY CERTIFY THAT ON THIS DAY, BEFORE ME AN OFFICER DULY AUTHORIZED TO TAKE ACKNOWLEDGEMENTS, PERSONALLY APPEARED GERALD H. GOULD AND EDWARD SHAPIRO, RESPECTIVELY PRESIDENT AND SECRETARY OF LEE COUNTY LAND AND TITLE COMPANY, TO ME KNOWN TO BE THE PERSONS DESCRIBED IN AND WHO EXECUTED THE FOREGOING DEDICATION AS PRESIDENT AND SECRETARY RESPECTIVELY OF LEE COUNTY LAND AND TITLE COMPANY FOR THE PURPOSES THEREIN SET FORTH.

WITNESS MY HAND AND OFFICIAL SEAL AT FORT MYERS, SAID COUNTY AND STATE THIS 10th DAY OF April A.D 1961

Jean Welchler
NOTARY PUBLIC STATE OF FLORIDA AT LARGE
My Commission Expires July 19, 1964

APPROVALS

THIS PLAT APPROVED THIS 12 DAY OF April A.D. 1961 IN OPEN MEETING OF THE BOARD OF COUNTY COMMISSIONERS OF LEE COUNTY, FLORIDA.

APPROVED Gloria S. Hunter
Chairman

D. L. Farahee
Clerk

County Attorney

Stacy Bruce
County Road Superintendent

260798

I HEREBY CERTIFY THAT THIS PLAT OF LEHIGH ACRES, HAS BEEN EXAMINED BY ME AND FROM MY EXAMINATION I FIND THAT SAID PLAT COMPLIES IN FORM WITH THE REQUIREMENTS OF SURVEY LAWS OF FLORIDA.

I FURTHER CERTIFY THAT SAID PLAT WAS FILED FOR RECORD AT FORT MYERS THIS 13 DAY OF April A.D. 1961, AND DULY RECORDED IN PLAT BOOK 15 ON PAGES 1-101 OF PUBLIC RECORDS OF LEE COUNTY, FLORIDA.

D. L. Farahee
Clerk Of The Circuit Court In And For Lee County

SURVEYORS CERTIFICATE

I THE UNDERSIGNED CERTIFY THAT THE PLAT AS SHOWN IS A CORRECT REPRESENTATION OF THE LANDS PLATTED.

R. V. Jamme

PLAT OF

LEHIGH ACRES

LEE COUNTY, FLORIDA

LEE COUNTY LAND AND TITLE COMPANY

DEVELOPERS

DESCRIPTION

TOWNSHIP 43 SOUTH, RANGE 27 EAST

The Southeast 1/4 of Section 36

TOWNSHIP 44 SOUTH, RANGE 27 EAST

The South 1/2 and the North 1/2 of the North 1/2 of Section 1; the East 1/2 and the East 1/2 of the Northwest 1/4 and the Southwest 1/4 of the Northwest 1/4 of Section 2; the South 1/2 of Section 7, 8 and 9; all of Sections 11 through 21 inclusively and all of Sections 23 through 28 inclusively; all of Section 29 except that part lying South of the South Right of Way of Able Canal; all of Sections 35 and 36 inclusively; all of Section 32 except the following: Units 1, 2 and 3 of Section 32 as recorded on pages 124, 125 and 126 of Plat Book 9. Units 4, 5, 7, 10, 11 and 18 of Section 32 as recorded on Pages 50 through 55 inclusively of Plat Book 12. Unit 20 of Section 32 as recorded on Pages 86 through 89 inclusively of Plat Book 10. Part of Unit 3 as recorded on Page 23 of Plat Book 12. Addition to Unit 20 as recorded on Pages 36 and 37 of Plat Book 13 all of the Public Records of Lee County, and all of section 33 except the following: Units 4 and 10 of Section 33 as recorded on Page 52 of Plat Book 12. Unit 5 of Section 33 as recorded on Page 29 of Plat Book 12. Bear Lake Subdivision as recorded on Pages 38 and 39 of Plat Book 13. Replat of Lots 1 through 6, Block 4e, Section 33, Township 44 South, Range 27 East as recorded on Page 84 of Plat Book 12 all of the Public Records of Lee County and that part of Section 34 described as Follows; Beginning at the Northwest Corner of Section 34, Township 44 South, Range 27 East go North 89°-12'-26" East, 1919.76 feet; thence South 00° 06'-48" East, 1468.91 feet; thence South 89°-12'-26" West, 1919.76 feet; thence North 00°-06'-48" West, 1468.91 feet to the Point of beginning; and that part of section 34, Twp. 44 S., Rge 27 E., described as follows: commencing at the Southwest corner of Section 34; thence North 00°-06'-48" West along the West line of said Section 1403.40 feet to the Northerly right of way line of Joel Boulevard; thence North 88°-58'-52" East along said Northerly right of way 15.80 feet to the P.C. of a curve; thence Northeasterly on a tangential curve to the left, with a radius of 676.18 feet, a distance of 400.21 feet, to the P.T. of said curve, thence North 55°-05'-58" East, continuing along said Northerly right of way of Joel Boulevard, 1012.50 feet to the Southwesterly right of way of David Avenue, said point also being the actual point of beginning of the tract of land to be described; thence North 34°-54'-02" West along said southwesterly right of way line 1610 feet; thence North 55°-05'-58" East, a distance of 200 feet; thence South 34°-54'-02" East, a distance of 90 feet; thence North 55°-05'-58" East, a distance of 175 feet; thence North 87° 55'-40" East, a distance of 184.45 feet; thence South 34°-54'-02" East, a distance of 185 feet; thence South 55°-05'-58" West, a distance of 280 feet; thence South 34°-54'-02" East, a distance of 375 feet; thence South 55°-05'-58" West, a distance of 200 feet; thence South 34°-54'-02" East, a distance of 860 feet; thence South 55°-05'-58" West, a distance of 50 feet, to the actual Point of Beginning; and all of Section 22 lying West of the West right of way line of Gerald Avenue as shown on the Plat of a Subdivision of Part of Lehigh Acres recorded on Pages 50 through 55 inclusively of Plat Book 12 of the Public Records of Lee County, Florida.

TOWNSHIP 44 SOUTH, RANGE 26 EAST

All of Sections 1, 2, 11, 12, 13, 23, 24, 26, 27, 32, 33, 34, 35, and 36; and the South 1/2 of Sections 29 and 30; and the South 100 feet of the North 1/2 of Section 30; and the East 1/2 of Section 14; the South 1/2, and that part of the North 1/2 of Section 28, lying East of Buckingham Road; all of Block E, Lot 1, Block 38 of Buckingham Park, south section as recorded in Plat Book 9, Page 99 of the Public Records of Lee County, Florida, and all that part of Section 22 lying South and West of the southerly right of way of Homestead Road, except Lots 1 through 7 inclusively, Block 37 of Buckingham Park south section as recorded in Plat Book 9, Page 99 of the Public Records of Lee County, Florida; and that part of Section 31, lying North of the Northerly right of way of State Road 82; and all of Section 25 except Units 3 and 6 and Units 11 and 12 as recorded in Plat Book 13, Pages 21, 22 and 23 of the Public Records of Lee County, Florida.

TOWNSHIP 45 SOUTH, RANGE 26 EAST

All of Sections 1, 2, 3 and 12, all of that part of sections 4, 5, 6, 9, 10, 11, 13 and 14 lying North of the Northerly right of way of State Road 82.

PART IV #26-17

Recommended Action

Engineering Review:

By: _____ 2/10/2026 **Approved _X_ Denied __**

District Field Review:

By: _____ **Approved ___ Denied ___**

District Manager: _____ **Date**: _____

Comments:

The referenced permit application for a boring under Dog Canal (47-12-2) at 1617 Hines Ave and 7897 16th Ter, in Lehigh Acres, submitted by Comcast, has been reviewed and is recommended for approval with the following stipulations:

1. Permanent above-ground cable markers must be placed at the canal right-of-way (R/W) line on each side of the crossing.

2. Any disturbed canal R/W must be restored with sod and maintained until fully established. Seeding will not be permitted.

3. Within 30 days of completing the improvements, provide LA-MSID with a copy of the survey record drawings indicating the location and elevation of utilities within the canal R/W.

4. The permittee will be responsible for any necessary repairs, alterations, or relocation of the utility if required for future maintenance or repairs of the canal.

5. Lee County DOT must review and approve any utility work proposed within the County Road R/W.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: PER #26-40 Chick-Fil-A 1110 Homestead Rd N.

Meeting Date: March 16, 2026

Agenda Item # 2I

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve PER #26-40 Chick-Fil-A at 1110 Homestead Rd N., with any listed stipulations.

What the action accomplishes: Allows Lemonade MM Lehigh Acres, LLC to build a Chick-Fil-A restaurant at 1110 Homestead Rd N.

Fiscal Impact: Permit Fees: $1,500.00

Staff comments: Approved according to the plans submitted February 19, 2026.

Staff review and recommended approval:

[x] **MANAGER** [ ] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

LEHIGH ACRES - MUNICIPAL SERVICES IMPROVEMENT DISTRICT
PERMIT

THIS PERMIT is granted this 16th day of March, 2026, by Lehigh Acres Municipal Services Improvement District (hereinafter referred to as "DISTRICT"), 601 East County Lane, Lehigh Acres, Florida 33936, to Lemonade MM Lehigh Acres LLC, C/O Madison Marquette, 100 High St., Suite 410, Boston, MA 02110, hereinafter referred to as the "Permittee") as a non-exclusive permit for a proposed approximately 5,614 SF commercial development on (1.82 +/- acres), located at 1110 Homestead Road N., with an on-site detention system and a stormwater outfall to the existing storm drain located at the corner of Homestead Road and Ashlar Ave., in Lehigh Acres, FL.

W I T N E S S E T H:

1. In the event the DISTRICT is not the fee title owner of the real property (the "Property") to which this Permit is applicable, the Permittee agrees, prior to commencement, to obtain all necessary consents from the fee title owners of the Property.

2. This Permit does not constitute a waiver of the necessity by the Permittee to obtain such other necessary and appropriate permits required by other governmental bodies or agencies; therefore the Permittee, prior to commencement, is required to obtain any and all other applicable federal, state, and local permits required in connection with Permittee's use of the Property and agrees that at all times it will comply with the requirements of all federal, state and local laws, ordinances, rules and regulations applicable or pertaining to the use of the Property by the Permittee.

3. Permittee understands and agrees that the use of the Property pursuant to this Permit is subordinate to the rights and interests of the DISTRICT and to the extent applicable that of the fee title owner of the Property. Further, Permittee does hereby stipulate that the Permittee is not relying upon any representations whatsoever by the DISTRICT regarding the DISTRICT'S right, title, interest, or ownership as to the Property for which this Permit is applicable.

4. The Permittee, by accepting this permit, specifically agrees to allow authorized District Personnel, upon presentation of credentials or other documents as may be required by law, access to the premises, at reasonable times, where the permitted activity is located or conducted for the purpose of:
    a. Having access to and copying any records that must be kept under the conditions of the permit:
    b. Reviewing the facility, equipment, practices, or operations regulated or required under this permit; and
    c. Sampling or monitoring any substances or parameters at any location reasonably necessary to assure compliance with this permit or District rules.

5. The DISTRICT specifically reserves the right to maintain its facilities located over, under or upon the Property; to make improvements; add additional facilities; maintain, construct or alter its roads; maintain any facilities, devices, or improvements on the Property which aid in or are necessary to the DISTRICT'S operation; and the right to enter upon the Property at all times for such purposes. Permittee understands that in the exercise of such rights and interest, the DISTRICT may require Permittee, at no cost to DISTRICT, to relocate, alter, or remove the Permittee's facilities and equipment or other improvements made by Permittee pursuant to this Permit which interfere with or prevent the DISTRICT, in the DISTRICT'S opinion, from properly and faithfully constructing, improving and maintaining its facilities. If the Permittee should fail to relocate, alter, or remove the Permittee's facilities, equipment or other improvements made by the Permittee pursuant to this Permit within a reasonable time period following receipt of notice from the DISTRICT requesting same, then in that event the DISTRICT shall have the right to enter upon the lands and make such relocation, alterations or

removal of the Permittee's facilities, equipment/or other improvements at no cost to DISTRICT.

6. Permittee agrees that it will not use the Property in any manner which interferes with the DISTRICT'S use of the Property or causes a hazardous condition to exist.

7. The DISTRICT assumes no responsibility for the ownership, operation and/or maintenance of the Permittee's facilities as permitted herein.

8. The Permittee agrees to backfill any excavation it makes within the Property and to repair or replace (i) any below-grade authorized facilities and features located therein, including but not limited to: water, sewer, stormwater or irrigation facilities, structures or appurtenances and (ii) as to at or above-grade facilities and features, this requirement shall only be applicable to grasses and sod, sidewalks, and roadway improvements (the term roadway improvements shall include but not be limited to curbing, roadway base, sub grade and asphaltic or concrete surfaces) which are removed or damaged as a direct result of said excavation. Such repair or replacement shall substantially restore these features to their condition as it existed immediately prior to the event precipitating the excavation.

9. All below-grade Permittee installations, including water and sewer lines, whether sub aqueous or not, together with appurtenant facilities, shall be located underground and have a minimum ground cover of thirty inches (30") below original design profile grade, including District waterways and roadways.

10. All Permittee activity related to sub aqueous utility, water and/or sewer lines, including appurtenant facilities, shall be conducted in such fashion that the primary functional purpose of the canal or lake body facility, specifically flood control, within which the activity is taking place is not adversely affected. More specifically, sub aqueous construction shall conform to the criteria given in the Lehigh Acres Municipal Services Improvement District (Lehigh Acres – MSID) Administrative Policy and Procedures.

11. Permittee shall, at no expense to DISTRICT and within a reasonable time following notice, adjust the positions and elevations of its facilities as may be required in connection with future improvements to, or construction of, works of the DISTRICT.

12. Permittee does hereby indemnify and hold harmless the DISTRICT, its Board of Commissioners, officers, personnel, and it's Engineers against any claims, losses, damages (including consequential), expenses, or legal fees that might arise out or result from the implementation of the proposed project by the Permittee or that may be caused by a failure of DISTRICT facilities or property.

13. The Permittee shall be obligated at the time of submission of its Permit application, to provide a detail of any other existing improvements located over, under, upon or through the Property and the Permittee's accompanying plans and specifications shall clearly indicate how the Permittee will comply with required clearances between the requested permitted activity and existing improvements located over, under and upon the Property, including compliance with other permitted users duly adopted and published standards.

14. Permittee shall reimburse the DISTRICT for its legal, engineering and other expenses incurred as a result of the implementation of the proposed project.

15. If Permittee should violate any of the terms or conditions of the Permit and shall not correct or remedy same within ten (10) days of receiving written notice of said violation from the DISTRICT, then in that event, the DISTRICT may, at its option, revoke, cancel and terminate this Permit.

16. This Permit may not be assigned by the Permittee without the prior written approval of the DISTRICT.

17. Unless otherwise adjusted, modified or terminated as set forth hereinabove, including the Conditions attached hereto, this Permit shall continue in full force and effect so long as Permittee complies with the terms of this Permit.

18. In any litigation, including breach, enforcement or interpretation arising out of this Permit, the prevailing party to this Permit shall be entitled to recover reasonable attorney's fees, costs and expenses related to said action from the non-prevailing party.

19. That attached hereto are Supplemental Conditions to this Permit which are incorporated herein and made a part hereof.

LEHIGH ACRES – MUNICIPAL SERVICES IMPROVEMENT DISTRICT

BY: _____

NAME TYPED: David E. Lindsay

TITLE: District Manager

SUPPLEMENTAL CONDITIONS TO PERMIT NO. 26-40

A. GENERAL CONDITIONS

1. Notification shall be given by the Permittee to the District Engineer, AIM Engineering, at 239-332-4569 forty-eight (48) hours prior to commencement of the permitted activity. The District Engineer shall establish points of construction, if applicable, which will require inspection by the District Engineer prior to construction of the permitted activity. When the Permittee considers the permitted activity complete, the Permittee shall notify the District Engineer and schedule an on-site final inspection which shall be held in the presence of a representative of the Permittee and the District Engineer.

2. The permitted activity shall be implemented, constructed and/or installed in full accordance with the approved plans and specifications within six (6) full calendar years from the date of permit approval. Deviations from the approved plans or specifications shall be coordinated with and approved by the District Engineer prior to implementation of any such deviation.

3. When working in a District road right-of-way, not more than one-half (½) of the road or street shall be closed and traffic shall be controlled so as to provide for the safety and minimum hindrance to the public. All traffic control operations shall conform to the most current issue of the Florida Department of Transportation publication "Manual on Traffic Controls and safe Practices for Street and Highway Construction, Maintenance and Utility Operations."

4. The District Engineer shall be the final authority as to the quality and quantity of the material and works required to satisfy the terms and conditions of this Permit as they relate to District improvements and facilities.

5. Upon completion of the permitted activity and after its final review and acceptance by the District Engineer THE PERMITTEE SHALL DELIVER TO THE DISTRICT OFFICE ONE COMPLETE SET OF THE FINAL "RECORD DRAWINGS." FAILURE TO PROVIDE THE FINAL RECORD DRAWINGS MAY RESULT IN THE REVOCATION, CANCELLATION AND TERMINATION OF THIS PERMIT.

6. The above Conditions shall be continuing obligations of the Permittee and shall be complied with in the event of any subsequent maintenance, repair, replacement or modification of the permitted improvement.

Stipulations:

1). Permit #26-40 is approved according to the plans and calculations submitted February 19th, 2026.

2). Grading must be closely monitored during construction in the southeast corner where the 6.33' long overflow spillway will be built at elevation 26.72'. The grading must allow for the overflow water to exit the proposed dry retention pond and flow into the existing inlet grate at elevation 25.95' without overflowing onto the sidewalk and Homestead Road N.

3). Maintenance of the entire on-site storm water management system, including the underground storm vaults, overflow spillway and bleeder pipe is the responsibility of the Permittee.

4). A permanent benchmark disc should be established on or near the spillway structure with the elevation and datum clearly labeled on it.

5). An "As-Built" survey showing the completed improvements (including perimeter berm, all pipe inverts and outfall structure elevations) must be submitted to Lehigh Acres - MSID within 30 days following completion of the improvements.

6). Please provide Lehigh Acres - MSID with copies of all applicable permits from other governing agencies prior to commencing construction.

02 - 19 - 26
26 - 40

APPLICATION FOR A PERMIT

*Note: The review of permit applications will be conducted per the Lehigh Acres Municipal Services Improvement District's Administrative Policy and Procedures

AGENT NAME: Jenny Baez **PHONE #:** (+19543148468

PROJECT NAME: CFA- Lehigh Acres

TOTAL ACREAGE: 1.82 Acres

Property Address (STRAP associated with this address will be used as the primary STRAP for this project):					
Mailing Address:	1110 HOMESTEAD RD N				
City:	LEHIGH ACRES,	**State:**	FL	**Zip:**	33936

STRAP NUMBERS (*attach additional sheet if necessary*)**:**

3	1	-	4	4	-	2	7	-	0	5	-	0	0	0	0	6	-	3	0	B	0
		-			-			-			-						-				
		-			-			-			-						-				
		-			-			-			-						-				
		-			-			-			-						-				

Brief description of proposed project:

Application for:

Chick fil a Restaurant with a drive thru

D.

Name of Authorized Agent(s):			Bowman Consulting Group				
Mailing Address:	Street:		900 SE 3rd ave. Suite 300				
City	Fort Lauderdale		State: FL			Zip:	33316
Phone No.	Area Code:	(954)	Number:	314- 8468		Ext.	
Fax No.	Area Code:	( )	Number:	-			
E-mail Address:	ftl@bowman.com						

E.

Name of Property Owner:			Lemonade MM Lehigh Acres LLC				
Mailing Address:	Street:		1110 HOMESTEAD RD N				
City	LEHIGH ACRES		State:	FL		Zip:	33936
Phone No.	Area Code:	( )	Number:	-		Ext.	
Fax No.	Area Code:	( )	Number:	-			
E-mail Address:							

*A disclosure of the name of all persons or entities having an ownership interest in the property is required (attached).

PART III

DISCLOSURE OF OWNERSHIP INTEREST FORM FORM:

3 1 - 4 4 - 2 7 - 0 5 - 0 0 0 0 6 - 3 0 B 0

PRIMARY STRAP NUMBER:

1. If the property is owned in the fee simple by an **INDIVIDUAL**, tenancy by the entirety, tenancy in common, or joint tenancy, list all parties with an ownership interest as well as the percentage of such interest.

Name and Address	Percentage of Ownership
Josh Anderson // Kemonade MM Lehigh Acres LLC	100 %

Name and Address	Percentage of Interest

Date of Contract (mm/dd/yy): / /

6. If any contingency clause or contract terms involve additional parties, list all individuals or officers, if a corporation, partnership, or trust.

Name and Address	Percentage of Interest

For any changes of ownership of changes in contracts for purchase subsequent to the date of the application, but prior to the date of final certificate for compliance, a supplemental disclosure of interest must be filed.

The above is a full disclosure of all parties of interest in this application, to the best of my knowledge and belief.

_____
SIGNATURE OF APPLICANT

_____
PRINTED OR TYPED NAME OF APPLICANT

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this _____ day of_____(m), _____(y), by _____ who is personally know to me or has produced _____ as identification.

_____
SIGNATURE OF NOTARY PUBLIC

_____

PART V

PROFESSIONAL CONSULTANT FORM

ATTORNEY

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

ENGINEER

Name:	Bill Pfeffer						
Mailing Address:	900 SE 3rd ave. Suite 300						
City:	Fort Lauderdale	State:		FL		Zip:	33316
Business No.	Area Code:	(954)	Number:	314-8468		Ext.	
Other Contact:	ftl@bowman.com						

OTHER REPRESENTATIVE/PROFESSIONAL

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

PART VI

SUBMITTALS

NO. OF COPIE		SUBMITTAL DOCUMENT
SUB'D	REQ'D	
		ITEMS REQUIRED FOR INITIAL SUBMITTA
	2	**Construction/Development Plans** signed and sealed by a L Florida Professional Engineer including: 1. Location map showing the location of the proposed project in r adjacent streets and LA-MSID facilities. 2. Details of proposed works that fully communicate the work and desired (i.e. Existing Conditions Survey, Proposed Site Plan, Pavi Grading & Drainage Plan, Cross Sections, Element Details, and Control Plan sheets). 3. Sufficient survey data to determine elevations, property and ROV limits of existing and proposed conditions, and notation of utiliz vertical datum (NAVD/NGVD).

	2	**Stormwater Drainage Calculations** signed and sealed by a I Florida Professional Engineer including, but not limited to, co stormwater routing modeling data (ICPR4, Cascade, etc.) to v compliance with the Design Criteria (input summary data belo **Water Quality Volume (Required):** _____(AC **Water Quality Volume (Provided):** _____(AC **Control Elevation:** _____(FT-N **Minimum Road/Parking Elevation:** _____(FT-N *(above the 10-year, 1-day design storm peak stage elevation)* **Minimum Perimeter Berm Elevation:** _____(FT-N *(above the 25-year, 3-day design storm peak stage elevation)* **Minimum Finished Floor Elevation:** _____(FT-N *(above the 100-year, 3-day design storm peak stage elevation a previously experienced high-water levels external to project)*
	2	**Flash Drive / CD** containing plans in both AutoCAD (.dwg drawn in "State Plane Coordinates") and PDF formats.
	2	**Permit Application** (PARTS I & II)
	2	**Disclosure of Owner Form (signed and notarized)** (PART
	2	**Professional Consultant Form & Submittals** (PARTS V &
	2	**Copy of Approved FDEP Notice of Intent (NOI)** for all s (1) acre and greater in size.
	2	**Copy of approved SWFMD permit**, for all sites required to permit, **or FDEP "10/2 Self-Certification"** approval.
	1	**Check for appropriate Permit Fees** made out to LA-MSID.

April 23, 2024

LETTER OF AUTHORIZATION

RE: Proposed Chick-fil-A Lehigh (Store #5587)
1110 W Homestead Road, Lehigh Acres, Florida 33936
STRAP: 31-44-27-05-00006.30B0 // Folio ID: 10390037

TO WHOM IT MAY CONCERN:

This letter is to designate the following Parties are to act as authorized agent/applicants on behalf of the property owner associated with the proposed redevelopment project: LEMONADE MM LEHIGH ACRES LLC (hereinafter the "*Property Owner*"):

Chick-Fil-A, Inc
Clint Mattson
5200 Buffington Road, Atlanta, GA 30349

Bowman Consulting Group (Bowman)
Jenny Baez, Andres Mizrahi, Kristhian Morales
910 SE 17th Street, Suite 300, Fort Lauderdale FL 33316

The Property owner grants the above-mentioned Parties authorization to make application submittals and negotiate conditions in matters related to government regulations and permitting activities with the State of Florida, Lee County, SFMWD and other government entities in relation to the above-mentioned site (associated with STRAP: 31-44-27-05-00006.30B0 // Folio ID: 10390037).

Joshua Anderson
Signature
Joshua Anderson
Vice President
Lemonade MM Lehigh Acres LLC
c/or Roseview
100 High Street Suite 410
Boston, MA 02110

STATE OF WASHINGTON
COUNTY OF KING

Sworn to and subscribed before me, the undersigned Notary Public, this 23RD day of APRIL, 2024

BERMALYN D MANDAC 22032929 NOTARY PUBLIC COMMISSION EXPIRES 07-01-26 STATE OF WASHINGTON

Bermandac
Notary Public

BERMALYN D MANDAC
Printed Name

DISCLOSURE OF INTEREST
AFFIDAVIT

BEFORE ME this day appeared Lemonade MM Lehigh Acres LLC, who, being first duly sworn and deposed says:

1. That I am the record owner, or a legal representative of the record owner, of the property that is located at 1110 HOMESTEAD RD N, Lehigh Acres, FL 33936 and is the subject of an Application for zoning action (hereinafter the "Property").

2. That I am familiar with the legal ownership of the Property and have full knowledge of the names of all individuals that have an ownership interest in the Property or a legal entity owning an interest in the Property.

[OPTIONAL PROVISION IF APPLICANT IS CONTRACT PURCHASER: In addition, I am familiar with the individuals that have an ownership interest in the legal entity that is under contract to purchase the Property.]

3. That, unless otherwise specified in paragraph 6 below, no Lee County Employee, County Commissioner, or Hearing Examiner has an Ownership Interest in the Property or any legal entity (Corporation, Company, Partnership, Limited Partnership, Trust, etc.) that has an Ownership Interest in the Property or that has contracted to purchase the Property.

4. That the disclosure identified herein does not include any beneficial Ownership Interest that a Lee County Employee, County Commissioner, or Hearing Examiner may have in any entity registered with the Federal Securities Exchange Commission or registered pursuant to Chapter 517, whose interest is for sale to the general public.

5. That, if the Ownership Interest in the Property changes and results in this affidavit no longer being accurate, the undersigned will file a supplemental Affidavit that identifies the name of any Lee County Employee, County Commissioner, or Hearing Examiner that subsequently acquires an interest in the Property.

6. Disclosure of Interest held by a Lee County Employee, County Commissioner, or Hearing Examiner.

Name and Address	Percentage of Ownership
Lemonade MM Lehigh Acres LLC	100%

Under penalty of perjury, I declare that I have read the foregoing and the facts alleged are true to the best of my knowledge and belief.

Agent to Property Owner

Property Owner

Andres Mizrahi – Authorized Agent

Print Name

************NOTE: NOTARY PUBLIC IS NOT REQUIRED FOR ADMINISTRATIVE APPROVALS************
ALL OTHER APPLICATION TYPES MUST BE NOTARIZED

STATE OF FLORIDA
COUNTY OF ~~LEE~~ BROWARD

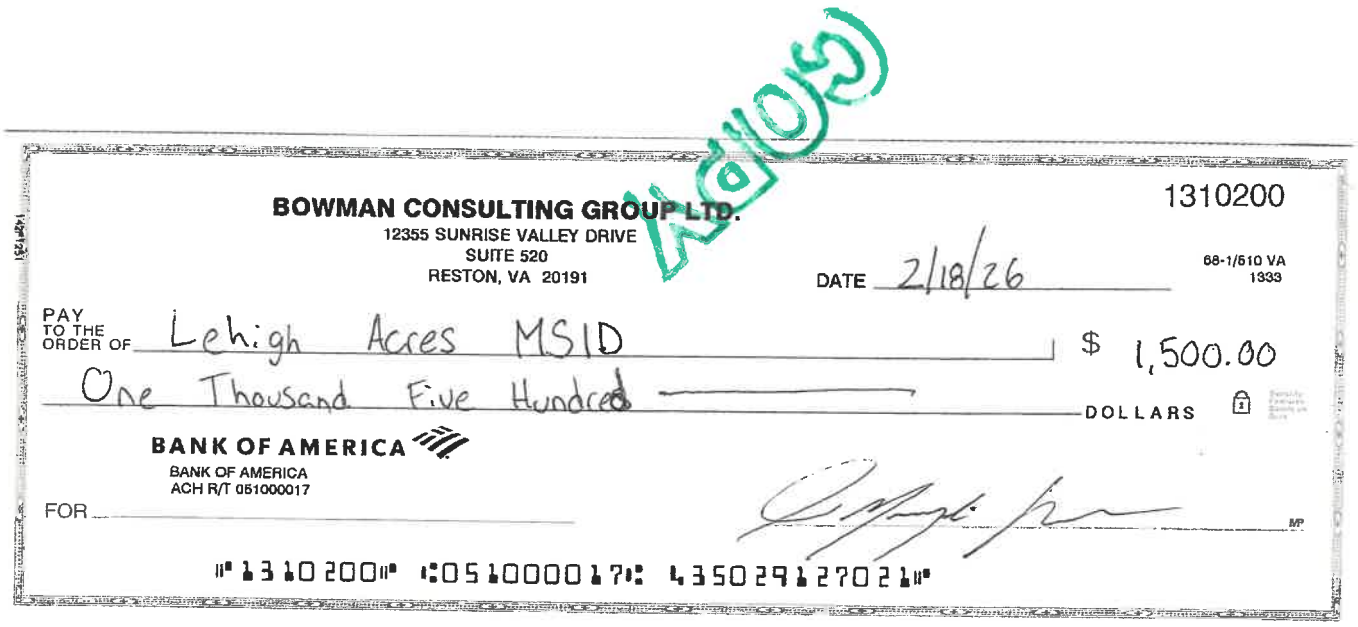
The foregoing instrument was sworn to (or affirmed) and subscribed before me by means of ☒ physical presence or ☐ online notarization, on 2/18/2026 (date) by Andres Mizrahi (name of person providing oath or affirmation), who is personally known to me ~~or who has produced~~ N/A ~~(type of identification) as identification~~.

STAMP/SEAL

Shailah Giovanna Steck
Comm.: HH 354492
Expires: January 26, 2027
Notary Public - State of Florida

Signature of Notary Public

Shailah G Steck



BOWMAN CONSULTING GROUP LTD.
12355 SUNRISE VALLEY DRIVE
SUITE 520
RESTON, VA 20191

1310200

DATE 2/18/26

68-1/510 VA
1333

PAY TO THE ORDER OF Lehigh Acres MSID $ 1,500.00

One Thousand Five Hundred DOLLARS

BANK OF AMERICA
BANK OF AMERICA
ACH R/T 051000017

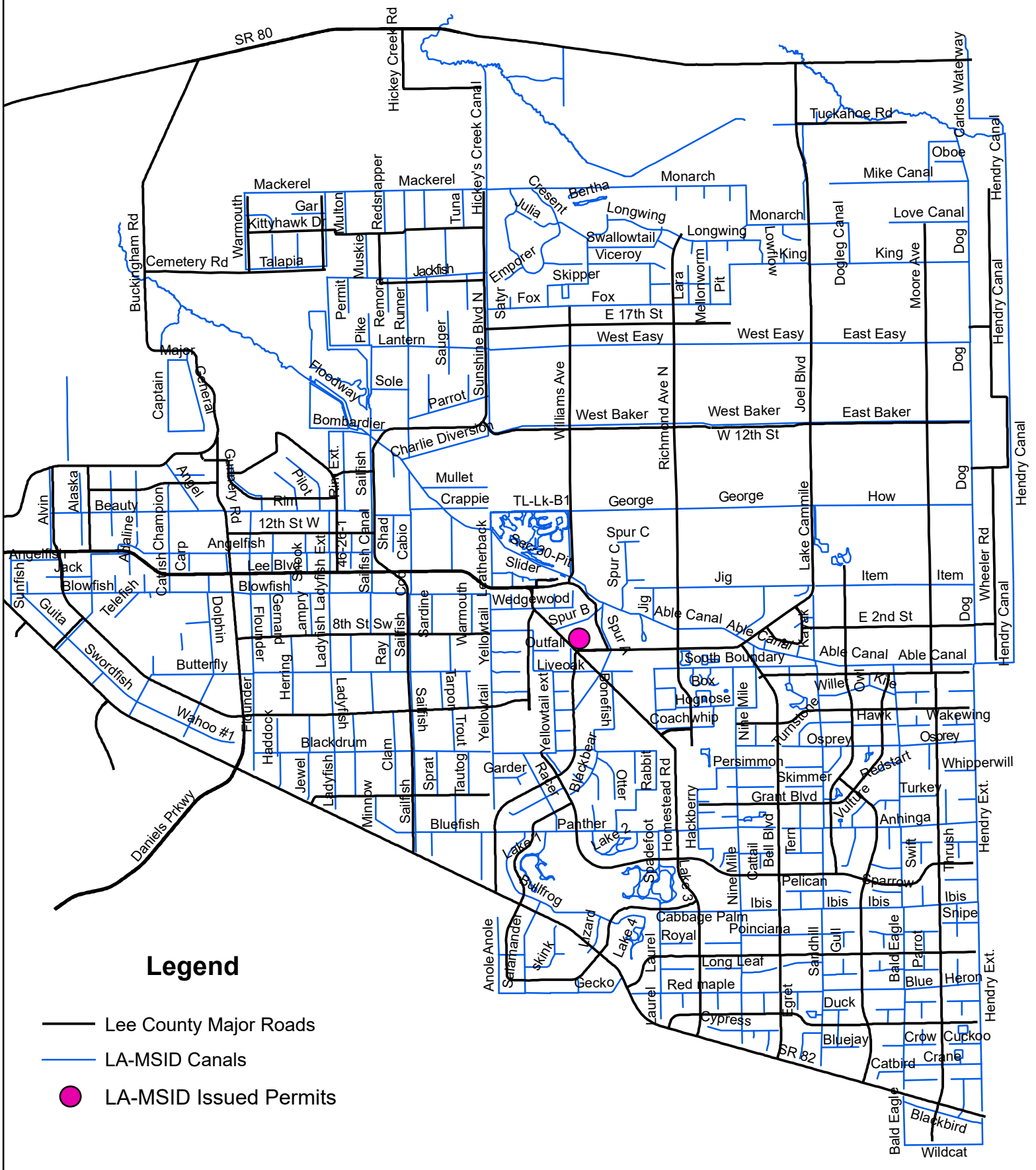
FOR

⑈1310200⑈ ⑆051000017⑆ 435029127021⑈

Permit #26-40

RECEIVED
FEB 19 2026
LEHIGH ACRES
MUNICIPAL SERVICES
IMPROVEMENT DISTRICT

Permit # 2026-40



Legend

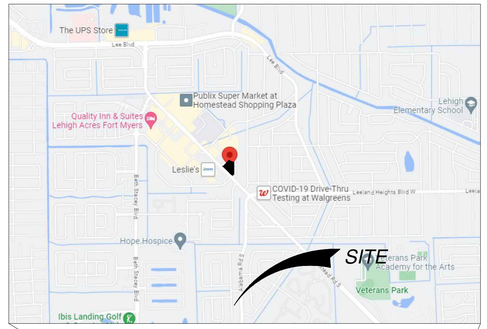
- Lee County Major Roads
- LA-MSID Canals
- LA-MSID Issued Permits

SITE DEVELOPMENT PLANS
FOR
LEHIGH ACRES
CHICK-FIL-A

PARCEL ID: 314427050000630B0

STORE #05587

1110 HOMESTEAD RD N
LEHIGH ACRES, LEE COUNTY, FLORIDA





LOCATION MAP
1" = 2000'

LEGAL DESCRIPTION

COMMENCING AT THE SOUTHEAST CORNER OF SECTION 31, TOWNSHIP 44 SOUTH, RANGE 27 EAST, GO NORTH ALONG THE EAST BOUNDARY OF SAID SECTION 31 ON A BEARING OF NORTH 00°52'08" EAST, A DISTANCE OF 1,618.87 FEET TO THE NORTH RIGHT-OF-WAY OF HOMESTEAD ROAD, THENCE ALONG THE NORTHEAST RIGHT-OF-WAY OF HOMESTEAD ROAD ON A BEARING OF NORTH 45°11'33" WEST, A DISTANCE OF 319.87 FEET TO THE POINT OF BEGINNING OF THE LAND HEREIN TO BE DESCRIBED: THENCE CONTINUING NORTH 45°11'33" WEST, 368.66 FEET, THENCE NORTH 44°48'27" EAST, 250.01 FEET TO A POINT ON A CURVE TO THE LEFT,SAID CURVE HAVING A DELTA ANGLE OF 43°21'56", A RADIUS OF 200.00 FEET, A CHORD BEARING OF SOUTH 67°26'54" EAST, A CHORD DISTANCE OF 147.79 FEET, AND AN ARC DISTANCE OF 151.37 FEET TO THE POINT OF TANGENCY OF SAID CURVE, THENCE SOUTH 00°52'08" WEST, A DISTANCE OF 293.84 FEET TO THE POINT OF CURVE OF A CURVE TO THE RIGHT, SAID CURVE HAVING A DELTA ANGLE OF 43°56'19" AND A RADIUS OF 100.00 FEET FOR AN ARC DISTANCE OF 76.68 FEET, THENCE SOUTH 44°48'27" WEST, A DISTANCE OF 25.00 FEET TO THE POINT OF BEGINNING OF THE LANDS DESCRIBED.

THE ABOVE DESCRIPTION BEING THE SAME PROPERTY WHICH IS DESCRIBED AS TRACT B, LOT 6, SUNSHINE SHOPPING PLAZA, A MAP OF WHICH IS ATTACHED TO DEED RECORDED IN OFFICIAL RECORDS BOOK 364, PAGE 882, OF THE PUBLIC RECORDS OF LEE COUNTY, FLORIDA.

CONTAINING 79,472 SQ. FT. OR 1.824 ACRES, MORE OR LESS.

DATUM NOTE

1. THE ELEVATIONS SHOWN HEREON ARE REFERENCED TO THE NORTH AMERICA VERTICAL DATUM OF 1988 (N.A.V.D. 88) AND ARE BASED UPON THE FOLLOWING LEE COUNTY PUBLISHED BENCHMARKS:

   "TBM 18-52"
   LOCATED AT THE INTERSECTION HUNTDALE ST. AND INWOOD AV S AT THE NORTH SIDE OF THE CATCH BASIN.

   ELEVATION: 23.313' (N.A.V.D. 88)

2. ELEVATION DATUM CONVERSION NGVD29 = NAVD88 + 1.22'

FLOOD ZONE NOTE

1. THE SUBJECT PROPERTY LIES WITHIN FLOOD ZONE "X", PER THE FEDERAL EMERGENCY MANAGEMENT AGENCY (F.E.M.A.) ON FLOOD INSURANCE RATE MAP NUMBER 12071C0475F, EFFECTIVE DATE: 08/28/2008.

GENERAL NOTES

1. CONTRACTOR SHALL HAVE ONE SIGNED COPY OF THE APPROVED PLANS AND THE APPROPRIATE STANDARDS AND SPECIFICATIONS ALONG WITH A COPY OF ANY PERMITS AND AGREEMENTS NEEDED FOR THE JOB ON-SITE AT ALL TIMES.
2. CONTRACTOR SHALL MEET OR EXCEED ALL SITE WORK SPECIFICATIONS AND APPLICABLE STATE AND FEDERAL REGULATIONS FOR ALL MATERIALS AND CONSTRUCTION.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ASPECTS OF SAFETY DURING CONSTRUCTION.
4. THE CONTRACTOR SHALL NOTIFY THE ENGINEER IF ANY SITUATION THAT IS NOT IDENTIFIED IN THE PLANS OR SPECIFICATIONS IS ENCOUNTERED.
5. NO REVISION SHALL BE MADE TO THESE PLANS WITH OUT THE APPROVAL OF THE ENGINEER OF RECORD.
6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING ROADWAYS FREE AND CLEAR OF ALL CONSTRUCTION DEBRIS AND DIRT TRACKED FROM THE SITE.
7. ANY REFERENCE TO PUBLISHED STANDARDS SHALL REFER TO THE LATEST REVISION OF SAID STANDARD, UNLESS SPECIFICALLY STATED OTHERWISE.
8. ALL NECESSARY INSPECTIONS AND/OR CERTIFICATIONS REQUIRED BY CODES AND/OR UTILITY SERVICE COMPANIES SHALL BE PERFORMED PRIOR TO ANNOUNCED BUILDING POSSESSION AND THE FINAL CONNECTION OF SERVICES.

REFUSE & SOLID WASTE DISPOSAL FACILITIES		
	REQUIRED (SQ FT)	PROVIDED (SQ FT)
GARBAGE COLLECTION	80	110
RECYCLABLE COLLECTION	48	110
TOTAL	128	220

CONTACTS

WATER & SEWER
FLORIDA GOVERNMENT UTILITY AUTHORITY
PAUL ARNETT
407-629-6900
PAUL.ARNETT@FGUA.COM

GAS
TECO PEOPLES NATURAL GAS
5901 ENTERPRISE PARKWAY
FORT MYERS, FL 33905
ERICKA M. AUGUTIS
239.699.9824
EMAUGUTIS@TECOENERGY.COM

ELECTRIC
LEE COUNTY ELECTRIC CO-OP (LCEC)
4980 BAYLINE DR N
FORT MYERS, FL
TOM BAILEY
239.656.2414
TOM.BAILEY@LCEC.NET

TELEPHONE/CABLE
SUMMIT BROADBAND
24017 PRODUCTION CIRCLE
BONITA SPRINGS, FL 34135
STEVE TRACY
239.319.1555
STEVEN.TRACY@SUMMITBB.COM

TENENT
CHICK-FIL-A
5200 BUFFINGTON ROAD
ATLANTA, GA 30349

OWNER
LEMONADE MM LEHIGH ACRES, LLC
C/O MADISON MARQUETTE
100 HIGH ST., SUITE 410
BOSTON, MA 02110
JULIE CHASE
617.956.4377
JULIE.CHASE@MADISONMARQUETTE.COM

ENGINEER
BOWMAN
900 SE 3RD AVENUE, SUITE 300
FT. LAUDERDALE, FL 33316
ANDRES MIZRAHI,
SENIOR PROJECT MANAGER
954.884.5262
AMIZRAHI@BOWMAN.COM

SURVEYOR
BOWMAN
1410 N. WESTSHORE BLVD., SUITE 111
TAMPA, FL 33607
813.474.7424

HEALTH DEPARTMENT
DEPT OF BUSINESS AND PROFESSIONAL REGULATIONS DIVISION OF HOTELS AND RESTAURANTS
1940 NORTH MONROE STREET
TALLAHASSEE, FL 32399-1011
DHR.PLANREVIEW@MYFLORIDALICENSE.COM

EROSION CONTROL
FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION
2600 BLAIR STONE ROAD
TALLAHASSEE, FL 32399
NPDES STORMWATER PROGRAM
866.336.6312
NPDES-STORMWATER@DEP.STATE.FL.US

STORMWATER
SOUTH FLORIDA WATER MANAGEMENT DISTRICT
2301 MCGREGOR BOULEVARD
FORT MYERS, FL 33901
ELIZABETH BLESSING (ENVIRONMENTAL)
239.338.2929 EXT. 7781
EQUINTER@SFWMD.GOV
SERGIO GUZMAN (ENGINEERING)
239.338.2929 EXT.7771
SGUZMAN@SFWMD.GOV

Sheet List Table	
Sheet Number	Sheet Title
C-1.0	COVER SHEET
C-1.1	EXISTING CONDITIONS PLAN
C-1.2	DEMOLITION PLAN
C-2.0	SITE PLAN
C-3.0	GRADING AND DRAINAGE PLAN
C-3.1	EROSION CONTROL PLAN – PHASE 1
C-3.2	EROSION CONTROL PLAN – PHASE II
C-3.3	EROSION CONTROL DETAILS
C-3.4	CROSS SECTIONS KEY MAP
C-3.5	CROSS SECTIONS
C-3.6	CROSS SECTIONS
C-4.0	CFA STANDARD DETAILS
C-4.1	CFA STANDARD DETAILS
C-4.2	CFA STANDARD DETAILS
C-4.3	CFA STANDARD DETAILS
C-5.0	SITE AND DRAINAGE DETAILS
C-5.1	SITE AND DRAINAGE DETAILS
C-5.2	SITE AND DRAINAGE DETAILS
C-5.3	SITE AND DRAINAGE DETAILS
C-5.4	SITE AND DRAINAGE DETAILS
C-5.5	SITE AND DRAINAGE DETAILS
C-5.6	SITE AND DRAINAGE DETAILS
C-6.0	TRUCK TURNING PLAN
C-6.1	FIRE TRUCK TURNING PLAN
C-6.2	SITE CIRCULATION PLAN
PS-1.0	UTILITY PLAN
PS-1.1	UTILITY PROFILES
PS-1.2	UTILITY DETAILS
PS-1.3	UTILITY DETAILS
PS-1.4	UTILITY DETAILS

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com

© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

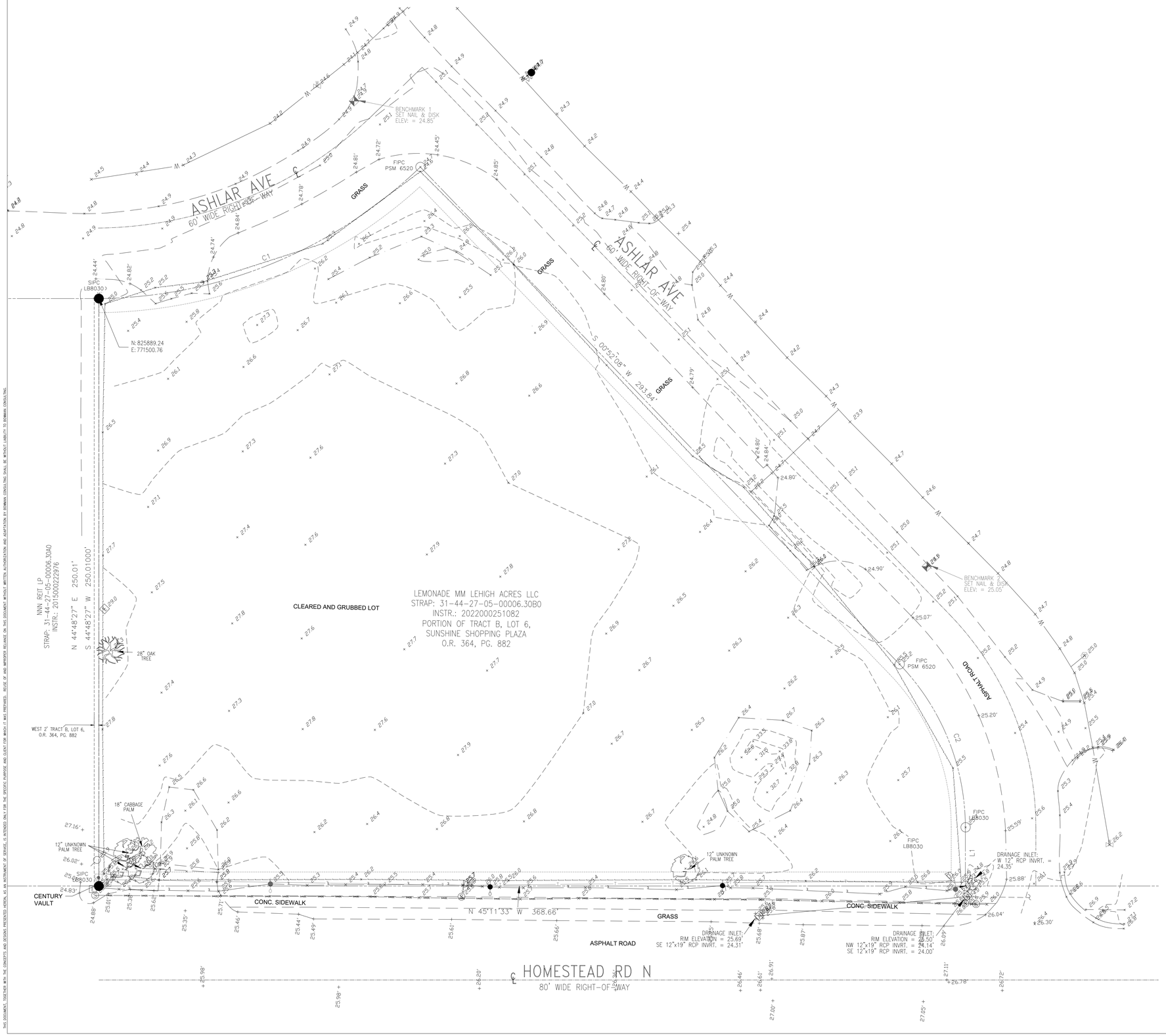
CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
COVER SHEET

SHEET NUMBER
C-1.0

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.



LEGEND

- PROPERTY LINE
- EX. CONCRETE SIDEWALK
- EX. CONCRETE D CURB
- EX. CONCRETE F CURB
- EX. EDGE OF SIDEWALK
- EX. SIGN
- EX. ELECTRIC HAND HOLE
- EX. SEWER CLEAN OUT
- EX. LIGHT POLE
- EX. TREE
- EX. HEDGE
- EX. FENCE
- EX. WATER METER
- EX. BACKFLOW PREVENTER
- EX. IRRIGATION VALVE
- EX. UNKNOWN UTILITY
- EX. STORM INLET
- EX. HEADWALL
- EX. STORM SEWER
- EX. SANITARY SEWER
- EX. TOP OF BANK

GRAPHIC SCALE
20 0 10 20 40
(IN FEET)
1 inch = 20 ft.

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast, it's free, it's the law.
www.callsunshine.com

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com

© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

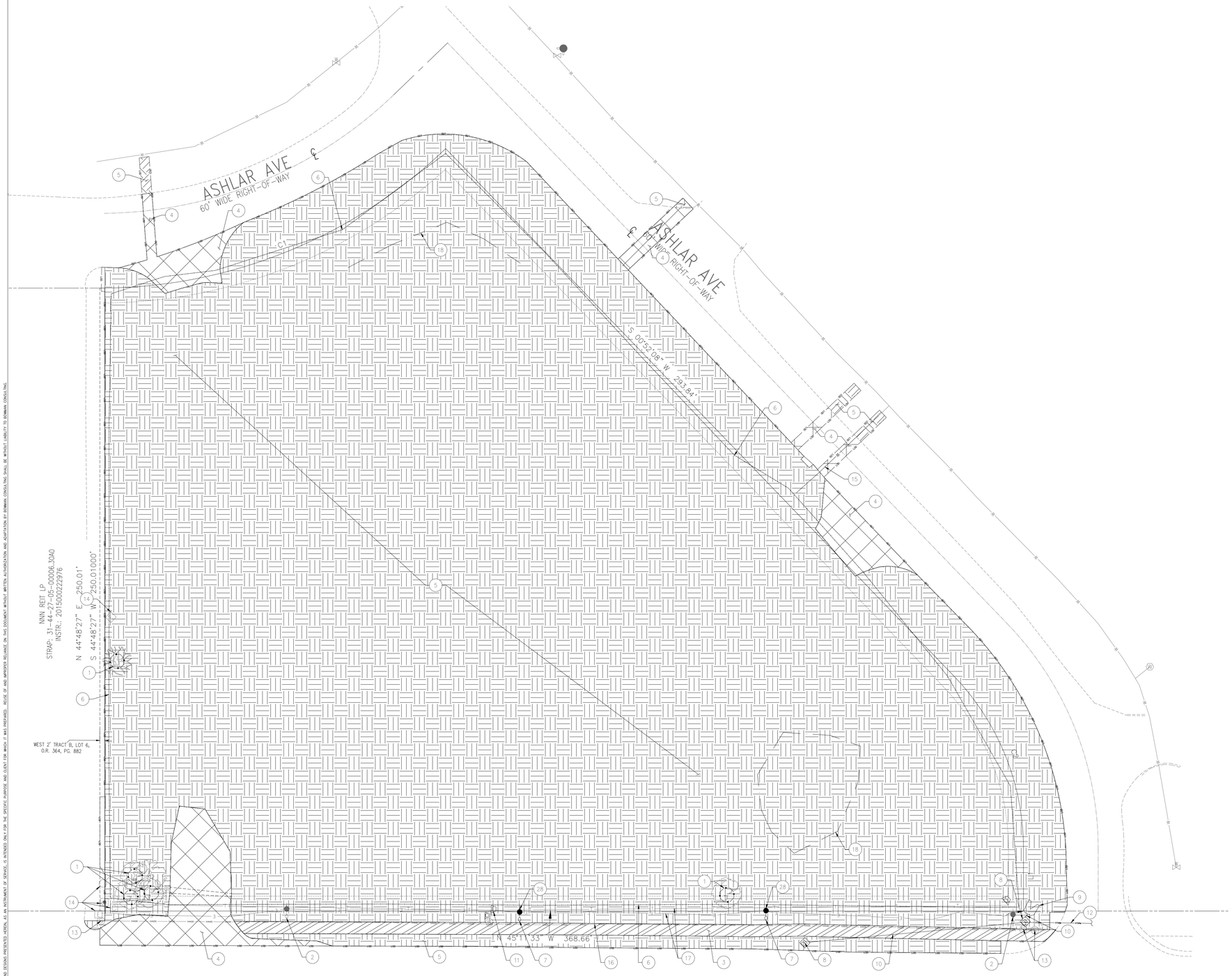
CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
EXISTING CONDITIONS PLAN

SHEET NUMBER
C-1.1

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.



LEGEND

DEMO ASPHALT DRIVE		PROPERTY LINE	
DEMO CONCRETE		EX. CONCRETE SIDEWALK	
DEMO OPEN SPACE		EX. CONCRETE D CURB	
DEMO BUILDING		EX. CONCRETE F CURB	
LIMIT OF DISTURBANCE	LOD LOD	EX. EDGE OF SIDEWALK	
DEMO CURB		EX. SIGN	
DEMO TREE		EX. ELECTRIC HAND HOLE	
DEMO UTILITY		EX. SEWER CLEAN OUT	
TREE PROTECTION BARRIER		EX. LIGHT POLE	
		EX. TREE	
		EX. HEDGE	
		EX. FENCE	
		EX. WATER METER	
		EX. BACKFLOW PREVENTER	
		EX. IRRIGATION VALVE	
		EX. UNKNOWN UTILITY	
		EX. STORM INLET	
		EX. HEADWALL	
		EX. STORM SEWER	
		EX. SANITARY SEWER	
		EX. TOP OF BANK	

DEMOLITION NOTES

1. TREE TO REMAIN
2. LIGHT POLE TO REMAIN
3. CONCRETE PAVEMENT / SIDEWALK TO BE REMOVED
4. ASPHALT PAVEMENT TO BE REMOVED
5. OPEN SPACE TO BE CLEARED AND GRUBBED
6. FENCE TO BE REMOVED
7. LIGHT POLE WITH HAND HOLE TO BE REMOVED AND REPLACED
8. DRAINAGE STRUCTURE TO BE REMOVED
9. MES TO BE REMOVED
10. DRAINAGE PIPE TO BE REMOVED
11. IRRIGATION VALVE TO BE REMOVED
12. DRAINAGE PIPE TO REMAIN
13. SANITARY STRUCTURE TO REMAIN
14. ELECTRICAL/COMM STRUCTURE TO REMAIN
15. WATER SERVICE TO BE REMOVED AND CAPPED
16. SEWER MAIN TO REMAIN
17. BURIED UTILITIES TO REMAIN
18. DIRT MOUND TO BE REMOVED
19. SWALE TO BE REMOVED

GENERAL DEMOLITION NOTES

1. THE LOCATION OF THE UTILITIES SHOWN HAVE BEEN DETERMINED BY INFORMATION GATHERED AND SHALL NOT BE USED AS EXACT. CONTRACTOR SHALL CONTACT THE APPROPRIATE UTILITY COMPANIES TO VERIFY EXACT LOCATIONS PRIOR TO DEMOLITION.
2. THE CONTRACTOR SHALL COORDINATE WITH THE PROPER UTILITY COMPANIES FOR REMOVAL AND RELOCATIONS OF THE RESPECTIVE UTILITY. THE CONTRACTOR SHALL VERIFY ANY WORK THAT MAY BE DONE BY THE UTILITY COMPANIES.
3. CONTRACTOR SHALL PROTECT THE PUBLIC WITH BEST MANAGEMENT PRACTICES.
4. CONTRACTOR SHALL PROTECT AND MAINTAIN ALL STRUCTURES, PAVEMENT, AND VEGETATION THAT IS NOT TO BE DISTURBED AND IS RESPONSIBLE FOR ANY DAMAGES TO THEM.
5. THE CONTRACTOR SHALL PROPERLY DISPOSE OF ALL MATERIALS RESULTING FROM THE WORK, ACCORDING TO GOVERNING AUTHORITIES AND SHALL OBTAIN THE PROPER PERMITS REQUIRED FOR DISPOSAL AND DEMOLITION.
6. THE CONTRACTOR SHALL INSTALL EROSION AND SEDIMENT CONTROL DEVICES PRIOR TO DEMOLITION.
7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING SERVICES TO ANY NECESSARY UTILITIES DURING CONSTRUCTION.
8. FOR ALL ITEMS NOTED TO BE REMOVED, REMOVE NOT ONLY THE ABOVE GROUND ELEMENTS, BUT ALSO REMOVE ALL UNDERGROUND ELEMENTS AS WELL INCLUDING, BUT NOT LIMITED TO; FOUNDATIONS, GRAVEL FILLS, TREE ROOTS, PIPES, TANKS, ETC.
9. BACKFILL ALL EXCAVATIONS RESULTING FROM THE DEMOLITION WORK MEETING THE REQUIREMENTS FOR FILL OUTLINED IN THE GEOTECHNICAL INVESTIGATION REPORT FOR THIS SITE.
10. ASBESTOS AND ANY OTHER HAZARDOUS MATERIAL SHALL BE PROPERLY PERMITTED AND REMOVED BY THE CONTRACTOR. CONTRACTOR SHALL SECURE ALL PERMITS FOR DEMOLITION AND REMOVAL OF MATERIALS FROM THE SITE.
11. LIMIT SAW-CUT AND PAVEMENT REMOVAL TO ONLY THOSE AREAS WHERE IT IS REQUIRED AS SHOWN ON THESE PLANS. ANY DAMAGE TO ANY SURROUNDING AREAS SHALL BE REPAIRED / REPLACED AT THE RESPONSIBILITY OF THE CONTRACTOR. CONTRACTOR SHALL PROTECT ADJACENT STRUCTURES, PAVEMENT, UTILITIES, LANDSCAPE, ETC. FROM DAMAGE DURING CONSTRUCTION.
12. CONTRACTOR SHALL ENSURE THAT SERVICES TO ALL UTILITIES TO BE REMOVED HAS BEEN DISCONTINUED AND SHUT OFF. ALL UTILITY LINES SHALL BE CAPPED PER UTILITY COMPANY STANDARDS.
13. PERIMETER TREES & LANDSCAPING TO REMAIN WHEREVER POSSIBLE.

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast. It's free. It's the law.
www.callsunshine.com

GRAPHIC SCALE
20 0 10 20 40
(IN FEET)
1 inch = 20 ft.

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

FOR PERMIT

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

SHEET
DEMOLITION PLAN

SHEET NUMBER
C-1.2

GENERAL SITE PLAN NOTES

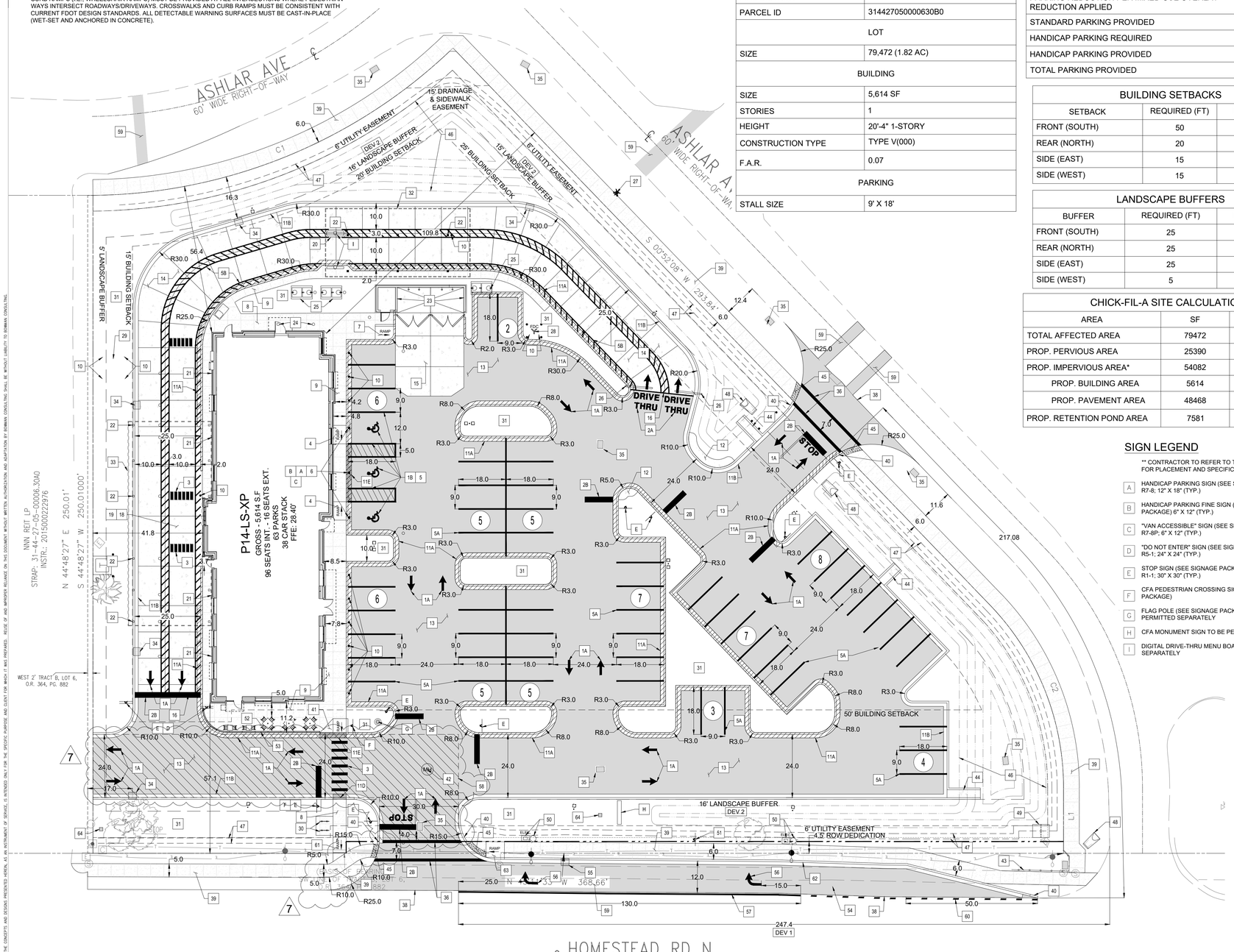
1. ALL CONSTRUCTION IN THE FDOT ROW SHALL CONFORM TO THE LATEST EDITIONS OF THE FDOT DESIGN STANDARDS (INDICES), THE FDOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AND THE FDOT UTILITY ACCOMMODATION MANUAL.
2. ALL RELOCATIONS, MODIFICATIONS, OR CLOSURES OF CONNECTIONS TO THE STATE HIGHWAY SYSTEM SHALL CONFORM TO F.A.C. (FLORIDA ADMINISTRATIVE CODE) RULE 14-96.
3. GC TO UPDATE/ADD/REMOVE ANY SIGNS OR PAVEMENT MARKINGS THAT NEED TO BE UPDATED/ADDED/REMOVED IN THE FIELD WITH THE PROPOSED CHANGES.
4. ALL AFFECTED SIDEWALKS, RAMPS AND CROSSWALKS WILL BE BUILT AND INSPECTED TO MEET CURRENT ADA REQUIREMENTS.
5. SIDEWALK AND CURB REPLACEMENT LIMITS MUST EXTEND TO THE NEAREST CONSTRUCTION JOINT TO PREVENT SMALL SECTIONS OF EXISTING PANELS/CURB FROM BEING LEFT IN PLACE.
6. PREFORMED THERMOPLASTIC MATERIAL PAVEMENT MARKINGS SHALL BE APPLIED FOR ALL PAVEMENT MARKINGS WITHIN THE COUNTY RIGHT-OF-WAY.
7. INSTALLATION OF ALL TRAFFIC SIGNS WITHIN THE COUNTY ROW MUST COMPLY WITH THE CURRENT LEE COUNTY DESIGN STANDARD FOR SIGN INSTALLATION.
8. ALL SIDEWALK CONSTRUCTION/RECONSTRUCTION MUST BE CONSTRUCTED WITH A CROSS SLOPE OF 2% OR LESS, AND UNLESS AT A CURB RAMP, MUST HAVE A RUNNING SLOPE OF 5% OR LESS. ALL SIDEWALK CONSTRUCTION/RECONSTRUCTION MUST CONFORM TO LATEST EDITION OF FLORIDA GREENBOOK AND ADA STANDARDS, INCLUDING CURB RAMPS AND DETECTABLE WARNING SURFACES. CROSSWALKS AND CURB RAMPS (I.E., WHEELCHAIR RAMPS) MUST BE PROVIDED AT ALL INTERSECTIONS WHERE PEDESTRIAN WAYS INTERSECT ROADWAYS/DRIVEWAYS. CROSSWALKS AND CURB RAMPS MUST BE CONSISTENT WITH CURRENT FDOT DESIGN STANDARDS. ALL DETECTABLE WARNING SURFACES MUST BE CAST-IN-PLACE (WET-SET AND ANCHORED IN CONCRETE).

DEVIATIONS

DEV 1 — DEVIATION 1 REQUESTS RELIEF FROM TABLE 1 OF LDC SECTION 10-285, CONNECTION SEPARATION, WHICH REQUIRES A MINIMUM CONNECTION SEPARATION DISTANCE OF 330 FEET ON MINOR ARTERIAL ROADWAYS IN FUTURE URBAN AREAS WHERE THE POSTED SPEED LIMIT IS LESS THAN 45 MILES PER HOUR, TO ALLOW FOR A MINIMUM CONNECTION SEPARATION DISTANCE OF 119 FEET FOR THE PROPOSED ACCESS LOCATED ON HOMESTEAD ROAD N.

DEV 2 — DEVIATION 2 REQUESTS RELIEF FROM LDC SECTION 34-1353(E)(1), CONVENIENCE FOOD AND BEVERAGE STORES, AUTOMOTIVE SERVICE STATIONS, FAST FOOD RESTAURANTS, AND CAR WASHES, WHICH REQUIRES LANDSCAPING ADJACENT TO RIGHTS-OF-WAY EXTERNAL TO THE DEVELOPMENT PROJECT TO BE LOCATED WITHIN A LANDSCAPE BUFFER THAT IS A MINIMUM 25 FEET IN WIDTH, TO ALLOW FOR A LANDSCAPE BUFFER THAT IS A MINIMUM WIDTH OF 15 FEET WITH ENHANCED PLANTINGS.

DEV 3 — DEVIATION 3 REQUESTS RELIEF FROM LDC SECTION 34-1353(E)(2), CONVENIENCE FOOD AND BEVERAGE STORES, AUTOMOTIVE SERVICE STATIONS, FAST FOOD RESTAURANTS, AND CAR WASHES, WHICH REQUIRES FOR PROJECTS WITH AN OPEN DRAINAGE SYSTEM, AN UNDULATING BERM WITH A MAXIMUM SLOPE OF 3:1 MUST BE CONSTRUCTED AND MAINTAINED AT A MINIMUM HEIGHT OF TWO FEET AS MEASURED FROM THE GRADE OF THE PARKING LOT, TO ALLOW FOR NO BERM TO BE REQUIRED.



SITE SUMMARY	
JURISDICTION	LEE COUNTY
ZONING	C-2 (COMMERCIAL)
FLU	FUTURE URBAN AREA: INTENSIVE DEVELOPMENT
OVERLAY DISTRICT	MIXED USE OVERLAY
USE	DRIVE THRU RESTAURANT
TYPE OF CONSTRUCTION	COMMERCIAL
FIRE DISTRICT	LEHIGH ACRES FIRE DISTRICT
PARCEL ID	314427050000630B0
LOT	
SIZE	79,472 (1.82 AC)
BUILDING	
SIZE	5,614 SF
STORIES	1
HEIGHT	20'-4" 1-STORY
CONSTRUCTION TYPE	TYPE V(000)
F.A.R.	0.07
PARKING	
STALL SIZE	9' X 18'

PARKING CALCULATIONS			
USE	QTY	RATIO	REQUIRED
SQUARE FOOTAGE	5,614 SF	13 PARKING SPACE FOR EVERY 1,000 SF	73 SPACES (7)
FLUM NON-RESIDENTIAL MIXED-USE PARKING REDUCTION (INTENSIVE) = 0.50			37
TOTAL PARKING REQUIRED (WITHOUT REDUCTION APPLIED)			73
PARKING REDUCTION PER MIXED-USE OVERLAY REDUCTION APPLIED			37
STANDARD PARKING PROVIDED			60
HANDICAP PARKING REQUIRED			3
HANDICAP PARKING PROVIDED			3
TOTAL PARKING PROVIDED			63

BUILDING SETBACKS		
SETBACK	REQUIRED (FT)	PROVIDED (FT)
FRONT (SOUTH)	50	57.1
REAR (NORTH)	20	56.4
SIDE (EAST)	15	109.8
SIDE (WEST)	15	41.8

LANDSCAPE BUFFERS		
BUFFER	REQUIRED (FT)	PROVIDED (FT)
FRONT (SOUTH)	25	16.0
REAR (NORTH)	25	16.3
SIDE (EAST)	25	15.0
SIDE (WEST)	5	15.0

CHICK-FIL-A SITE CALCULATIONS			
AREA	SF	ACRES	%
TOTAL AFFECTED AREA	79472	1.82	100.0
PROP. PERVIOUS AREA	25390	0.58	31.9
PROP. IMPERVIOUS AREA*	54082	1.24	68.1
PROP. BUILDING AREA	5614	0.13	7.1
PROP. PAVEMENT AREA	48468	1.11	61.0
PROP. RETENTION POND AREA	7581	0.17	9.5

SIGN LEGEND

** CONTRACTOR TO REFER TO THE SIGNAGE PACKAGE FOR PLACEMENT AND SPECIFICATIONS OF ALL SIGNS **

- A — HANDICAP PARKING SIGN (SEE SIGNAGE PACKAGE) R7-8; 12" X 18" (TYP.)
- B — HANDICAP PARKING FINE SIGN (SEE SIGNAGE PACKAGE) 6" X 12" (TYP.)
- C — "VAN ACCESSIBLE" SIGN (SEE SIGNAGE PACKAGE) R7-8P; 6" X 12" (TYP.)
- D — "DO NOT ENTER" SIGN (SEE SIGNAGE PACKAGE) R5-1; 24" X 24" (TYP.)
- E — STOP SIGN (SEE SIGNAGE PACKAGE) R1-1; 30" X 30" (TYP.)
- F — CFA PEDESTRIAN CROSSING SIGN (SEE SIGNAGE PACKAGE)
- G — FLAG POLE (SEE SIGNAGE PACKAGE) TO BE PERMITTED SEPARATELY
- H — CFA MONUMENT SIGN TO BE PERMITTED SEPARATELY
- I — DIGITAL DRIVE-THRU MENU BOARDS TO BE PERMITTED SEPARATELY

LEGEND

PROP. ASPHALT DRIVE
PROP. CONC. SIDEWALK
PROP. CONC. PAVEMENT
PROP. CROSS ACCESS EASEMENT
PROP. TYPE "F" CURB
PROP. TYPE "D" CURB
PROP. CLEARANCE BAR
PROP. PARKING STRIPE
PROP. HANDICAP MARKING
PROP. SITE SIGNAGE
PROP. DIRECTIONAL ARROW
PROP. BOLLARD
PROP. PARKING COUNT (9)
PROP. GREASE TRAP (G)
PROP. LIGHT / LIGHT POLE

PROPERTY LINE
EX. CONCRETE SIDEWALK
EX. CONCRETE D CURB
EX. CONCRETE F CURB
EX. EDGE OF SIDEWALK
EX. SIGN
EX. ELECTRIC HAND HOLE
EX. SEWER CLEAN OUT
EX. LIGHT POLE
EX. TREE
EX. HEDGE
EX. FENCE
EX. WATER METER
EX. BACKFLOW PREVENTER
EX. IRRIGATION VALVE
EX. UNKNOWN UTILITY

SITE PLAN DESIGN NOTES & KEY PLAN

1A DIRECTIONAL ARROW (1A C-4.0)
1B PAINTED HANDICAP PARKING SYMBOL (1B C-4.0)
2A DRIVE-THRU GRAPHICS (2A C-4.0)
2B STOP BAR GRAPHIC (2B C-4.0)
3 CROSSWALK MARKINGS (3 C-4.0)
4 SIDEWALK ACCESSIBLE RAMP (1 C-4.1)
5 STANDARD OR HANDICAP PARKING STALL PER CODE (5 C-4.0)
5A 4" SOLID WHITE STRIPING
5B 4" SOLID YELLOW STRIPING
6 BOLLARD MOUNTED SIGN (7 C-4.0)
7 RETURNED CURB RAMP (10 C-4.0)
8 CONCRETE SIDEWALK (4 C-4.1)
9 CONCRETE SIDEWALK w/ CURB & GUTTER (5 C-4.1)
10 CONCRETE BOLLARD (7 C-4.1)
11 CONCRETE CURB & GUTTER (8 C-4.1)
11A SPILLING CURB & GUTTER
11B CATCHING CURB & GUTTER
11C DEPRESSED SPILLING CURB & GUTTER
11D DEPRESSED CATCHING CURB & GUTTER
11E SPILLING GUTTER SECTION AT ACCESSIBLE RAMP
11F CATCHING GUTTER SECTION AT ACCESSIBLE RAMP
11G MOUNTABLE CURB & GUTTER
12 LANDSCAPE & IRRIGATION PROTECTOR (9 C-4.1)
13 TYPICAL HMAC PAVEMENT SECTION (1 C-4.2)
14 CONCRETE PAVEMENT DRIVE-THRU LANE (3 C-4.2)
15 CONCRETE APRON AT TRASH ENCLOSURE (4 C-4.2)
16 PAVEMENT EDGE DETAIL (START & END OF DRIVE-THRU LANES) (5 C-4.2)
17 CONCRETE PAVEMENT SECTIONS (6 C-4.2)
18 DRIVE-THRU PLAN - FLUSH WITH FFE (1 C-4.3)
19 DRIVE-THRU ISOMETRIC (2 C-4.3)
20 DRIVE-THRU ORDER POINT ISLAND (3 C-4.3)
21 BUILDING DOWNSPOUT CONNECTION (TO SITE DRAINAGE SYSTEM) (5 C-4.3)
22 CANOPY DOWNSPOUT CONNECTION (TO SITE DRAINAGE SYSTEM) (6 C-4.3)
23 SCREENED REFUSE ENCLOSURE (REFER TO ARCH PLANS FOR ADDITIONAL DETAILS) (7 C-4.3)
24 CLEAN-OUT (OUTSIDE OF BUILDING) (8 C-4.3)
25 GREASE TRAP
26 DRIVE-THRU CLEARANCE BAR (REFER TO SIGNAGE PACKAGE)
27 FIRE HYDRANT
28 FIRE DEPARTMENT CONNECTION
29 ELECTRICAL TRANSFORMER
30 BIKE RACK (BELSON OUTDOORS MODEL # ORN-2-ST-G)
31 LANDSCAPED AREA
32 FREE-STANDING ORDER POINT CANOPY
33 FREE-STANDING OUTSIDE MEAL DELIVERY CANOPY
34 FDOT TYPE 9 CURB INLET
35 FDOT TYPE C GRATE INLET
36 THERMOPLASTIC 12" SOLID PEDESTRIAN CROSSWALK LINE PER FDOT STANDARD INDEX 711-001 PAGE 2 OF 13
37 CONNECT TO EXISTING SIDEWALK (SEE GENERAL SITE PLAN NOTE #5)
38 CONNECT TO EXISTING PAVEMENT
39 6" THICK CONCRETE SIDEWALK PER FDOT STANDARD INDEX 522-001
40 3' CURB TRANSITION
41 CURB RAMP W/ FLARED SIDES (9 C-4.0)
42 48" STORM MANHOLE
43 FDOT HANDRAILS (1 C-5.5)
44 3' WIDE CONCRETE FLUME
45 FDOT DETECTABLE WARNING PER FDOT STANDARD INDEX STANDARD INDEX 522-002
46 RETENTION POND
47 BERM (MIN. EL. = 26.75')
48 MITERED END SECTION
49 CONCRETE OVERFLOW BERM WITH 3" BLEEDER (7 C-5.0)
50 RELOCATE EXISTING LIGHT POLE WITH HAND HOLE
51 RELOCATE EXISTING SIGN (SEE GENERAL SITE PLAN NOTE #7)
52 TYPICAL PATIO AREA
53 ALUMINUM HANDRAIL
54 REPLACE EXISTING INLET TOP WITH MANHOLE TOP
55 FDOT TYPE 3 CURB INLET
56 THERMOPLASTIC DIRECTIONAL ARROW PER FDOT STANDARD INDEX 711-001
57 THERMOPLASTIC 6" WHITE SOLID LINE PER FDOT STANDARD INDEX 711-001
58 CROSS ACCESS EASEMENT
59 ASPHALT PAVEMENT (MATCH SPECS OF EXISTING PAVEMENT)
60 THERMOPLASTIC 6" WHITE (2'-4") SOLID LINE PER FDOT STANDARD INDEX 711-001
61 6' CONCRETE SIDEWALK RAMP @ 8.33% MAX.
62 FDOT TYPE F CURB AND GUTTER
63 FDOT CURB RAMP CR-G
64 YARD INLET

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG — It's fast, it's free, it's the law. — www.callsunshine.com

GRAPHIC SCALE
20 0 10 20 40
(IN FEET)
1 inch = 20 ft.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman
Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com

© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU
1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE		
NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
SITE PLAN

SHEET NUMBER
C-2.0

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING, SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

LEGEND

PROP. ASPHALT DRIVE	PROPERTY LINE
PROP. CONC. SIDEWALK	EX. CONCRETE SIDEWALK
PROP. CONC. PAVEMENT	EX. CONCRETE D CURB
PROP. TYPE "F" CURB	EX. CONCRETE F CURB
PROP. TYPE "D" CURB	EX. EDGE OF SIDEWALK
PROP. PARKING STRIPE	EX. SIGN
PROP. HANDICAP MARKING	EX. ELECTRIC HAND HOLE
PROP. TYPE C INLET	EX. SEWER CLEAN OUT
PROP. GREASE TRAP	EX. LIGHT POLE
PROP. GRADE	EX. TREE
PROP. TYPE 9 CURB INLET	EX. HEDGE
PROP. STORM SEWER	EX. FENCE
PROP. CLEANOUT	EX. WATER METER
PROP. SANITARY SEWER	EX. BACKFLOW PREVENTER
PROP. WATER	EX. IRRIGATION VALVE
PROP. ELECTRIC	EX. UNKNOWN UTILITY
PROP. GAS LINE	

DATUM NOTE

1. THE ELEVATIONS SHOWN HEREON ARE REFERENCED TO THE NORTH AMERICA VERTICAL DATUM OF 1988 (N.A.V.D. 88) AND ARE BASED UPON THE FOLLOWING LEE COUNTY PUBLISHED BENCHMARKS:

   "TBM 18-52"
   LOCATED AT THE INTERSECTION HUNTDALE ST. AND INWOOD AV S AT THE NORTH SIDE OF THE CATCH BASIN.

   ELEVATION: 23.313' (N.A.V.D. 88)

2. ELEVATION DATUM CONVERSION NGVD29 = NAVD88 + 1.22'

STORMWATER SUMMARY	
SEASONAL HIGH WATER TABLE (SHWT)	21.00'
CONTROL ELEVATION	26.72'
MIN. FINISHED FLOOR EL.	27.40'
PROP. FINISHED FLOOR EL.	28.40'
FEMA FLOOD EL.	N/A (ZONE X)
MIN. PARKING LOT EL.	25.32'
PROP. MIN. PARKING LOT EL.	25.51'

STORM EVENT	RAINFALL DEPTH	PEAK STAGE EL.
5 YEAR - 1 DAY	5.5"	25.31'
25 YEAR - 3 DAY	11.0"	26.72'
100 YEAR - 3 DAY	14.0"	27.38'
ALLOWABLE DISCHARGE RATE	0.09 CFS	
PEAK DISCHARGE RATE	0.29 CFS*	

* 3" BLEEDER UTILIZED FOR DISCHARGE RATE BEING OVER THE MAXIMUM ALLOWABLE

STORM DRAIN PIPE SCHEDULE

PIPE NAME	SIZE	LENGTH	SLOPE
EX STRM-1	EX. 12' X 19"' ELLIPTICAL RCP *		
STRM-1	18" HDPE	55.5'	0.00%
STRM-2	18" HDPE	112.5'	0.00%
STRM-3	24" HDPE	3.5'	0.00%
STRM-4	18" HDPE	154.0'	0.00%
STRM-5	18" HDPE	94.7'	0.00%
STRM-6	12" HDPE	16.9'	4.43%
STRM-7	18" HDPE	111.1'	0.00%
STRM-8	18" HDPE	80.6'	0.00%
STRM-9	8" HDPE	9.6'	1.00%
STRM-10	8" HDPE	12.1'	1.00%
STRM-11	6" HDPE	19.6'	1.00%
STRM-12	8" HDPE	7.6'	1.00%
STRM-13	6" HDPE	10.1'	1.00%
STRM-14	8" HDPE	4.6'	1.00%
STRM-15	6" HDPE	19.7'	1.00%
STRM-16	8" HDPE	22.4'	1.00%
STRM-17	6" HDPE	19.8'	1.00%
STRM-18	8" HDPE	10.3'	1.00%
STRM-19	6" HDPE	10.0'	1.00%
STRM-20	8" HDPE	9.1'	0.70%
STRM-21	8" HDPE	27.0'	1.00%
STRM-22	6" HDPE	3.0'	1.03%
STRM-23	8" HDPE	19.0'	1.00%
STRM-24	6" HDPE	3.0'	1.04%
STRM-25	15" RCP CLASS III	49.3'	0.00%

* GC TO FIELD VERIFY EX. PIPE SIZE/LOCATION/MATERIAL/INVERT

STORM DRAIN PIPE SCHEDULE

PIPE NAME	SIZE	LENGTH	SLOPE
STRM-26	18" HDPE	17.4'	0.50%
STRM-27	24" HDPE	5.0'	0.00%
STRM-28	24" HDPE	5.0'	0.00%
STRM-30	6" HDPE	10.1'	1.00%
STRM-31	8" HDPE	23.7'	1.00%
STRM-32	4" HDPE	14.3'	3.50%
STRM-33	4" HDPE	1.3'	1.00%
STRM-34	4" HDPE	5.9'	1.00%
STRM-35	4" HDPE	19.7'	1.00%
STRM-36	8" HDPE	13.7'	1.00%
STRM-37	6" HDPE	19.8'	1.00%
STRM-38	8" HDPE	13.2'	1.00%
STRM-39	6" HDPE	10.0'	1.00%
STRM-40	12 "x 18" ELLIPTICAL RCP	16.5'	13.66%
STRM-41	15" RCP CLASS III	79.7'	0.00%
STRM-42	6" HDPE	3.0'	1.04%
STRM-43	4" HDPE	4.6'	1.00%
STRM-44	4" HDPE	1.1'	1.00%
STRM-45	8" HDPE	19.0'	1.00%
STRM-46	12" ADS N-12	4.1'	0.00%
STRM-47	12" ADS N-12	2.5'	0.00%
STRM-48	12" ADS N-12	2.5'	0.00%
STRM-49	18" HDPE	21.9'	0.00%
STRM-50	12' x 18"' ELLIPTICAL RCP	189.9'	0.35%
STRM-51	15" RCP CLASS III	57.3'	0.00%

SOIL CLASSIFICATION NOTE

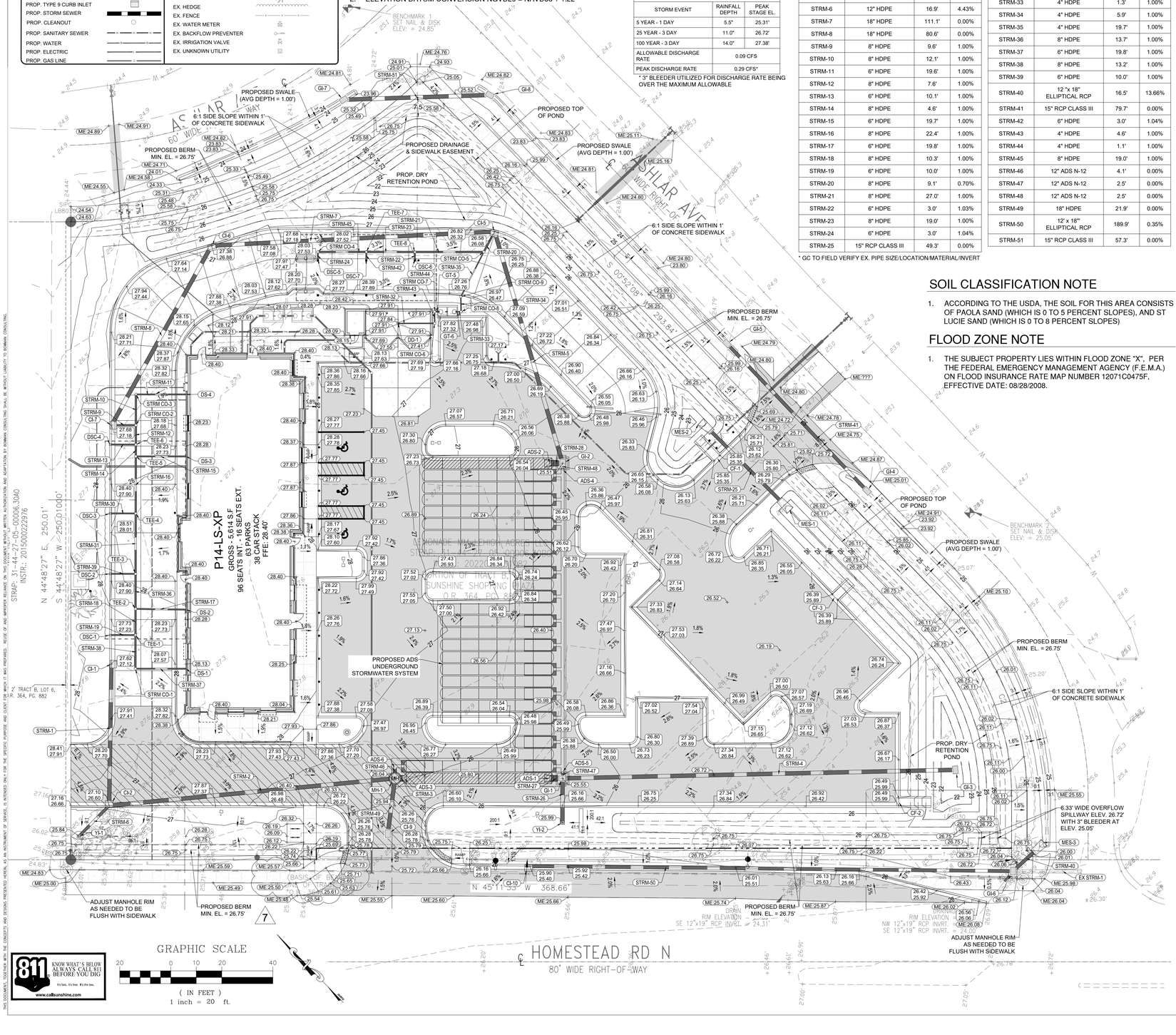
1. ACCORDING TO THE USDA, THE SOIL FOR THIS AREA CONSISTS OF PAOLA SAND (WHICH IS 0 TO 5 PERCENT SLOPES), AND ST LUCIE SAND (WHICH IS 0 TO 8 PERCENT SLOPES)

FLOOD ZONE NOTE

1. THE SUBJECT PROPERTY LIES WITHIN FLOOD ZONE "X", PER THE FEDERAL EMERGENCY MANAGEMENT AGENCY (F.E.M.A.) ON FLOOD INSURANCE RATE MAP NUMBER 12071C0475F, EFFECTIVE DATE: 08/28/2008.

STORMWATER STRUCTURE TABLE

STRUCTURE ID	STRUCTURE TYPE	RIM ELEV.	PIPE INVERTS	PIPE INFORMATION
ADS-1	CONNECTION TO UNDERGROUND ADS SYSTEM		(SE) = 21.36	5 LF, 24" HDPE @ 0.00%
ADS-2	CONNECTION TO UNDERGROUND ADS SYSTEM		(SE) = 21.36	5 LF, 24" HDPE @ 0.00%
ADS-3	CONNECTION TO UNDERGROUND ADS SYSTEM		(NW) = 21.36	4 LF, 24" HDPE @ 0.00%
ADS-4	CONNECTION TO UNDERGROUND ADS SYSTEM		(NE) = 22.37	2 LF, 12" ADS N-12 @ 0.00%
ADS-5	CONNECTION TO UNDERGROUND ADS SYSTEM		(SW) = 22.37	3 LF, 12" ADS N-12 @ 0.00%
ADS-6	CONNECTION TO UNDERGROUND ADS SYSTEM		(SW) = 22.37	4 LF, 12" ADS N-12 @ 0.00%
CF-1	3' WIDE CONCRETE FLUME	RIM = 25.35		
CF-2	3' WIDE CONCRETE FLUME	RIM = 25.39		
CF-3	3' WIDE CONCRETE FLUME	RIM = 26.06		
CI-1	FDOT TYPE 9 CURB INLET W/ 48" DIA. TYPE P STRUCTURE BOTTOM	RIM = 27.12	(SW) = 20.63	55 LF, 18" HDPE @ 0.00%
CI-2	FDOT TYPE 9 CURB INLET W/ 48" DIA. TYPE P STRUCTURE BOTTOM	RIM = 26.60	(NE) = 20.63 (SE) = 19.06 (W) = 20.63	55 LF, 18" HDPE @ 0.00% 113 LF, 18" HDPE @ 0.00% 17 LF, 12" HDPE @ 4.43%
CI-5	FDOT TYPE 9 CURB INLET W/ 72" DIA. TYPE J STRUCTURE BOTTOM	RIM = 26.08	(W) = 22.12 (NW) = 20.63 (SW) = 20.63	9 LF, 8" HDPE @ 0.70% 111 LF, 18" HDPE @ 0.00% 95 LF, 18" HDPE @ 0.00%
CI-6	FDOT TYPE 9 CURB INLET W/ 48" DIA. TYPE P STRUCTURE BOTTOM	RIM = 26.88	(SE) = 20.63 (W) = 20.63	111 LF, 18" HDPE @ 0.00% 81 LF, 18" HDPE @ 0.00%
CI-7	FDOT TYPE 9 CURB INLET W/ 48" DIA. TYPE P STRUCTURE BOTTOM	RIM = 27.18	(E) = 20.63 (SE) = 23.04	81 LF, 18" HDPE @ 0.00% 10 LF, 8" HDPE @ 1.00%
CI-9	GRATE INLET	RIM = 25.61	(NE) = 20.63	22 LF, 18" HDPE @ 0.00%
CI-10	FDOT TYPE 3 CURB INLET W/ 48" DIA. TYPE P STRUCTURE BOTTOM	RIM = 25.40	(SE) = 21.40	190 LF, 12' x 18"' ELLIPTICAL RCP @ 0.35%
DD-1	DUMPSTER DRAIN	RIM = 27.84	(SE) = 24.38	14 LF, 4" HDPE @ 3.50%
DS-1	BUILDING DOWNSPOUT CONNECTION		(NW) = 24.39	20 LF, 6" HDPE @ 1.00%
DS-2	BUILDING DOWNSPOUT CONNECTION		(NW) = 24.15	20 LF, 6" HDPE @ 1.00%
DS-3	BUILDING DOWNSPOUT CONNECTION		(NW) = 23.56	20 LF, 6" HDPE @ 1.00%
DS-4	BUILDING DOWNSPOUT CONNECTION		(NW) = 23.46	20 LF, 6" HDPE @ 1.00%
DSC-1	CANOPY DOWNSPOUT CONNECTION		(SE) = 24.15	10 LF, 6" HDPE @ 1.00%
DSC-2	CANOPY DOWNSPOUT CONNECTION		(SE) = 23.91	10 LF, 6" HDPE @ 1.00%
DSC-3	CANOPY DOWNSPOUT CONNECTION		(SE) = 23.68	10 LF, 6" HDPE @ 1.00%
DSC-4	CANOPY DOWNSPOUT CONNECTION		(SE) = 23.41	10 LF, 6" HDPE @ 1.00%
DSC-5	CANOPY DOWNSPOUT CONNECTION		(NE) = 22.86	3 LF, 6" HDPE @ 1.04%
DSC-6	CANOPY DOWNSPOUT CONNECTION		(NE) = 22.48	3 LF, 6" HDPE @ 1.03%
DSC-7	CANOPY DOWNSPOUT CONNECTION		(NE) = 22.67	3 LF, 6" HDPE @ 1.04%
GI-1	FDOT TYPE C INLET W/ 60" DIA. TYPE P STRUCTURE BOTTOM	RIM = 25.55	(NW) = 21.36 (SE) = 20.63 (NE) = 22.37 (SW) = 21.05	5 LF, 24" HDPE @ 0.00% 154 LF, 18" HDPE @ 0.00% 3 LF, 12" ADS N-12 @ 0.00% 17 LF, 18" HDPE @ 0.50%
GI-2	FDOT TYPE C INLET W/ 60" DIA. TYPE P STRUCTURE BOTTOM	RIM = 25.51	(NW) = 21.36 (NE) = 20.63 (SW) = 22.37	5 LF, 24" HDPE @ 0.00% 95 LF, 18" HDPE @ 0.00% 2 LF, 12" ADS N-12 @ 0.00%
GI-3	FDOT TYPE C INLET W/ 48" DIA. TYPE P STRUCTURE BOTTOM	RIM = 22.19	(NW) = 20.63	154 LF, 18" HDPE @ 0.00%
GI-4	FDOT TYPE C INLET W/ 48" DIA. TYPE P STRUCTURE BOTTOM	RIM = 24.05	(N) = 21.00	80 LF, 15" RCP CLASS III @ 0.00%
GI-5	FDOT TYPE C INLET W/ 48" DIA. TYPE P STRUCTURE BOTTOM	RIM = 23.80	(S) = 21.00	80 LF, 15" RCP CLASS III @ 0.00%
GI-6	EXISTING GRATE INLET	RIM = 25.95	(E) = 21.95 (SE) = 24.00 (NW) = 20.74	17 LF, 12 "x 18" ELLIPTICAL RCP @ 13.66% 27 LF, EX. 12' X 19"' ELLIPTICAL RCP @ 1.26% 190 LF, 12' x 18"' ELLIPTICAL RCP @ 0.35%
GI-7	GRATE INLET	RIM = 23.81	(SE) = 21.00	57 LF, 15" RCP CLASS III @ 0.00%
GI-8	GRATE INLET	RIM = 23.80	(NW) = 21.00	57 LF, 15" RCP CLASS III @ 0.00%
GT-5	1,250 GAL. GREASE TRAP INLET		(NW) = 23.82	1 LF, 4" HDPE @ 1.00%
GT-6	1,250 GAL. GREASE TRAP OUTLET		(SE) = 23.65	1 LF, 4" HDPE @ 1.00%
MES-1	15" FES		(N) = 23.20	49 LF, 15" RCP CLASS III @ 0.00%
MES-2	15" FES		(S) = 23.20	49 LF, 15" RCP CLASS III @ 0.00%
MES-3	12" MITERED END SECTION		(W) = 24.21	17 LF, 12 "x 18" ELLIPTICAL RCP @ 13.66%
MH-1	STORM MANHOLE W/ 48" DIA. TYPE P STRUCTURE BOTTOM	RIM = 26.04	(SE) = 21.36 (NW) = 19.06 (NE) = 22.37 (SW) = 20.63	4 LF, 24" HDPE @ 0.00% 113 LF, 18" HDPE @ 0.00% 4 LF, 12" ADS N-12 @ 0.00% 22 LF, 18" HDPE @ 0.00%
STRM CO-1	8"X6" 90 DEG. FITTING WITH STRM CLEANOUT	RIM = 27.31	(SE) = 24.19 (NE) = 24.19	20 LF, 6" HDPE @ 1.00% 13 LF, 6" HDPE @ 1.00%
STRM CO-2	8"X8"X6" TEE WITH STRM CLEANOUT	RIM = 27.37	(NW) = 23.14 (SW) = 23.14 (NE) = 23.14	10 LF, 8" HDPE @ 1.00% 8 LF, 8" HDPE @ 1.00% 12 LF, 8" HDPE @ 1.00%
STRM CO-3	8"X6" 90 DEG. FITTING WITH STRM CLEANOUT	RIM = 27.50	(SW) = 23.26 (SE) = 23.26	12 LF, 8" HDPE @ 1.00% 20 LF, 6" HDPE @ 1.00%
STRM CO-4	8"X6" 90 DEG. FITTING WITH STRM CLEANOUT	RIM = 27.50	(SE) = 22.83 (SW) = 22.83	19 LF, 8" HDPE @ 1.00% 3 LF, 6" HDPE @ 1.04%
STRM CO-5	8"X8"X4" WYE WITH STRM CLEANOUT	RIM = 26.16	(E) = 22.18 (NW) = 22.18 (W) = 23.38	9 LF, 8" HDPE @ 0.70% 27 LF, 8" HDPE @ 1.00% 20 LF, 4" HDPE @ 1.00%
STRM CO-6	4" 90 DEG. FITTING WITH STRM CLEANOUT	RIM = 27.57	(NW) = 23.88 (NE) = 23.88	14 LF, 4" HDPE @ 3.50% 5 LF, 4" HDPE @ 1.00%
STRM CO-7	4" 90 DEG. FITTING WITH STRM CLEANOUT	RIM = 27.63	(SW) = 23.83 (SE) = 23.83	5 LF, 4" HDPE @ 1.00% 1 LF, 4" HDPE @ 1.00%
STRM CO-8	4" 90 DEG. FITTING WITH STRM CLEANOUT	RIM = 27.51	(NE) = 23.64 (NW) = 23.64	6 LF, 4" HDPE @ 1.00% 1 LF, 4" HDPE @ 1.00%
STRM CO-9	4" 22.5 DEG. FITTING (DEFLECTED AS NEEDED) WITH STRM CLEANOUT	RIM = 27.12	(E) = 23.58 (SW) = 23.58	20 LF, 4" HDPE @ 1.00% 6 LF, 4" HDPE @ 1.00%
TEE-1	8"X8"X6" STRM TEE		(NE) = 24.05 (NW) = 24.05 (SW) = 24.05	10 LF, 8" HDPE @ 1.00% 10 LF, 6" HDPE @ 1.00% 13 LF, 6" HDPE @ 1.00%
TEE-2	8"X8"X6" STRM TEE		(NE) = 23.95 (SW) = 23.95 (SE) = 23.95	14 LF, 8" HDPE @ 1.00% 10 LF, 8" HDPE @ 1.00% 20 LF, 6" HDPE @ 1.00%
TEE-3	8"X8"X6" STRM TEE		(NE) = 23.82 (SW) = 23.81 (NW) = 23.81	24 LF, 8" HDPE @ 1.00% 14 LF, 8" HDPE @ 1.00% 10 LF, 6" HDPE @ 1.00%
TEE-4	8"X8"X6" STRM TEE		(NE) = 23.58 (SW) = 23.58 (NW) = 23.58	22 LF, 8" HDPE @ 1.00% 24 LF, 8" HDPE @ 1.00% 10 LF, 6" HDPE @ 1.00%
TEE-5	8"X8"X6" STRM TEE		(NE) = 23.36 (SW) = 23.36 (SE) = 23.36	5 LF, 8" HDPE @ 1.00% 22 LF, 8" HDPE @ 1.00% 20 LF, 6" HDPE @ 1.00%
TEE-6	8"X8"X6" STRM TEE		(NE) = 23.21 (SW) = 23.31 (NW) = 23.31	8 LF, 8" HDPE @ 1.00% 5 LF, 8" HDPE @ 1.00% 10 LF, 6" HDPE @ 1.00%
TEE-7	8"X8"X6" STRM TEE		(SE) = 22.64 (NW) = 22.64 (SW) = 22.64	19 LF, 8" HDPE @ 1.00% 19 LF, 8" HDPE @ 1.00% 3 LF, 6" HDPE @ 1.04%
TEE-8	8"X8"X6" STRM TEE		(SE) = 22.45 (NW) = 22.45 (SW) = 22.45	27 LF, 8" HDPE @ 1.00% 19 LF, 8" HDPE @ 1.00% 3 LF, 6" HDPE @ 1.03%
YI-1	YARD INLET	RIM = 25.84	(E) = 21.38	17 LF, 12" HDPE @ 4.43%
YI-2	YARD INLET	RIM = 25.99	(NE) = 21.14	17 LF, 18" HDPE @ 0.50%



THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

Chick-fil-A
5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman
Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
GRADING AND DRAINAGE PLAN

SHEET NUMBER
C-3.0

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

CONSTRUCTION SEQUENCE

1. CONDUCT PRE-CONSTRUCTION MEETING WITH THE CITY/COUNTY TO DISCUSS EROSION AND SEDIMENT CONTROLS AND CONSTRUCTION PHASING.
2. INSTALL AND POST SWPPP AND SITE COMPLIANCE SIGNAGE PUBLICLY VISIBLE.
3. INSTALL INLET PROTECTION, SILT DIKES, AND SILT FENCE ON THE SITE AS SHOWN.
4. INSTALL CONSTRUCTION FENCES AND TEMPORARY TRAFFIC AND PEDESTRIAN CONTROL DEVICES.
5. PREPARE TEMPORARY PARKING AND STORAGE AREAS.
6. DEMO EXISTING STRUCTURES, PAVEMENT, AND SPECIFIED UTILITIES.
7. BEGIN GRADING THE SITE.
8. BEGIN CONSTRUCTION OF UTILITIES.
9. BEGIN SUBGRADE PREPARATION AND CONSTRUCTION OF STRUCTURES.
10. BEGIN INSTALLATION OF CURB, GUTTER, AND PAVING.
11. COMPLETE PERMANENT STABILIZATION ON AREAS WHERE CONSTRUCTION HAS BEEN COMPLETED.
12. COMPLETE FINAL GRADING AND INSTALLATION OF PERMANENT STABILIZATION OVER ALL AREAS.
13. OBTAIN CONCURRENCE FROM THE OWNER AND THE CITY/COUNTY THAT THE SITE HAS BEEN FULLY STABILIZED.
14. REMOVE ALL REMAINING TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES.
15. STABILIZE ALL AREAS DISTURBED BY BMP REMOVAL.

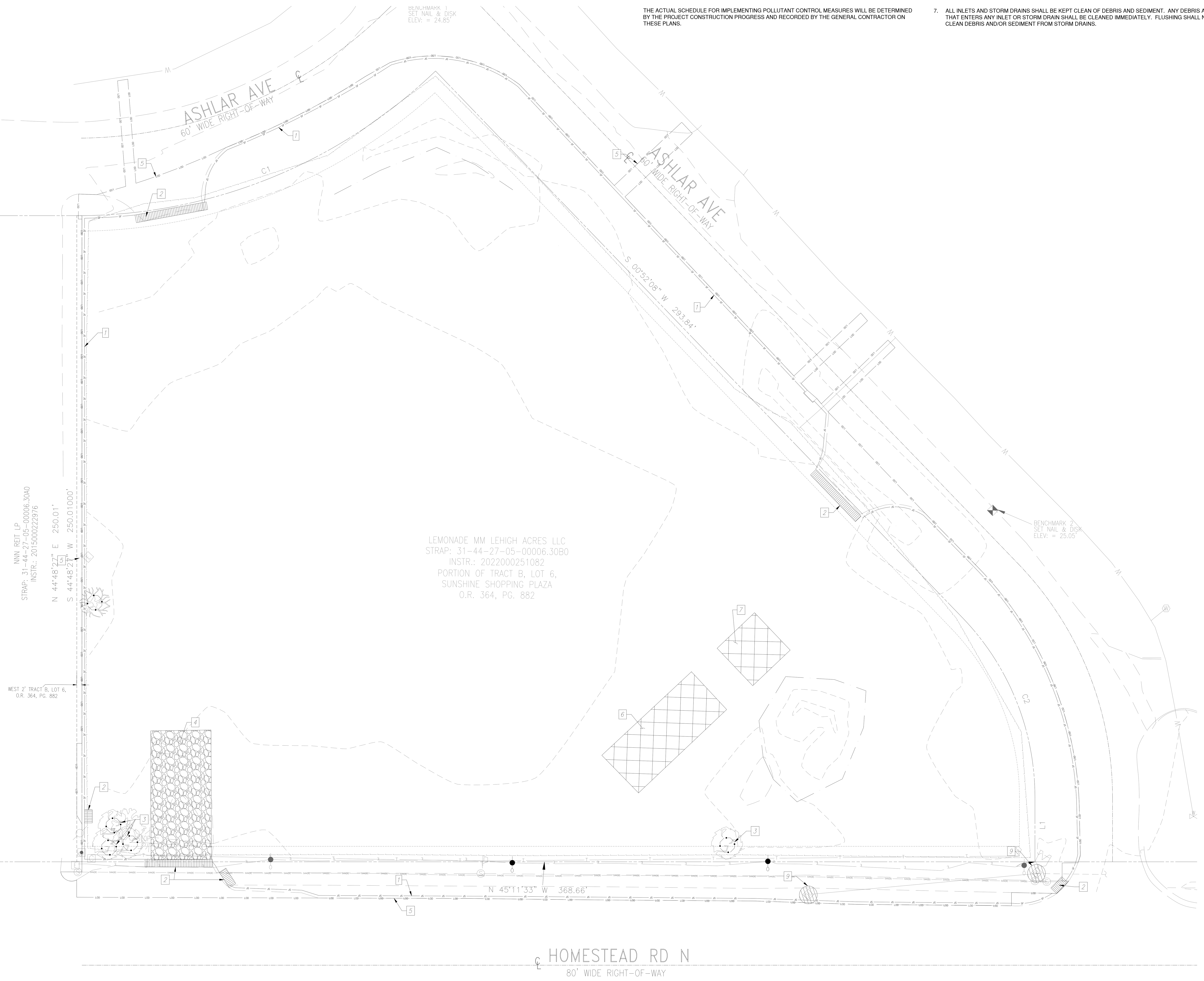
CONTRACTOR MAY COMPLETE CONSTRUCTION RELATED ACTIVITIES CONCURRENTLY ONLY IF ALL PRECEDING BMPS HAVE BEEN COMPLETELY INSTALLED.

THE ACTUAL SCHEDULE FOR IMPLEMENTING POLLUTANT CONTROL MEASURES WILL BE DETERMINED BY THE PROJECT CONSTRUCTION PROGRESS AND RECORDED BY THE GENERAL CONTRACTOR ON THESE PLANS.

BMP MAINTENANCE NOTES

ALL MEASURES STATED ON THESE PLANS SHALL BE MAINTAINED IN FULLY FUNCTIONAL CONDITION UNTIL NO LONGER REQUIRED FOR COMPLETED PHASE OF WORK OF FINAL STABILIZATION OF THE SITE. SEDIMENTATION CONTROL MEASURES SHALL BE CHECKED BY A QUALIFIED PERSON IN ACCORDANCE WITH THE CONSTRUCTION FDEP GENERIC PERMIT, AND REPAIRED IN ACCORDANCE WITH THE FOLLOWING:

1. INLET PROTECTION DEVICES AND BARRIERS SHALL BE REPAIRED OR REPLACED IF THEY SHOW SIGNS OF DETERIORATION.
2. ALL SEEDED/SODDED AREAS SHALL BE CHECKED REGULARLY TO SEE THAT A GOOD STAND OF GRASS IS MAINTAINED. AREAS SHALL BE FERTILIZED, WATERED AND REPAIRED AS NEEDED.
3. SILT FENCES SHALL BE REPAIRED TO THEIR ORIGINAL CONDITIONS IF DAMAGED. SEDIMENT SHALL BE REMOVED FROM THE SILT FENCE WHEN IT REACHES ONE-HALF THE HEIGHT OF THE FENCE.
4. THE CONSTRUCTION EXIT SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOW OF SEDIMENT FROM THE SITE. THIS MAY REQUIRE PERIODIC TOP DRESSING OF THE EXIT AS CONDITIONS DEMAND.
5. THE TEMPORARY PARKING AND STORAGE AREA SHALL BE KEPT IN A GOOD CONDITION. THIS MAY REQUIRE PERIODIC TOP DRESSING OF THE AREA AS CONDITIONS DEMAND.
6. PRIOR TO LEAVING THE SITE, ALL VEHICLES SHALL BE CLEANED OF DEBRIS. ANY DEBRIS AND/OR SEDIMENT LEAVING THE SITE SHALL BE CLEANED IMMEDIATELY.
7. ALL INLETS AND STORM DRAINS SHALL BE KEPT CLEAN OF DEBRIS AND SEDIMENT. ANY DEBRIS AND/OR SEDIMENT THAT ENTERS ANY INLET OR STORM DRAIN SHALL BE CLEANED IMMEDIATELY. FLUSHING SHALL NOT BE USED TO CLEAN DEBRIS AND/OR SEDIMENT FROM STORM DRAINS.



LEGEND

Symbol	
PROP. SILT DIKE ON PAVEMENT	PROPERTY LINE
TEMPORARY PARKING AND STAGING AREAS	EX. CONCRETE SIDEWALK
LIMIT OF DISTURBANCE	EX. CONCRETE D CURB
SILT FENCE	EX. CONCRETE F CURB
CURB INLET FILTER	EX. EDGE OF SIDEWALK
TREE PROTECTION BARRIER	EX. SIGN
INLET FILTER	EX. ELECTRIC HAND HOLE
	EX. SEWER CLEAN OUT
	EX. LIGHT POLE
	EX. TREE
	EX. HEDGE
	EX. FENCE
	EX. WATER METER
	EX. BACKFLOW PREVENTER
	EX. IRRIGATION VALVE
	EX. UNKNOWN UTILITY
	EX. STORM INLET
	EX. HEADWALL
	EX. STORM SEWER
	EX. SANITARY SEWER
	EX. TOP OF BANK

LIMITS OF DISTURBANCE SHOWN OFFSET 1FT FROM PROPERTY LINE FOR READIBILITY PURPOSES

EROSION CONTROL NOTES

1. CONST. SILT FENCE (1/C-3.3)
2. CONST. SILT DIKE ON PAVEMENT (3/C-3.3)
3. CONST. TREE BARRIER (5/C-3.3)
4. CONST. STABILIZED CONSTRUCTION EXIT (6/C-3.3)
5. LIMIT OF DISTURBANCE
6. TEMPORARY PARKING AREA
7. TEMPORARY STORAGE AREA
8. CONST. CURB INLET FILTER (4/C-3.3)
9. CONST. INLET FILTER (2/C-3.3)

GENERAL EROSION CONTROL NOTES

1. CONTRACTOR SHALL IMPLEMENT BEST MANAGEMENT PRACTICES AS REQUIRED AND THAT CONFORM TO FEDERAL, STATE, OR LOCAL REQUIREMENTS. ADDITIONAL BEST MANAGEMENT PRACTICES SHALL BE IMPLEMENTED AS DIRECTED BY PERMITTING AGENCY AND OWNER OR AS DICTATED BY CONDITIONS AT NO ADDITIONAL COST TO OWNER THROUGHOUT ALL PHASES OF CONSTRUCTION.
2. PERMIT(S) FOR ANY CONSTRUCTION ACTIVITY MUST BE MAINTAINED ON SITE AT ALL TIMES.
3. CONTRACTOR SHALL MINIMIZE CLEARING TO THE MAXIMUM EXTENT PRACTICAL OR AS REQUIRED BY THE FDEP GENERIC PERMIT.
4. GENERAL CONTRACTOR SHALL DENOTE ON PLAN THE TEMPORARY PARKING AND STORAGE AREA WHICH SHALL ALSO BE USED AS THE EQUIPMENT MAINTENANCE AND CLEANING AREA, EMPLOYEE PARKING AREA, AND AREA FOR LOCATING PORTABLE FACILITIES, OFFICE TRAILERS, AND TOILET FACILITIES.
5. ALL WASH WATER SHALL BE DETAINED AND PROPERLY TREATED OR DISPOSED.
6. SUFFICIENT OIL AND GREASE ABSORBING MATERIALS AND FLOTATION BOOMS SHALL BE MAINTAINED ON SITE OR READILY AVAILABLE TO CONTAIN AND CLEAN-UP FUEL OR CHEMICAL SPILLS AND LEAKS.
7. DUST ON THE SITE SHALL BE CONTROLLED. THE USE OF MOTOR OILS AND OTHER PETROLEUM BASED OR TOXIC LIQUIDS FOR DUST SUPPRESSION OPERATIONS IS PROHIBITED.
8. RUBBISH, TRASH, GARBAGE, LITTER, OR OTHER SUCH MATERIALS SHALL BE DEPOSITED INTO SEALED CONTAINERS. MATERIALS SHALL BE PREVENTED FROM LEAVING THE PREMISES THROUGH THE ACTION OF WIND OR STORM WATER DISCHARGE INTO DRAINAGE DITCHES OR WATERS OF THE STATE.
9. ALL STORM WATER POLLUTION PREVENTION MEASURES PRESENTED ON THIS PLAN SHALL BE INITIATED AS SOON AS PRACTICABLE.
10. DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS TEMPORARILY STOPPED FOR AT LEAST 7 DAYS, SHALL BE TEMPORARILY SEEDED.
11. DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS PERMANENTLY STOPPED SHALL BE SODDED/LANDSCAPED PER PLANS. THESE AREA SHALL BE SODDED/LANDSCAPED NO LATER THAN 7 DAYS AFTER THE LAST CONSTRUCTION ACTIVITY OCCURRING IN THESE AREAS. REFER TO THE GRADING PLAN AND/OR LANDSCAPE PLAN.
12. IF THE ACTION OF VEHICLES TRAVELING OVER THE CONSTRUCTION ENTRANCES IS NOT SUFFICIENT TO REMOVE THE MAJORITY OF DIRT OR MUD, THEN THE TIRES MUST BE WASHED BEFORE THE VEHICLES ENTER A PUBLIC ROAD. IF WASHING IS USED, PROVISIONS MUST BE MADE TO INTERCEPT THE WASH WATER AND TRAP THE SEDIMENT BEFORE IT IS CARRIED OFF THE SITE.
13. ALL MATERIALS SPILLED, DROPPED, WASHED, OR TRACKED FROM VEHICLES ONTO ROADWAYS OR INTO STORM DRAINS MUST BE REMOVED IMMEDIATELY.
14. CONTRACTORS OR SUBCONTRACTORS WILL BE RESPONSIBLE FOR REMOVING ANY SEDIMENT THAT MAY HAVE COLLECTED IN THE STORM SEWER DRAINAGE SYSTEMS IN CONJUNCTION WITH THE STABILIZATION OF THE SITE.
15. ON-SITE AND OFFSITE SOIL STOCKPILE AND BORROW AREAS SHALL BE PROTECTED FROM EROSION AND SEDIMENTATION THROUGH IMPLEMENTATION OF BEST MANAGEMENT PRACTICES. STOCKPILE AND BORROW AREA LOCATIONS SHALL BE NOTED ON THE SITE MAP AND PERMITTED IN ACCORDANCE WITH FDEP GENERIC PERMIT REQUIREMENTS.
16. SLOPES SHALL BE LEFT IN A ROUGHENED CONDITION DURING THE GRADING PHASE TO REDUCE RUNOFF VELOCITIES AND EROSION.
17. DUE TO CONSTRUCTION ACTIVITIES DURING THE DEVELOPMENT OF THE PROJECT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADJUSTING THE EROSION AND SEDIMENT CONTROL MEASURES TO PREVENT EROSION AND SEDIMENTATION.
18. CONTRACTOR SHALL DESIGNATE/IDENTIFY AREAS INSIDE THE LIMITS OF DISTURBANCE, FOR WASTE DISPOSAL AND DELIVERY AND MATERIAL STORAGE.
19. CONTRACTOR TO LIMIT DISTURBANCE OF SITE IN STRICT ACCORDANCE WITH THE EROSION CONTROL SEQUENCING SHOWN ON THIS PLAN, NO UNNECESSARY OR IMPROPERLY SEQUENCED CLEARING AND/OR GRADING SHALL BE PERMITTED.
20. ALL EXISTING SIGNALIZATION EQUIPMENT TO REMAIN IS ASSUMED TO BE IN GOOD WORKING ORDER UNLESS CITY/COUNTY IS NOTIFIED IN WRITING PRIOR TO THE START OF CONSTRUCTION. ANY SUBSEQUENT DAMAGE TO THE SIGNAL EQUIPMENT SHALL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
21. THE CONTRACTOR SHALL ENSURE THAT A FOREMAN OR SUPERVISOR WHO HAS BEEN CERTIFIED UNDER FLORIDA STORMWATER, EROSION AND SEDIMENTATION CONTROL INSPECTOR TRAINING PROGRAM IS AVAILABLE IN PERSON OR BY PHONE AT ALL TIMES DURING THE CONSTRUCTION ACTIVITIES. (ONCE CONTRACTOR IS SELECTED, A QUALIFIED FOREMAN/SUPERVISOR WILL BE DESIGNATED AND AVAILABLE AT THE PRE-CONSTRUCTION MEETING)
22. ADDITIONAL SEDIMENT AND EROSION CONTROL MEASURES MAY BE REQUIRED, DURING ANY PHASE OF DEVELOPMENT, AT THE DISCRETION OF THE CITY/COUNTY'S INSPECTOR.

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast. It's free. It's the law.
www.callsunshine.com

GRAPHIC SCALE
20 0 10 20 40
(IN FEET)
1 inch = 20 ft.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman
Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

**1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY**

FSU# 05587

REVISION SCHEDULE

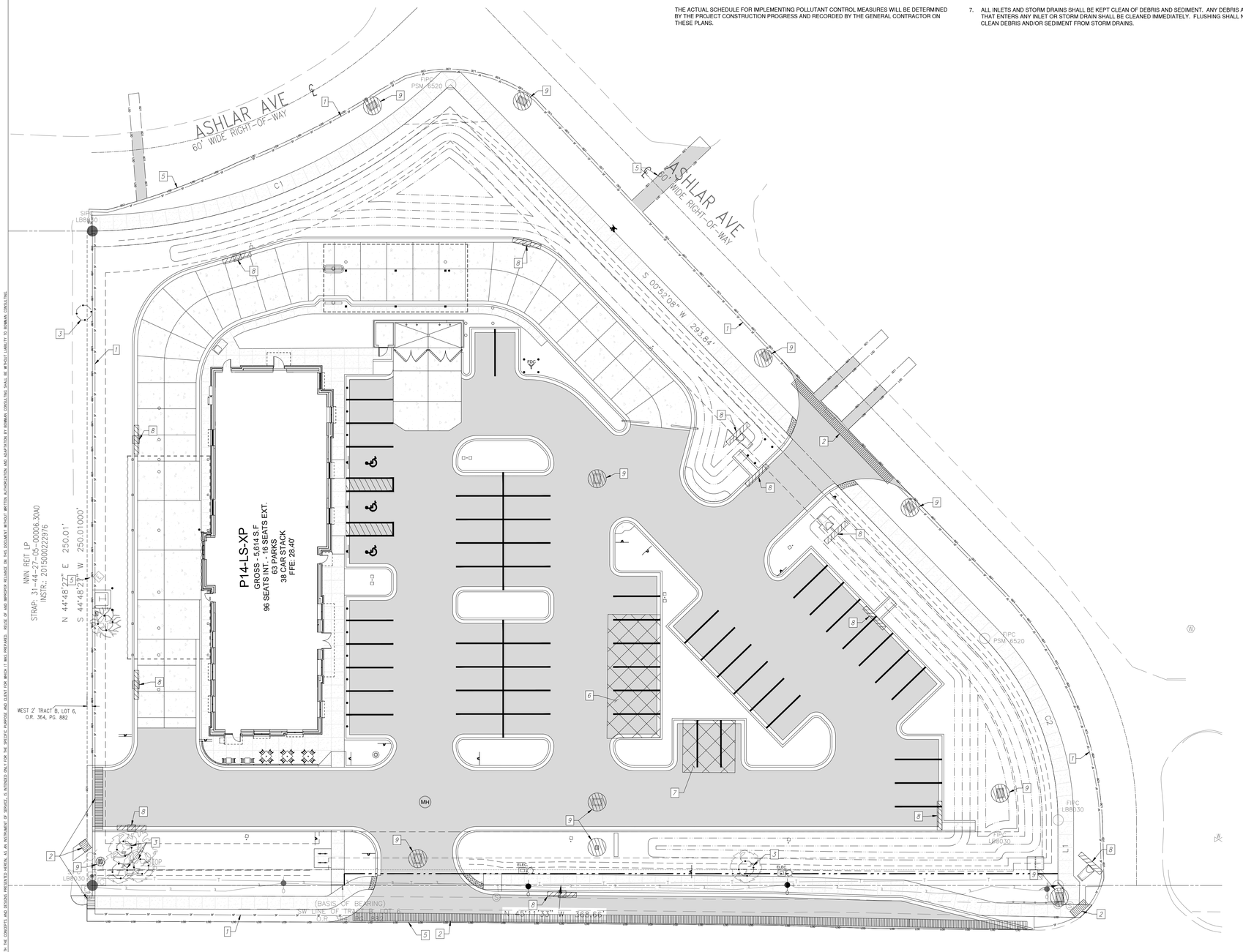
NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
EROSION CONTROL PLAN - PHASE 1

SHEET NUMBER
C-3.1



CONSTRUCTION SEQUENCE

1. CONDUCT PRE-CONSTRUCTION MEETING WITH THE CITY/COUNTY TO DISCUSS EROSION AND SEDIMENT CONTROLS AND CONSTRUCTION PHASING.
2. INSTALL AND POST SWPPP AND SITE COMPLIANCE SIGNAGE PUBLICLY VISIBLE.
3. INSTALL INLET PROTECTION, SILT DIKES, AND SILT FENCE ON THE SITE AS SHOWN.
4. INSTALL CONSTRUCTION FENCES AND TEMPORARY TRAFFIC AND PEDESTRIAN CONTROL DEVICES.
5. PREPARE TEMPORARY PARKING AND STORAGE AREAS.
6. DEMO EXISTING STRUCTURES, PAVEMENT, AND SPECIFIED UTILITIES.
7. BEGIN GRADING THE SITE.
8. BEGIN CONSTRUCTION OF UTILITIES.
9. BEGIN SUBGRADE PREPARATION AND CONSTRUCTION OF STRUCTURES.
10. BEGIN INSTALLATION OF CURB, GUTTER, AND PAVING.
11. COMPLETE PERMANENT STABILIZATION ON AREAS WHERE CONSTRUCTION HAS BEEN COMPLETED.
12. COMPLETE FINAL GRADING AND INSTALLATION OF PERMANENT STABILIZATION OVER ALL AREAS.
13. OBTAIN CONCURRENCE FROM THE OWNER AND THE CITY/COUNTY THAT THE SITE HAS BEEN FULLY STABILIZED.
14. REMOVE ALL REMAINING TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES.
15. STABILIZE ALL AREAS DISTURBED BY BMP REMOVAL.

CONTRACTOR MAY COMPLETE CONSTRUCTION RELATED ACTIVITIES CONCURRENTLY ONLY IF ALL PRECEDING BMPS HAVE BEEN COMPLETELY INSTALLED.

THE ACTUAL SCHEDULE FOR IMPLEMENTING POLLUTANT CONTROL MEASURES WILL BE DETERMINED BY THE PROJECT CONSTRUCTION PROGRESS AND RECORDED BY THE GENERAL CONTRACTOR ON THESE PLANS.

BMP MAINTENANCE NOTES

ALL MEASURES STATED ON THESE PLANS SHALL BE MAINTAINED IN FULLY FUNCTIONAL CONDITION UNTIL NO LONGER REQUIRED FOR COMPLETED PHASE OF WORK OF FINAL STABILIZATION OF THE SITE. SEDIMENTATION CONTROL MEASURES SHALL BE CHECKED BY A QUALIFIED PERSON IN ACCORDANCE WITH THE CONSTRUCTION FDEP GENERIC PERMIT, AND REPAIRED IN ACCORDANCE WITH THE FOLLOWING:

1. INLET PROTECTION DEVICES AND BARRIERS SHALL BE REPAIRED OR REPLACED IF THEY SHOW SIGNS OF DETERIORATION.
2. ALL SEEDED/SODDED AREAS SHALL BE CHECKED REGULARLY TO SEE THAT A GOOD STAND OF GRASS IS MAINTAINED. AREAS SHALL BE FERTILIZED, WATERED AND REPAIRED AS NEEDED.
3. SILT FENCES SHALL BE REPAIRED TO THEIR ORIGINAL CONDITIONS IF DAMAGED. SEDIMENT SHALL BE REMOVED FROM THE SILT FENCE WHEN IT REACHES ONE-HALF THE HEIGHT OF THE FENCE.
4. THE CONSTRUCTION EXIT SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOW OF SEDIMENT FROM THE SITE. THIS MAY REQUIRE PERIODIC TOP DRESSING OF THE EXIT AS CONDITIONS DEMAND.
5. THE TEMPORARY PARKING AND STORAGE AREA SHALL BE KEPT IN A GOOD CONDITION. THIS MAY REQUIRE PERIODIC TOP DRESSING OF THE AREA AS CONDITIONS DEMAND.
6. PRIOR TO LEAVING THE SITE, ALL VEHICLES SHALL BE CLEANED OF DEBRIS. ANY DEBRIS AND/OR SEDIMENT LEAVING THE SITE SHALL BE CLEANED IMMEDIATELY.
7. ALL INLETS AND STORM DRAINS SHALL BE KEPT CLEAN OF DEBRIS AND SEDIMENT. ANY DEBRIS AND/OR SEDIMENT THAT ENTERS ANY INLET OR STORM DRAIN SHALL BE CLEANED IMMEDIATELY. FLUSHING SHALL NOT BE USED TO CLEAN DEBRIS AND/OR SEDIMENT FROM STORM DRAINS.

LEGEND

- PROP. SILT DIKE ON PAVEMENT
- TEMPORARY PARKING AND STAGING AREAS
- LIMIT OF DISTURBANCE (LOD)
- SILT FENCE (SF)
- CURB INLET FILTER
- TREE PROTECTION BARRIER
- INLET FILTER
- PROPERTY LINE
- EX. CONCRETE SIDEWALK
- EX. CONCRETE D CURB
- EX. CONCRETE F CURB
- EX. EDGE OF SIDEWALK
- EX. SIGN
- EX. ELECTRIC HAND HOLE (ELEC)
- EX. SEWER CLEAN OUT
- EX. LIGHT POLE
- EX. TREE
- EX. HEDGE
- EX. FENCE
- EX. WATER METER
- EX. BACKFLOW PREVENTER
- EX. IRRIGATION VALVE
- EX. UNKNOWN UTILITY
- EX. STORM INLET
- EX. HEADWALL
- EX. STORM SEWER
- EX. SANITARY SEWER
- EX. TOP OF BANK (TOB)

LIMITS OF DISTURBANCE SHOWN OFFSET 1FT FROM PROPERTY LINE FOR READIBILITY PURPOSES

EROSION CONTROL NOTES

1. CONST. SILT FENCE (C-33)
2. CONST. SILT DIKE ON PAVEMENT (C-33)
3. CONST. TREE BARRIER (C-33)
4. CONST. STABILIZED CONSTRUCTION EXIT (C-33)
5. LIMIT OF DISTURBANCE
6. TEMPORARY PARKING AREA
7. TEMPORARY STORAGE AREA
8. CONST. CURB INLET FILTER (C-33)
9. CONST. INLET FILTER (C-33)

GENERAL EROSION CONTROL NOTES

1. CONTRACTOR SHALL IMPLEMENT BEST MANAGEMENT PRACTICES AS REQUIRED AND THAT CONFORM TO FEDERAL, STATE, OR LOCAL REQUIREMENTS. ADDITIONAL BEST MANAGEMENT PRACTICES SHALL BE IMPLEMENTED AS DIRECTED BY PERMITTING AGENCY AND OWNER OR AS DICTATED BY CONDITIONS AT NO ADDITIONAL COST TO OWNER THROUGHOUT ALL PHASES OF CONSTRUCTION.
2. PERMIT(S) FOR ANY CONSTRUCTION ACTIVITY MUST BE MAINTAINED ON SITE AT ALL TIMES.
3. CONTRACTOR SHALL MINIMIZE CLEARING TO THE MAXIMUM EXTENT PRACTICAL OR AS REQUIRED BY THE FDEP GENERIC PERMIT.
4. GENERAL CONTRACTOR SHALL DENOTE ON PLAN THE TEMPORARY PARKING AND STORAGE AREA WHICH SHALL ALSO BE USED AS THE EQUIPMENT MAINTENANCE AND CLEANING AREA, EMPLOYEE PARKING AREA, AND AREA FOR LOCATING PORTABLE FACILITIES, OFFICE TRAILERS, AND TOILET FACILITIES.
5. ALL WASH WATER SHALL BE DETAINED AND PROPERLY TREATED OR DISPOSED.
6. SUFFICIENT OIL AND GREASE ABSORBING MATERIALS AND FLOTATION BOOMS SHALL BE MAINTAINED ON SITE OR READILY AVAILABLE TO CONTAIN AND CLEAN-UP FUEL OR CHEMICAL SPILLS AND LEAKS.
7. DUST ON THE SITE SHALL BE CONTROLLED. THE USE OF MOTOR OILS AND OTHER PETROLEUM BASED OR TOXIC LIQUIDS FOR DUST SUPPRESSION OPERATIONS IS PROHIBITED.
8. RUBBISH, TRASH, GARBAGE, LITTER, OR OTHER SUCH MATERIALS SHALL BE DEPOSITED INTO SEALED CONTAINERS. MATERIALS SHALL BE PREVENTED FROM LEAVING THE PREMISES THROUGH THE ACTION OF WIND OR STORM WATER DISCHARGE INTO DRAINAGE DITCHES OR WATERS OF THE STATE.
9. ALL STORM WATER POLLUTION PREVENTION MEASURES PRESENTED ON THIS PLAN SHALL BE INITIATED AS SOON AS PRACTICABLE.
10. DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS TEMPORARILY STOPPED FOR AT LEAST 7 DAYS, SHALL BE TEMPORARILY SEEDED.
11. DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS PERMANENTLY STOPPED SHALL BE SODDED/LANDSCAPED PER PLANS. THESE AREA SHALL BE SODDED/LANDSCAPED NO LATER THAN 7 DAYS AFTER THE LAST CONSTRUCTION ACTIVITY OCCURRING IN THESE AREAS. REFER TO THE GRADING PLAN AND/OR LANDSCAPE PLAN.
12. IF THE ACTION OF VEHICLES TRAVELING OVER THE CONSTRUCTION ENTRANCES IS NOT SUFFICIENT TO REMOVE THE MAJORITY OF DIRT OR MUD, THEN THE TIRES MUST BE WASHED BEFORE THE VEHICLES ENTER A PUBLIC ROAD. IF WASHING IS USED, PROVISIONS MUST BE MADE TO INTERCEPT THE WASH WATER AND TRAP THE SEDIMENT BEFORE IT IS CARRIED OFF THE SITE.
13. ALL MATERIALS SPILLED, DROPPED, WASHED, OR TRACKED FROM VEHICLES ONTO ROADWAYS OR INTO STORM DRAINS MUST BE REMOVED IMMEDIATELY.
14. CONTRACTORS OR SUBCONTRACTORS WILL BE RESPONSIBLE FOR REMOVING ANY SEDIMENT THAT MAY HAVE COLLECTED IN THE STORM SEWER DRAINAGE SYSTEMS IN CONJUNCTION WITH THE STABILIZATION OF THE SITE.
15. ON-SITE AND OFFSITE SOIL STOCKPILE AND BORROW AREAS SHALL BE PROTECTED FROM EROSION AND SEDIMENTATION THROUGH IMPLEMENTATION OF BEST MANAGEMENT PRACTICES. STOCKPILE AND BORROW AREA LOCATIONS SHALL BE NOTED ON THE SITE MAP AND PERMITTED IN ACCORDANCE WITH FDEP GENERIC PERMIT REQUIREMENTS.
16. SLOPES SHALL BE LEFT IN A ROUGHENED CONDITION DURING THE GRADING PHASE TO REDUCE RUNOFF VELOCITIES AND EROSION.
17. DUE TO CONSTRUCTION ACTIVITIES DURING THE DEVELOPMENT OF THE PROJECT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADJUSTING THE EROSION AND SEDIMENT CONTROL MEASURES TO PREVENT EROSION AND SEDIMENTATION.
18. CONTRACTOR SHALL DESIGNATE/IDENTIFY AREAS INSIDE THE LIMITS OF DISTURBANCE, FOR WASTE DISPOSAL AND DELIVERY AND MATERIAL STORAGE.
19. CONTRACTOR TO LIMIT DISTURBANCE OF SITE IN STRICT ACCORDANCE WITH THE EROSION CONTROL SEQUENCING SHOWN ON THIS PLAN, NO UNNECESSARY OR IMPROPERLY SEQUENCED CLEARING AND/OR GRADING SHALL BE PERMITTED.
20. ALL EXISTING SIGNALIZATION EQUIPMENT TO REMAIN IS ASSUMED TO BE IN GOOD WORKING ORDER UNLESS CITY/COUNTY IS NOTIFIED IN WRITING PRIOR TO THE START OF CONSTRUCTION. ANY SUBSEQUENT DAMAGE TO THE SIGNAL EQUIPMENT SHALL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
21. THE CONTRACTOR SHALL ENSURE THAT A FOREMAN OR SUPERVISOR WHO HAS BEEN CERTIFIED UNDER FLORIDA STORMWATER, EROSION AND SEDIMENTATION CONTROL INSPECTOR TRAINING PROGRAM IS AVAILABLE IN PERSON OR BY PHONE AT ALL TIMES DURING THE CONSTRUCTION ACTIVITIES. (ONCE CONTRACTOR IS SELECTED, A QUALIFIED FOREMAN/SUPERVISOR WILL BE DESIGNATED AND AVAILABLE AT THE PRE-CONSTRUCTION MEETING)
22. ADDITIONAL SEDIMENT AND EROSION CONTROL MEASURES MAY BE REQUIRED, DURING ANY PHASE OF DEVELOPMENT, AT THE DISCRETION OF THE CITY/COUNTY'S INSPECTOR.

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast, it's free. It's the law.
www.callsunshine.com

GRAPHIC SCALE
20 0 10 20 40
(IN FEET)
1 inch = 20 ft.

Chick-fil-A
5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman
Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com

© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
EROSION CONTROL PLAN - PHASE II

SHEET NUMBER
C-3.2

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

STATE OF FLORIDA E&SC DESIGNER & REVIEWER MANUAL; LATEST EDITION: JULY 2013

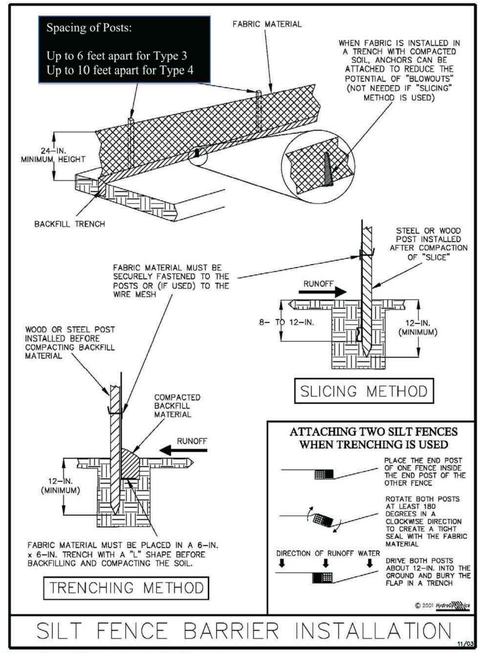
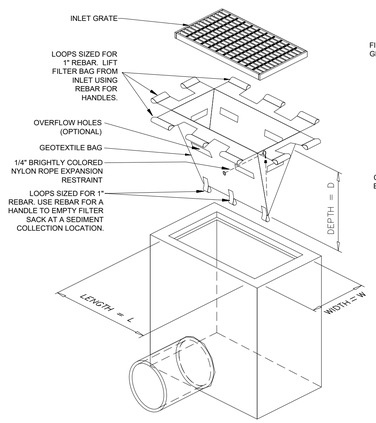


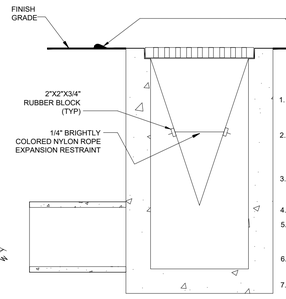
Figure V-2: Illustration of a Silt Fence Barrier

Permission is given by HydroDynamics Incorporated to copy and reproduce this detail

1 C-3.3 SILT FENCE DETAIL
NOT TO SCALE



ISOMETRIC VIEW



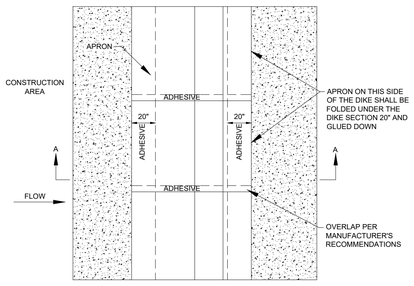
SECTION VIEW
PROFILE VIEW OF INSTALLED FILTER SACK

1. REMOVE TRAPPED SEDIMENT WHEN BRIGHTLY COLORED EXPANSION RESTRAINT CAN NO LONGER BE SEEN.
2. GEOTEXTILE SHALL BE A WOVEN POLYPROPYLENE FABRIC THAT MEETS OR EXCEEDS REQUIREMENTS IN THE SPECIFICATIONS TABLE.
3. PLACE AN OIL ADSORBENT PAD OR PILLOW OVER INLET GRATE WHEN OIL SPILLS ARE A CONCERN.
4. INSPECT PER REGULATORY REQUIREMENTS.
5. THE WIDTH, "W", OF THE FILTER SACK SHALL MATCH THE INSIDE WIDTH OF THE GRATED INLET BOX.
6. THE DEPTH, "D", OF THE FILTER SACK SHALL BE BETWEEN 18 INCHES AND 36 INCHES.
7. THE LENGTH, "L", OF THE FILTER SACK SHALL MATCH THE INSIDE LENGTH OF THE GRATED INLET BOX.

LOW TO MODERATE FLOW GEOTEXTILE FABRIC SPECIFICATION TABLE		
PROPERTIES	TEST METHOD	UNITS
GRAB TENSILE STRENGTH	ASTM D-4632	300 LBS
GRAB TENSILE ELONGATION	ASTM D-4632	20 %
PUNCTURE	ASTM D-4833	120 LBS
MULLEN BURST	ASTM D-3786	800 PSI
TRAPEZOID TEAR	ASTM D-5533	120 LBS
UV RESISTANCE	ASTM D-4355	80 %
APPARENT OPENING SIZE	ASTM D-4751	40 US SIEVE
FLOW RATE	ASTM D-4491	40 GAL/MIN/SQ.FT
PERMITTIVITY	ASTM D-4491	0.55 SEC -1

MODERATE TO HIGH FLOW GEOTEXTILE FABRIC SPECIFICATION TABLE		
PROPERTIES	TEST METHOD	UNITS
GRAB TENSILE STRENGTH	ASTM D-4632	265 LBS
GRAB TENSILE ELONGATION	ASTM D-4632	20 %
PUNCTURE	ASTM D-4833	135 LBS
MULLEN BURST	ASTM D-3786	420 PSI
TRAPEZOID TEAR	ASTM D-4533	45 LBS
UV RESISTANCE	ASTM D-4355	90 %
APPARENT OPENING SIZE	ASTM D-4751	20 US SIEVE
FLOW RATE	ASTM D-4491	200 GAL/MIN/SQ.FT
PERMITTIVITY	ASTM D-4491	1.5 SEC -1

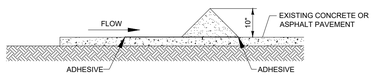
2 C-3.3 INLET FILTER DETAIL
NOT TO SCALE



DIKE PLAN VIEW

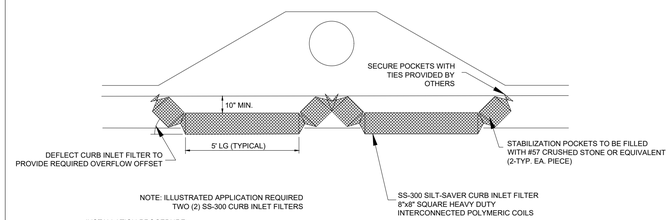
NOTES:

1. INSTALLED SILT DIKE UNIT SHALL HAVE CONTINUOUS AND FIRM CONTACT WITH PAVEMENT.
2. ADHESIVES SHALL BE LIQUID NAIL OR APPROVED EQUAL FOR CONCRETE PAVEMENT APPLICATIONS AND EMULSIFIED ASPHALT FOR ASPHALT APPLICATIONS. ADHESIVE SHALL BE PLACED WHERE THE UNITS OVERLAP AND A 20" STRIP ALONG BOTH EDGES.



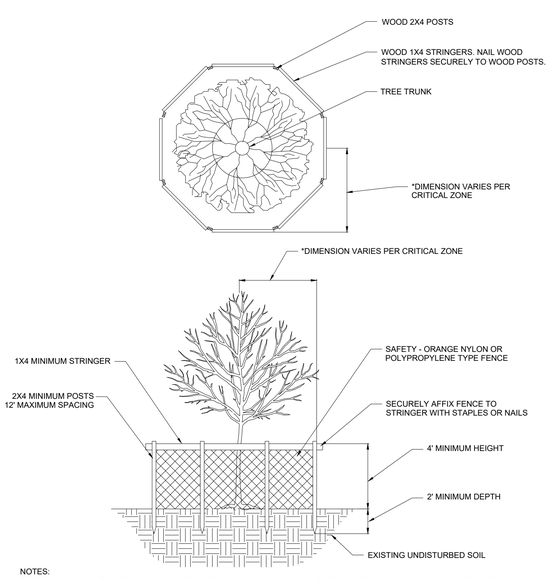
SECTION A-A

3 C-3.3 SILT DIKE ON EXISTING PAVEMENT
NOT TO SCALE



INSTALLATION PROCEDURE:
1. IDENTIFY OPENING DIMENSIONS TO DETERMINE THE NUMBER OF SS-300 CURB INLET FILTERS THAT WILL BE REQUIRED.
2. COMPLETELY FILL THE STABILIZATION CHAMBER ON EACH END OF EACH SS-300 CURB INLET FILTER WITH #57 CRUSHED STONE.
3. SECURE ENDS OF THE STAVILIZATION POCKETS WITH TIES TO BE PROVIDED BY OTHERS.
4. PLACE THE SS-300 CURB INLET FILTER(S) IN FRONT OF THE CURB INLET OR OPENING TO PREVENT THE MIGRATION OF SILT INTO THE STORM DRAIN SYSTEM.

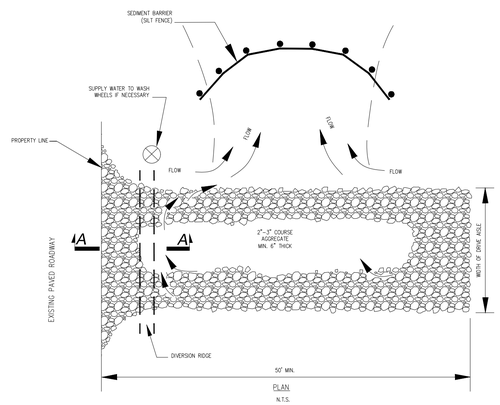
4 C-3.3 CURB INLET FILTER
NOT TO SCALE



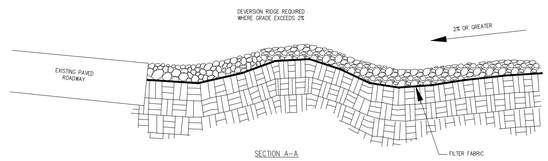
NOTES:
CRITICAL PROTECTION ZONE: THE AREA SURROUNDING A TREE WITHIN A CIRCLE DESCRIBED BY A RADIUS OF ONE FOOT FOR EACH INCH OF THE TREE TRUNK DIAMETER AT 54" ABOVE FINISHED GRADE. FOR GROUPS OF TREES, PLACE BARRICADES BETWEEN TREES AND CONSTRUCTION ACTIVITY.

* TREE PROTECTION BARRICADE SHALL BE LOCATED TO PROTECT A MINIMUM OF 75% OF THE CRITICAL PROTECTION ZONE.

5 C-3.3 TREE BARRIER
NOT TO SCALE



PLAN
N.T.S.



SECTION A-A
N.T.S.

NOTE:
USE SILT FENCE ONLY

NOTES:

1. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE TOP DRESSING, REPAIR AND/OR CLEAN-OUT OF ANY MEASURES USED TO TRAP SEDIMENT.
2. WHEN NECESSARY, WHEELS SHALL BE CLEANED PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY
3. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON A AREA STABILIZED WITH CRUSHED STONE THAT DRAINS INTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN.
4. ANY SEDIMENT TRACKED ONTO ROADWAYS SHALL BE SWEPT BACK ONTO SITE AT THE END OF EACH WORKING DAY.

6 C-3.3 STABILIZED CONSTRUCTION EXIT
NOT TO SCALE

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2026 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU
1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

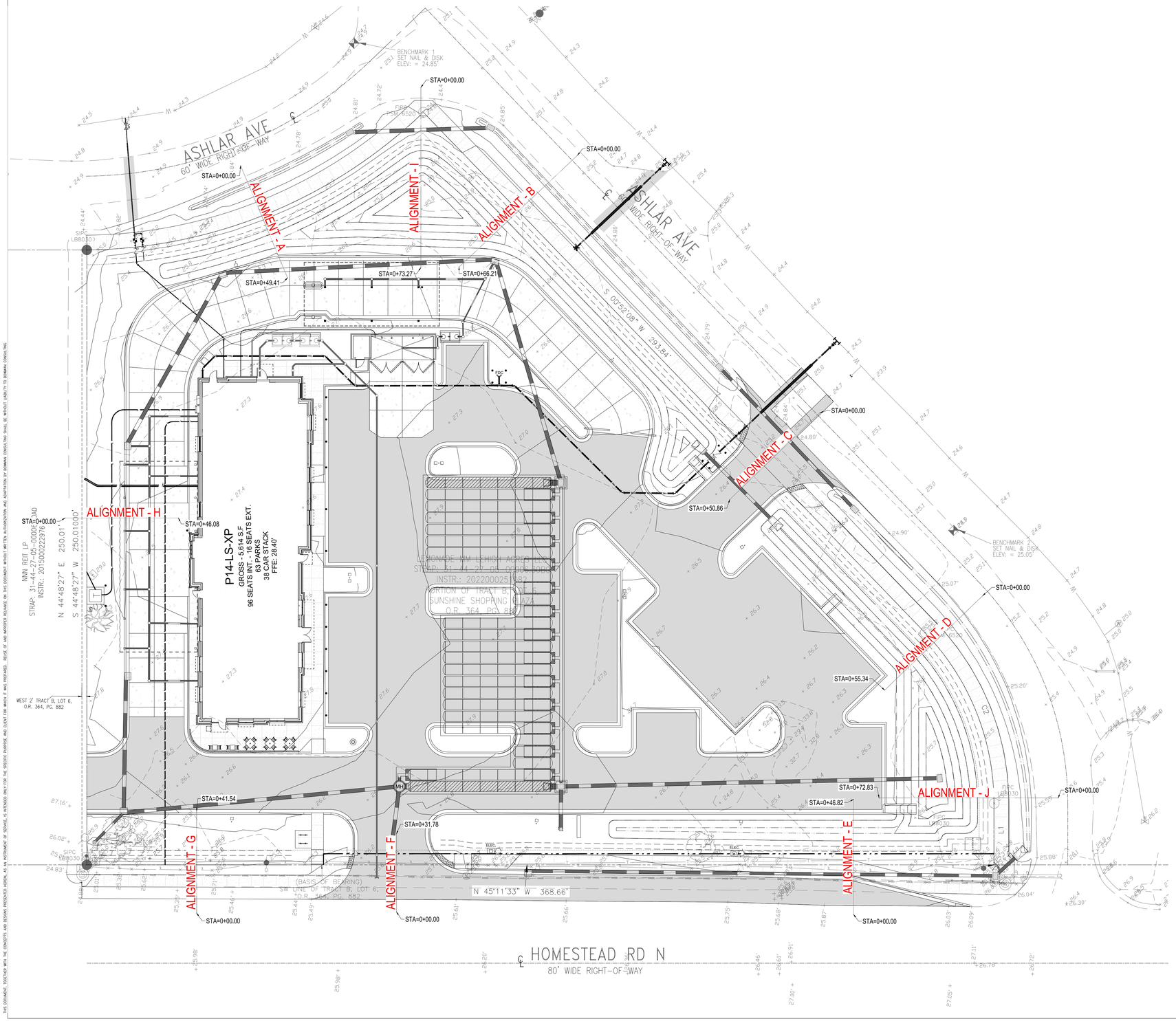
NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
EROSION CONTROL DETAILS

SHEET NUMBER
C-3.3



LEGEND

PROP. ASPHALT DRIVE		PROPERTY LINE	
PROP. CONC. SIDEWALK		EX. CONCRETE SIDEWALK	
PROP. CONC. PAVEMENT		EX. CONCRETE D CURB	
PROP. TYPE "F" CURB		EX. CONCRETE F CURB	
PROP. TYPE "D" CURB		EX. EDGE OF SIDEWALK	
PROP. PARKING STRIPE		EX. SIGN	
PROP. HANDICAP MARKING		EX. ELECTRIC HAND HOLE	
PROP. TYPE C INLET		EX. SEWER CLEAN OUT	
PROP. GREASE TRAP		EX. LIGHT POLE	
PROP. GRADE		EX. TREE	
PROP. TYPE 9 CURB INLET		EX. HEDGE	
PROP. STORM SEWER		EX. FENCE	
PROP. CLEANOUT		EX. WATER METER	
PROP. SANITARY SEWER		EX. BACKFLOW PREVENTER	
PROP. WATER		EX. IRRIGATION VALVE	
PROP. ELECTRIC		EX. UNKNOWN UTILITY	
PROP. GAS LINE			

GRAPHIC SCALE
20 0 10 20 40
(IN FEET)
1 inch = 20 ft.

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast. It's free. It's the law.
www.callsunshine.com

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

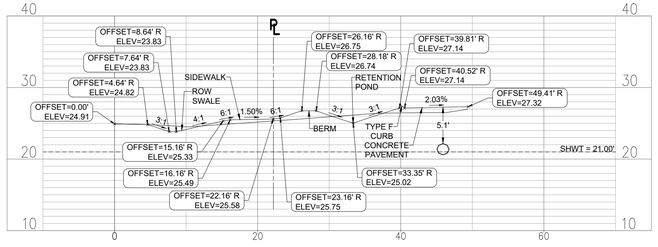
NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

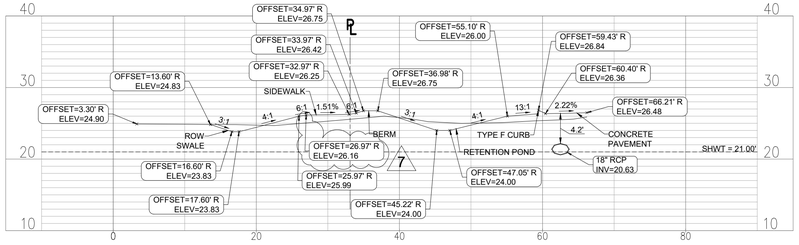
FOR PERMIT

SHEET
CROSS SECTIONS KEY MAP

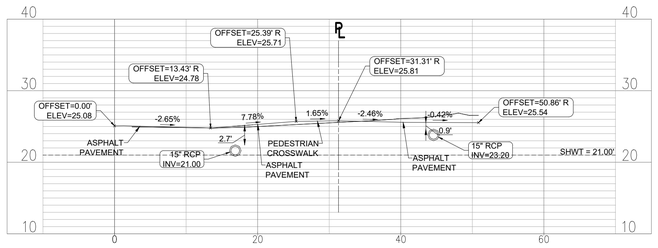
SHEET NUMBER
C-3.4



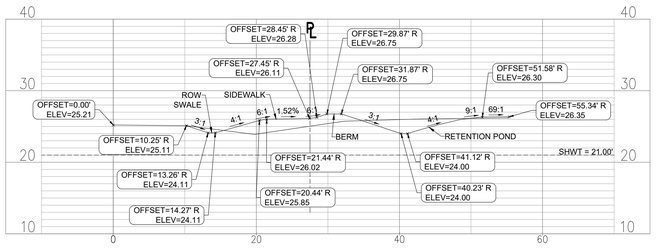
ALIGNMENT A
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=20'



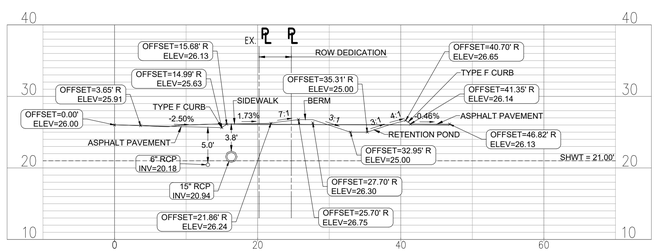
ALIGNMENT B
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=20'



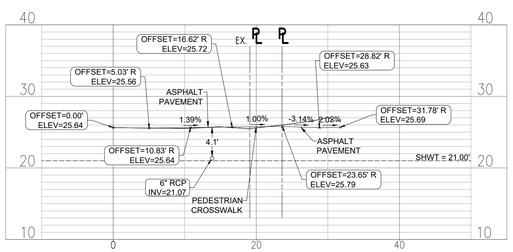
ALIGNMENT C
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=20'



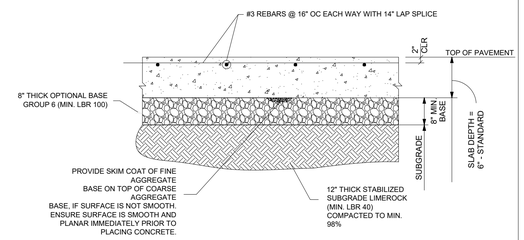
ALIGNMENT D
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=20'



ALIGNMENT E
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=20'

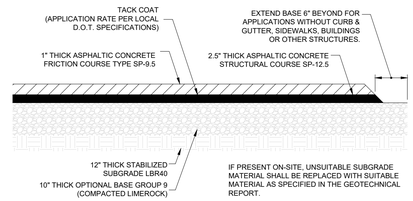


ALIGNMENT F
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=20'



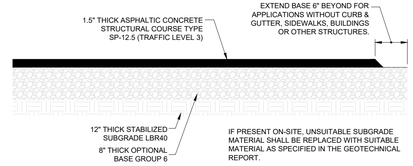
6" CONCRETE PAVEMENT SECTION

NOT TO SCALE



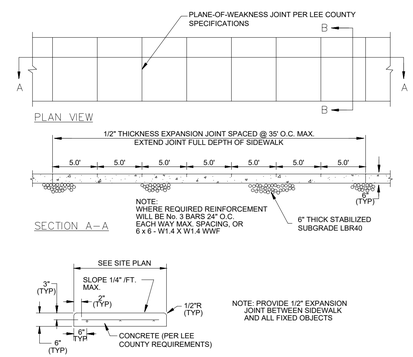
LEE COUNTY DOT MAINTAINED ARTERIAL ROAD ASPHALT PAVEMENT SECTION (HOMESTEAD RD)

NOT TO SCALE



COUNTY ROAD ASPHALT PAVEMENT SECTION (ASHLAR RD)

NOT TO SCALE



ROW CONCRETE SIDEWALK

NOT TO SCALE

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast. It's free. It's the law.
www.callsunshine.com

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

Chick-fil-A
5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman
Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

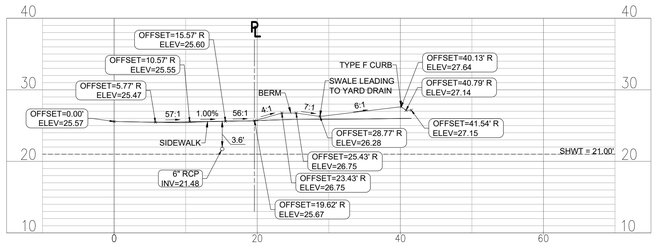
NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

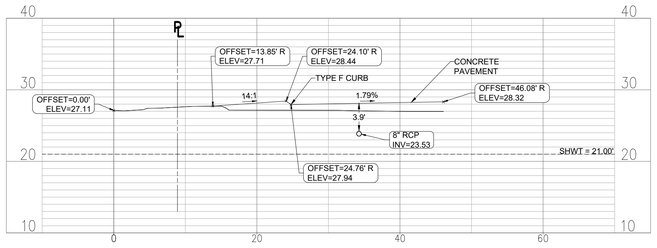
FOR PERMIT

SHEET
CROSS SECTIONS

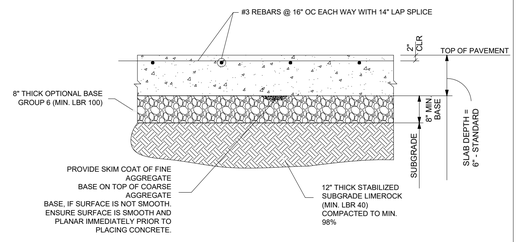
SHEET NUMBER
C-3.5



ALIGNMENT G
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=20'

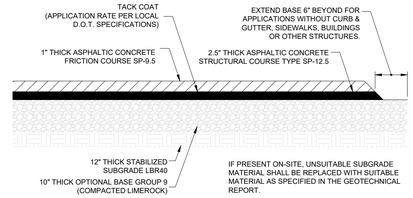


ALIGNMENT H
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=20'



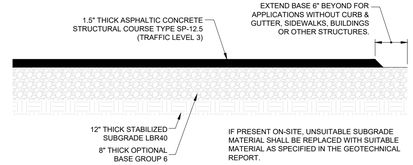
6" CONCRETE PAVEMENT SECTION

NOT TO SCALE



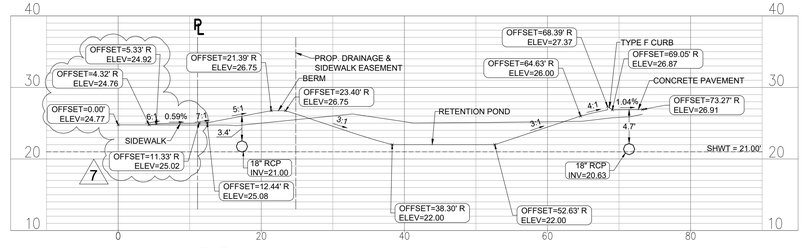
LEE COUNTY DOT MAINTAINED ARTERIAL ROAD ASPHALT PAVEMENT SECTION (HOMESTEAD RD)

NOT TO SCALE

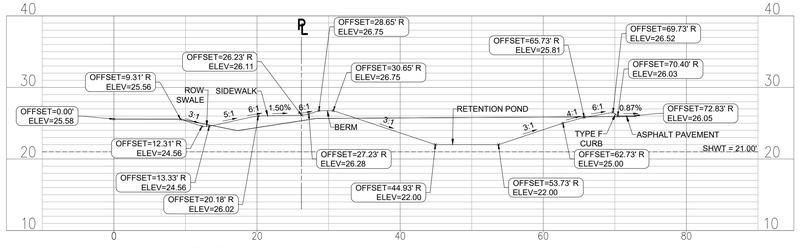


COUNTY ROAD ASPHALT PAVEMENT SECTION (ASHLAR RD)

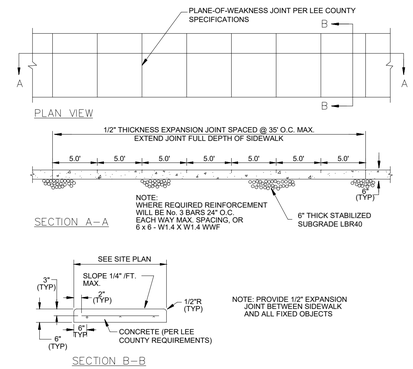
NOT TO SCALE



ALIGNMENT I
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=20'



ALIGNMENT J
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=20'



ROW CONCRETE SIDEWALK

NOT TO SCALE

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
811 Safe. It's free. It's the law.
www.callsunshine.com

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

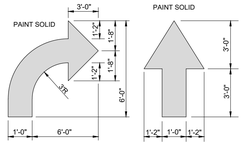
NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

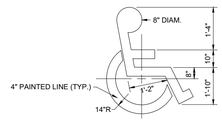
FOR PERMIT

SHEET
CROSS SECTIONS

SHEET NUMBER
C-3.6



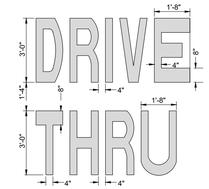
1A C-4.0 DIRECTIONAL ARROW
NOT TO SCALE



1B C-4.0 ACCESSIBILITY SYMBOL
NOT TO SCALE

NOTES:
1. GENERAL CONTRACTOR SHALL REFER TO PARKING LOT STRIPING SPECIFICATIONS (SEE DETAIL 5/C-4.0).
2. CONTRACTOR SHALL USE WHITE REFLECTIVE PAINT ON ASPHALT & YELLOW REFLECTIVE PAINT ON CONCRETE, UNLESS UPON VERIFICATION BY THE GENERAL CONTRACTOR IT IS DETERMINED THAT LOCAL, STATE, OR ADA CODES DIFFER, IN WHICH CASE THESE CODES SHALL GOVERN.

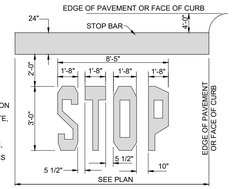
1 C-4.0 PAVEMENT MARKINGS - 1
NOT TO SCALE



2A C-4.0 DRIVE-THRU
NOT TO SCALE

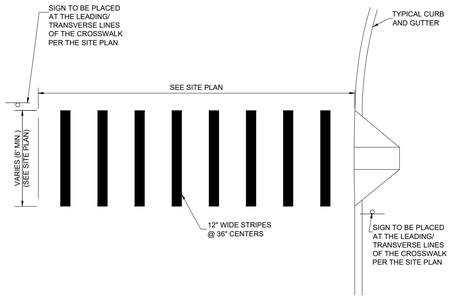
NOTES:
1. GENERAL CONTRACTOR SHALL REFER TO PARKING LOT STRIPING SPECIFICATIONS.
2. CONTRACTOR SHALL USE WHITE REFLECTIVE PAINT ON ASPHALT & YELLOW REFLECTIVE PAINT ON CONCRETE, UNLESS UPON VERIFICATION BY THE GENERAL CONTRACTOR IT IS DETERMINED THAT LOCAL, STATE, OR ADA CODES DIFFER, IN WHICH CASE THESE CODES SHALL GOVERN.

NOTE:
IF STOP SIGN IS PROPOSED, "STOP" LETTERING GRAPHIC IS NOT REQUIRED.



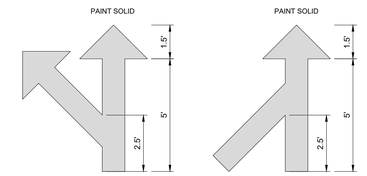
2B C-4.0 STOP BAR
NOT TO SCALE

2 C-4.0 PAVEMENT MARKINGS - 2
NOT TO SCALE



NOTES:
1. GENERAL CONTRACTOR SHALL REFER TO PARKING LOT STRIPING SPECIFICATIONS (SEE DETAIL 5/C-4.0).
2. CONTRACTOR SHALL USE WHITE REFLECTIVE PAINT ON ASPHALT AND YELLOW REFLECTIVE PAINT ON CONCRETE.

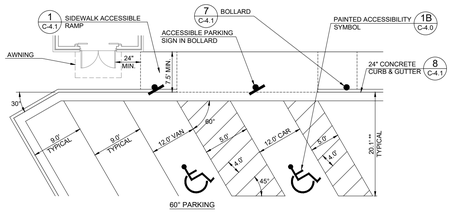
3 C-4.0 CROSSWALK MARKINGS
NOT TO SCALE



MULTI-LANE SPLIT MULTI-LANE MERGE

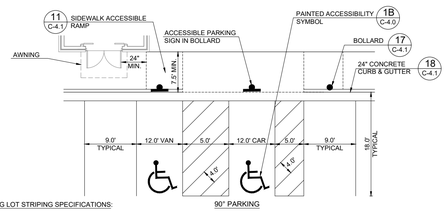
NOTES:
1. GENERAL CONTRACTOR SHALL REFER TO PARKING LOT STRIPING SPECIFICATIONS (SEE DETAIL 5/C-4.0).
2. CONTRACTOR SHALL USE WHITE REFLECTIVE PAINT ON ASPHALT & YELLOW REFLECTIVE PAINT ON CONCRETE.

4 C-4.0 MULTI-LANE DIRECTIONAL GRAPHICS
NOT TO SCALE



60° PARKING

** 60° PARKING STALL CAN BE REDUCED TO 18' MINIMUM IF SITE CONDITIONS REQUIRE IT.



90° PARKING

NOTES

1. ACCESSIBLE PARKING AND ACCESSIBLE AISLES SHALL NOT EXCEED 1.5% IN SLOPE IN ANY DIRECTION. IF ONLY ONE ACCESS AISLE IS INSTALLED, IT IS TO BE A VAN SIZE.
2. PARKING STALL DIMENSIONING SHALL BE IN ACCORDANCE WITH APPLICABLE GOVERNING AUTHORITY'S AND ADA STANDARDS. SEE SITE PLAN FOR COMPLETE STRIPING LAYOUT.
3. GENERAL CONTRACTOR SHALL REFER TO CHICK-FIL-A PARKING LOT STRIPING SPECIFICATIONS, THIS DETAIL.
4. CONTRACTOR SHALL USE 4" WIDE WHITE REFLECTIVE PAINT FOR STRIPING ON ASPHALT PARKING LOTS.
5. CONTRACTOR SHALL USE 4" WIDE YELLOW REFLECTIVE PAINT FOR STRIPING ON CONCRETE PARKING LOTS.
6. ACCESSIBLE PARKING AND ACCESSIBLE AISLE STRIPING COLOR SHALL BE IN ACCORDANCE WITH APPLICABLE GOVERNING AUTHORITY REQUIREMENTS.
7. NO WHEEL STOPS TO BE INSTALLED WHEN PARKING IS ADJACENT TO SIDEWALK.
8. ADA SIGNS IN BOLLARDS AND BOLLARDS SHALL BE INSTALLED WHEN PARKING IS ADJACENT TO FLUSH CURB OR A RAMP.
9. ALL DIMENSIONS ARE TO CENTER OF STRIPE UNLESS OTHERWISE NOTED.
10. STRIPING IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

5 C-4.0 PARKING STALL LAYOUT AND STRIPING
NOT TO SCALE

PARKING LOT STRIPING SPECIFICATIONS:

STANDARDS:
1. ALWAYS FOLLOW ALL APPLICABLE GOVERNING AUTHORITY'S STANDARDS.
2. SURFACES SHOULD BE CLEAN, DRY, AND FREE FROM LOOSE OR PEELING PAINT. REMOVE ALL OIL, DUST, GREASE, DIRT, AND OTHER FOREIGN MATERIAL TO ENSURE ADEQUATE ADHESION. DO NOT APPLY WHEN AIR OR SURFACE TEMPERATURES ARE BELOW 40°F.
3. APPLY SHERWIN-WILLIAMS SETFAST PREMIUM ALKYD ZONE MARKING PAINT A300 WHITE OR A303 YELLOW USING EITHER AIRLESS OR CONVENTIONAL LINE STRIPING EQUIPMENT. USE THE FOLLOWING SETTINGS AS A GUIDE-ACTUAL SETTINGS DEPEND ON ATMOSPHERIC CONDITIONS AT THE TIME OF APPLICATION:

AIRLESS

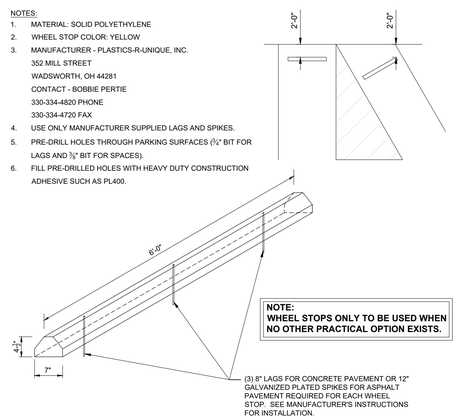
PRESSURE	1800-2700 PSI
HOSE	1/4"-3/8" ID
TIP	0.015"-0.017"
FILTER	60 MESH
REDUCTION	ONLY IF NECESSARY, UP TO 1PT/GAL VM&P NAPTHA R1K3

CONVENTIONAL

GUN	BLINKS 21 (BLEEDER) OR EQUIVALENT
FLUID NOZZLE	#68
AIR NOZZLE	INTERNAL MIX, #709
ATOMIZATION PRESSURE	45-80 PSI
FLUID PRESSURE	40-70 PSI
REDUCTION	ONLY IF NECESSARY, UP TO 1PT/GAL VM&P NAPTHA R1K3

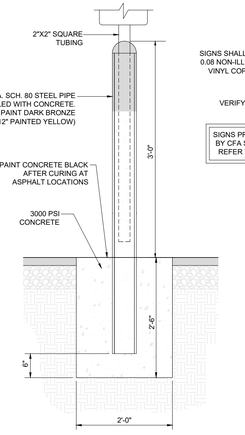
- SHERWIN WILLIAMS, H&C SHARK GRIP SLIP RESISTANT ADDITIVE TO BE MEASURED AND ADDED TO ALL PAINT PER MANUFACTURER'S WRITTEN SPECIFICATIONS. MIX THOROUGHLY PER MANUFACTURER'S RECOMMENDATIONS SO THAT NO CLUMPING IS APPARENT AND UNTIL EVEN DISTRIBUTION IS ACHIEVED. MAINTAIN EVEN DISTRIBUTION OF ADDITIVE IN PAINT THROUGHOUT THE APPLICATION PROCESS.
- MIX PAINT THOROUGHLY BY BOXING, STIRRING, OR POWER AGITATION BEFORE USE. APPLY AT 15 MILS WET TO ACHIEVE A SPREAD RATE OF 400-500 LINEAL FEET OF STANDARD 4" STRIPE PER GALLON. APPLIED AT THIS RATE AT 70 DEGREES F AND 50% RELATIVE HUMIDITY, PAINT WILL DRY WITH NO TRAFFIC PICKUP AFTER 20 MINUTES.
- GENERAL CONTRACTOR TO RE-STRIPE THE LOT 45 DAYS AFTER OPENING.

NOTES:
1. MATERIAL: SOLID POLYETHYLENE
2. WHEEL STOP COLOR: YELLOW
3. MANUFACTURER - PLASTICS-R-UNIQUE, INC.
   352 MILL STREET
   WADSWORTH, OH 44281
   CONTACT - BOBBIE PERTIE
   330-334-4820 PHONE
   330-334-4720 FAX
4. USE ONLY MANUFACTURER SUPPLIED LAGS AND SPIKES.
5. PRE-DRILL HOLES THROUGH PARKING SURFACES (⅝" BIT FOR LAGS AND ⅜" BIT FOR SPACES).
6. FILL PRE-DRILLED HOLES WITH HEAVY DUTY CONSTRUCTION ADHESIVE SUCH AS PL400.



NOTE:
WHEEL STOPS ONLY TO BE USED WHEN NO OTHER PRACTICAL OPTION EXISTS.

6 C-4.0 SOLID PLASTIC WHEEL STOP
NOT TO SCALE

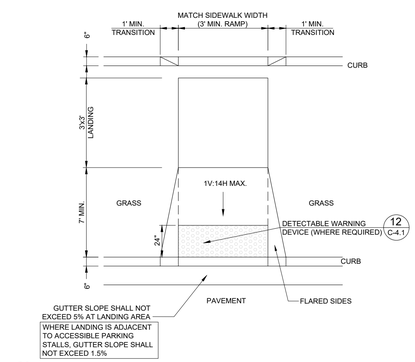


NOTE:
SIGNS SHALL BE FABRICATED USING S/F 0.08 NON-ILLUMINATED ALUMINUM WITH VINYL COPY APPLIED TO THE FIRST SURFACE.

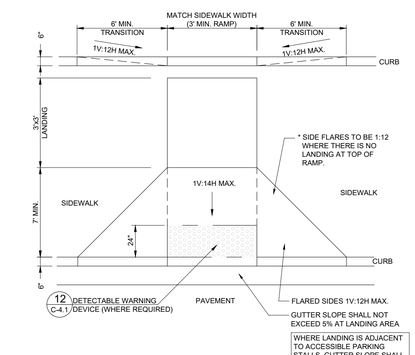
NOTE:
VERIFY COLORS WITH LOCAL MUNICIPALITY.

SIGNS PROVIDED AND INSTALLED BY CFA SIGNAGE CONTRACTOR. REFER TO SIGNAGE PACKAGE.

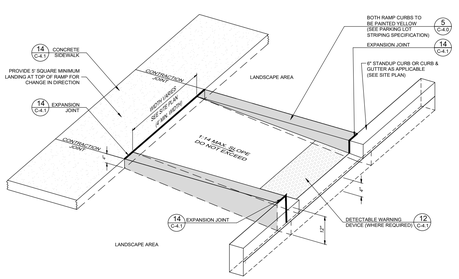
7 C-4.0 BOLLARD MOUNTED SIGN
NOT TO SCALE



8 C-4.0 CURB RAMP w/ SHORT FLARED SIDES
NOT TO SCALE



9 C-4.0 CURB RAMP w/ FLARED SIDES
NOT TO SCALE



10 C-4.0 RETURNED CURB RAMP
NOT TO SCALE

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING, SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

Chick-fil-A
5200 Buffington Rd.
Atlanta Georgia,
30349-2998
Bowman
Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2026 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU
1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

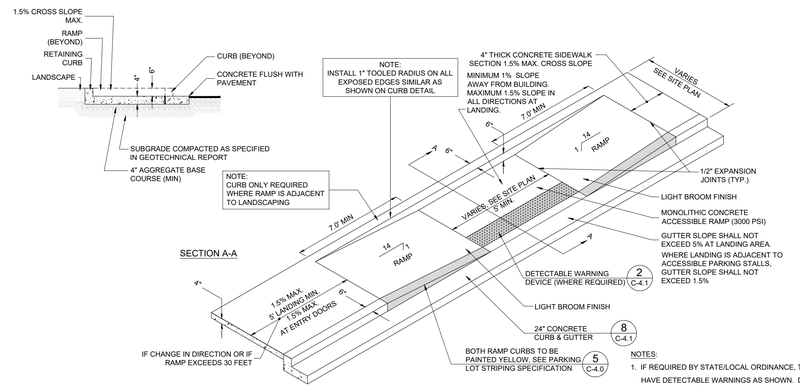
NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
CFA STANDARD DETAILS

SHEET NUMBER
C-4.0

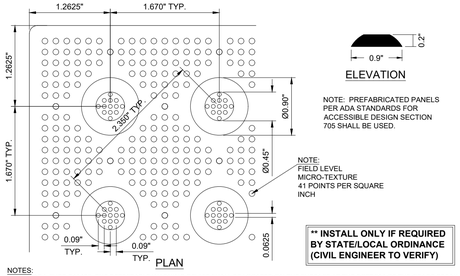


1 C-4.1 SIDEWALK ACCESSIBLE RAMP

NOT TO SCALE

NOTES:

1. IF REQUIRED BY STATE/LOCAL ORDINANCE, THE SURFACE OF THE RAMP LANDING SHALL HAVE DETECTABLE WARNINGS AS SHOWN. DETECTABLE WARNINGS SHALL CONSIST OF RAISED TRUNCATED DOMES. SEE DETECTABLE WARNINGS DETAIL.
2. CONSTRUCT PER A.D.A. STANDARDS.
3. REFER TO PLANS FOR ADJACENT SLOPES.
4. THE CROSS SLOPE OF THE RAMP SURFACE SHALL BE NO GREATER THAN 1.5%.
5. CONSTRUCTION OF THE RAMP SHALL MATCH ADJOINING SIDEWALK DETAIL.
6. CONCRETE SHALL BE P.C.C. 3000 PSI.



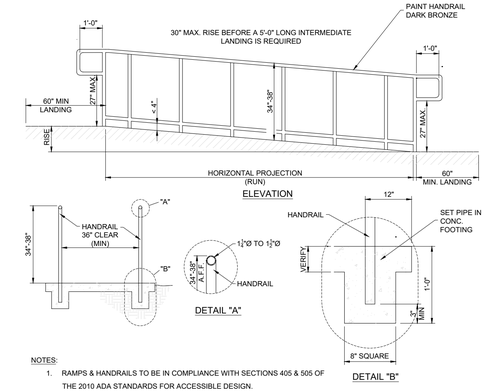
PLAN

NOTES:

1. AT ADA CURB RAMPS, THE DETECTABLE WARNING DEVICE SHALL BE LOCATED ON THE SURFACE AT THE BOTTOM OF THE GRADE BREAK OR AT THE LOWER LANDING. FOR LANDINGS AND BLENDED TRANSITIONS, DETECTABLE WARNING DEVICES SHALL BE LOCATED ON THE LANDING OR BLENDED TRANSITION AT THE BACK OF CURB.
2. DETECTABLE WARNING DEVICE SHALL BE 24 INCHES IN THE DIRECTION OF TRAVEL AND EXTEND THE FULL WIDTH OF THE RAMP OR FLUSH SURFACE.
3. DETECTABLE WARNING SURFACES SHALL BE CONSTRUCTED BY TEXTURING PRODUCTS CONFORMING TO PROWAG R305. TRANSITION SLOPES ARE NOT TO HAVE DETECTABLE WARNINGS. CONTRACTOR SHALL CONFIRM LOCAL CODES ARE MET.
4. WHERE A CURB RAMP IS CONSTRUCTED WITHIN AN EXISTING CURB, CURB & GUTTER AND/OR SIDEWALK, THE EXISTING CURB & GUTTER SHALL BE REMOVED TO THE NEAREST JOINT BEYOND THE CURB TRANSITIONS OR TO THE EXTENT THAT NO REMAINING SECTION OF CURB OR CURB & GUTTER IS LESS THAN 9' LONG. THE EXISTING SIDEWALK SHALL BE REMOVED TO THE NEAREST JOINT BEYOND THE TRANSITION SLOPE WALK AROUND OR TO THE EXTENT THAT NO REMAINING SECTION OF SIDEWALK IS LESS THAN 5'.
5. THE PLAN MUST PROVIDE FOR DETECTABLE WARNING SURFACE COLORS OR MATERIALS THAT PROVIDE THE NECESSARY CONTRAST, EITHER DARK-ON-LIGHT, OR LIGHT-ON-DARK. STANDARD DOME COLOR IS BRICK RED.
6. TRUNCATED DOMES TO BE INSTALLED USING ARMOR TILE CAST IN PLACE DOME TACTILE TILE. PART NUMBER ADA-2424 OR OTHER EQUIVALENT APPROVED MATERIAL. PREFERRED MANUFACTURER ARMOR TILE TACTILE SYSTEMS LANCE MITCHELL(919)622-4615 UNLESS PAVERS ARE REQUIRED, CONTRACTOR TO VERIFY THAT CURB RAMPS MEET LOCAL CODES AND ADA REQUIREMENTS.

2 C-4.1 DETECTABLE WARNING DEVICE

NOT TO SCALE

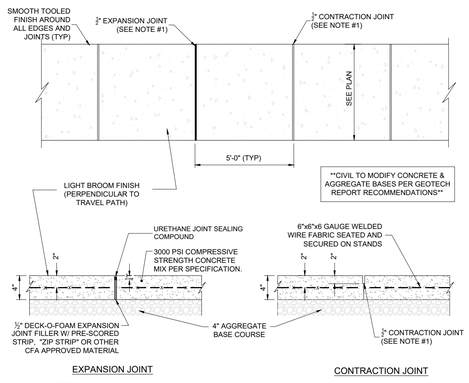


NOTES:

1. RAMPS & HANDRAILS TO BE IN COMPLIANCE WITH SECTIONS 405 & 505 OF THE 2010 ADA STANDARDS FOR ACCESSIBLE DESIGN.
2. VERIFY ALL STATE & LOCAL REQUIREMENTS FOR RAMPS & HANDRAILS.
3. USE ONLY COMMERCIAL GRADE ALUMINUM FENCING WITH DARK BRONZE POWDER COAT FINISH.

3 C-4.1 TYPICAL ADA RAMP & HANDRAIL

NOT TO SCALE



EXPANSION JOINT

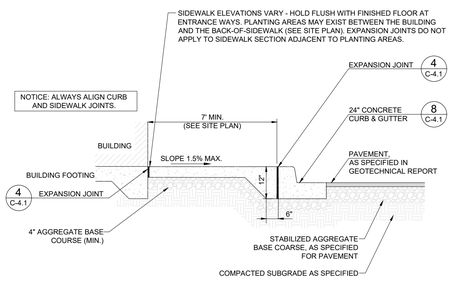
CONTRACTION JOINT

NOTES:

1. CONTRACTION JOINTS AT 5'-0" O.C. TOOLED ¼" WIDE, 1" DEEP OR MAX. ¼ DEEP, WHICHEVER IS GREATER. EXPANSION JOINTS AT 20' MAX. & ALL P.C.s, UNLESS APPROVED OR INDICATED OTHERWISE ON PLAN VIEW JOINT PATTERN.

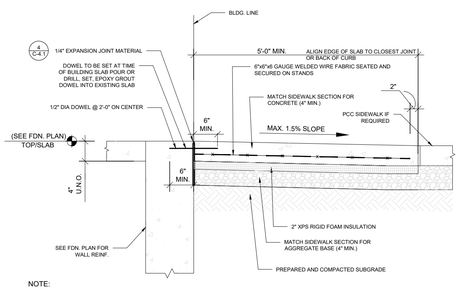
4 C-4.1 CONCRETE SIDEWALK

NOT TO SCALE



5 C-4.1 CONCRETE SIDEWALK w/ CURB & GUTTER

NOT TO SCALE

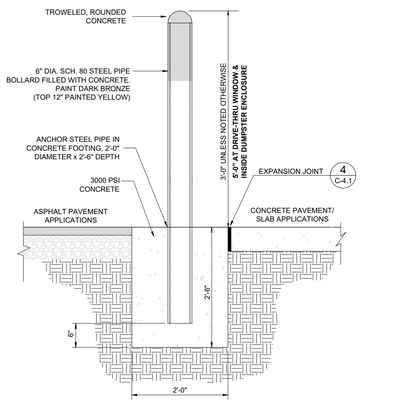


NOTE:

1. CONTRACTOR TO REFER TO STRUCTURAL PLANS FOR ADDITIONAL DETAILS. NOTIFY ENGINEER WITH ANY DISCREPANCIES FROM THIS DETAIL.
2. CONTRACTOR HAS THE OPTION TO DRILL, SET AND EPOXY GROUT DOWEL, IF NOT INSTALLED WHEN THE BUILDING SLAB WAS POURED.
3. REFER TO ARCHITECTURAL DRAWINGS FOR DOOR LOCATIONS. DOOR STOOP SHALL EXTEND 12" PAST EACH SIDE OF DOOR MIN. STOOP IS TO BE SEPARATED ON ALL SIDES WITH 1/4" EXPANSION JOINT MATERIAL, TOPPED WITH SEALANT.
4. USE A 10'X5' SLAB MINIMUM AT A DOUBLE DOOR ENTRY.
5. USE A 5'X5' SLAB MINIMUM AT A SINGLE DOOR ENTRY.
5. DOWELS TO BE INSTALLED INTO THE BUILDING FOUNDATION ONLY. THE REMAINING THREE SIDES OF THE SLAB DO NOT GET TIED TO THE SURROUNDING SIDEWALK.

6 C-4.1 ENTRY DOOR FROST SLAB DETAIL

NOT TO SCALE

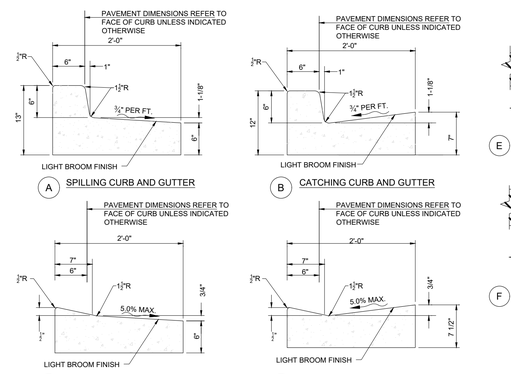


NOTE:

1. BOLLARD HEIGHT SHALL BE 5'-0" AT DRIVE-THRU WINDOW AND INSIDE DUMPSTER ENCLOSURE.

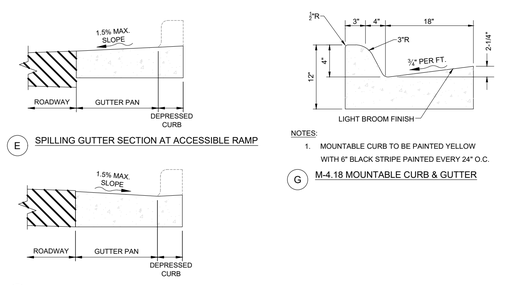
7 C-4.1 CONCRETE BOLLARD

NOT TO SCALE



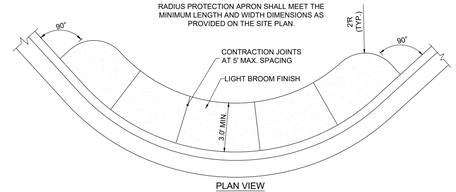
8 C-4.1 CONCRETE CURB & GUTTER

NOT TO SCALE



NOTES:

1. CONCRETE FOR CURBING SHALL HAVE A MIN. COMPRESSIVE STRENGTH OF 3,000 PSI @ 28 DAYS.
2. CONSTRUCTION STAKING FOR CURBING INSTALLATION SHALL BE REFERENCED (CUT OR FILL) TO THE TOP OF CURB.
3. AT CONTRACTOR'S OPTION THE GUTTER THICKNESS MAY BE INCREASED AT THE EDGE OF PAVEMENT TO MAKE BOTTOM OF GUTTER PARALLEL WITH PAVING OF BASE COURSE.
4. CONTRACTION JOINTS @ 10'-0" O.C. TOOLED ½" (± ⅛", -0) WIDE, 1" OR MAX. D/4 DEPTH WHICHEVER IS GREATER. EXPANSION JOINTS @ 40'-0" MAX., AT ENDS OF RADIUS, AND A MINIMUM OF 5 FEET FROM INLET STRUCTURES, UNLESS APPROVED OR INDICATED OTHERWISE ON PLANS.
5. GUTTER SLOPE AT ACCESSIBLE SPACES TO MATCH ADJACENT PAVEMENT, TRAVERSE & LONGITUDINAL.
6. CONSTRUCTION STAKING FOR CURBING INSTALLATION SHALL BE REFERENCED (CUT OR FILL) TO THE TOP OF CURB.
7. LIGHT BROOM FINISH; PERPENDICULAR TO CURB.



NOTES:

1. MOUNTABLE CURB TO BE PAINTED YELLOW WITH 6" BLACK STRIPE PAINTED EVERY 24" O.C.

G M-4.18 MOUNTABLE CURB & GUTTER

NOTE:

1. GENERAL CONTRACTOR SHALL REFERENCE GEOTECHNICAL REPORT FOR PAVEMENT SECTION REQUIREMENTS.

9 C-4.1 LANDSCAPE & IRRIGATION PROTECTOR

NOT TO SCALE

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

Chick-fil-A
5200 Buffington Rd.
Atlanta Georgia,
30349-2998
Bowman
Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

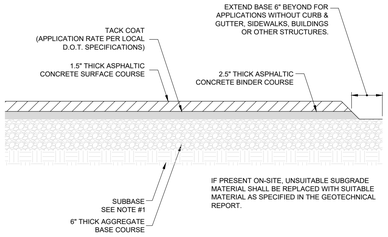
NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

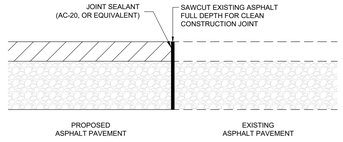
SHEET
CFA STANDARD DETAILS

SHEET NUMBER
C-4.1

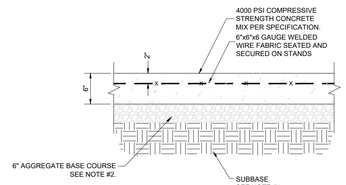


NOTE:
1. GENERAL CONTRACTOR SHALL REFERENCE SITE SPECIFIC GEOTECHNICAL REPORT FOR PAVEMENT, AGGREGATE, AND SUBGRADE SECTION REQUIREMENTS.
2. MINIMUM PAVEMENT THICKNESS SHOULD BE 8" GRADED AGGREGATE BASE, 2.5"ASPHALT PAVEMENT BINDER, 1.5" ASPHALT PAVEMENT SURFACE COURSE, AND TACK COAT.

1 C-4.2 TYPICAL HMAC PAVEMENT SECTION
NOT TO SCALE

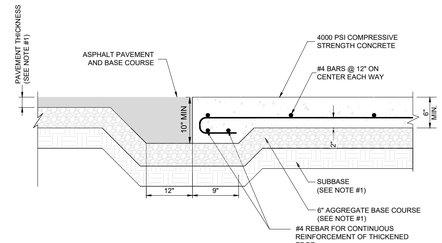


2 C-4.2 BUTT JOINT
NOT TO SCALE



NOTE:
1. GENERAL CONTRACTOR SHALL REFERENCE GEOTECHNICAL REPORT FOR PAVEMENT SECTION REQUIREMENTS.
2. MINIMUM PAVEMENT THICKNESS SHALL BE 6" CONCRETE, 6" AGGREGATE BASE.

3 C-4.2 CONCRETE PAVEMENT DRIVE-THRU LANE
NOT TO SCALE



NOTE:
1. GENERAL CONTRACTOR SHALL REFERENCE GEOTECHNICAL REPORT FOR PAVEMENT SECTION REQUIREMENTS.

4 C-4.2 CONCRETE APRON AT TRASH ENCLOSURE
NOT TO SCALE

NOTE:
1. GENERAL CONTRACTOR SHALL REFERENCE GEOTECHNICAL REPORT FOR PAVEMENT SECTION REQUIREMENTS.

5 C-4.2 PAVEMENT EDGE DETAIL (START & END OF DRIVE-THRU LANES)
NOT TO SCALE

NOTES:
1. DESIGN PER GEOTECH REPORT PREPARED BY UNITED CONSULTING, DATED 09/01/2023.
2. PAVEMENTS & SUBGRADES INCLUDING MATERIALS & COMPACTION SHALL MEET STANDARDS & SPECIFICATIONS OF THE GOVERNING DEPARTMENT OF TRANSPORTATION.
3. JOINTING & SPACING SHALL BE PER CONCRETE JOINT DETAILS.
4. CONCRETE SHALL HAVE A MIN. COMPRESSIVE STRENGTH OF 4,000 PSI @ 28 DAYS.

NOTE:
1. GENERAL CONTRACTOR SHALL REFERENCE GEOTECHNICAL REPORT FOR PAVEMENT SECTION REQUIREMENTS.
2. MINIMUM PAVEMENT THICKNESS SHALL BE 6" CONCRETE, 6" AGGREGATE BASE.

6 C-4.2 CONCRETE PAVEMENT
NOT TO SCALE

NOTE:
1. GENERAL CONTRACTOR SHALL REFERENCE GEOTECHNICAL REPORT FOR PAVEMENT SECTION REQUIREMENTS.

7 C-4.2 TRANSVERSE AND LONGITUDINAL CONTRACTION JOINT
NOT TO SCALE

NOTE:
1. GENERAL CONTRACTOR SHALL REFERENCE GEOTECHNICAL REPORT FOR PAVEMENT SECTION REQUIREMENTS.

8 C-4.2 TRANSVERSE AND LONGITUDINAL DOWELED CONSTRUCTION JOINT
NOT TO SCALE

THIS DOCUMENT, CONSTRUCTION AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2026 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU
1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

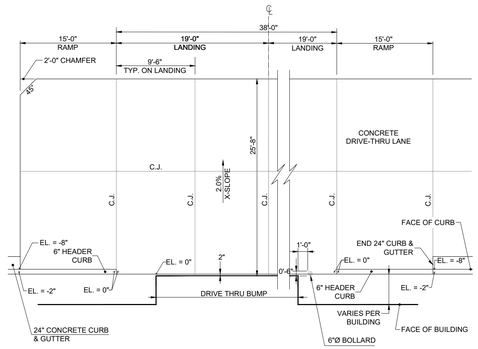
NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
CFA STANDARD DETAILS

SHEET NUMBER
C-4.2

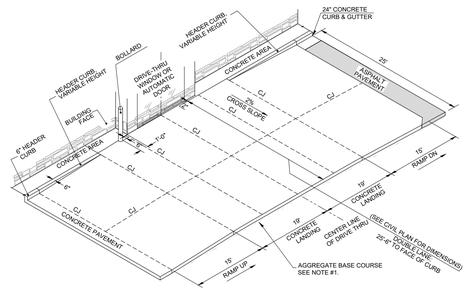


NOTE:

1. ALL ELEVATIONS ON THIS DETAIL ARE RELATIVE TO FFE = 0'.
2. FOR LC FOOTPRINT DECREASE LANDING FROM 38' TO 30'. LANDING SHOULD REMAIN CENTERED ON DRIVE-THRU WINDOW.

1 C-4.3 DRIVE-THRU PLAN - FLUSH WITH FFE

NOT TO SCALE

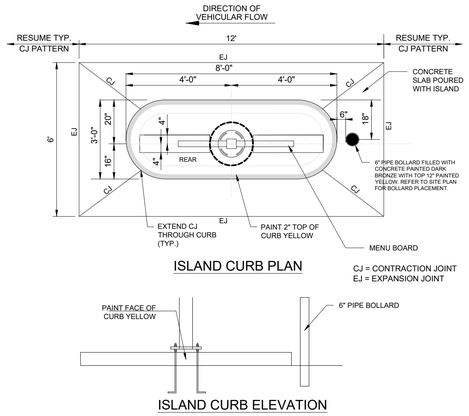


NOTE:

1. GENERAL CONTRACTOR SHALL REFERENCE GEOTECHNICAL REPORT FOR PAVEMENT SECTION REQUIREMENTS.
2. FOR LC FOOTPRINT DECREASE LANDING FROM 38' TO 30'. LANDING SHOULD REMAIN CENTERED ON DRIVE-THRU WINDOW.

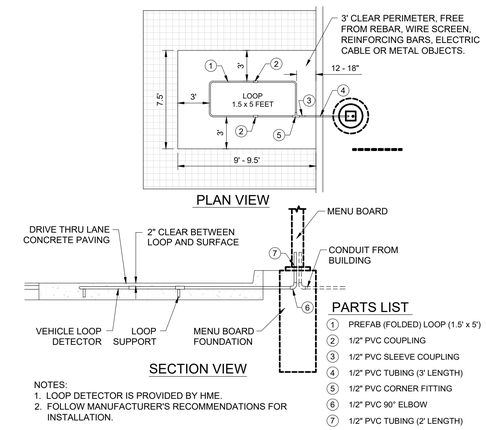
2 C-4.3 DRIVE-THRU ISOMETRIC

NOT TO SCALE



3 C-4.3 DRIVE-THRU ORDER POINT ISLAND

NOT TO SCALE



PARTS LIST

1	PREFAB (FOLDED) LOOP (1.5' x 5')
2	1/2" PVC COUPLING
3	1/2" PVC SLEEVE COUPLING
4	1/2" PVC TUBING (3' LENGTH)
5	1/2" PVC CORNER FITTING
6	1/2" PVC 90° ELBOW
7	1/2" PVC TUBING (2' LENGTH)

NOTES:

1. LOOP DETECTOR IS PROVIDED BY HME.
2. FOLLOW MANUFACTURER'S RECOMMENDATIONS FOR INSTALLATION.

4 C-4.3 MENU BOARD LOOP DETECTION SYSTEM

NOT TO SCALE

NOTES:

1. FOR ALL DEPTHS OF COVER LESS THAN TWO (2) FEET, PIPE MUST BE SCHEDULE 40 PVC. FOR DEPTHS OF COVER GREATER THAN TWO (2) FEET, FLEXIBLE PIPE MAY BE USED. REFER TO SPECIFICATIONS FOR ALLOWABLE PIPE TYPES.
2. A WATERTIGHT CONNECTION SHALL BE MAINTAINED WITH ANY TRANSITION FROM SCHEDULE 40 PVC PIPE TO ANY OTHER PIPE TYPE.
3. THE DOWNSPOUT COLLECTOR DRAIN SHALL BE INSTALLED BEFORE THE DOWNSPOUTS ARE INSTALLED ON THE BUILDING.
4. IF NECESSARY, ADJUST FOOTING TO ALLOW DOWNSPOUT TO BE INSTALLED TIGHT AGAINST BUILDING.

5 C-4.3 BUILDING DOWNSPOUT CONNECTION

NOT TO SCALE

NOTES:

1. CANOPY FOOTING SHALL BE A MINIMUM 2'-0" BELOW DRIVE-THRU LANE PAVEMENT SURFACE. REFER TO CIVIL PLANS FOR EXACT CANOPY FOOTING ELEVATIONS.
2. FLEXIBLE FITTING (NON-SHEAR FERNCO) TO BE UTILIZED BETWEEN PIPE TRANSITIONS AS NECESSARY.
3. CONTRACTOR TO COORDINATE COLUMN DOWNSPOUT ORIENTATION WITH CANOPY VENDOR. ADJUST DOWNSPOUT PIPING AS NECESSARY TO MAKE CONNECTION TO COLLECTOR PIPE.

6 C-4.3 CANOPY DOWNSPOUT CONNECTION

NOT TO SCALE

CONTRACTOR TO REFER TO CIVIL PLANS FOR ORIENTATION & LOCATION OF STORAGE ROOM & RAMP.

REFUSE ENCLOSURE PLAN NOTES:

1. SEE SITE PLAN FOR LAYOUT OF CONCRETE APPROACH PAD.
2. SEE GRADING PLAN FOR ELEVATIONS ON DUMPSTER PAD & DRAINAGE OF INTERIOR TRASH ENCLOSURE.
3. PROVIDE POSITIVE DRAINAGE ON APPROACH PAD AWAY FROM DUMPSTER. (SEE GRADING PLAN)
4. SEE ARCHITECTURAL PLANS FOR ADDITIONAL DIMENSIONS & DETAILS ON ENCLOSURE; COORDINATE & VERIFY SIZE WITH ARCHITECT & STRUCTURAL ENGINEER.
5. GENERAL CONTRACTOR SHALL DRILL HOLES FOR GATE PINS IN BOTH THE OPEN & CLOSED POSITION.

7 C-4.3 SCREENED REFUSE ENCLOSURE

NOT TO SCALE

NOTES:

1. SEE PLUMBING PLANS FOR GREASE TRAP DETAIL.
2. CLEAN-OUT COVERS SHALL BE PROVIDED ON ALL CLEAN-OUTS REGARDLESS OF LOCATION.
3. CONCRETE COLLAR ALSO REQUIRED FOR SINGLE CLEAN-OUTS IN PAVEMENT.

8 C-4.3 CLEAN-OUT (OUTSIDE OF BUILDING)

NOT TO SCALE

NOTE: SEE PLUMBING PLANS FOR GREASE TRAP DETAIL

9 C-4.3 THICKENED PAVEMENT @ STRUCTURES

NOT TO SCALE

10 C-4.3 STORM STRUCTURE WEEP HOLE DETAILS

NOT TO SCALE

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2026 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

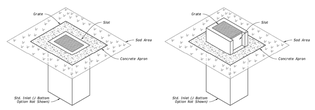
SHEET
CFA STANDARD DETAILS

SHEET NUMBER
C-4.3

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING GROUP, LTD. SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

GENERAL NOTES:

1. Work this Index with Index 425-001 and Index 425-010.
2. Chamfer all exposed edges and corners ¾" chamfer or radius to ¾" radius.
3. All reinforcing to Grade 60 bars with 3" minimum cover unless otherwise noted. Cut or bend bars for 150 clearance around pipe opening. Provide one additional #4 bar above and at each side of pipe opening.
4. Use Concrete Apron on Inlets without slots and inlets with non-traversable slots only when called for in the Plans.
5. Quantities are for informational and estimating purposes only.

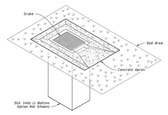


DITCH BOTTOM INLET TYPE C TRAVERSABLE

(Without Slot - Type D, E, and H Similar, Pipe Connection Rod Shown)

DITCH BOTTOM INLET TYPE C NON-TRAVERSABLE

(Slot = 7" Shown - Type D, E, and H Similar, Pipe Connection Rod Shown)



DITCH BOTTOM INLET TYPE C TRAVERSABLE

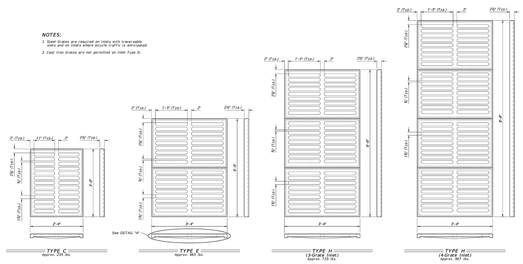
(Single Slot = 7" Shown, Double Slot, Type D, and E Similar, Pipe Connection Rod Shown)

TABLE OF CONTENTS:

Sheet	Description
1	General Notes and Contents
2	Type C – Dimensional, Reinforcing, and Grate Details
3	Type D – Dimensional, Reinforcing, and Grate Details
4	Type E – Dimensional, Reinforcing, and Grate Details
5	Type H (2 & 3 Grate – Dimensional, Reinforcing, and Steel Grate Details
6	Type H (4 Grate) – Dimensional, Reinforcing, and Steel Grate Details
7	Cast Iron Grate Details
8	Non-Traversable Inlet Details
9	Traversable Inlet Without Slot Details
10	Traversable Inlet With Slot Details
11	Case 1 – Add Traversable Slots to Existing Inlets
12	Case 2 – Add Traversable Slots (Partial) to Existing Inlets
13	Case 3 – Add Traversable Slots (Partial) to Existing Inlets and Ditch Block
14	Alternate A Structure Bottom – Top Slab Details

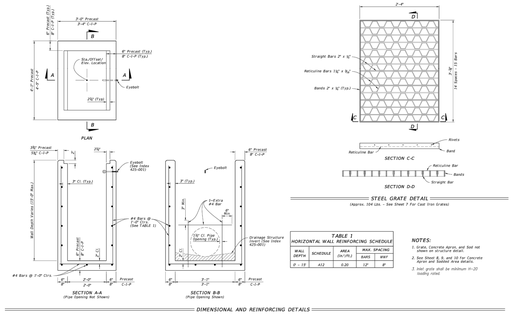
NOTES:

1. Steel Grates are required on inlets with traversable slots and on inlets where bicycle traffic is anticipated.
2. Cast Iron Grates are not permitted on Inlet Type D.



DETAIL "A"
(Typical Section)

CAST IRON GRATE DETAILS



DIMENSIONAL AND REINFORCING DETAILS

TABLE 1
HORIZONTAL WALL REINFORCING SCHEDULE

WALL DEPTH	SCHEDULE	AREA (sq.in/ft.)	WALL SPACING BARS	WHY
P ≤ 18	411	0.20	12"	P

NOTES:

1. Grate, Concrete Apron, and Bed not shown on structure detail.
2. See Sheet B, E, and I2 for Concrete Aprin and Slotted Area details.
3. Inlet grate shall be minimum H-20 loading rating.

STEEL GRATE DETAIL

(Approx. 704 Lbs. - See Sheet 7 for Cast Iron Grates)

1 DITCH BOTTOM INLET TYPE C

C-5.0

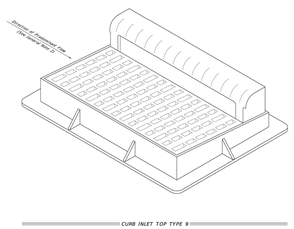
NOT TO SCALE

GENERAL NOTES:

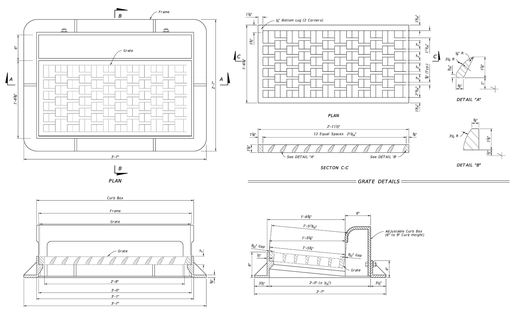
1. Work this Index with Index 425-001 and Index 425-010.
2. Orient grate with vanes directed toward predominant flow.
3. Provide 1¼" minimum cover for steel in slab tops unless otherwise shown. Tops may be either cast-in-place or precast concrete.
4. Place top slab openings such that 2 edges of inlet frame will be located directly above bottom wall or riser wall for alternate B applications.
5. When used on a structure with dimensions larger than those detailed on Sheet 3 and risers are not applied, construct the top slab using Index 425-010 with the slab opening adjusted to 24"x36". The "Special Top Slab" on Index 425-010 is not permitted.
6. Frame may be adjusted with one to six courses of brick.
7. Install grates with approximately equal openings that satisfy AASHTO H1-93 loading are permitted. Provide reversible (right or left) grates.

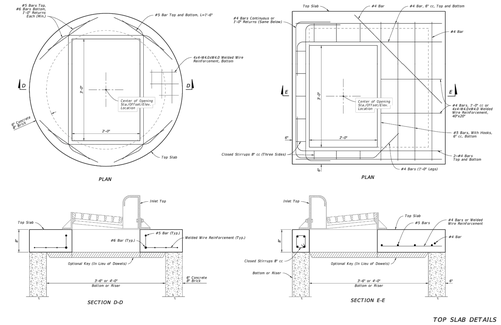
TABLE OF CONTENTS:

Sheet	Description
1	General Notes and Contents
2	Frame and Grate Details
3	Top Slab Details



CURB INLET TOP TYPE 9





TOP SLAB DETAILS

2 CURB INLET TOP TYPE 9

C-5.0

NOT TO SCALE

Chick-fil-A

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

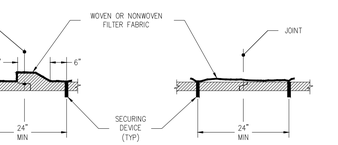
Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com

© 2025 Bowman Consulting Group Ltd

Seal

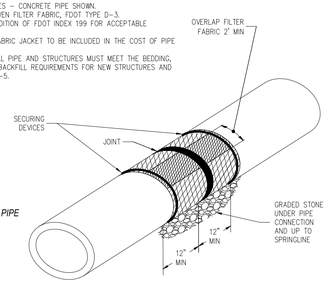
WILLIAM PFEFFER, P.E.
LICENSE NO. 73058



PIPE SECTIONS

NOTE:

1. SECURING DEVICES MAY BE ELIMINATED BY DOUBLE WRAPPING PIPE WITH FILTER FABRIC.
2. FOR ALL PIPE TYPES – CONCRETE PIPE SHOWN.
3. WOVEN OR NONWOVEN FILTER FABRIC, FDOT TYPE D-3.
4. SEE THE LATEST EDITION OF FDOT INDEX 199 FOR ACCEPTABLE SECURING DEVICES.
5. COST OF FILTER FABRIC JACKET TO BE INCLUDED IN THE COST OF PIPE CULVERTS.
6. INSTALLATION OF ALL PIPE AND STRUCTURES MUST MEET THE BEDDING, INSTALLATION AND BACKFILL REQUIREMENTS FOR NEW STRUCTURES AND PIPE ON DETAIL SW-5.



3 FILTER FABRIC FOR STORM PIPE

C-5.0

NOT TO SCALE

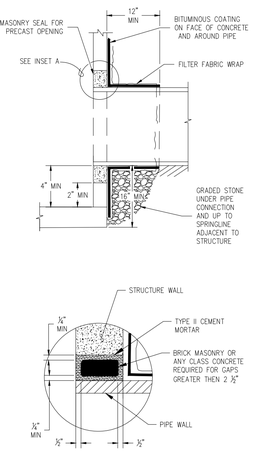
THIS DETAIL COVERS ALL PIPE CONNECTIONS TO STRUCTURES.

NEW PIPE CONNECTION NOTES:

1. CONNECTION SHALL BE MADE IN A DRY TRENCH.
2. EXISTING STRUCTURES SHALL BE CORED FOR NEW PIPE CONNECTIONS.
3. PIPE BEDDING AT CONNECTION TO BE ¾" TO ¾" GRADED STONE (100% PASSING 1" SIEVE).
4. BACKFILL (6" MAX. SIZE) MATERIAL TO BE PLACED AND COMPACTED PER APPROPRIATE SPEC'S DEPENDING ON THE SURFACE IMPROVEMENTS OR A MINIMUM OF 95% PER AASHTO T-180.
5. BITUMINOUS COATING REQUIRED (ANY SUITABLE BITUMINOUS MATERIAL MAY BE FIELD APPLIED) BITUMINOUS COATING TO EXTEND 2" BEYOND FILTER FABRIC.
6. ALTERNATE CONNECTION AND/OR MODIFICATION TO THIS DETAIL MUST BE APPROVED BY THE TOWN OF JUPITER PRIOR TO INSTALLATION.
7. INSTALLATION OF ALL PIPE AND STRUCTURES MUST MEET THE BEDDING, INSTALLATION AND BACKFILL REQUIREMENTS FOR NEW STRUCTURES AND PIPE ON DETAIL SW-5.

EXISTING PIPE CONNECTION REPAIR NOTES:

1. CONNECTION REPAIR SHALL BE MADE IN A DRY TRENCH.
2. REPAIR BOTH THE INSIDE AND OUTSIDE OF THE STRUCTURE PENETRATIONS.
3. CHIP AWAY ANY LOOSE MATERIAL.
4. SAWCUT OR CUT BACK THE EXTREMITIES OF THE REPAIR LOCATION TO A DEPTH OF ¾" TO AVOID FEATHER EDGING.
5. MINIMUM MUD THICKNESS FOR REPAIR IS ¾".
6. WHERE BREAKING OUT IS NOT REQUIRED, ROUGHEN THE SURFACE AND REMOVE ANY LAITANCE BY LIGHT SCABBING OR ABRASIVE BLASTING.
7. CLEAN SURFACE TO REMOVE ANY DUST, UNSOUND OR CONTAMINATED MATERIAL, PLASTER, PAINT, OIL, GREASE, CORROSION DEPOSITS OR ALGAE.
8. APPLY THOROC 1440 OR APPROVED EQUAL IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS.
9. ALTERNATE CONNECTION AND/OR MODIFICATION TO THIS DETAIL MUST BE APPROVED BY THE TOWN OF JUPITER PRIOR TO INSTALLATION.
10. ITEMS 3 THROUGH 6 UNDER NEW PIPE CONNECTION NOTES ABOVE APPLY TO REPAIR ON OUTSIDE OF STRUCTURE.
11. INSTALLATION OF ALL PIPE AND STRUCTURES MUST MEET THE BEDDING, INSTALLATION AND BACKFILL REQUIREMENTS FOR NEW STRUCTURES AND PIPE ON DETAIL SW-5.



4 PIPE TO STRUCTURE CONNECTION

C-5.0

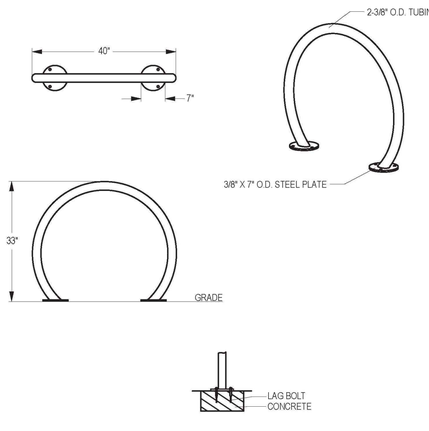
NOT TO SCALE

BELSON OUTDOORS

111 North River Road
North Aurora, IL 60542
Phone: (800) 323-5664
Fax: (630) 897-0573
sales@belson.com

Model # ORN-2-SF-G — Dimension Sheet

ORION™ | 2 BIKES

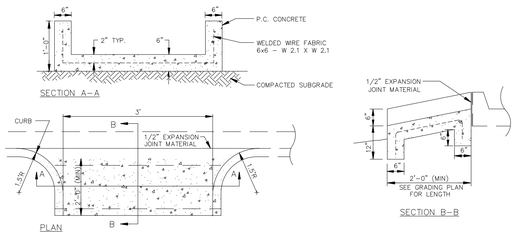


www.belson.com

5 BICYCLE RACK

C-5.0

NOT TO SCALE

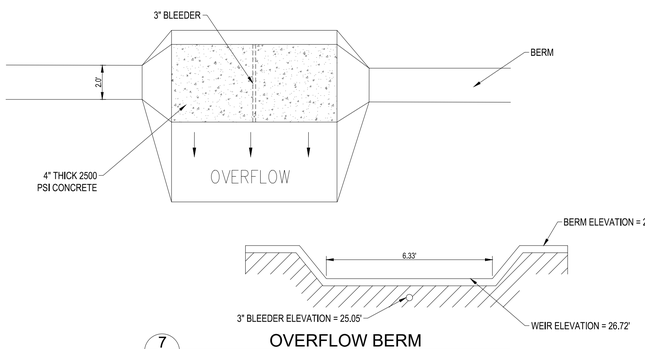


6 CONCRETE FLUME

C-5.0

NOT TO SCALE

PLEASE NOTE THIS DETAIL IS NOT TO SCALE, AND IS INTENDED AS A GENERAL DETAIL. SIDESLOPES ON THE OUTSIDE OF THE WRA MAY VARY SIGNIFICANTLY FROM THE SLOPE DEPICTED HERE.



7 OVERFLOW BERM

C-5.0

NOT TO SCALE

CHICK-FIL-A
LEHIGH ACRES FSU
1110 HOMESTEAD RD N LEHIGH ACRES, FL 33936 LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

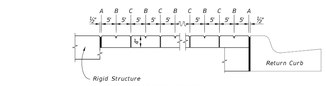
SHEET
SITE AND DRAINAGE DETAILS

SHEET NUMBER
C-5.0

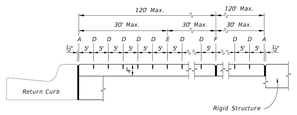
THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

GENERAL NOTES:

1. Construct sidewalks in accordance with Specification 522. Use 6" concrete for Sidewalks and Curb Ramps Located within Curb Returns (See Plan View). Install all other concrete with thickness as shown, unless otherwise detailed in the Plans.
2. Include detectable warnings on sidewalk curb ramps in accordance with Index 522-002.
3. For Driveways see Index 522-003.
4. Bond breaker material can be any impermeable coated or sheet membrane or preformed material having a thickness of not less than 6 mils and not more than ½".
5. Construct sidewalks with Edge Beam through the limits of any surface mounted Pedestrian/Bicycle Railing or Pipe Guiderail shown in the plans. (See RAILING DETAIL)



OPEN JOINTS



SAWED JOINTS

LONGITUDINAL SECTION

LEGEND:

A- ½" Expansion Joints (Preformed Joint Filler) between the sidewalk and; driveways, sidewalk-intersections, and all other fixed objects (e.g. drainage inlets and utility poles).

B- ⅛" Dummy Joints, Tooled

C- ⅛" Formed Open Joints

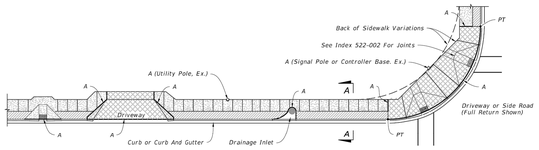
D- 9/16" Saw Cut Joints, 1½" Deep (within 96 hours) Max. 5' Centers

E- 9/16" Saw Cut Joints, 1½" Deep (within 12 hours) Max. 30' Centers Joint(s) Required When Length Exceeds 30'

F- ½" Expansion Joint When Run Of Sidewalk Exceeds 120'. Intermediate locations when called for in the plans or at locations as directed by the Engineer.

G- Cold Joint With Bond Breaker, Tooled

SIDEWALK JOINTS



PLAN

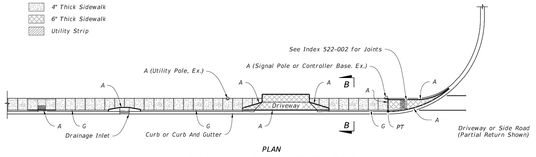
SIDEWALK WITH UTILITY STRIP

LEGEND:

4" Thick Sidewalk

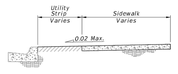
6" Thick Sidewalk

Utility Strip

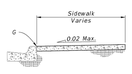


PLAN

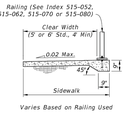
SIDEWALK WITHOUT UTILITY STRIP



SECTION A-A

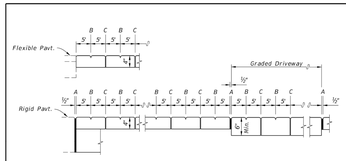


SECTION B-B

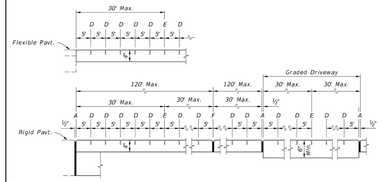


Varies Based on Railing Used

RAILING DETAIL



OPEN JOINTS



SAWED JOINTS

LONGITUDINAL SECTION

LEGEND:

A- ½" Expansion Joints (Preformed Joint Filler) between the sidewalk and driveways, sidewalk-intersections, and all other fixed objects (e.g. drainage inlets and utility poles).

B- ⅛" Dummy Joints, Tooled

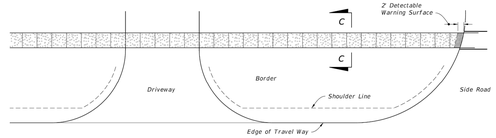
C- ⅛" Formed Open Joints

D- 9/16" Saw Cut Joints, 1½" Deep (within 96 hours) Max. 5' Centers

E- 9/16" Saw Cut Joints, 1½" Deep (within 12 hours) Max. 30' Centers Joint(s) Required When Length Exceeds 30'

F- ½" Expansion Joint When Run Of Sidewalk Exceeds 120'. Intermediate locations when called for in the plans or at locations as directed by the Engineer.

SIDEWALK JOINTS

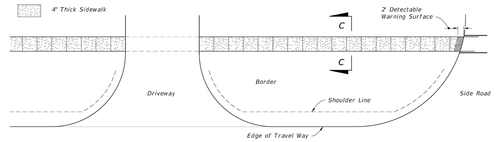


PLAN

CONTINUOUS SIDEWALK

LEGEND:

4" Thick Sidewalk



PLAN

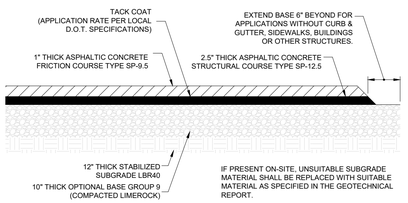
DISCONTINUOUS SIDEWALK



SECTION C-C

1 C-5.1 FDOT CONCRETE SIDEWALK

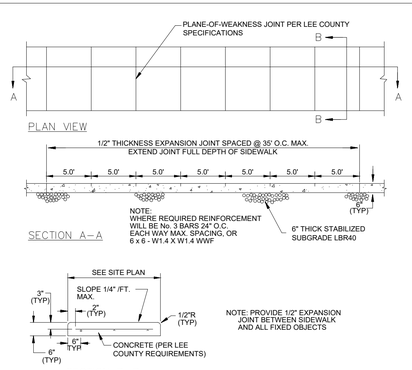
NOT TO SCALE



LEE COUNTY DOT MAINTAINED ARTERIAL ROAD ASPHALT PAVEMENT SECTION (HOMESTEAD RD)

2 C-5.1

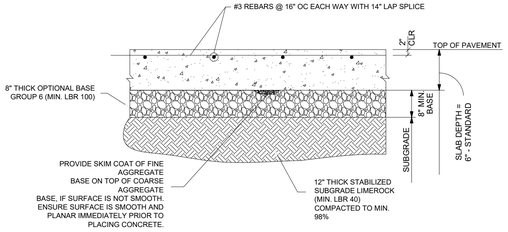
NOT TO SCALE



ROW CONCRETE SIDEWALK

3 C-5.1

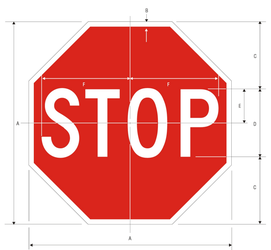
NOT TO SCALE



6" CONCRETE PAVEMENT SECTION

7 C-5.1

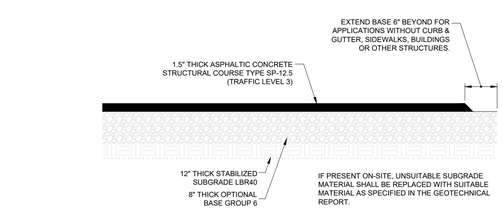
NOT TO SCALE



STOP SIGN DETAIL

4 C-5.1

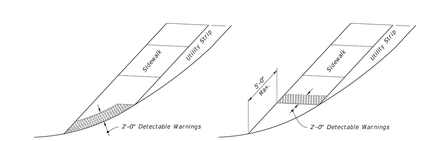
NOT TO SCALE



LOCAL COUNTY ROAD ASPHALT PAVEMENT SECTION (ASHLAR RD)

5 C-5.1

NOT TO SCALE



DETECTABLE WARNING ON FLUSH SHOULDER SIDEWALKS

4. Detectable Warnings:

A. Install detectable warnings in accordance with Specification 527.

B. Place detectable warnings across the full width of the ramp or landing, to a minimum depth of 2 feet measured perpendicular to the curb line and no greater than 5 feet from the back of the curb or edge of pavement.

C. If detectable warnings are shown in the Plans on slopes greater than 5%, align the truncated domes with the centerline of the ramp; otherwise, the truncated domes are not required to be aligned.

FDOT DETECTABLE WARNING

6 C-5.1

NOT TO SCALE

Chick-fil-A

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com

© 2026 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU
1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
SITE AND DRAINAGE DETAILS

SHEET NUMBER
C-5.1

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING, INC. SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

GENERAL NOTES:

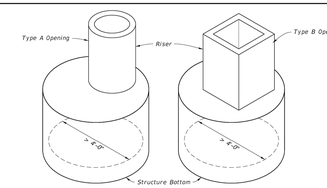
1. Work this Index with Specification 425 and Index 425-001.
2. Type P standard structure bottoms are 4'-0"diameter and smaller (Alt. A) and 3'-6" square (Alt. B) . Larger standard structure bottoms are designated Type J. Risers are permitted for all structures.
3. Walls of circular structures (Alt. A) constructed in place may be of brick or reinforced concrete. Construct precast and rectangular structures (Alt. B) with reinforced concrete only.
4. Wall thickness and reinforcement are for either reinforced cast-in-place or precast concrete units except that precast circular units may be furnished with walls in accordance with ASTM C478 (See Table 1).
5. Top and bottom slab thickness and reinforcement are for precast and cast-in-place construction. Use Class II concrete, except when Class IV concrete is shown in the Plans.
6. Alt. A or Alt. B structure bottoms may be used in conjunction with curb inlet tops Types 1, 2, 3, 4, 5, 6, 9, and 10, and any manhole or junction box. Alt. B structure bottoms may be used in conjunction with curb inlet Types 7 & 8, or any ditch bottom inlet.
7. Rectangular structures may be rotated as directed by the Engineer in order to facilitate connections between the structure walls and pipes.
8. Use straight embedment reinforcement in top and bottom slabs ,except when ACI hooks are specifically required.
9. Construct corner fillets as shown for rectangular structures used with circular risers and inlet throats, and when used on skew with rectangular risers, inlets, and inlet throats. Construct fillets in the top slab of the Alt. A structure bottoms when used with the Type B risers. Reinforce each fillet with two #5 bars.
10. Units larger than specified standards may be substituted at the contractor's option when these units will not cause or increase the severity of utility conflicts. Furnish such larger units at no additional cost to the Department. Larger Alt. A units cannot replace Alt. B units without approval of the Engineer. This Note applies to this Index only.

REINFORCEMENT NOTES:

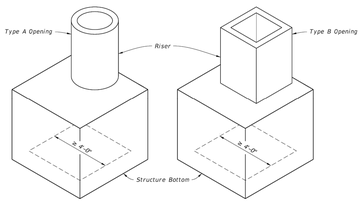
1. Locate wall reinforcement in rectangular structures as shown in the WALL REINFORCEMENT SPLICE DETAILS in Index 425-001.
2. Provide a minimum 2"clear cover for all reinforcement unless otherwise noted and except for 2¾"diameter ASTM C478 units.
3. Additional bars used to restrain hole formers for precast structures with grouted pipe connections may be left flush with the hole surface.
4. Cut or bend reinforcement at pipe openings to maintain cover.
5. Remove exposed ends of reinforcing at precast pipe openings and grouted joints to 1" below the concrete surface and seal with a Type F Epoxy meeting the requirements of Specification 926.
6. Equivalent area smooth or deformed welded wire reinforcement may be substituted in accordance with Index 425-001.

TABLE OF CONTENTS:

Sheet	Description
1	General Notes and Contents
2	Dimensional and Reinforcing Details
3	Tables 1, 2, 3, and 4
4	Tables 5 and 6

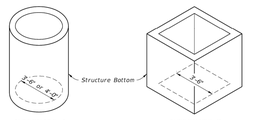


ALTERNATE A



ALTERNATE B

TYPE J

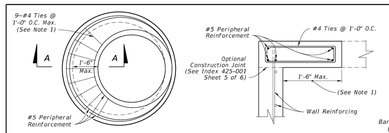


ALTERNATE A ALTERNATE B

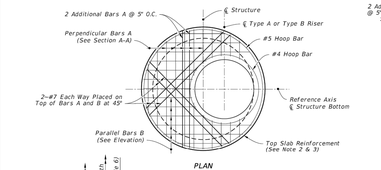
TYPE P

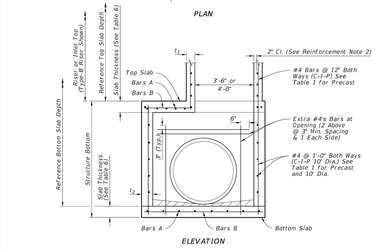
LAST REVISION 11/01/20	REVISION	DESCRIPTION:	FDOT FY 2023-24 STANDARD PLANS	STRUCTURE BOTTOMS TYPE J AND P	INDEX 425-010	SHEET 1 of 4

10/6/2022 1:43:14 PM



SPECIAL TOP SLAB

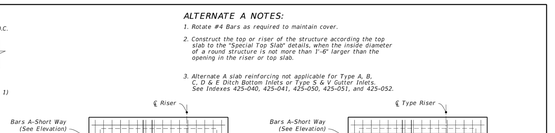


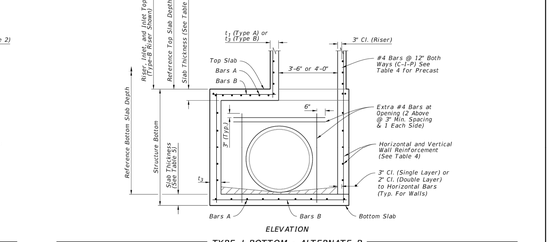


TYPE J BOTTOM - ALTERNATE A

ALTERNATE A NOTES:

1. Rotate #4 Bars as required to maintain cover.
2. Construct the top or riser of the structure according the top slab to the "Special Top Slab" details, when the inside diameter of a round structure is not more than 1'-6" larger than the opening in the riser or top slab.
3. Alternate A slab reinforcing not applicable for Type A, B, C, D & E Ditch Bottom Inlets or Type S & V Gutter Inlets. See Indexes 425-040, 425-041, 425-050, 425-051, and 425-052.





TYPE J BOTTOM - ALTERNATE B

DIMENSIONAL AND REINFORCING DETAILS

LAST REVISION 11/01/20	REVISION	DESCRIPTION:	FDOT FY 2023-24 STANDARD PLANS	STRUCTURE BOTTOMS TYPE J AND P	INDEX 425-010	SHEET 2 of 4

10/6/2022 1:43:15 PM

TABLE 1 - ALTERNATE A - STRUCTURES

TYPE	STRUCTURE/RISER DIAMETER (ft)	CAST-IN-PLACE ITEMS CLASS II CONCRETE t1 RISER (in.)	t2 BOTTOM (in.)	As (in²/ft.)	PRECAST ITEMS CLASS II CONCRETE t1 RISER (in.)	t2 BOTTOM (in.)	As (in²/ft.)	ASTM C478 t1 or t2 (in.)	As*** (in²/ft.)
P	3'-6"	6	8	0.20	6	8	0.20	4**	0.105
P	4'-0"	6	8	0.20	6	8	0.20	5**	0.120
J	5'-0"	–	8	0.20	–	8	0.20	6**	0.150
J	6'-0"	–	8	0.20	–	8	0.20	6	0.180
J	7'-0"	–	8	0.20	–	8	0.20	7	0.210
J	8'-0"	–	8	0.20	–	8	0.20	8	0.240
J	10'-0"	–	10	0.40/#*	–	10	0.40/#*	10	0.300
J	12'-0"	–	10	0.40/#*	–	12	0.40/#*	12	0.360

t_1 and t_2 - Wall Thickness.
A_s- Vertical and horizontal areas of reinforcement.
##Provide 0.20 $in.^2/ft.$ at each face, 12" max. bar spacing.
**Modified minimum wall thickness.
***Min. total circumferential reinforcement for continuous steel hoops:
A_2 = 0.40 sq. in. for riser section height equal or less than 2'-0" (2 hoop min.)
A_s = 0.60 sq. in. for riser section height more than 2'-0" up to 4'-0" (3 hoop min.)
Areas of reinforcing for precast items are based on Grade 60 reinforcing.
No reduction in the area of reinforcement is allowed for welded wire fabric in Table 1.
Area of vertical reinforcing may be reduced in accordance with ASTM C478.

TABLE 2 - ALTERNATE B SQUARE AND RECTANGULAR STRUCTURES

TYPE	WALL LENGTH (FT)	MAX. DEPTH (FT)	WALL THICKNESS (t_2) C-I-P (in.)	6 Riser & Bottom	PRECAST (in.)
P	≤ 3'-6"	40	6		6
J	4'-0"	40	8		6
J	5'-0"	22	–		6
J	6'-0"	15	–		6
J	5'-0" to 9'-0"	40	8		8
J	10'-0"	26	8		8
J	10'-0" to 12'-0"	40	10		9
J	16'-0"	35	–		9
J	16'-0"	40	10		10
J	20'-0"	25	–		9
J	20'-0"	30	10		10

See Table 4 for Reinforcing Schedule.

TABLE 3 - REINFORCING SCHEDULE

SCHEDULE	GRADE 60 BARS OR 65 KSI & 70 KSI WELDED WIRE REINFORCING GRADE 60 AREA (in² /ft)	MAXIMUM SPACING GR 60 BARS (in.)	65 KSI (in.)	70 KSI (in.)	UWR EQUIV. AREA
A12	0.20	12	8	8	
A6	0.20	6	5	4½	
B10	0.24	10	8	7½	
B5.5	0.24	5½	5	4	
C6.5	0.37	6½	6	5	
C3.5	0.37	3½	3	2½	
D7	0.53	7	6	5	
D4.5	0.53	4½	4	3½	
E5	0.73	5	4	4	
E3	0.73	3	3	3	
F5	1.06	5	4	4	
F3.5	1.06	3½	3	3	
G5	1.45	5	4	4	
G3.5	1.45	3½	3	3	
H4	1.75	4	3	3	

TABLE 4 - WALL DESIGNS - RECTANGULAR STRUCTURES

VERTICAL REINFORCING WALL DEPTH	VERTICAL REINFORCING SCHEDULE	HORIZONTAL REINFORCING WALL DEPTH	HORIZONTAL REINFORCING SCHEDULE	WALL THICKNESS
SIZE: 3'-6" & RISER				
≥1.17" – 40"	A12	≥1.17" < 10"	B10	6"/8"
		10" < 18"	B5.5	6"/8"
		18" < 29"	C6.5	6"/8"
		29" - 40"	C3.5	6"/8"
SIZE: 4'-0"				
≥1.17" – 40"	A12	≥1.17" < 6"	B10	6"/8"
		6" < 10"	B5.5	6"/8"
		10" < 20"	C6.5	6"/8"
		20" < 28"	C3.5	6"/8"
		28" - 40"	D4.5	6"/8"
SIZE: 5'-0"				
≥1.17" – 40"	A12	≥1.17" < 5"	B5.5	6"/8"
		5" < 9"	C6.5	6"/8"
		9" < 15"	C3.5	6"/8"
		15" < 22"	D4.5	6"/8"
		22" - 40"	E3	8"
SIZE: 6'-0"				
≥1.17" < 26"	A12	≥1.17" < 9"	C3.5	6"/8"
		9" < 15"	D4.5	6"/8"
		15" < 26"	E3	8"
	Inside/Outside		Inside/Outside	
26' - 40"	A12 / A12	26' - 40"	D7 / D7	8"
SIZE: 7'-0"				
	Inside/Outside		Inside/Outside	
≥1.17" < 25"	A12 / A12	≥1.17" < 7"	B10 / B10	8"
26' - 40"	B10 / B10	7" < 10"	B5.5 / B5.5	8"
		10" < 20"	C6.5 / C6.5	8"
		20" < 30"	D7 / D7	8"
		30' - 40"	E5 / E5	8"
SIZE: 8'-0"				
	Inside/Outside		Inside/Outside	
≥1.17" < 20"	A12 / A12	≥1.17" < 6"	B5.5 / B5.5	8"
20' - 40"	C6.5 / C6.5	6" < 13"	C6.5 / C6.5	8"
		13" < 22"	D7 / D7	8"
		22" < 31"	E5 / E5	8"
		31' - 40"	F5 / F5	8"
SIZE: 9'-0"				
	Inside/Outside		Inside/Outside	
≥1.17" < 12"	A12 / A12	≥1.17" < 8"	C6.5 / C6.5	8"
12" < 28"	C6.5 / C6.5	8" < 15"	D7 / D7	8"
28' - 40"	D7 / D7	15" < 23"	E5 / E5	8"
		23' - 40"	F5 / F5	8"
3@CE: 10'-0"				
	Inside/Outside		Inside/Outside	
≥1.17" < 10"	B10 / B10	≥1.17" < 10"	D7 / D7	8"
10" < 21"	C6.5 / C6.5	10" < 17"	E5 / E5	8"
21" < 26"	D7 / D7	17" < 26"	F5 / F5	8"
26' - 40"	C6.5 / C6.5	26' - 40"	F5 / F5	10"
SIZE: 10'-0" (Precast Only)				
	Inside/Outside		Inside/Outside	
26' – 40'	D7 / D7	26' – 40'	F5 / F5	9"
SIZE: 12'-0"				
	Inside/Outside		Inside/Outside	
≥1.17" < 14"	B10 / B10	≥1.17" < 10"	C6.5 / C6.5	10"
14" < 25"	C6.5 / C6.5	10" < 17"	D7 / D7	10"
25' - 40'	D7 / D7	17" < 24"	E5 / E5	10"
		24' - 40'	F5 / F5	10"
SIZE: 12'-0" (Precast Only)				
	Inside/Outside		Inside/Outside	
≥1.17" < 12"	B10 / B10	≥1.17" < 10"	D7 / D7	9"
12" < 24"	C6.5 / C6.5	10" < 17"	D4.5 / D4.5	9"
24' - 40'	D7 / D7	17" < 23"	E5 / E5	9"
		23" < 32"	F5 / F5	9"
		32' - 40'	G5 / G5	9"
SIZE: 16'-0"				
	Inside/Outside		Inside/Outside	
≥1.17" < 11"	C6.5 / C6.5	≥1.17" < 17"	D7 / D7	10"
11" < 20"	D7 / D7	17" < 20"	E5 / E5	10"
20" < 28"	E5 / E5	20" < 28"	F5 / F5	10"
28' - 40'	F5 / F5	28' - 40'	G5 / G5	10"
SIZE: 16'-0" (Precast Only)				
	Inside/Outside		Inside/Outside	
≥1.17" < 10"	C6.5 / C6.5	≥1.17" < 9"	D7 / D7	9"
10" < 18"	D7 / D7	9" < 13"	D4.5 / D4.5	9"
18" < 25"	E5 / E5	13" < 19"	E5 / E5	9"
25' - 35'	F5 / F5	19" < 27"	F5 / F5	9"
		27" - 35"	G5 / G5	9"
SIZE: 20'-0"				
	Inside/Outside		Inside/Outside	
≥1.17" < 10"	C6.5 / C6.5	≥1.17" < 8"	D7 / D7	10"
10" < 17"	D7 / D7	8" < 12"	E5 / E5	10"
17" - 30"	E5 / E5	12" < 20"	F5 / F5	10"
		20" - 30"	G5 / G5	10"
SIZE: 20'-0" (Precast Only)				
	Inside/Outside		Inside/Outside	
≥1.17" < 8"	C6.5 / C6.5	≥1.17" < 8"	D4.5 / D4.5	9"
8" < 13"	D7 / D7	8" < 12"	E5 / E5	9"
13' - 25'	E5 / E5	12" < 19"	F5 / F5	9"
		19' - 25'	G5 / G5	9"

TABLE 4 NOTES:

1. Wall depth is measured to the top of the bottom slab for boxes and to the top of the intermediate slab for risers.
2. Wall height is the distance between top of lower slab to bottom of upper slab. Maximum wall height is 12' for wall lengths exceeding 5', or 10' for wall lengths exceeding 12'.
3. Wall lengths exceeding 6'-0" require two layers of reinforcing (See Table 4) with 2" of cover from the horizontal bars to the inside and outside faces for each layer.
4. Wall lengths exceeding the dimensions or depths shown in Table 4, or 12" diameter require a special design.
5. Wall thickness and reinforcing for rectangular structures is based on the longer wall length.

TABLES 1, 2, 3, AND 4

LAST REVISION 11/01/20	REVISION	DESCRIPTION:	FDOT FY 2023-24 STANDARD PLANS	STRUCTURE BOTTOMS TYPE J AND P	INDEX 425-010	SHEET 3 of 4

10/19/2022 1:37:31 AM

TABLE 5 - SLAB DESIGNS - SQUARE AND RECTANGULAR STRUCTURES
(ALL SLABS 8" THICK EXCEPT AS NOTED - REINFORCING PARALLEL TO SHORT WAY AND LONG WAY)

SHORT-WAY SLAB DEPTH	SHORT-WAY SCHEDULE (Bars A)	LONG-WAY SLAB DEPTH	LONG-WAY SCHEDULE (Bars B)
SIZE: 3'-6" x UNLIMITED			
≥0.5" < 8'	B10	≥0.5" < 24'	B10
8' < 13'	B5.5	24'-40'	B5.5
13' < 31'	C6.5		
31'-40'	D7		
SIZE: 4' x UNLIMITED			
≥0.5" < 7'	B5.5	≥0.5" < 15'	B10
7' < 19'	C6.5	15' < 29'	B5.5
19' < 31'	D7	29'-40'	C6.5
31'-40'	E5		
SIZE: 5' x 5'			
≥0.5" < 3'	C6.5	≥0.5" < 3'	C6.5
3' < 7'	B5.5	3' < 13'	C6.5
7' < 22'	C6.5	13' < 22'	D7
22' < 29'	D7	22' < 29'	D4.5
29'-40'	E5	29'-40'	E5
SIZE: 5' x 6'			
≥0.5" < 12'	C6.5	≥0.5" < 3'	C6.5
12' < 26'	D7	3' < 9'	B5.5
26'-40'	E5	9' < 23'	C3.5
		23' < 35'	D4.5
		35'-40'	E5
SIZE: 5' x 7'			
≥0.5" < 10'	C6.5	≥0.5" < 10'	B5.5
10' < 20'	D7	10' < 31'	C3.5
20' < 34'	E5	31'-40'	D4.5
34'-40'	F5		
SIZE: 5' x 8'			
≥0.5" < 7'	C6.5	≥0.5" < 8'	B10
7' < 13'	D7	8' < 17'	B5.5
13' < 24'	E5	17' < 29'	C6.5
24'-40'	F5	29'-40'	C3.5
SIZE: 5' x 9'			
≥0.5" < 8'	C6.5	≥0.5" < 14'	B10
8' < 14'	D7	14' < 24'	B5.5
14' < 25'	E5	24' < 34'	C6.5
25'-40'	F5	34'-40'	C3.5
SIZE: 5' x UNLIMITED			
≥0.5" < 8'	C6.5	≥0.5" < 14'	B10
8' < 14'	D7	14' < 24'	B5.5
14' < 25'	E5	24' < 34'	C6.5
25'-40'	F5	34'-40'	C3.5

SHORT-WAY SLAB DEPTH	SHORT-WAY SCHEDULE (Bars A)	LONG-WAY SLAB DEPTH	LONG-WAY SCHEDULE (Bars B)
SIZE: 6' x 6'			
≥0.5" < 13'	C6.5	≥0.5" < 10'	D4.5
13' < 23'	D7	10' < 18'	D4.5
23'-40'	E5	18' < 27'	E5
		27' < 33'	E3
		33'-40'	F5
SIZE: 6' x 7'			
≥0.5" < 8'	C6.5	≥0.5" < 8'	C6.5
8' < 16'	D7	8' < 12'	C3.5
16' < 28'	E5	12' < 21'	D4.5
28'-40'	F5	21' < 28'	E5
		28' < 35'	E3
		35'-40'	F5
SIZE: 6' x 8'			
≥0.5" < 6'	C6.5	≥0.5" < 6'	B5.5
6' < 13'	D7	6' < 11'	C6.5
13' < 22'	E5	11' < 17'	C3.5
22' < 35'	F5	17' < 27'	D4.5
35'-40'	G5	22' < 32'	E5
		32'-40'	E3
SIZE: 6' x 9'			
≥0.5" < 8'	D7	≥0.5" < 8'	B5.5
8' < 14'	E5	8' < 14'	C6.5
14' < 24'	F5	14' < 21'	C3.5
24'-34'	G5	21' < 25'	D4.5
		25'-34'	E5
SIZE: 6' x UNLIMITED			
≥0.5" < 8'	D7	≥0.5" < 8'	B5.5
8' < 14'	E5	8' < 14'	C6.5
14' < 24'	F5	14' < 21'	C3.5
24'-34'	G5	21' < 25'	D4.5
		25'-34'	E5
SIZE: 7' x 7'			
≥0.5" < 8'	C6.5	≥0.5" < 4'	C6.5
8' < 15'	D7	4' < 7'	C3.5
15' < 26'	E5	7' < 11'	D4.5
26'-40'	F5	11' < 22'	E3
		22' < 32'	F3.5
		32'-40'	G3.5
SIZE: 7' x 8'			
≥0.5" < 5'	C6.5	≥0.5" < 5'	C6.5
5' < 11'	D7	5' < 8'	C3.5
11' < 19'	E5	8' < 13'	D4.5
19' < 30'	F5	13' < 22'	E3
30'-40'	G5	22' < 30'	F3.5
		30'-40'	G3.5
SIZE: 7' x 9'			
≥0.5" < 9'	D7	≥0.5" < 7'	C6.5
9' < 15'	E5	7' < 10'	C3.5
15' < 25'	F5	10' < 14'	D4.5
25' - 34'	G5	14' < 21'	E5
		21' < 29'	F5
		29'-34'	F3.5

SHORT-WAY SLAB DEPTH	SHORT-WAY SCHEDULE (Bars A)	LONG-WAY SLAB DEPTH	LONG-WAY SCHEDULE (Bars B)
SIZE: 8' x 8'			
≥0.5" < 10'	D7	≥0.5" < 9'	D4.5
10' < 19'	E5	9' < 13'	E5
19'-30'	F5	13' < 18'	F5
		18' < 23'	F3.5
		23'-30'	G3.5
SIZE: 8' x 9'			
≥0.5" < 8'	D7	≥0.5" < 7'	D7
8' < 14'	E5	7' < 9'	D4.5
14' < 23'	F5	9' < 19'	E3
23'-31'	G3.5	19' < 26'	F5
		26' < 35'	F3.5
		35'-40'	G3.5
SIZE: 9' x 9'			
≥0.5" < 8'	D7	≥0.5" < 7'	D4
8' < 14'	E5	7' < 10'	E5
14' < 22'	F5	10' < 17'	F3.5
		17' < 22'	G3.5
SIZE: 9'x9'x10" SLAB THICKNESS			
22' < 30'	F5	22' < 31'	F3.5
30'-40'	G5	31'-40'	G3.5
SIZE: 10'x10'x10" SLAB THICKNESS			
≥0.5" < 7'	C6.5	0.5' < 6'	C6.5
7' < 10'	D7	6' < 9'	D4.5
10' < 18'	E5	9' < 15'	E5
18' < 27'	F5	15' < 22'	F5
27'-32'	G5	22'-32'	G3.5
SIZE: 12'x12'x12" SLAB THICKNESS			
≥0.5" < 10'	D7	≥0.5" < 8'	D7
10' < 16'	E5	8' < 14'	E5
16' < 25'	F5	14' < 22'	F5
25'-35'	G5	22' < 30'	G5
		30'-35'	H4

SLAB AND WALL DESIGN TABLE NOTES

1. Size is the inside dimension(s) of a structure.
2. Slab reinforcement is appropriate for top, intermediate, and bottom slabs.
3. Bottom Slabs for precast 3'-6" x 3'-6" rectangular structures at 15' depth or less, may be 6" thick.
4. Slab depth is measured from finished grade to top of slab.
5. Reinforcing schedules with larger areas of steel may be substituted for schedules with smaller bar or wire spacing, except that Schedule B10 may not be substituted for Schedule A6. See Index 425-001 for allowable bar spacing adjustments when larger areas of reinforcing are substituted.

TABLE 6 - SLAB DESIGNS ROUND STRUCTURES

SLAB DEPTH	SLAB THICKNESS	REINF. (2-WAY) SCHEDULE
SIZE: 3'-6" DIAMETER		
2'-15"	6" Precast	C6.5
0.5" < 30"	8"	A6
30'-40"	8"	B5.5
SIZE: 4'-0" DIAMETER		
≥0.5" < 19"	8"	A6
19' < 30"	8"	B5.5
30'-40"	8"	C6.5
SIZE: 5'-0" DIAMETER		
≥0.5" < 15"	8"	B5.5
15' < 26"	8"	C6.5
26' < 35"	8"	D7
35'-40"	8"	D4.5
SIZE: 6'-0" DIAMETER		
≥0.5" < 9"	8"	B5.5
9' < 15"	8"	C6.5
15' < 22"	8"	C3.5
22' < 30"	8"	D4.5
30'-40"	8"	E5
SIZE: 7'-0" DIAMETER		
≥0.5" < 8"	8"	C3.5
8' < 16"	8"	D4.5
16' < 23"	8"	E5
23' < 27"	8"	E3
27'-40"	8"	F3.5
SIZE: 8'-0" DIAMETER		
≥0.5" < 10"	8"	D4.5
10' < 16"	8"	E5
16' < 19"	8"	E3
19' < 29"	8"	F3.5
29'-40"	10"	F5
SIZE: 10'-0" DIAMETER		
≥0.5" < 12"	10"	D4.5
12' < 20"	10"	E5
20' < 28"	10"	F5
28'-40"	10"	G3.5
SIZE: 12'-0" DIAMETER		
≥0.5" < 8"	10"	D4.5
8' < 13"	10"	E5
13' < 18"	10"	F5
18' < 26"	10"	G3.5
26'-40"	12"	G3.5

TABLES 5 AND 6

LAST REVISION 11/01/20	REVISION	DESCRIPTION:	FDOT FY 2023-24 STANDARD PLANS	STRUCTURE BOTTOMS TYPE J AND P	INDEX 425-010	SHEET 4 of 4

1 C-5.2 FDOT STRUCTURE BOTTOMS TYPE J AND P

NOT TO SCALE

Chick-fil-A
5200 Buffington Rd.
Atlanta Georgia,
30349-2998
Bowman
Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 864-5353
www.bowman.com
© 2026 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

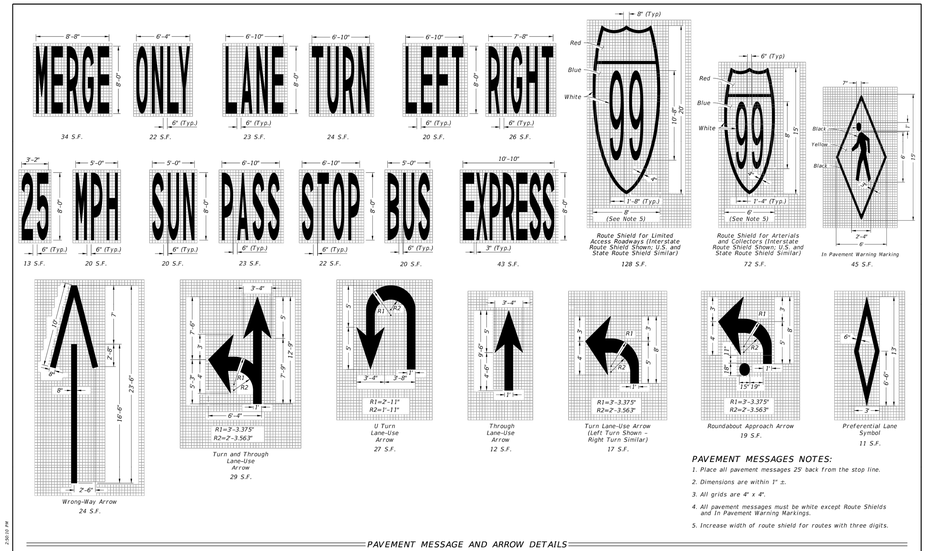
REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
SITE AND DRAINAGE DETAILS
SHEET NUMBER
C-5.2



Route Shield for Limited Access Roadways (Interstate Route Shield Shown; U.S. and State Route Shield Similar)

Route Shield for Arterials and Collectors (Interstate Route Shield Shown; U.S. and State Route Shield Similar)

In Pavement Warning Marking

Turn and Through Lane-Use Arrow

U Turn Lane-Use Arrow

Through Lane-Use Arrow

Turn Lane-Use Arrow (Left Turn Shown – Right Turn Similar)

Roundabout Approach Arrow

Preferential Lane Symbol

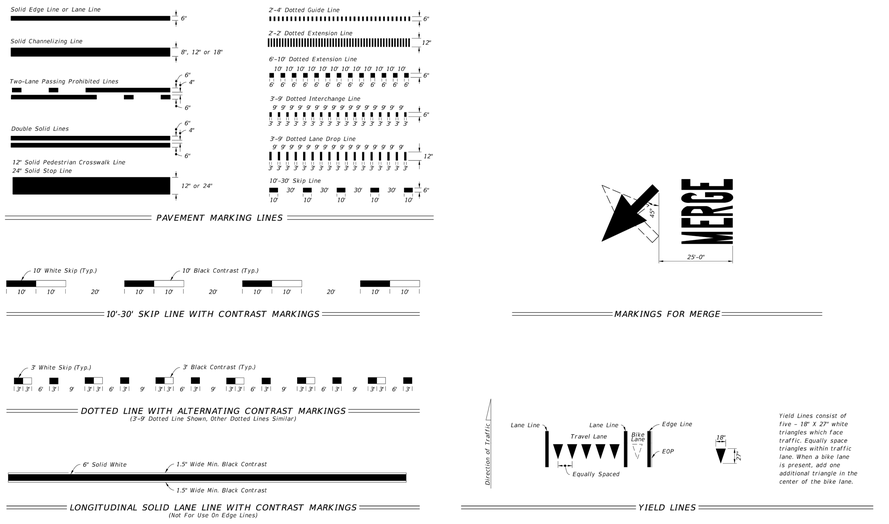
Wrong-Way Arrow

PAVEMENT MESSAGES NOTES:

1. Place all pavement messages 25' back from the stop line.
2. Dimensions are within 1" ±.
3. All grids are 4" x 4".
4. All pavement messages must be white except Route Shields and In Pavement Warning Markings.
5. Increase width of route shield for routes with three digits.

PAVEMENT MESSAGE AND ARROW DETAILS

LAST REVISION 11/01/21	REVISION	DESCRIPTION:	FDOT FY 2023-24 STANDARD PLANS	PAVEMENT MARKINGS	INDEX 711-001	SHEET 1 of 13



PAVEMENT MARKING LINES

10'-30' SKIP LINE WITH CONTRAST MARKINGS

MARKINGS FOR MERGE

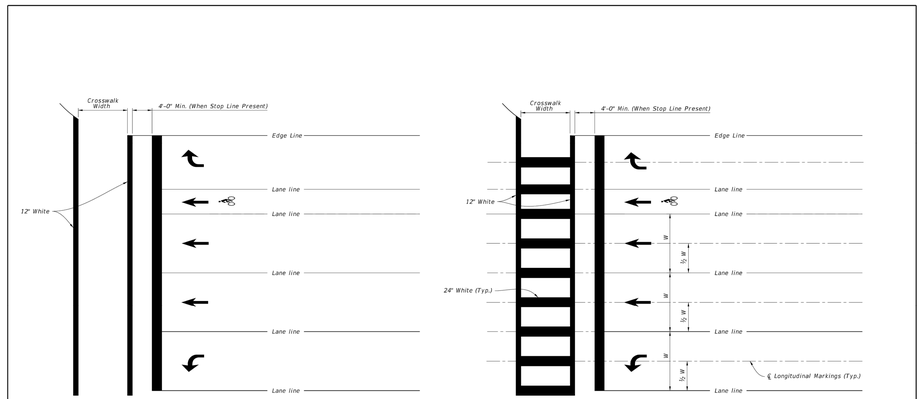
DOTTED LINE WITH ALTERNATING CONTRAST MARKINGS
(3'-9' Dotted Line Shown, Other Dotted Lines Similar)

LONGITUDINAL SOLID LANE LINE WITH CONTRAST MARKINGS
(Not For Use On Edge Lines)

YIELD LINES

Yield Lines consist of five – 18" X 27" white triangles which face traffic. Equally space triangles within traffic lane. When a bike lane is present, add one additional triangle in the center of the bike lane.

LAST REVISION 11/01/22	REVISION	DESCRIPTION:	FDOT FY 2023-24 STANDARD PLANS	PAVEMENT MARKINGS	INDEX 711-001	SHEET 2 of 13



STANDARD CROSSWALK DETAILS

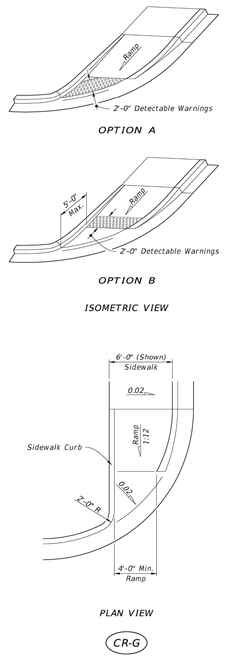
SPECIAL EMPHASIS CROSSWALK DETAILS

NOTES:

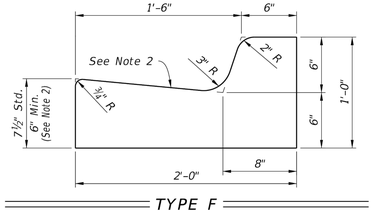
1. For crosswalk width, exceed width of the adjacent sidewalk, but do not make width less than 6' for intersection crosswalks and 10' for midblock crosswalks. Measure width from the inside of the transverse crosswalk markings.
2. When the Special Emphasis Crosswalk is not perpendicular to the lane lines, make the longitudinal markings parallel to the lane lines.
3. Refer to Index 522-002 when Curb Ramps are present.

LAST REVISION 11/01/21	REVISION	DESCRIPTION:	FDOT FY 2023-24 STANDARD PLANS	PAVEMENT MARKINGS	INDEX 711-001	SHEET 9 of 13

1 C-5.3 FDOT PAVEMENT MARKINGS
NOT TO SCALE



2 C-5.3 FDOT CURB RAMP CR-G
NOT TO SCALE



TYPE F

3 C-5.3 FDOT TYPE F CURB
NOT TO SCALE

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 864-5253
www.bowman.com
© 2026 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
SITE AND DRAINAGE DETAILS

SHEET NUMBER
C-5.3



3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL
(42" Height shown, 54" Height Similar)

TABLE 1 - RAILING MEMBERS

MEMBER	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS
Post "A"	HSS2½x1½x5/16	2.50" x 1.50"	0.125"
Post "B"	HSS2½x1½x3/8	2.50" x 1.50"	0.188"
Top Rail	2½" NPS (Sch. 10)	2.875"	0.120"
	HSS3.000x0.120	3.000"	0.120"
End Hoops	2½" NPS (Sch. 10)	2.875"	0.120"
	HSS3.000x0.120	3.000"	0.120"
Top Rail Joint/Splice Sleeves	HSS2.500x0.125	2.500"	0.125"
Intermediate & Bottom Rail	HSS2x2x3/16	2.00" x 2.00"	0.188" (1)
Int. & Bottom Rail Post Connection Sleeve	HSS1.500x0.125	1.500"	0.125" (1)
Handrail Joint/Splice Sleeves	1" NPS (Sch. 40)	1.315"	0.133"
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"
Handrail Support Bar	3/4" Ø Round Bar	0.750"	N/A
Pickets (Type 1 Infill Panel)	3/4" Ø Round Bar	0.750"	N/A
Infill Panel Members (Types 2 - 5)	Varies (See Details)	Varies	Varies

TABLE 1 NOTES:
(1) 0.125" wall thickness permitted for rails with post spacings less than 5'-8", except that Post Connection Sleeve must be 1½" NPS (Sch. 40).

NOTES

DESIGN LOADS, GEOMETRY AND APPLICABILITY:
See the Instructions for Design Standards for the design loads, geometry and applicability requirements.

GENERAL:
Adequate foundation support shall be provided for anchorage and stability against overturning. See Index No. 851 for special requirements and modifications for use on bridges. The railing shown on these drawings requires a handrail for ramps steeper than a 5% grade to conform with the requirements of the Americans with Disabilities Act (ADA).

RAILS, PANELS AND POSTS:
Pipe Rails and Pickets shall be in accordance with ASTM A500 Grade B, C or D, or ASTM A53 Grade B for standard weight pipe (Schedule 40) or ASTM A36 for bars. Structural Tube shall be in accordance with ASTM A500 Grade A, B, C or D, or ASTM A501. Perforated panels (Type 5), U-Channels and filler plates shall be ASTM A36 or A1011 (Grade 36). Posts and End Rails shall be fabricated and installed plumb, ± 1° tolerance when measured at 3'-6" above the foundation. Pickets and vertical panel elements shall be fabricated parallel to the posts, except that Type 2, 3 & 5 panel infills may be fabricated parallel to the longitudinal grade. Corners and changes in horizontal longitudinal alignment shall be made continuous with a 9" bend radius or terminate at adjoining sections with mitered end sections when handrails are not required. For changes in tangential longitudinal alignment greater than 45°, posts shall be positioned at a maximum distance of 2'-0" each side of the corner and shall not be located at the corner apex. For curved longitudinal alignments the top and bottom rails and handrails shall be shop bent to match the alignment radius.

BASE PLATES AND RAIL CAPS:
Base Plates and Rail Cap Plates shall be in accordance with ASTM A36 or ASTM A709 Grade 36.

SHIM PLATES:
Shim Plates shall be aluminum in accordance with ASTM B209, Alloy 6061 or 6063. Shim plates shall be used for foundation height adjustments greater than ½" and localized irregularities greater than ½". Field trim shim plates when necessary to match the contours of the foundation. Beveled shim plates may be used in lieu of trimmed flat shim plates shown. Stacked shim plates must be bonded together with adhesive bonding material and limited to a maximum total thickness of ½", unless longer anchor bolts are provided for the exposed thread length.

ANCHOR BOLTS:
Anchor bolts shall be in accordance with ASTM F1554 Grade 36. Headless anchor bolts for Adhesive Anchors shall be threaded full length. Cutting of reinforcing steel is permitted for drilled hole installation. Expansion Anchors are not permitted. All anchor bolts shall have single self-locking hex nuts. Tack welding of the nut to the anchor bolt may be used in lieu of self-locking nuts. All nuts shall be in accordance with ASTM A563 or ASTM A194. Flat Washers shall be in accordance with ASTM F436 and Plate Washers (for long slotted holes only), shall be in accordance with ASTM A36 or ASTM A709 Grade 36. After the nuts have been snug tightened, the anchor bolt threads shall be distorted to prevent removal of the nuts. Distorted threads and tack welds shall be coated with a galvanizing compound in accordance with the Specifications.

RESILIENT AND NEOPRENE PADS:
Resilient and Neoprene pads shall be in accordance with Specification Section 932 except that testing of the finished pads shall not be required, Neoprene pads shall be durometer hardness 60 to 80.

JOINTS:
All welded joints are to be ground smooth. Expansion joints shall be spaced at a maximum 40'-0". Field splices similar to the expansion joint detail may be approved by the Engineer to facilitate handling, but top rail must be continuous across a minimum of two posts.

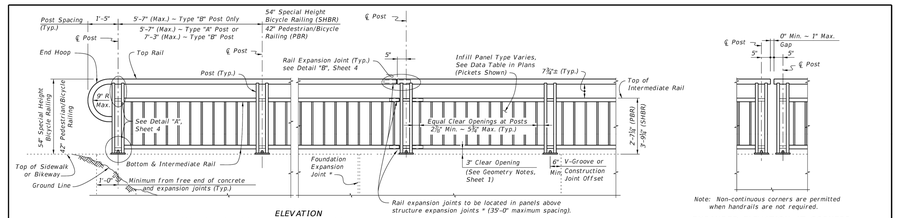
WELDING:
All welding shall be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal shall be E60XX or E70XX. Nondestructive testing of welds is not required.

COATINGS:
The steel railing shall be hot-dip galvanized after fabrication in accordance with Section 962 of the Specifications mill finish unless otherwise noted in the Contract Documents. All nuts, bolts and washers shall be hot-dip galvanized in accordance with Specification Section 962.

SHOP DRAWINGS:
Complete details addressing project specific geometry (line & grade) showing post and expansion joint locations, post and panel type, anchor bolt installation "Case" or lengths, must be submitted by the Contractor for the Engineer's approval prior to fabrication of the railing. Shop drawings shall be in accordance with the Specifications.

PAYMENT:
Railing shall be paid for per linear foot (Item No. 515-2-abb). Payment will be plan quantity measured as the length along the center line of the top rail, and includes rails, posts, pickets, panels, rail splice assembly, base plates, anchor bolts, nuts, washers, resilient or neoprene pads and all incidental materials and labor required to complete installation of the railing.

LAST REVISION	DESCRIPTION:	FDOT DESIGN STANDARDS 2013	STEEL PEDESTRIAN/BICYCLE RAILING	INDEX NO.	SHEET NO.
07/01/12				852	1

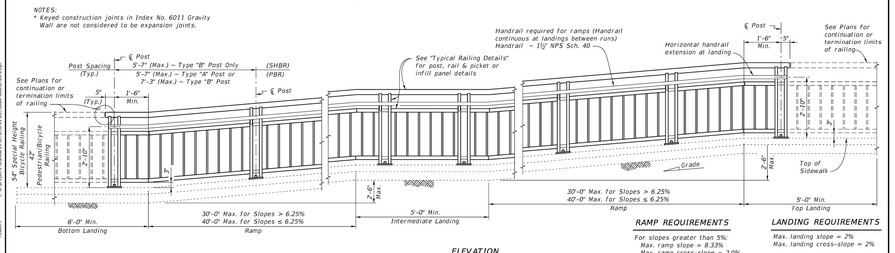


ELEVATION
(Showing Outside Face of Railing with Type "A" Posts)

EXPANDED ELEVATION AT CORNERS

TYPICAL RAILING DETAILS & RAILINGS ON GRADES 0% TO 5%
(Type 1 - Picket Railing Shown, Other Types Similar)

DETAIL FOR NON-CONTINUOUS RAILING AT CORNERS



ELEVATION
(Showing Inside Face of Railing with Type "A" Posts)

RAMP REQUIREMENTS	LANDING REQUIREMENTS
For slopes greater than 5%:	Max. landing slope = 2%
Max. ramp slope = 8.33%	Max. landing cross-slope = 2%
Max. ramp cross-slope = 2.0%	

RAILINGS ON GRADES STEEPER THAN 5%
(Type 1 - Picket Railing Shown, Other Types Similar)

LAST REVISION	DESCRIPTION:	FDOT DESIGN STANDARDS 2013	STEEL PEDESTRIAN/BICYCLE RAILING	INDEX NO.	SHEET NO.
01/01/11				852	2

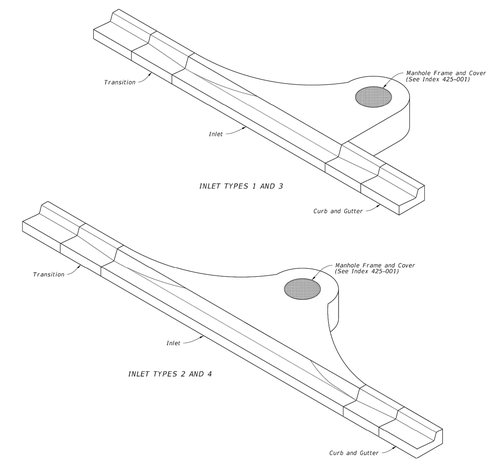
1 C-5.5 PEDESTRIAN HANDRAIL
NOT TO SCALE

GENERAL NOTES:

1. Work this Index with Index 425-001 and Index 425-010.
2. Conform finished grade and slope of the Inlet Tops to the finished cross slope and grade of the adjacent sidewalk and/or border.
3. Provide 1½" minimum cover for steel in the Inlet Top.
4. Construction of Inlet Tops are either precast or cast-in-place.
5. For precast units, the rear wall and apron may be precast as a separate piece from the top slab. Provide a minimum of 7 – #4 dowels, otherwise install in accordance with Index 425-001 "OPTIONAL CONSTRUCTION JOINTS".
6. These inlets are designed for use with standard curb and gutter Type E and Type F.
7. Use only round concrete support posts.

TABLE OF CONTENTS:

Sheet	Description
1	General Notes and Contents
2	Top Dimensional and Reinforcing Details
3	Transverse Dimensional and Reinforcing Details



CURB INLET TOPS

LAST REVISION	DESCRIPTION:	FDOT FY 2025-26 STANDARD PLANS	CURB INLET TOP TYPES 1, 2, 3, AND 4	INDEX NO.	SHEET NO.
11/01/20				425-020	1 of 3

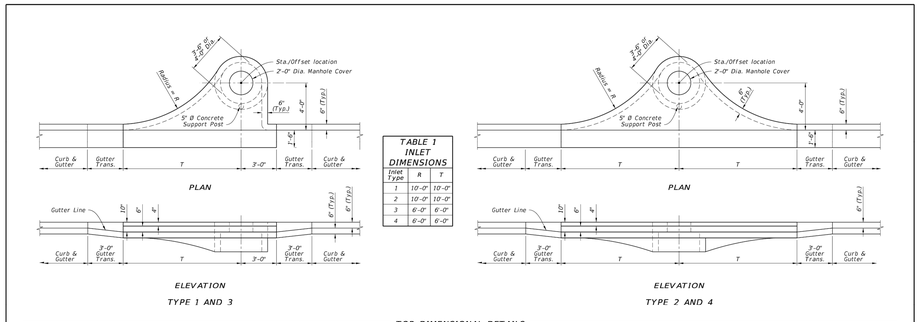
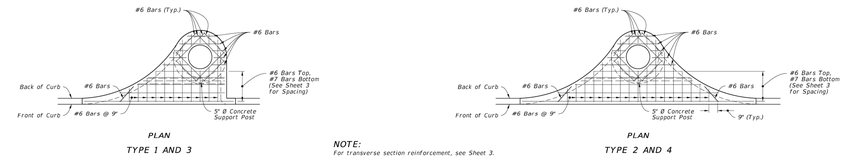


TABLE 1 INLET DIMENSIONS		
Inlet Type	R	T
1	10'-0"	10'-0"
2	10'-0"	10'-0"
3	6'-0"	6'-0"
4	6'-0"	6'-0"

TOP DIMENSIONAL DETAILS



NOTE:
For transverse section reinforcement, see Sheet 3.

TOP REINFORCING DETAILS

TOP DIMENSIONAL AND TOP REINFORCING DETAILS

LAST REVISION	DESCRIPTION:	FDOT FY 2025-26 STANDARD PLANS	CURB INLET TOP TYPES 1, 2, 3, AND 4	INDEX NO.	SHEET NO.
11/01/20				425-020	2 of 3

2 C-5.5 FDOT TYPE 2 CURB INLET
NOT TO SCALE

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING, SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

Chick-fil-A
5200 Buffington Rd.
Atlanta Georgia,
30349-2998
Bowman
Certificate of Authorization License No. 31462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU
1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
SITE AND DRAINAGE DETAILS

SHEET NUMBER
C-5.4

PROJECT INFORMATION	
ENGINEERED PRODUCT MANAGER	KARL GUTMANN 239-994-6478 KARL.GUTMANN@ADSPIPE.COM
ADS SALES REP	RICK PFAFFENDORF 813-727-2545 RICK.PFAFFENDORF@ADSPIPE.COM
PROJECT NO.	S450292
PROJECT SITE COORDINATOR:	JAKOB LAMBERT 941-290-7120 JAKOB.LAMBERT@ADSPIPE.COM

Advanced Drainage Systems, Inc.

ADS SiteAssist™ FOR STORMTECH INSTALLATION INSTRUCTIONS VISIT OUR APP

CHICK-FIL-A STORE #05587

LEHIGH ACRES, FL, USA

SC-800 STORMTECH CHAMBER SPECIFICATIONS

1. CHAMBERS SHALL BE STORMTECH SC-800.
2. CHAMBERS SHALL BE ARCH-SHAPED AND SHALL BE MANUFACTURED FROM VIRGIN, IMPACT-MODIFIED POLYPROPYLENE COPOLYMERS.
3. CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
4. CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNOBSTRUCTED INTERNAL SPACE WITH NO INTERNAL SUPPORTS THAT WOULD IMPEDE FLOW OR LIMIT ACCESS FOR INSPECTION.
5. THE STRUCTURAL DESIGN OF THE CHAMBERS, THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET FOR: 1) LONG-DURATION DEAD LOADS AND 2) SHORT-DURATION LIVE LOADS, BASED ON THE AASHTO DESIGN TRUCK WITH CONSIDERATION FOR IMPACT AND MULTIPLE VEHICLE PRESENCES.
6. CHAMBERS SHALL BE DESIGNED AND ALLOWABLE LOAD CONFIGURATIONS DETERMINED IN ACCORDANCE WITH ASTM F2787, "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS". LOAD CONFIGURATIONS SHALL INCLUDE: 1) INSTANTANEOUS (<1 MIN) AASHTO DESIGN TRUCK LIVE LOAD ON MINIMUM COVER 2) MAXIMUM PERMANENT (75-YR) COVER LOAD AND 3) ALLOWABLE COVER WITH PARKED (1-WEEK) AASHTO DESIGN TRUCK.
7. REQUIREMENTS FOR HANDLING AND INSTALLATION:
   - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
   - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 2".
   - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT SHALL BE GREATER THAN OR EQUAL TO 550 LBS/FT/%. THE ASC IS DEFINED IN SECTION 6.2.8 OF ASTM F2418. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.
8. ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. UPON REQUEST BY THE SITE DESIGN ENGINEER OR OWNER, THE CHAMBER MANUFACTURER SHALL SUBMIT A STRUCTURAL EVALUATION FOR APPROVAL BEFORE DELIVERING CHAMBERS TO THE PROJECT SITE AS FOLLOWS:
   - THE STRUCTURAL EVALUATION SHALL BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER.
   - THE STRUCTURAL EVALUATION SHALL DEMONSTRATE THAT THE SAFETY FACTORS ARE GREATER THAN OR EQUAL TO 1.95 FOR DEAD LOAD AND 1.75 FOR LIVE LOAD, THE MINIMUM REQUIRED BY ASTM F2787 AND BY SECTIONS 3 AND 12.12 OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS FOR THERMOPLASTIC PIPE.
   - THE TEST DERIVED CREEP MODULUS AS SPECIFIED IN ASTM F2418 SHALL BE USED FOR PERMANENT DEAD LOAD DESIGN EXCEPT THAT IT SHALL BE THE 75-YEAR MODULUS USED FOR DESIGN.
9. CHAMBERS AND END CAPS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY.
10. MANIFOLD SIZE TO BE DETERMINED BY SITE DESIGN ENGINEER. SEE TECH NOTE #6.32 FOR MANIFOLD SIZING GUIDANCE. DUE TO THE ADAPTATION OF THIS CHAMBER SYSTEM TO SPECIFIC SITE AND DESIGN CONSTRAINTS, IT MAY BE NECESSARY TO CUT AND COUPLE ADDITIONAL PIPE TO STANDARD MANIFOLD COMPONENTS IN THE FIELD.
11. ADS DOES NOT DESIGN OR PROVIDE MEMBRANE LINER SYSTEMS. TO MINIMIZE THE LEAKAGE POTENTIAL OF LINER SYSTEMS, THE MEMBRANE LINER SYSTEM SHOULD BE DESIGNED BY A KNOWLEDGEABLE GEOTEXTILE PROFESSIONAL AND INSTALLED BY A QUALIFIED CONTRACTOR.

IMPORTANT - NOTES FOR THE BIDDING AND INSTALLATION OF THE SC-800 SYSTEM

1. STORMTECH SC-800 CHAMBERS SHALL NOT BE INSTALLED UNTIL THE MANUFACTURER'S REPRESENTATIVE HAS COMPLETED A PRE-CONSTRUCTION MEETING WITH THE INSTALLERS.
2. STORMTECH SC-800 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/SC-800/DC-780 CONSTRUCTION GUIDE".
3. CHAMBERS ARE NOT TO BE BACKFILLED WITH A DOZER OR AN EXCAVATOR SITUATED OVER THE CHAMBERS. STORMTECH RECOMMENDS 3 BACKFILL METHODS:
   - STONESHOOTER LOCATED OFF THE CHAMBER BED.
   - BACKFILL AS ROWS ARE BUILT USING AN EXCAVATOR ON THE FOUNDATION STONE OR SUBGRADE.
   - BACKFILL FROM OUTSIDE THE EXCAVATION USING A LONG BOOM HOE OR EXCAVATOR.
4. THE FOUNDATION STONE SHALL BE LEVELED AND COMPACTED PRIOR TO PLACING CHAMBERS.
5. JOINTS BETWEEN CHAMBERS SHALL BE PROPERLY SEATED PRIOR TO PLACING STONE.
6. MAINTAIN MINIMUM - 6" (150 mm) SPACING BETWEEN THE CHAMBER ROWS.
7. EMBEDMENT STONE SURROUNDING CHAMBERS MUST BE A CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE; AASHTO M43 #3, 357, 4, 467, 5, 56, OR 57.
8. THE CONTRACTOR MUST REPORT ANY DISCREPANCIES WITH CHAMBER FOUNDATION MATERIALS BEARING CAPACITIES TO THE SITE DESIGN ENGINEER.
9. ADS RECOMMENDS THE USE OF "FLEXSTORM CATCH IT" INSERTS DURING CONSTRUCTION FOR ALL INLETS TO PROTECT THE SUBSURFACE STORMWATER MANAGEMENT SYSTEM FROM CONSTRUCTION SITE RUNOFF.

NOTES FOR CONSTRUCTION EQUIPMENT

1. STORMTECH SC-800 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/SC-800/DC-780 CONSTRUCTION GUIDE".
2. THE USE OF CONSTRUCTION EQUIPMENT OVER SC-800 CHAMBERS IS LIMITED:
   - NO EQUIPMENT IS ALLOWED ON BARE CHAMBERS.
   - NO RUBBER TIRED LOADERS, DUMP TRUCKS, OR EXCAVATORS ARE ALLOWED UNTIL PROPER FILL DEPTHS ARE REACHED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/SC-800/DC-780 CONSTRUCTION GUIDE".
   - WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT CAN BE FOUND IN THE "STORMTECH SC-310/SC-740/SC-800/DC-780 CONSTRUCTION GUIDE".
3. FULL 36" (900 mm) OF STABILIZED COVER MATERIALS OVER THE CHAMBERS IS REQUIRED FOR DUMP TRUCK TRAVEL OR DUMPING.

USE OF A DOZER TO PUSH EMBEDMENT STONE BETWEEN THE ROWS OF CHAMBERS MAY CAUSE DAMAGE TO THE CHAMBERS AND IS NOT AN ACCEPTABLE BACKFILL METHOD. ANY CHAMBERS DAMAGED BY THE "DUMP AND PUSH" METHOD ARE NOT COVERED UNDER THE STORMTECH STANDARD WARRANTY.

CONTACT STORMTECH AT 1-800-821-6710 WITH ANY QUESTIONS ON INSTALLATION REQUIREMENTS OR WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT.

©2025 ADS, INC.

PROPOSED LAYOUT

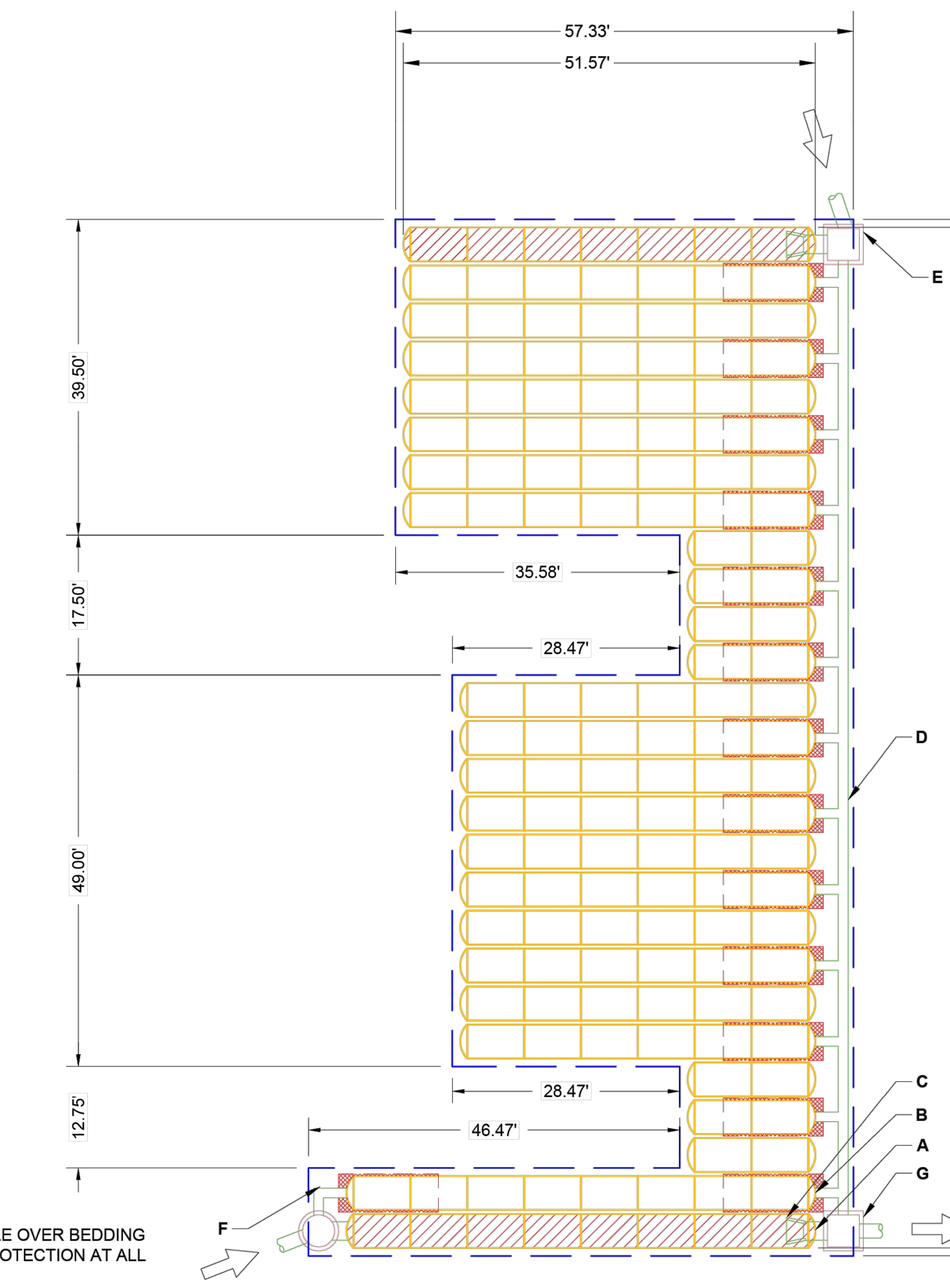
146	STORMTECH SC-800 CHAMBERS
54	STORMTECH SC-800 END CAPS
6	STONE ABOVE (in)
9	STONE BELOW (in)
40	% STONE VOID
12929	**INSTALLED SYSTEM VOLUME (CF) ABOVE INVERT 21.00 (PERIMETER STONE INCLUDED)**
6,134	SYSTEM AREA (ft²)
524	SYSTEM PERIMETER (ft)

PROPOSED ELEVATIONS

31.92	MAXIMUM ALLOWABLE GRADE (TOP OF PAVEMENT/UNPAVED)
25.67	MINIMUM ALLOWABLE GRADE (UNPAVED WITH TRAFFIC)
25.17	MINIMUM ALLOWABLE GRADE (UNPAVED NO TRAFFIC)
25.17	MINIMUM ALLOWABLE GRADE (BASE OF FLEXIBLE PAVEMENT)
25.17	MINIMUM ALLOWABLE GRADE (TOP OF RIGID PAVEMENT)
24.42	TOP OF STONE
23.92	TOP OF SC-800 CHAMBER
22.37	12" TOP MANIFOLD/CONNECTION INVERT
21.36	24" ISOLATOR ROW PLUS CONNECTION INVERT
21.17	BOTTOM OF SC-800 CHAMBER
20.42	BOTTOM OF STONE

*INVERT ABOVE BASE OF CHAMBER

PART TYPE	ITEM ON LAYOUT	DESCRIPTION	INVERT*	MAX FLOW
PRE-CORED END CAP	A	24" BOTTOM PRE-CORED END CAP, PART#: SC800EPE24BPC / TYP OF ALL 24" BOTTOM CONNECTIONS AND ISOLATOR PLUS ROWS	2.30"	
PRE-CORED END CAP	B	12" TOP PRE-CORED END CAP, PART#: SC800EPE12TPC / TYP OF ALL 12" TOP CONNECTIONS	14.40"	
FLAMP	C	INSTALL FLAMP ON 24" ACCESS PIPE / PART#: SC74024RAMP (TYP 2 PLACES)		
MANIFOLD	D	12" x 12" TOP MANIFOLD, ADS N-12	14.40"	4.6 CFS IN
CONCRETE STRUCTURE	E	(DESIGN BY ENGINEER / PROVIDED BY OTHERS)		
CONCRETE STRUCTURE	F	(DESIGN BY ENGINEER / PROVIDED BY OTHERS)		2.3 CFS IN
CONCRETE STRUCTURE	G	(DESIGN BY ENGINEER / PROVIDED BY OTHERS)		2.0 CFS OUT



ISOLATOR ROW PLUS (SEE DETAIL)

PLACE MINIMUM 12.50' OF ADSPLUS625 WOVEN GEOTEXTILE OVER BEDDING STONE AND UNDERNEATH CHAMBER FEET FOR SCOUR PROTECTION AT ALL CHAMBER INLET ROWS

BED LIMITS

CHICK-FIL-A STORE #05587
LEHIGH ACRES, FL, USA
DATE: 04/02/2025 | SVO
PROJECT #: S450292 | CHECKED: XXX

DATE	DRW	CHK	DESCRIPTION
04/09/25	AMC	---	REVISED AS PER EPM NOTES.

StormTech® Chamber System
1-800-821-6710 | WWW.STORMTECH.COM
4640 TRUEMAN BLVD
HILLIARD, OH 43026
1-800-733-7473

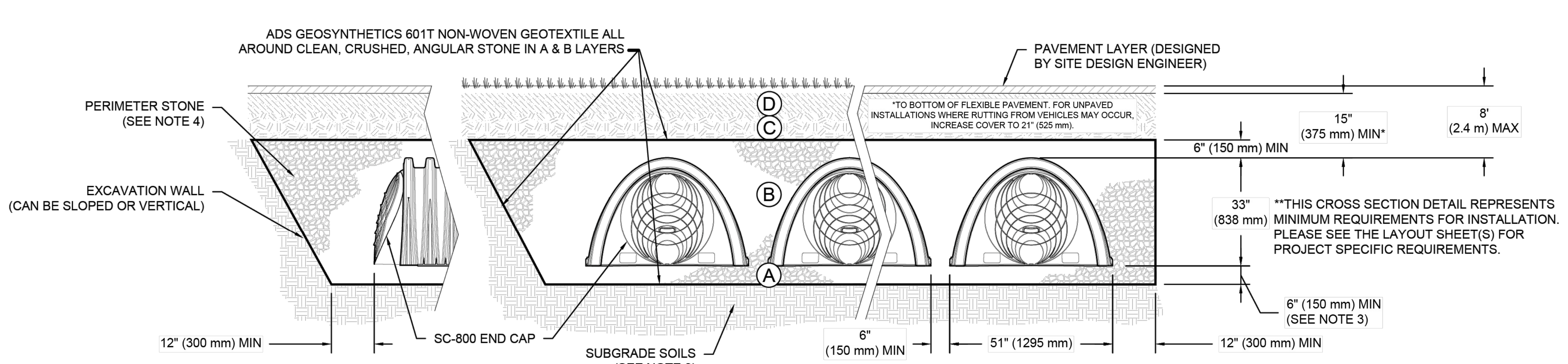
THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS/STORMTECH UNDER THE DIRECTION OF THE PROJECT'S ENGINEER OF RECORD ("EOR") OR OTHER PROJECT REPRESENTATIVE. THIS DRAWING IS NOT INTENDED FOR USE IN BIDDING OR CONSTRUCTION WITHOUT THE EOR'S PRIOR APPROVAL. EOR SHALL REVIEW THIS DRAWING PRIOR TO BIDDING AND/OR CONSTRUCTION. IT IS THE ULTIMATE RESPONSIBILITY OF THE EOR TO ENSURE THAT THE PRODUCT(S) DEPICTED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE LAWS, REGULATIONS, AND PROJECT REQUIREMENTS.

SHEET 2 OF 5

ACCEPTABLE FILL MATERIALS: STORMTECH SC-800 CHAMBER SYSTEMS

MATERIAL LOCATION		DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	**FINAL FILL:** FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER.	ANY SOIL/ROCK MATERIALS, NATIVE SOILS, OR PER ENGINEER'S PLANS. CHECK PLANS FOR PAVEMENT SUBGRADE REQUIREMENTS.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
C	**INITIAL FILL:** FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 15" (375 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	GRANULAR WELL-GRADED SOIL/AGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE. MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER.	AASHTO M145[1] A-1, A-2-4, A-3 OR AASHTO M43[1] 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 12" (300 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 6" (150 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. ROLLER GROSS VEHICLE WEIGHT NOT TO EXCEED 12,000 lbs (53 kN). DYNAMIC FORCE NOT TO EXCEED 20,000 lbs (89 kN).
B	**EMBEDMENT STONE:** FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE.	CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE[5]	AASHTO M43[1] 3, 357, 4, 467, 5, 56, 57	NO COMPACTION REQUIRED.
A	**FOUNDATION STONE:** FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE[5]	AASHTO M43[1] 3, 357, 4, 467, 5, 56, 57	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE.[2,3]

PLEASE NOTE:
1. THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
2. STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (150 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
3. WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.
4. ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.
5. WHERE RECYCLED CONCRETE AGGREGATE IS USED IN LAYERS 'A' OR 'B' THE MATERIAL SHOULD ALSO MEET THE ACCEPTABILITY CRITERIA OUTLINED IN TECHNICAL NOTE 6.20 "RECYCLED CONCRETE STRUCTURAL BACKFILL".

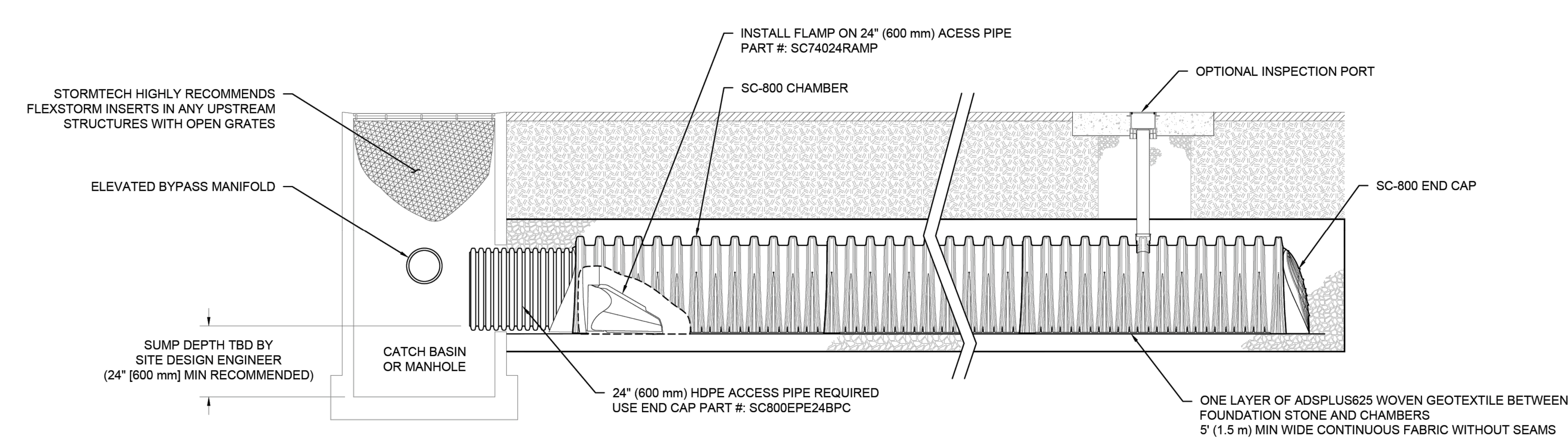


NOTES:

1. CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
2. SC-800 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
3. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS. REFERENCE STORMTECH DESIGN MANUAL FOR BEARING CAPACITY GUIDANCE.
4. PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
5. REQUIREMENTS FOR HANDLING AND INSTALLATION:
   - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
   - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 2".
   - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN SECTION 6.2.8 OF ASTM F2418 SHALL BE GREATER THAN OR EQUAL TO 550 LBS/FT/%. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.

CHICK-FIL-A STORE #05587
LEHIGH ACRES, FL, USA
DATE: 04/02/2025 | SVO
PROJECT #: S450292 | CHECKED: XXX
04/09/25 | AMC | --- | REVISED AS PER EPM NOTES.
StormTech® Chamber System
1-800-821-6710 | WWW.STORMTECH.COM
4640 TRUEMAN BLVD
HILLIARD, OH 43026
1-800-733-7473

SHEET 3 OF 5



SC-800 ISOLATOR ROW PLUS DETAIL
NTS

INSPECTION & MAINTENANCE

STEP 1) INSPECT ISOLATOR ROW PLUS FOR SEDIMENT
- A. INSPECTION PORTS (IF PRESENT)
  - A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
  - A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
  - A.3. USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
  - A.4. LOWER A CAMERA INTO ISOLATOR ROW PLUS FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
  - A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- B. ALL ISOLATOR PLUS ROWS
  - B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW PLUS
  - B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW PLUS THROUGH OUTLET PIPE
    - i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
    - ii) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
  - B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.

STEP 2) CLEAN OUT ISOLATOR ROW PLUS USING THE JETVAC PROCESS
- A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45" (1.1 m) OR MORE IS PREFERRED
- B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
- C. VACUUM STRUCTURE SUMP AS REQUIRED

STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.

STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

NOTES

1. INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
2. CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.

CHICK-FIL-A STORE #05587
LEHIGH ACRES, FL, USA
DATE: 04/02/2025 | SVO
PROJECT #: S450292 | CHECKED: XXX
04/09/25 | AMC | --- | REVISED AS PER EPM NOTES.
StormTech® Chamber System
1-800-821-6710 | WWW.STORMTECH.COM
4640 TRUEMAN BLVD
HILLIARD, OH 43026
1-800-733-7473

SHEET 4 OF 5

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com

© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

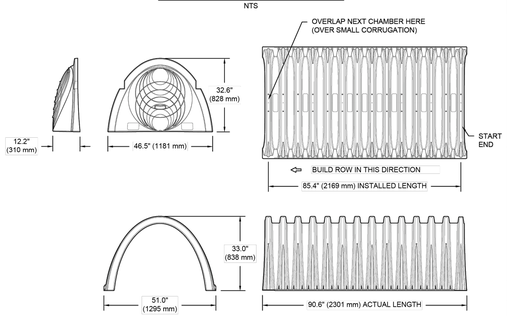
SHEET
SITE AND DRAINAGE DETAILS

SHEET NUMBER
C-5.5

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

SC-800 TECHNICAL SPECIFICATION

NTS



NOMINAL CHAMBER SPECIFICATIONS

SIZE (W X H X INSTALLED LENGTH)	51.0" X 33.0" X 85.4"	(1295 mm X 838 mm X 2169 mm)
CHAMBER STORAGE	50.6 CUBIC FEET	(1.43 m³)
MINIMUM INSTALLED STORAGE*	81.0 CUBIC FEET	(2.29 m³)
WEIGHT	81.8 lbs.	(37.1 kg)

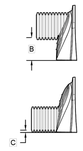
NOMINAL END CAP SPECIFICATIONS

SIZE (W X H X INSTALLED LENGTH)	46.5" X 32.6" X 10.5"	(1181 mm X 828 mm X 267 mm)
END CAP STORAGE	3.4 CUBIC FEET	(0.09 m³)
MINIMUM INSTALLED STORAGE**	15.4 CUBIC FEET	(0.43 m³)
WEIGHT	15.7 lbs.	(7.1 kg)

* ASSUMES 6" (152 mm) STONE ABOVE, BELOW, AND BETWEEN CHAMBERS
**ASSUMES 6" (152 mm) STONE ABOVE AND BELOW END CAPS, 6" (152 mm) BETWEEN ROWS, 12" (305 mm) BEYOND END CAPS

PRE-CORED HOLES AT BOTTOM OF END CAP FOR PART NUMBERS ENDING WITH "BPC"
PRE-CORED HOLES AT TOP OF END CAP FOR PART NUMBERS ENDING WITH "TPC"

PART #	STUB	B	C
SC800EPE06TPC	6" (150 mm)	21.4" (544 mm)	---
SC800EPE06BPC		---	0.9" (23 mm)
SC800EPE08TPC	8" (200 mm)	19.2" (488 mm)	---
SC800EPE08BPC		---	1.0" (25 mm)
SC800EPE10TPC	10" (250 mm)	17.0" (432 mm)	---
SC800EPE10BPC		---	1.2" (30 mm)
SC800EPE12TPC	12" (300 mm)	14.4" (366 mm)	---
SC800EPE12BPC		---	1.6" (41 mm)
SC800EPE15TPC	15" (375 mm)	11.3" (287 mm)	---
SC800EPE15BPC		---	1.7" (43 mm)
SC800EPE18TPC	18" (450 mm)	8.0" (203 mm)	---
SC800EPE18BPC		---	2.0" (51 mm)
SC800EPE24BPC	24" (600 mm)	---	2.3" (58 mm)
SC800EPE	NONE	SOLID END CAP	



NOTE: ALL DIMENSIONS ARE NOMINAL

CHICK-FIL-A STORE #05587
LEHIGH ACRES, FL, USA
DATE: 04/02/2025 | SVO | CHECKED: XXX
PROJECT # 5400292 | FOR REVIEW ONLY - NOT RELEASED FOR CONSTRUCTION

DATE	DRW.	CHK.	DESCRIPTION
04/09/25	AMC	--	REVISED AS PER EPM NOTES.

StormTech® Chamber System
1-800-821-6710 | WWW.STORMTECH.COM

PRELIMINARY DESIGN
THIS DRAWING HAS BEEN PREPARED, DIRECTED OR SUPERVISED BY THE FOLLOWING PROFESSIONAL ENGINEER FOR THE STORMTECH PROJECT DESIGN AS NOTED. IT IS NOT TO BE USED FOR CONSTRUCTION WITHOUT THE EOR'S PRIOR APPROVAL. EOR SHALL REVIEW THIS DRAWING PRIOR TO BIDDING AND/OR CONSTRUCTION. IT IS THE ULTIMATE RESPONSIBILITY OF THE EOR TO ENSURE THAT THE PRODUCTS DEPICTED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE LAWS, REGULATIONS, CODES, AND STANDARDS.

4640 TRUEMAN BLVD
HILLIARD, OH 43026
1-800-733-7473

SHEET
5 OF 5

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING, INC. SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

Chick-fil-A
5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman
Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2026 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU
1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

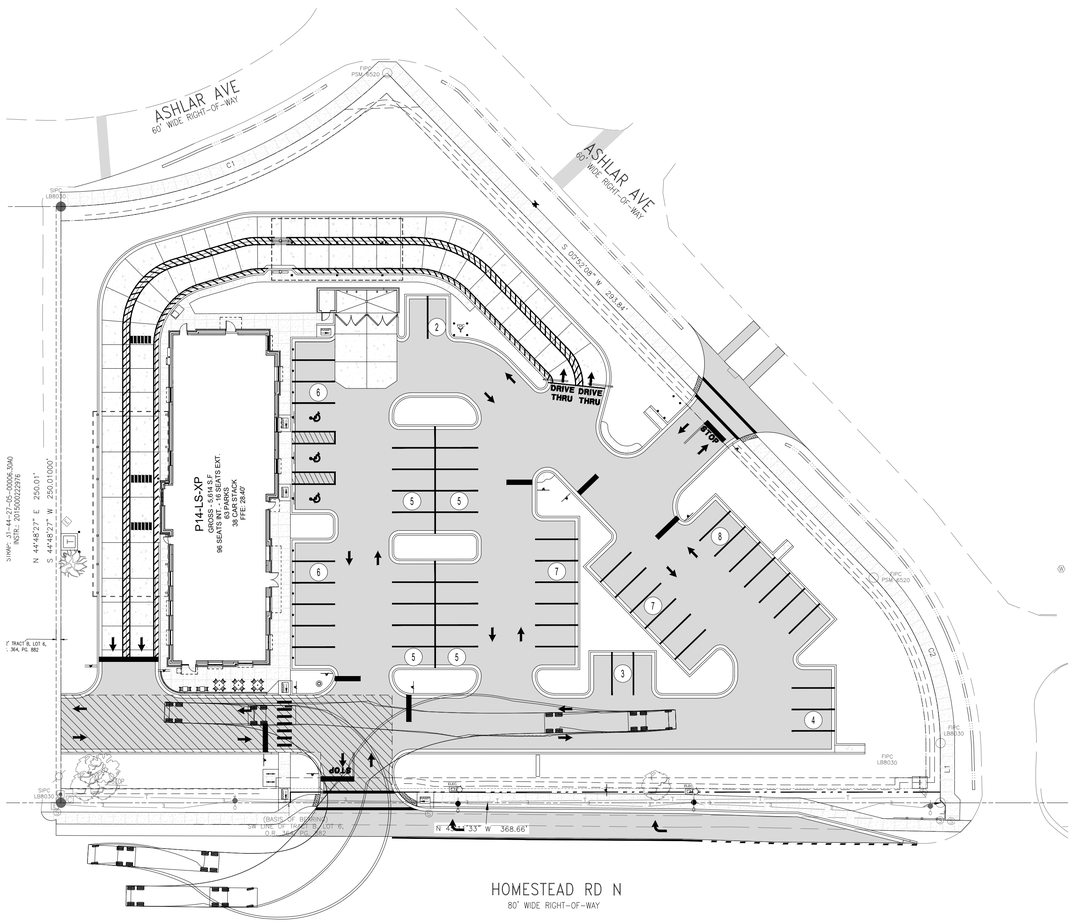
FOR PERMIT

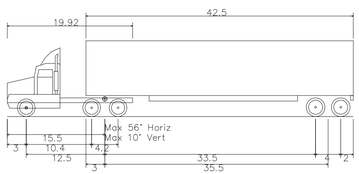
CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

SHEET
SITE AND DRAINAGE DETAILS
SHEET NUMBER
C-5.6

GENERAL DELIVERY NOTES

1. ALL DELIVERIES WILL BE DURING OFF HOURS.

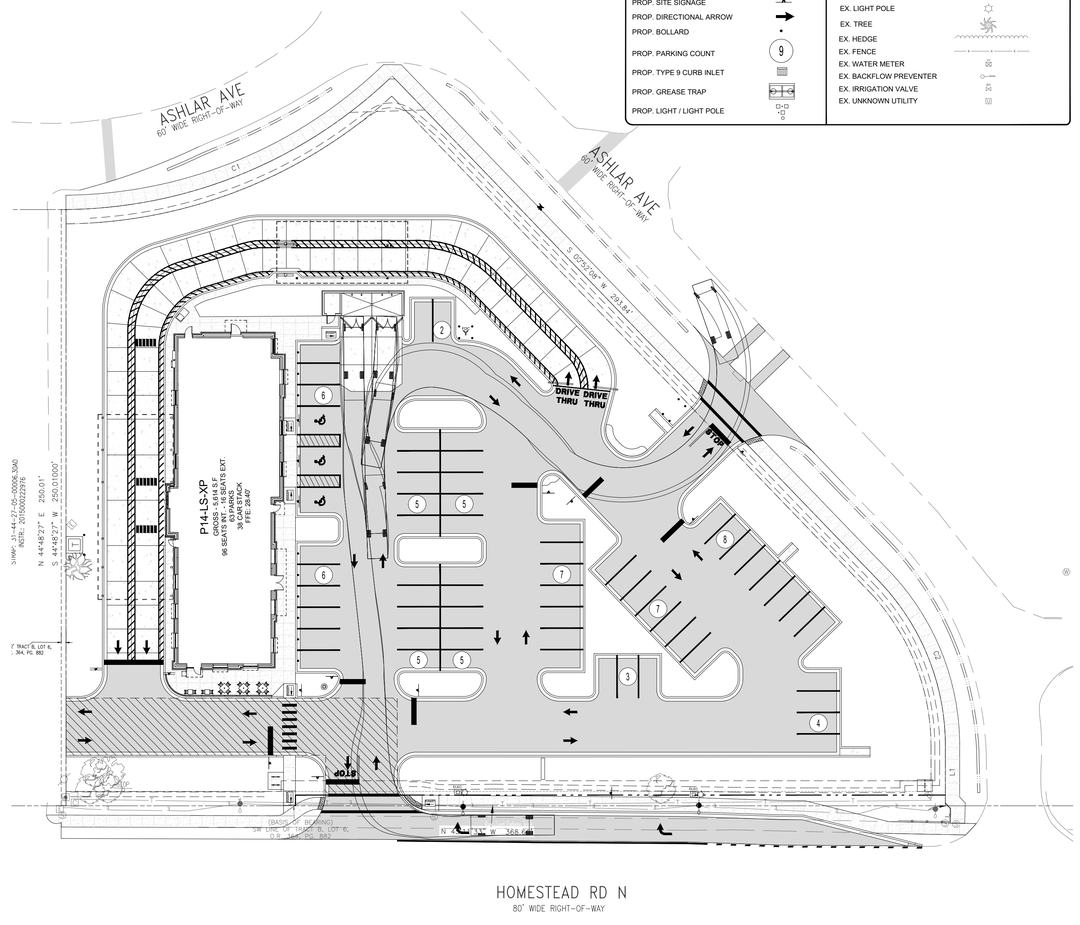


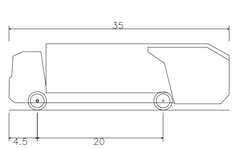


WB-50 - Intermediate Semi-Trailer

Overall Length	55.000ft
Overall Width	8.500ft
Overall Body Height	12.052ft
Min Body Ground Clearance	1.334ft
Max Track Width	8.500ft
Lock-to-lock time	6.00s
Curb to Curb Turning Radius	45.000ft

DELIVERY TRUCK





Rear-Load Garbage Truck

Overall Length	
Overall Width	
Overall Body Height	
Min Body Ground Clearance	
Track Width	
Lock-to-lock time	
Curb to Curb Turning Radius	

GARBAGE TRUCK

LEGEND

PROP. ASPHALT DRIVE	PROPERTY LINE
PROP. CONC. SIDEWALK	EX. CONCRETE SIDEWALK
PROP. CONC. PAVEMENT	EX. CONCRETE D CURB
PROP. TYPE "F" CURB	EX. CONCRETE F CURB
PROP. TYPE "D" CURB	EX. EDGE OF SIDEWALK
PROP. CLEARANCE BAR	EX. SIGN
PROP. PARKING STRIPE	EX. ELECTRIC HAND HOLE
PROP. HANDICAP MARKING	EX. SEWER CLEAN OUT
PROP. SITE SIGNAGE	EX. LIGHT POLE
PROP. DIRECTIONAL ARROW	EX. TREE
PROP. BOLLARD	EX. HEDGE
PROP. PARKING COUNT (9)	EX. FENCE
PROP. TYPE 9 CURB INLET	EX. WATER METER
PROP. GREASE TRAP	EX. BACKFLOW PREVENTER
PROP. LIGHT / LIGHT POLE	EX. IRRIGATION VALVE
	EX. UNKNOWN UTILITY

KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
811 It's fast. It's free. It's the law.
www.callsunshine.com

GRAPHIC SCALE
30 0 15 30 60
(IN FEET)
1 inch = 30 ft.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com

© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

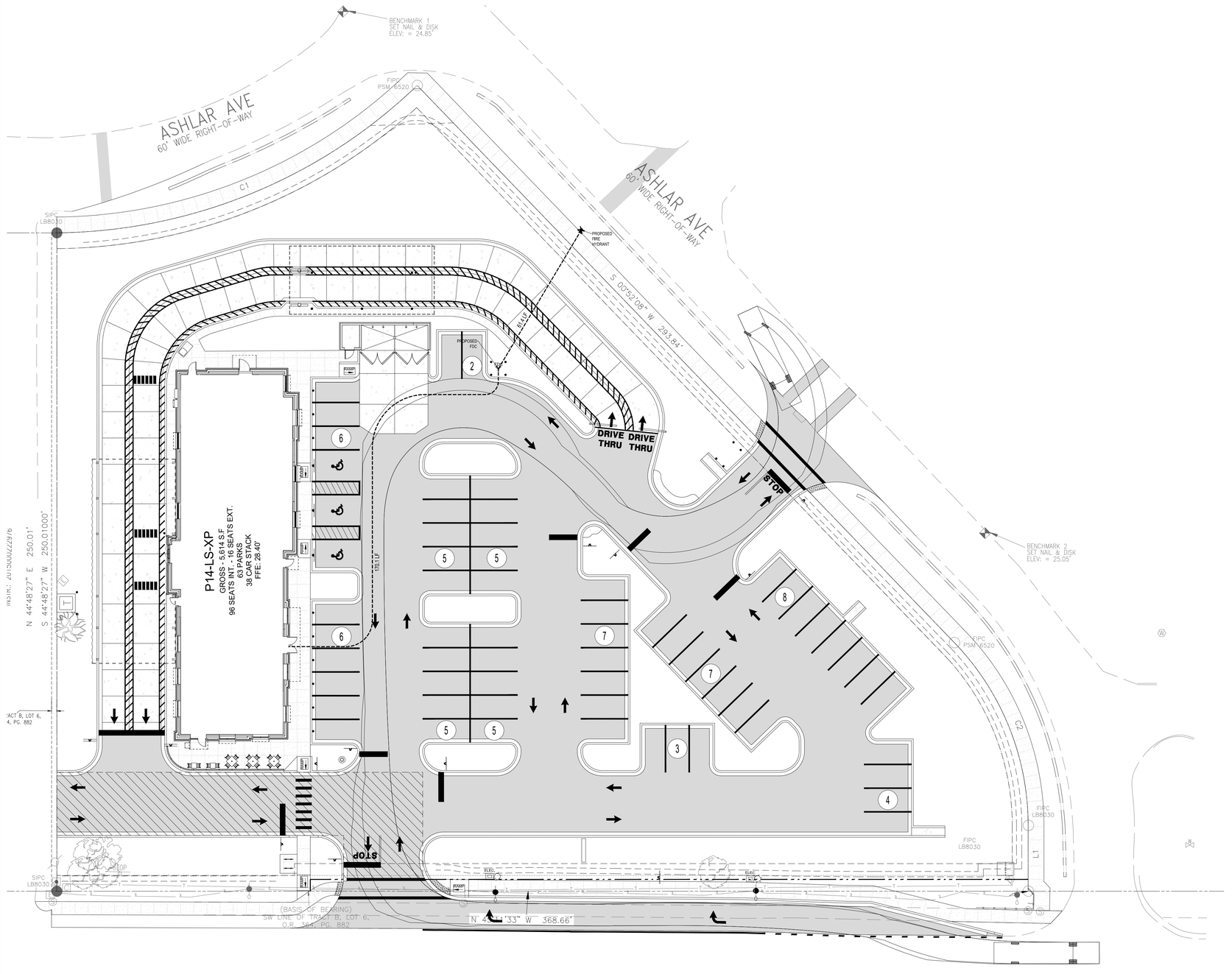
CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
TRUCK TURNING PLAN

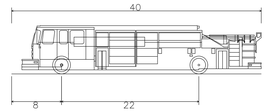
SHEET NUMBER
C-6.0

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.



LEGEND

PROP. ASPHALT DRIVE		PROPERTY LINE	
PROP. CONC. SIDEWALK		EX. CONCRETE SIDEWALK	
PROP. CONC. PAVEMENT		EX. CONCRETE D CURB	
PROP. TYPE "F" CURB		EX. CONCRETE F CURB	
PROP. TYPE "D" CURB		EX. EDGE OF SIDEWALK	
PROP. CLEARANCE BAR		EX. SIGN	
PROP. PARKING STRIPE		EX. ELECTRIC HAND HOLE	
PROP. HANDICAP MARKING		EX. SEWER CLEAN OUT	
PROP. SITE SIGNAGE		EX. LIGHT POLE	
PROP. DIRECTIONAL ARROW		EX. TREE	
PROP. BOLLARD		EX. HEDGE	
PROP. PARKING COUNT	9	EX. FENCE	
PROP. TYPE 9 CURB INLET		EX. WATER METER	
PROP. GREASE TRAP		EX. BACKFLOW PREVENTER	
PROP. LIGHT / LIGHT POLE		EX. IRRIGATION VALVE	
		EX. UNKNOWN UTILITY	



Pumper Fire Truck	
Overall Length	40.000ft
Overall Width	8.167ft
Overall Body Height	7.745ft
Min Body Ground Clearance	0.656ft
Track Width	8.167ft
Lock-to-lock time	5.00s
Max Wheel Angle	45.00°

FIRE FLOW CALCULATIONS

PROPOSED BUILDING MINIMUM FIRE FLOW REQUIREMENTS

CONSTRUCTION TYPE:	V(000) SPRINKLED
EFFECTIVE BUIILDING AREA:	5,614 SF
FIRE FLOW REQUIRED (PER 18.4.5.3.2):	1,500 GPM @20 PSI
FIRE FLOW DURATION:	2 HRS
FIRE HYDRANTS REQUIRED:	1

REFERANCE: NATIONAL FIRE PROTECTION ASSOCIATION 1 AND 101 2025 EDITION

FIRE DISTRICT : LEHIGH ACRES FIRE DISTRCIT

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast. It's free. It's the law.
www.callsunshine.com

GRAPHIC SCALE
20 0 10 20 40
(IN FEET)
1 inch = 20 ft.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

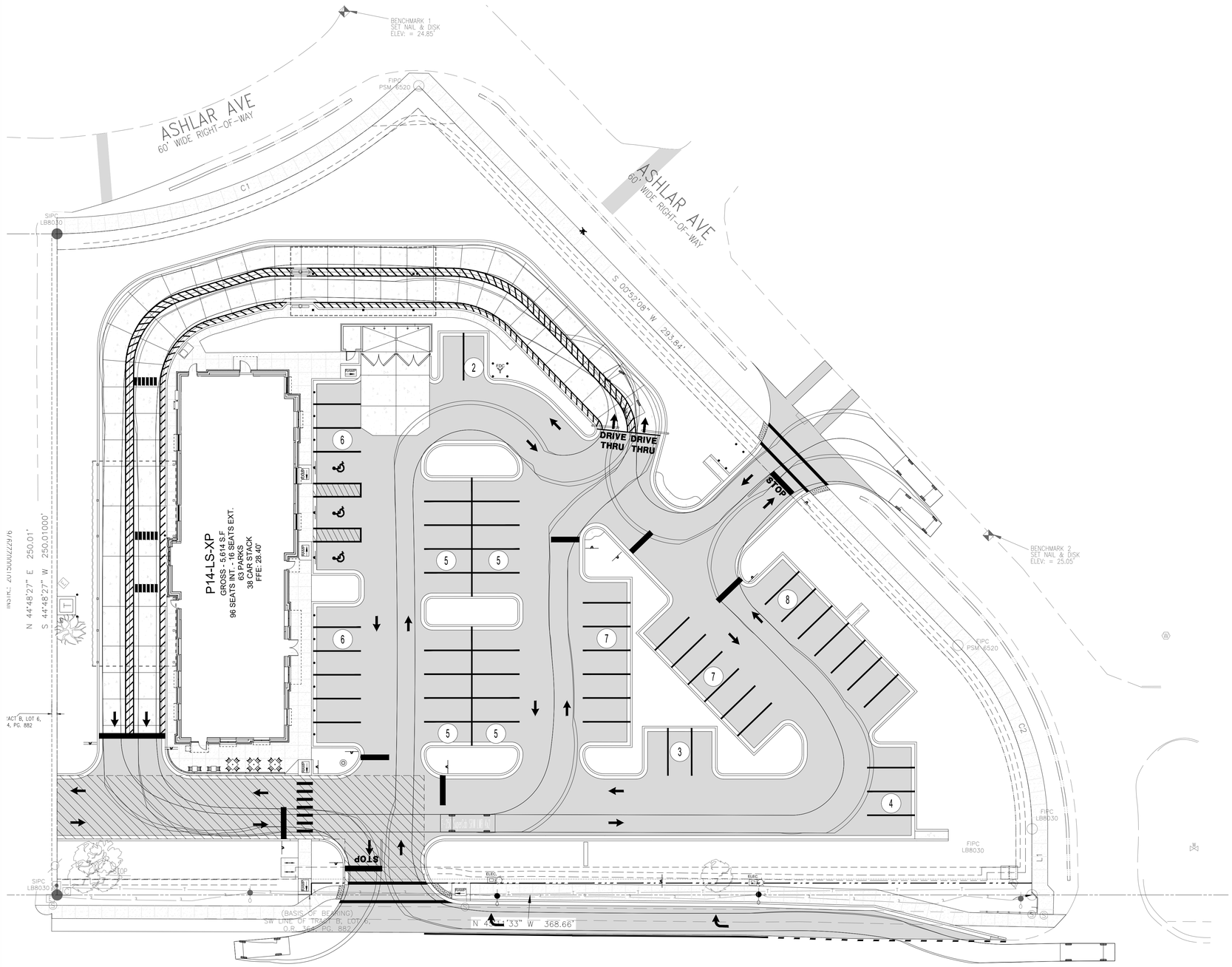
CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
FIRE TRUCK TURNING PLAN

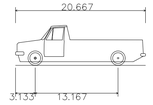
SHEET NUMBER
C-6.1

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.



LEGEND

PROP. ASPHALT DRIVE		PROPERTY LINE	
PROP. CONC. SIDEWALK		EX. CONCRETE SIDEWALK	
PROP. CONC. PAVEMENT		EX. CONCRETE D CURB	
PROP. TYPE "F" CURB		EX. CONCRETE F CURB	
PROP. TYPE "D" CURB		EX. EDGE OF SIDEWALK	
PROP. CLEARANCE BAR		EX. SIGN	
PROP. PARKING STRIPE		EX. ELECTRIC HAND HOLE	
PROP. HANDICAP MARKING		EX. SEWER CLEAN OUT	
PROP. SITE SIGNAGE		EX. LIGHT POLE	
PROP. DIRECTIONAL ARROW		EX. TREE	
PROP. BOLLARD		EX. HEDGE	
PROP. PARKING COUNT	9	EX. FENCE	
PROP. TYPE 9 CURB INLET		EX. WATER METER	
PROP. GREASE TRAP		EX. BACKFLOW PREVENTER	
PROP. LIGHT / LIGHT POLE		EX. IRRIGATION VALVE	
		EX. UNKNOWN UTILITY	



F-250 SuperCab SRW LWB 4x2
Overall Length
Overall Width
Overall Body Height
Min Body Ground Clearance
Track Width
Lock-to-lock time
Curb to Curb Turning Radius

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast. It's free. It's the law.
www.callsunshine.com

GRAPHIC SCALE
20 0 10 20 40
(IN FEET)
1 inch = 20 ft.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com

© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
SITE CIRCULATION PLAN

SHEET NUMBER
C-6.2

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

MISCELLANEOUS

1X CONNECT TO EXISTING POWER POLE

2X PRIMARY ELECTRIC SERVICE LINE TO BE CONSTRUCTED BY SERVICE PROVIDER

3X CONST. TRANSFORMER WITH METER

4X SECONDARY ELECTRIC SERVICE LINE TO BE CONSTRUCTED BY CONTRACTOR

5X GAS SERVICE LINE TO BE CONSTRUCTED BY CONTRACTOR

6X COORDINATE WITH TECO TO DETERMINE BEST METHOD TO EXTEND GAS SERVICE TO SUBJECT PROPERTY

7X TELEPHONE CONDUITS TO BE CONSTRUCTED BY CONTRACTOR

8X 3" SEWER VENT LINE

9X OPEN CUT EXISTING ASPHALT PAVEMENT AND REPAIR WITH ASPHALT PAVEMENT SPECS FOR LOCAL COUNTY ROADWAYS

10X RELOCATE EXISTING LIGHT POLE WITH ELECTRIC HAND HOLE

UTILITY NOTES

1. ALL WATER, WASTEWATER, AND RECLAIMED WATER INFRASTRUCTURE ON PRIVATE PROPERTY WILL BE OWNED, OPERATED, AND MAINTAINED BY OWNER AFTER POINT OF DEMARCATION.

SANITARY PIPE SCHEDULE

PIPE NAME	SIZE	LENGTH	SLOPE
ESS-1	8" PVC	119.5'	0.43%
ESS-2	8" PVC	255.4'	0.48%
SS-1	6" SCH 40 PVC	2.4'	1.03%
SS-2	6" SCH 40 PVC	9.1'	1.03%
SS-3	6" SCH 40 PVC	4.2'	1.03%
SS-4	6" SCH 40 PVC	2.0'	1.03%
SS-5	6" SCH 40 PVC	1.1'	1.03%
SS-6	6" SCH 40 PVC	2.2'	1.03%
SS-7	6" SCH 40 PVC	1.1'	1.03%
SS-8	6" SCH 40 PVC	13.4'	1.03%

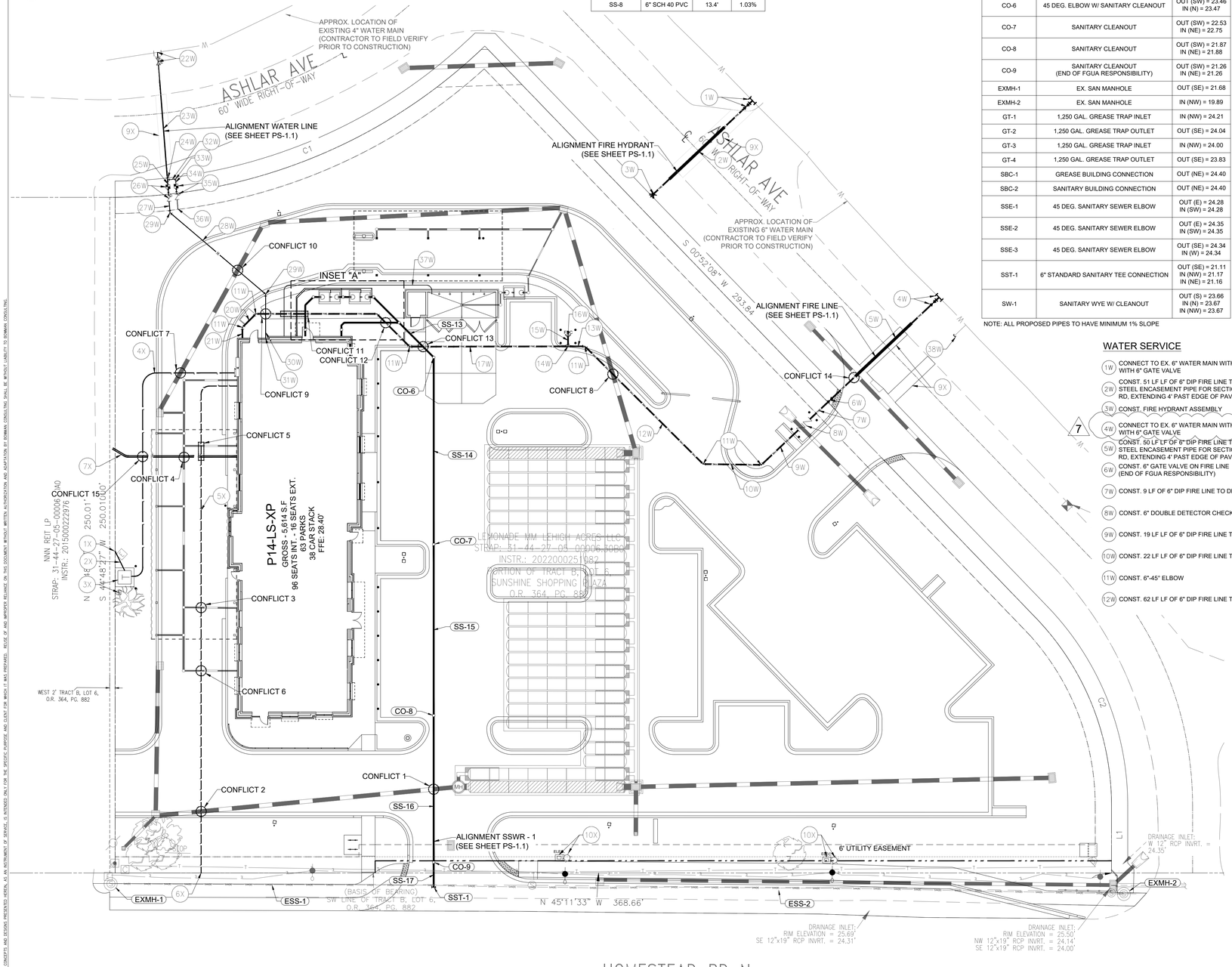
SANITARY PIPE SCHEDULE

PIPE NAME	SIZE	LENGTH	SLOPE
SS-9	6" SCH 40 PVC	2.4'	1.03%
SS-10	6" SCH 40 PVC	1.6'	1.03%
SS-11	6" SCH 40 PVC	1.4'	1.03%
SS-12	6" SCH 40 PVC	20.3'	3.30%
SS-13	6" SCH 40 PVC	18.2'	1.03%
SS-14	6" SCH 40 PVC	69.2'	1.03%
SS-15	6" SCH 40 PVC	63.3'	1.03%
SS-16	6" SCH 40 PVC	54.3'	1.12%
SS-17	6" SCH 40 PVC	9.2'	1.12%

SANITARY SEWER STRUCTURE TABLE

STRUCTURE ID	STRUCTURE TYPE	PIPE INVERTS
CO-1	TWO-WAY SANITARY CLEANOUT	OUT (NE) = 24.37 IN (SW) = 24.38
CO-2	45 DEG. ELBOW W/ SANITARY CLEANOUT	OUT (SE) = 24.23 IN (W) = 24.24
CO-3	SANITARY CLEANOUT	OUT (SE) = 24.02 IN (NW) = 24.03
CO-4	45 DEG. ELBOW W/ SANITARY CLEANOUT	OUT (S) = 23.81 IN (NW) = 23.82
CO-5	TWO-WAY SANITARY CLEANOUT	OUT (NE) = 24.37 IN (SW) = 24.38
CO-6	45 DEG. ELBOW W/ SANITARY CLEANOUT	OUT (SW) = 23.46 IN (N) = 23.47
CO-7	SANITARY CLEANOUT	OUT (SW) = 22.53 IN (NE) = 22.75
CO-8	SANITARY CLEANOUT	OUT (SW) = 21.87 IN (NE) = 21.88
CO-9	SANITARY CLEANOUT (END OF FGUA RESPONSIBILITY)	OUT (SW) = 21.26 IN (NE) = 21.26
EXMH-1	EX. SAN MANHOLE	OUT (SE) = 21.68
EXMH-2	EX. SAN MANHOLE	IN (NW) = 19.89
GT-1	1,250 GAL. GREASE TRAP INLET	IN (NW) = 24.21
GT-2	1,250 GAL. GREASE TRAP OUTLET	OUT (SE) = 24.04
GT-3	1,250 GAL. GREASE TRAP INLET	IN (NW) = 24.00
GT-4	1,250 GAL. GREASE TRAP OUTLET	OUT (SE) = 23.83
SBC-1	GREASE BUILDING CONNECTION	OUT (NE) = 24.40
SBC-2	SANITARY BUILDING CONNECTION	OUT (NE) = 24.40
SSE-1	45 DEG. SANITARY SEWER ELBOW	OUT (E) = 24.28 IN (SW) = 24.28
SSE-2	45 DEG. SANITARY SEWER ELBOW	OUT (E) = 24.35 IN (SW) = 24.35
SSE-3	45 DEG. SANITARY SEWER ELBOW	OUT (SE) = 24.34 IN (W) = 24.34
SST-1	6" STANDARD SANITARY TEE CONNECTION	OUT (SE) = 21.11 IN (NW) = 21.17 IN (NE) = 21.16
SW-1	SANITARY WYE W/ CLEANOUT	OUT (S) = 23.66 IN (N) = 23.67 IN (NW) = 23.67

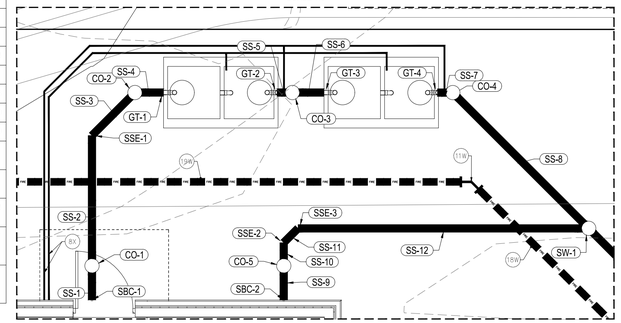
NOTE: ALL PROPOSED PIPES TO HAVE MINIMUM 1% SLOPE



LEGEND

PROP. ASPHALT DRIVE	PROPERTY LINE
PROP. CONC. SIDEWALK	EX. CONCRETE SIDEWALK
PROP. CONC. PAVEMENT	EX. CONCRETE D CURB
PROP. TYPE "F" CURB	EX. CONCRETE F CURB
PROP. TYPE "D" CURB	EX. EDGE OF SIDEWALK
PROP. TYPE 9 CURB INLET	EX. SIGN
PROP. STORM SEWER	EX. ELECTRIC HAND HOLE
PROP. TYPE C INLET	EX. SEWER CLEAN OUT
PROP. GREASE TRAP	EX. LIGHT POLE
PROP. CLEANOUT	EX. TREE
PROP. SANITARY SEWER	EX. HEDGE
PROP. WATER	EX. FENCE
PROP. ELECTRIC	EX. WATER METER
PROP. GAS LINE	EX. BACKFLOW PREVENTER
	EX. IRRIGATION VALVE
	EX. UNKNOWN UTILITY

INSET "A"
SCALE 1:5



WATER SERVICE

1W CONNECT TO EX. 6" WATER MAIN WITH 6" CUT-IN TEE WITH 6" GATE VALVE

2W CONST. 51 LF LF OF 6" DIP FIRE LINE TO HYDRANT (ADD 12" STEEL ENCASEMENT PIPE FOR SECTION UNDER ASHLAR RD, EXTENDING 4' PAST EDGE OF PAVEMENT)

3W CONST. FIRE HYDRANT ASSEMBLY

4W CONNECT TO EX. 6" WATER MAIN WITH 6" CUT-IN TEE WITH 6" GATE VALVE

5W CONST. 50 LF LF OF 6" DIP FIRE LINE TO DDCV (ADD 12" STEEL ENCASEMENT PIPE FOR SECTION UNDER ASHLAR RD, EXTENDING 4' PAST EDGE OF PAVEMENT)

6W CONST. 6" GATE VALVE ON FIRE LINE (END OF FGUA RESPONSIBILITY)

7W CONST. 9 LF OF 6" DIP FIRE LINE TO DDCV

8W CONST. 6" DOUBLE DETECTOR CHECK VALVE (DDCV)

9W CONST. 19 LF LF OF 6" DIP FIRE LINE TO BUILDING

10W CONST. 22 LF LF OF 6" DIP FIRE LINE TO BUILDING

11W CONST. 6"-45° ELBOW

12W CONST. 62 LF LF OF 6" DIP FIRE LINE TO BUILDING

13W CONST. 6 LF LF OF 6" DIP FIRE LINE TO BUILDING

14W CONST. 6"X6" TEE ON PROPOSED FIRE LINE

15W CONST. 4 LF LF OF 6" DIP FIRE LINE TO THE FDC

16W CONST. FIRE DEPARTMENT CONNECTION (FDC)

17W CONST. 59 LF LF OF 6" DIP FIRE LINE TO BUILDING

18W CONST. 17 LF LF OF 6" DIP FIRE LINE TO BUILDING

19W CONST. 45 LF LF OF 6" DIP FIRE LINE TO BUILDING

20W CONST. 7 LF LF OF 6" DIP FIRE LINE TO BUILDING

21W CONST. 4 LF LF OF 6" DIP FIRE LINE TO BUILDING

22W CONNECT TO EX. 4" WATER MAIN WITH 4"X2" TAPPING SLEEVE AND 2" CORP. STOP

23W CONST. 43 LF LF OF 2" POLY TUBE SERVICE LINE (ADD 6" STEEL ENCASEMENT PIPE FOR SECTION UNDER ASHLAR RD, EXTENDING 4' PAST EDGE OF PAVEMENT)

24W CONST. 2"X1" TEE

25W CONST. 2 LF OF 2" PVC POTABLE WATER SERVICE

26W CONST. 1 ½" WATER METER & 1 ½" BACKFLOW PREVENTER PER DETAILS 6.17 & 6.19

27W CONST. 4 LF OF 2" PVC POTABLE WATER SERVICE

28W CONST. 45 LF OF 2" PVC POTABLE WATER SERVICE

29W CONST. 2"-45° ELBOW

30W CONST. 16 LF OF 2" PVC POTABLE WATER SERVICE

31W 2" PVC WATER SERVICE BUILDING CONNECTION POINT

32W CONST. 3 LF OF 1" PVC IRRIGATION SERVICE

33W CONST. 1"-90° ELBOW

34W CONST. 2 LF OF 1" PVC IRRIGATION SERVICE

35W CONST. 1" IRRIGATION METER & 1" PRESSURE VACUUM BREAKER PER DETAILS 6.17 & 6.19

36W SEE LANDSCAPE PLANS FOR CONTINUATION

37W CONST. 75 LF OF ¾" PVC WATER SERVICE TO DUMPSTER

38W CONST. PLUG ON EXISTING WATER POINT OF CONNECTION

UTILITY CONFLICTS

CONFLICT 1
SEWER TO PASS OVER STORM WITH A MINIMUM CLEARANCE OF 12"
B.O.P. SEWER = 21.56'
T.O.P. STORM = 20.56'

CONFLICT 2
GAS SERVICE TO PASS OVER STORM WITH A MINIMUM CLEARANCE OF 12"
T.O.P. STORM = 20.56'

CONFLICT 3
GAS SERVICE TO PASS OVER STORM WITH A MINIMUM CLEARANCE OF 12"
T.O.P. STORM = 24.51'

CONFLICT 4
TELEPHONE SERVICE TO PASS OVER STORM WITH A MINIMUM CLEARANCE OF 12"
T.O.P. STORM = 24.06'

CONFLICT 5
GAS SERVICE TO PASS OVER TELEPHONE SERVICE/STORM WITH A MINIMUM CLEARANCE OF 12"
T.O.P. STORM = 23.92'

CONFLICT 6
GAS SERVICE TO PASS OVER ELECTRIC SERVICE WITH A MINIMUM CLEARANCE OF 12"
T.O.P. STORM = 24.75'

CONFLICT 7
ELECTRIC SERVICES TO PASS OVER STORM WITH A MINIMUM CLEARANCE OF 12"
T.O.P. STORM = 22.13'

CONFLICT 8
FIRE LINE TO PASS OVER STORM WITH A MINIMUM CLEARANCE OF 12"
T.O.P. STORM = 22.13'

CONFLICT 9
2" WATER SERVICE TO PASS OVER 6" FIRE LINE WITH A MINIMUM CLEARANCE OF 12"
T.O.P. FIRE = 22.50'

CONFLICT 10
2" WATER SERVICE TO PASS OVER STORM WITH A MINIMUM CLEARANCE OF 12"
T.O.P. STORM = 22.13'

CONFLICT 11
FIRE LINE TO PASS UNDER SEWER/SEWER VENT & WSTD WITH A MINIMUM CLEARANCE OF 12"
B.O.P. SEWER = 24.31'
T.O.P. FIRE = 22.50'

CONFLICT 12
FIRE LINE TO PASS UNDER SEWER WITH A MINIMUM CLEARANCE OF 12"
B.O.P. SEWER = 23.83'
T.O.P. FIRE = 22.50'

CONFLICT 13
FIRE LINE TO PASS UNDER SEWER WITH A MINIMUM CLEARANCE OF 12"
B.O.P. SEWER = 23.52'
T.O.P. FIRE = 22.50'

CONFLICT 14
FIRE LINE TO PASS UNDER STORM WITH A MINIMUM CLEARANCE OF 12"
B.O.P. STORM = 21.00'
T.O.P. FIRE = 19.87'

CONFLICT 15
TELEPHONE SERVICE LINE TO PASS OVER ELECTRIC SERVICE LINE WITH A MINIMUM CLEARANCE OF 12"

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast, It's free. It's the law.
www.callsunshine.com

GRAPHIC SCALE
20 0 10 20 40
(IN FEET)
1 inch = 20 ft.

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

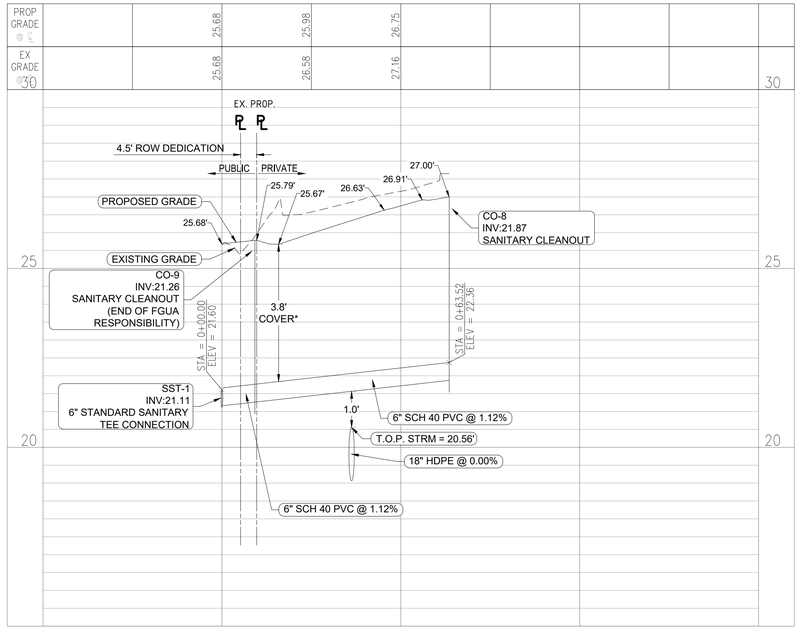
NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

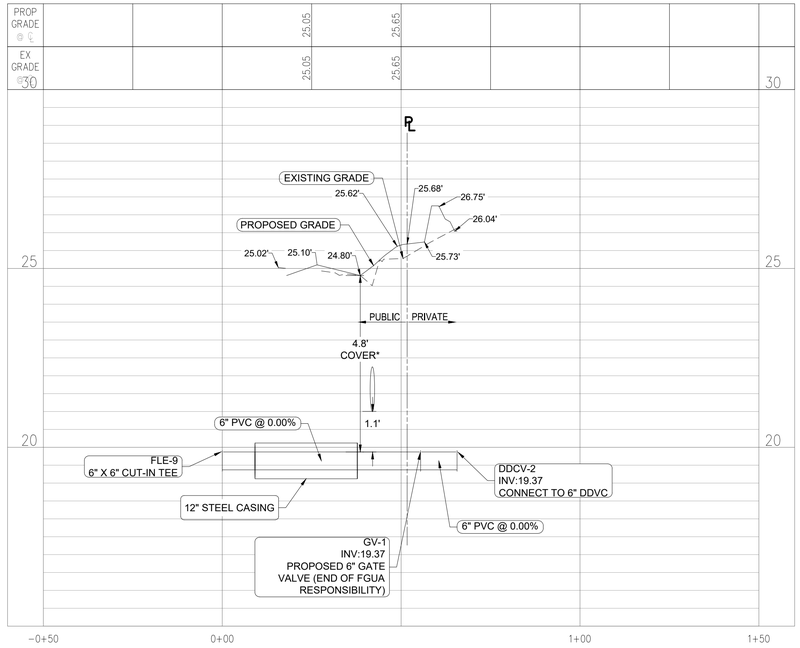
FOR PERMIT

SHEET
UTILITY PLAN

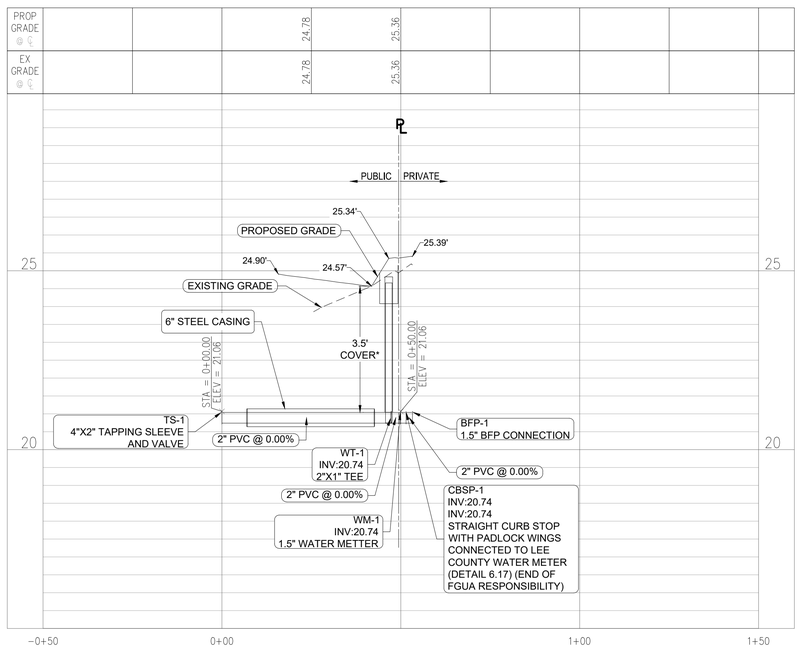
SHEET NUMBER
PS-1.0



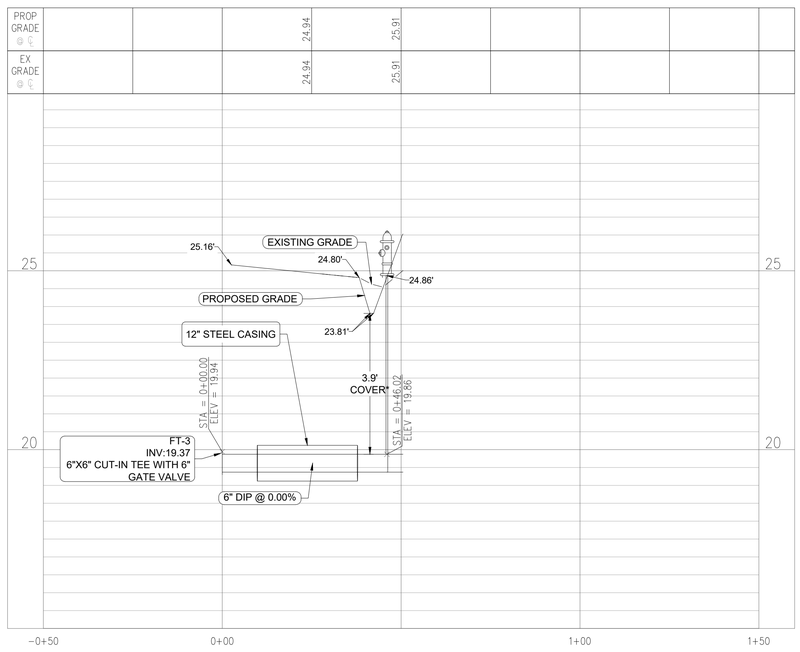
SSWR - 1 PROFILE VIEW
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=2'



FIRE LINE PROFILE VIEW
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=2'



WATER LINE PROFILE VIEW
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=2'



FIRE HYDRANT PROFILE VIEW
HORIZONTAL SCALE: 1"=20'
VERTICAL SCALE: 1"=2'

811 KNOW WHAT'S BELOW ALWAYS CALL 811 BEFORE YOU DIG
It's fast. It's free. It's the law.
www.callsunshine.com

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

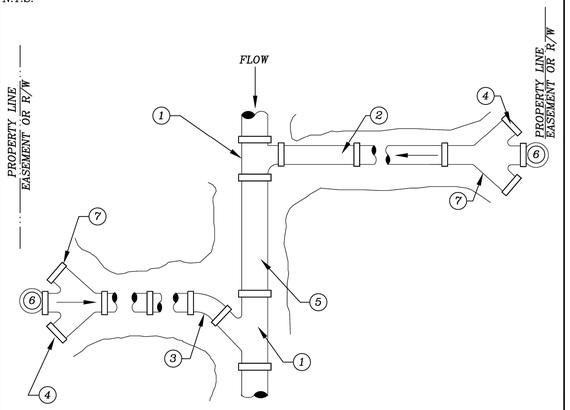
FOR PERMIT

SHEET
UTILITY PROFILES

SHEET NUMBER
PS-1.1

STANDARD DETAIL NO. 6.38
LEE COUNTY UTILITIES
STANDARD SEWER SERVICE LATERALS

N.T.S.



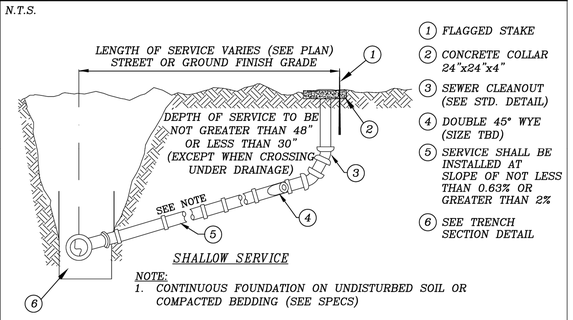
NOTE: 1. SEE SPECS FOR DETAILS OF PIPE AND FITTINGS.

1. STANDARD WYE OR SANITARY TEE
2. PVC SEWER SERVICE LINE
3. SHORT RADIUS 45°BEND WITH WYE FITTINGS
4. PLUG ENDS WITH HEAVY WALL PVC CAPS (TYP.)
5. 5' MIN. PIPE BETWEEN FITTINGS
6. 6" OR 8" CLEANOUT
7. DOUBLE 45° WYE (SIZE TBD)

REV: 08/28/2019

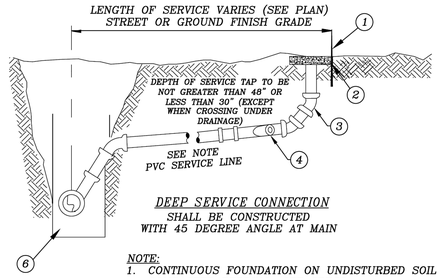
STANDARD DETAIL NO. 6.39
LEE COUNTY UTILITIES
STANDARD SEWER SERVICE LATERAL

N.T.S.



SHALLOW SERVICE

NOTE:
1. CONTINUOUS FOUNDATION ON UNDISTURBED SOIL OR COMPACTED BEDDING (SEE SPECS)



DEEP SERVICE CONNECTION
SHALL BE CONSTRUCTED WITH 45 DEGREE ANGLE AT MAIN

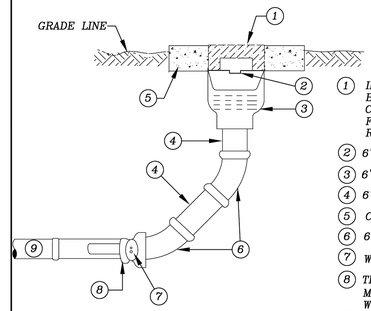
NOTE:
1. CONTINUOUS FOUNDATION ON UNDISTURBED SOIL OR COMPACTED BEDDING (SEE SPECS)

1. FLAGGED STAKE
2. CONCRETE COLLAR 24"x24"x4"
3. SEWER CLEANOUT (SEE STD. DETAIL)
4. DOUBLE 45° WYE (SIZE TBD)
5. SERVICE SHALL BE INSTALLED AT SLOPE OF NOT LESS THAN 0.63% OR GREATER THAN 2%
6. SEE TRENCH SECTION DETAIL

REV: 08/28/2019

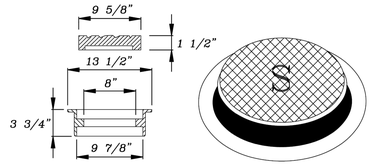
STANDARD DETAIL NO. 6.40
LEE COUNTY UTILITIES
6" OR 8" SEWER CLEAN-OUT DETAIL

N.T.S.



1. IN ALL CASES USE TRAFFIC BEARING HANDHOLE RING & COVER OR EQUAL ADJUSTED TO FINAL GRADE WITH CLEANOUT RECESSER
2. 6" OR 8" THREADED CAP
3. 6" OR 8" PVC FEMALE ADAPTER
4. 6" OR 8" PVC RISER
5. CONCRETE COLLAR 24"x24"x4"
6. 6" OR 8" 45° ELBOW
7. W/WATERTIGHT PLUG
8. THE CONNECTION MUST BE MADE TO THE DOUBLE 45° WYE (SIZE TBD)
9. 6" OR 8" PVC SEWER PIPE

REVERSIBLE HANDHOLE RING AND COVER



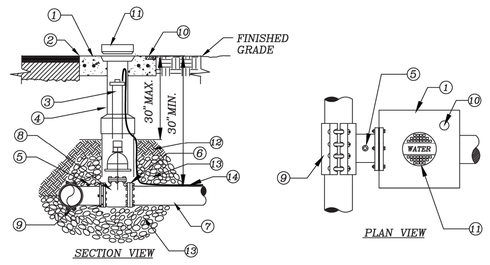
COVERS ARE MARKED WITH "S"

COVER TYPE	LOAD RATING	COVER WEIGHT	TOTAL WEIGHT
FE	HEAVY DUTY	20	45

REV: 08/28/2019

STANDARD DETAIL NO. 6.1
LEE COUNTY UTILITIES
TAPPING SLEEVE AND VALVE

N.T.S.

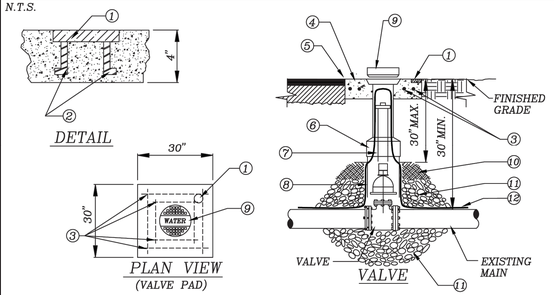


1. CONCRETE PAD (SEE VALVE INSTALLATION DETAIL)
2. SET TOP OF BOX FLUSH WITH FINISHED GRADE
3. EXTENSION STEM IF TOP OF OPERATING NUT IS MORE THAN 30"BELOW FINISH GRADE
4. VALVE BOX (SEE VALVE INSTALLATION DETAIL)
5. OUTLET FOR TESTING
6. MJ RESTRAINED
7. MAIN SIZE VARIES
8. TAPPING VALVE (SEE VALVE INSTALLATION DETAIL)
9. LCU APPROVED ALL STAINLESS STEEL PRESSURE RATED TAPPING SLEEVE. FOR SIZE ON SIZE TAPS A FLANGED TAPPING SLEEVE IS REQUIRED. SPRING LOADED TAPPING SLEEVE IS REQUIRED WHEN CONNECTING TO HDPE (SEE APPROVED MATERIAL LIST).
10. BRASS PLATE (SEE VALVE INSTALLATION DETAIL)
11. CAST IRON DROP COVER MARKED "WATER","SEWER", OR "REUSE"
12. COMPACTED SUITABLE EARTH BACKFILL
13. 3/4" GRANULAR MATERIAL #57 STONE
14. 12 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.2
LEE COUNTY UTILITIES
VALVE INSTALLATION

N.T.S.

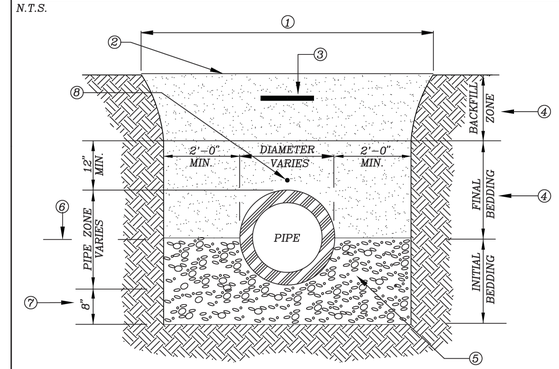


1. BRASS PLATE: SIZE OF VALVE, VALVE TYPE No. OF TURNS & DIRECTION TO OPEN VALVE M.F.G. & YEAR INSTALL SYSTEM "WATER" OR "SEWER" OR "REUSE" OR "FIRE"
2. ANCHOR
3. EIGHT (8) #4 BARS OVERLAP EACH CORNER BY 2"
4. 30"SQ X 4" THK.CONC.PAD SURROUNDING BOX, MIN. 3,000 P.S.I. POURED IN PLACE
5. SET TOP OF BOX FLUSH WITH FINISHED GRADE
6. HEAVY DUTY TRAFFIC BEARING CAST IRON VALVE BOX, ADJUSTABLE SCREW TYPE, 5 1/4" DIAMETER SHAFT THAT IS LCU APPROVED
7. EXTENSION STEM WITH 2" OPERATING NUT AS REQUIRED, IF NUT IS MORE THAN 30" BELOW FINISH GRADE
8. RISER NOT TO BEAR ON VALVE OR PIPE
9. CAST IRON DROP COVER MARKED "WATER" OR "SEWER" OR "REUSE OR "FIRE"
10. COMPACTED SUITABLE EARTH BACKFILL
11. 3/4" GRANULAR MATERIAL #57 STONE
12. 12 GAUGE DOUBLE INSULATED COPPER LOCATING WIRE (SEE LCU STANDARD DETAIL)

REV: 12/10/2015

STANDARD DETAIL NO. 6.5
LEE COUNTY UTILITIES
TRENCH CROSS SECTION

N.T.S.



1. EXCAVATED TRENCH
2. FINISHED GRADE
3. MARKING TAPE 12" DEPTH MAX.
4. MATERIALS CLEAN DRY SAND, FINE LIME ROCK OR PIT SHELL MECHANICALLY COMPACTED IN 6" LIFTS TO AT LEAST 95% OF MAX. DENSITY, OR 98% IF UNDER PAVED AREA OF ROADWAY.
5. #57 STONE LOCATED UNDER PIPE OR EXISTING UNDISTURBED SUITABLE MATERIAL
6. SPRING LINE
7. STANDARD 6" MIN TRENCH UNDERCUT AND BACKFILL WITH #57 STONE COMPACTED IN 6" LIFTS ADDITIONAL UNDERCUT AND CRUSHED ROCK BEDDING FOUNDATION WHEN DIRECTED BY L.C.U. THE CONTRACTOR IS TO REMOVE UNSTABLE MATERIAL FROM THE TRENCH FOUNDATION. THEN INSTALL STABILIZED CRUSHED ROCK BACKFILL WITH #57 STONE COMPACTED IN 6" LIFTS
8. LOCATING WIRE IS REQUIRED FOR ALL PRESSURIZED PIPELINES.

REV: 12/10/2015

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING, SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2026 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

FOR PERMIT

SHEET
UTILITY DETAILS

SHEET NUMBER
PS-1.2

STANDARD DETAIL NO. 6.12
LEE COUNTY UTILITIES
RESTRAINED LENGTH SCHEDULE

N.T.S.

DUCTILE IRON PIPE

PIPE SIZE (Inches)	MINIMUM RESTRAINED PIPE LENGTH (FEET)					
	HORIZONTAL BENDS				DEAD END/VALVE	HORIZONTAL TEE
	90°	45°	22-1/2°	11-1/4°		
4	17	7	4	2	29	6
6	23	10	5	2	40	17
8	29	12	6	3	53	29
10	35	14	7	4	63	38
12	41	17	8	4	74	49
16	51	21	11	5	94	68
24	69	29	14	7	131	105
30	81	34	17	8	156	129

PVC PIPE

PIPE SIZE (Inches)	MINIMUM RESTRAINED PIPE LENGTH (FEET)					
	HORIZONTAL BENDS				DEAD END/VALVE	HORIZONTAL TEE
	90°	45°	22-1/2°	11-1/4°		
4	20	8	4	2	45	8
6	29	12	6	3	63	25
8	36	15	8	4	83	43
10	44	18	9	5	99	58
12	51	21	11	5	116	74
16	63	26	13	7	149	103
24	87	36	18	9	208	158
30	102	42	21	10	248	194

A COMPLETE JOINT RESTRAINING SCHEDULE FOR ALL ENCOUNTERED VERTICAL & HORIZONTAL BENDS, VERTICAL OFFSETS, TEES, AND DEAD ENDS SHALL BE THE RESPONSIBILITY OF THE DESIGN ENGINEER.

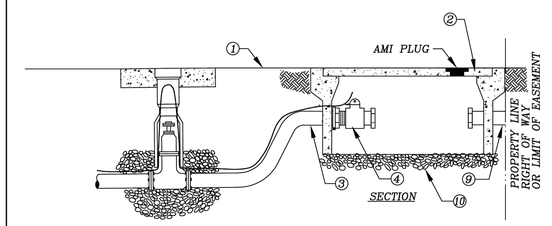
LENGTH FIGURES BASED ON FOLLOWING:

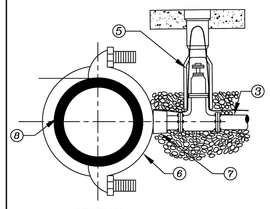
Pressure = 150 psi, FS = 1.5, trench type = 3, 30" cover on Bare pipe, Soil type = GP & SP

REV: 12/22/2022

STANDARD DETAIL NO. 6.17
LEE COUNTY UTILITIES
WATER SERVICE INSTALLATION 1-1/2" THROUGH 2"

N.T.S.





NOTE:

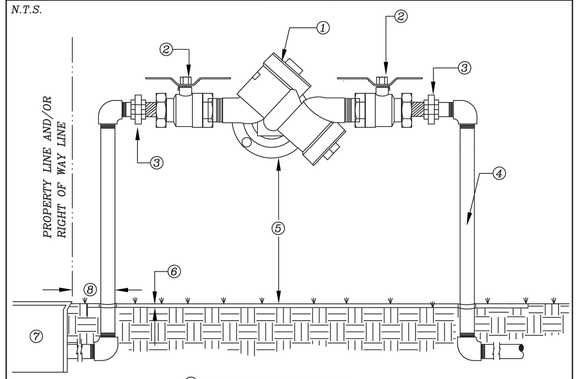
A. TUBING UNDER ROADWAY SHALL BE ENCASED IN 4" P.V.C. PIPE SCHEDULE 40 MINIMUM AND EXTEND 4 FEET BEYOND THE EDGES OF PAVEMENT OR BACK OF CURB.

1. FINISHED GRADE
2. 17" X 30" METER BOX WITH COVER TO BE LOCATED INSIDE R.O.W. @ PROPERTY LINE & CLEAR OF DRIVEWAYS. (SEE APPROVED MATERIALS LIST)
3. 2" POLYETHYLENE TUBING, PE 3408, 200PSI, SDR9, BLUE (SEE APPROVED MATERIAL LIST) WITH LOCATING WIRE
4. STRAIGHT CURB STOP WITH PADLOCK WINGS CONNECTED TO LEE COUNTY WATER METER (SEE APPROVED MATERIAL LIST)
5. 2" GATE VALVE WITH OPERATIONAL NUT (SEE DETAIL 6.1 OR 6.2)
6. DOUBLE STRAP SERVICE SADDLE PROVIDE SPRING LOADED TAPPING SADDLE WHEN TAPPING HDPE (SEE APPROVED MATERIAL LIST)
7. BRASS NIPPLE (MAX 3" LONG)
8. WATER MAIN
9. PRIVATE SERVICE LINE
10. 2" OF 57 STONE

REV: 4/24/2023

STANDARD DETAIL NO. 6.19
LEE COUNTY UTILITIES
3/4" TO 2" CROSS CONNECTION CONTROL ASSEMBLY

N.T.S.



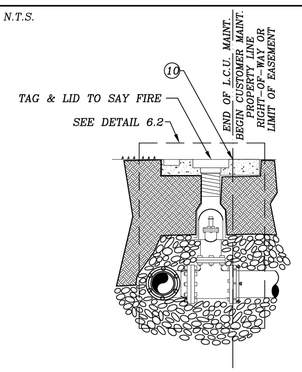
1. LCU APPROVED REDUCED PRESSURE BACKFLOW PREVENTION DEVICE (SEE APPROVED MATERIAL LIST)
2. FULL FLOW BRONZE BALL VALVE
3. UNION
4. ALL PIPING AND FITTINGS SHALL BE BRASS OR COPPER
5. 12"MIN.-24"MAX. UNLESS OTHERWISE APPROVED BY LCU
6. FINISHED GRADE
7. METER BOX
8. 2' MAXIMUM

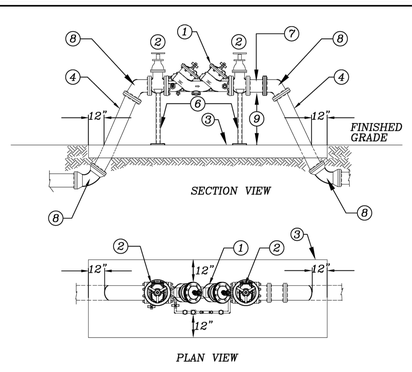
NOTES: 1. UNIONS REQUIRED
2. INITIAL AND ANNUAL DEVICE TESTING REQUIRED

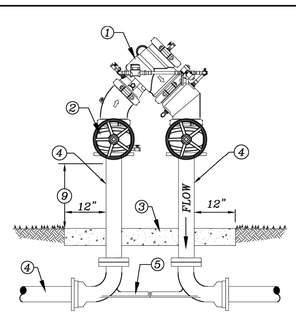
REV: 12/10/2015

STANDARD DETAIL NO. 6.21
LEE COUNTY UTILITIES
CROSS CONNECTION & COMPACT CROSS CONNECTION CONTROL ASSEMBLIES FOR FIRE LINE

N.T.S.







1. DOUBLE CHECK DETECTOR BACKFLOW PREVENTOR ASSEMBLY UL/FM APPROVED FOR FIRE SERVICE INSTALLATION WITH LOW FLOW BYPASS INDICATOR METER. (LEAD FREE) (SEE APPROVED MATERIAL LIST)
2. FLANGED GATE VALVE WITH RISING STEM AND RESILIENT SEAT (TYPICAL)
3. CONCRETE FOUNDATION 3,000 P.S.I. (TYPICAL) TO BE EXTEND 12" ON ALL SIDES FROM ABOVE GROUND PIPE, 4"THICK REINFORCED 6 X 6 10/10 WELDED WIRE MESH MINIMUM
4. D.I.P. TO POINT OF CONNECTION
5. EPOXY COATED D.I.P. VALVE SETTER ASSEMBLY
6. HOT DIPPED GALVANIZED OR S.S. ADJUSTABLE PIPE STANDS (SEE LCU STANDARD DETAIL #6.22)
7. 24" LONG FLANGE X PLAIN END SPOOL PIECE W/UNIFLANGE ON PLAIN END LINE UP
8. 45° OR 90° ELBOW M.J. W/ RETAINER GLANDS ABOVE GROUND FITTINGS TO BE FLANGED
9. 18"MIN. TO 24"MAX. FROM FINISH GRADE TO LOWEST POINT ALL FITTINGS
10. GATE VALVE @ PROPERTY LINE. END OF LCU OWNERSHIP AND MAINTENANCE.

NOTES:
A. ALL PAINTING TO BE IN ACCORDANCE WITH LCU STANDARDS. (SEE GENERAL CONDITIONS SECTION)
B. A REDUCED PRESSURE DETECTOR BACKFLOW ASSEMBLY SHALL BE USED WHEN HIGH HAZARDS, AS DEFINED BY AWWA M-14 (e.g. RISK OF CHEMICAL ADDITION, MEDICAL FACILITIES, INDUSTRIAL FACILITIES, PROPERTIES USING RECLAIMED WATER, ETC.) EXIST.
C. ALL ABOVE GROUND PIPE WILL HAVE FLANGED END DUCTILE IRON PIPE, PRESSURE CLASS 350. ALL NUTS AND BOLTS SHALL BE STAINLESS STEEL.
D. ALL PLANTING SHALL BE A MINIMUM OF 1.5' FROM THE EDGE OF SLAB, AND SHALL PROVIDE A 3' ACCESS OPENING.

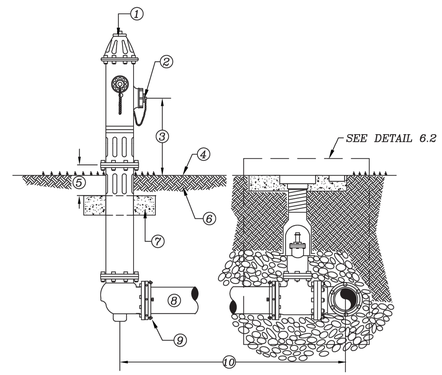
Lee County Southwest Florida

REV: 04/13/2023

April 13, 2023 6:33 AM S:\Utilities\ENGINEERING\ACAD\LCU-CAD-Stnds\151209 LCU STD DETAILS\CAD FILES\6.21_Cross Conn. Cntrl. 6.21A.dwg

STANDARD DETAIL NO. 6.24
LEE COUNTY UTILITIES
FIRE HYDRANT ASSEMBLY

N.T.S.



1. FIRE HYDRANT: STANDARD A.W.W.A. SAFETY YELLOW (SEE APPROVED MATERIAL LIST)
2. PUMPER CONNECTION FACING AS PER FIRE DISTRICT RECOMMENDATION
3. 18" MIN. – 24" MAX.
4. FINISHED GRADE
5. 6" MIN. CLEARANCE
6. COMPACTED FILL
7. 30" X 30" X 4" CONCRETE BREAK AWAY CONCRETE PAD, 3000 P.S.I.
8. PIPE TO BE D.I.P., PRESSURE CLASS 350
9. RESTRAINED JOINTS, FROM HYDRANT TO WATER MAIN
10. VARIABLE DISTANCE, IF GREATER THAN 100' A VALVE SHALL BE INSTALLED AT HYDRANT BASE

REV: 12/10/2015

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING, SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

Chick-fil-A
5200 Buffington Rd.
Atlanta Georgia,
30349-2998
Bowman
Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2026 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

FOR PERMIT

CHICK-FIL-A
LEHIGH ACRES FSU
1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

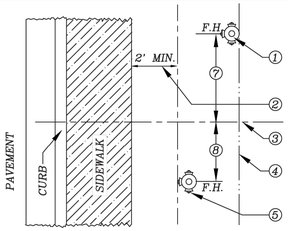
CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

SHEET
UTILITY DETAILS

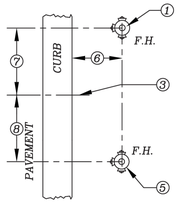
SHEET NUMBER
PS-1.3

STANDARD DETAIL NO. 6.25
LEE COUNTY UTILITIES
FIRE HYDRANT LOCATIONS/CLEARANCE

N.T.S.

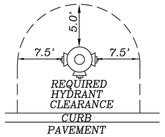


PARKWAY AREA WITH SIDEWALK
(PLAN VIEW)



PARKWAY AREA OR NO SIDEWALK
(PLAN VIEW)

1. PREFERRED LOCATION
2. F.H. TO BE LOCATED 2' MINIMUM FROM EDGE OF SIDEWALK
3. P.T. OR P.C. OF CURB RETURN
4. PROPERTY LINE RIGHT-OF-WAY OR LIMIT OF EASEMENT
5. ACCEPTABLE LOCATION WITH CURB RADIUS OF 20' OR MORE
6. BETWEEN 2'-0" MIN. AND 6'-0" MAX.
7. 6'-0"
8. 4'-0"



(PLAN VIEW)

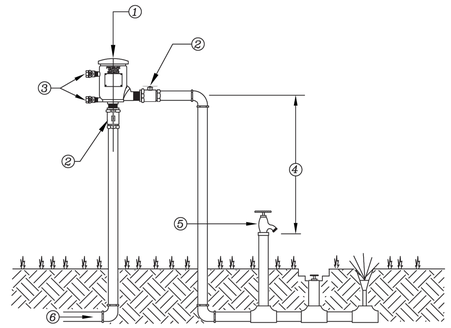
NOTES

1. OBSTRUCTIONS SUCH AS UTILITY POLES, STREET SIGNS, IRRIGATION BOXES, FENCES, ETC. SHALL NOT BE PLACED BETWEEN CURB AND HYDRANT.
2. SOME LOCATIONS APPLY AT EITHER END OF CURB RETURNS.
3. DIMENSION SHOWN ON CONSTRUCTION DRAWINGS SUPERCEDE LOCATIONS SHOWN HERE.

REV: 12/10/2015

STANDARD DETAIL NO. 6.26
LEE COUNTY UTILITIES
PRESSURE VACUUM BREAKER

N.T.S.

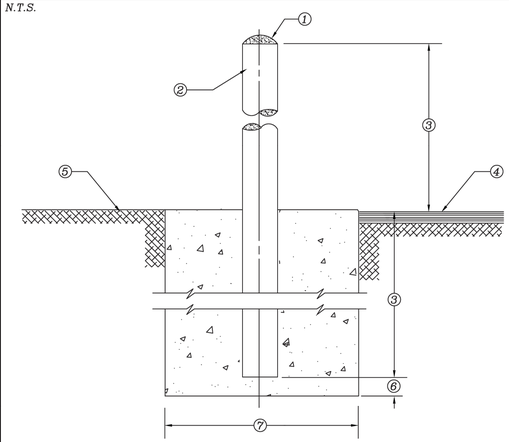


1. PRESSURE VACUUM BREAKER TO BE USED ON IRRIGATION LINES ONLY
2. RESILIENT SEATED FULL FLOW VALVE
3. TEST PORT
4. 12" MIN. ABOVE THE HIGHEST OUTLET
5. HOSE BIB
6. FLOW

REV: 12/10/2015

STANDARD DETAIL NO. 6.44
LEE COUNTY UTILITIES
BOLLARD POST DETAIL

N.T.S.



1. FILL AND TOP PIPE WITH CONCRETE
2. 6" DIA. x 6'-0" LONG PRESSURE CLASS 350 DUCTILE IRON PIPE
3. 3'-0" MINIMUM
4. PAVEMENT
5. FINISHED GRADE
6. 3"
7. 18" DIA. CONCRETE ENCASEMENT 3000 P.S.I.

NOTE:
1. PAINT BOLLARD TO LCU STANDARDS

REV: 12/10/2015

GENERAL NOTES:

1. ALL CONSTRUCTION SHALL MEET LEE COUNTY STANDARDS AND SPECIFICATIONS AS APPLICABLE.
2. AN FGUA REPRESENTATIVE IS TO BE PRESENT DURING CONNECTIONS TO EXISTING FACILITIES WITH 72-HOUR NOTIFICATION.
3. ALL UTILITIES WITHIN THE PROPERTY LINES WILL BE OWNED AND MAINTAINED BY THE PROPERTY OWNER.
4. ANY UTILITIES (PIPES, VALVES, AND ASSOCIATED HARDWARE) TO BE CONVEYED TO FGUA WILL BE LOCATED WITHIN AN EXISTING PUBLIC UTILITY EASEMENT, RIGHT-OF-WAY (ROW) AND/OR A DEDICATED FGUA UTILITY EASEMENT.
5. ANY REAL PROPERTY (LIFT STATIONS, PUMP HOUSES, WELLS, ETC.) WILL NOT BE LOCATED WITHIN THE ROW AND WILL BE CONVEYED TO FGUA VIA A SPECIFIC WARRANTY DEED.
6. LOW PRESSURE AIR TEST, AND VIDEO INSPECTION ARE REQUIRED FOR THE GRAVITY COLLECTION SYSTEM) - WATER SYSTEMS ARE TESTED TO A MINIMUM OF 150PSI FOR TWO-HOUR DURATION AS PER AWWA C 600 STANDARDS. FIRE SYSTEMS ARE NOT THE FGUA RESPONSIBILITY.
7. INSTALL 10-GAUGE TRACER WIRE ON ALL MAIN PIPELINES. WIRE TO EXTEND INTO EACH VALVE BOX WITH SUFFICIENT EXCESS TO BRING WIRE THREE FEET ABOVE FINISH GRADE.
8. ALL WATER AND/OR WASTEWATER LINES PASSING UNDER PAVED AREAS WILL BE CONSTRUCTED WITH A CASING AND SHALL APPLY WITH LOCAL OR STATE DOT STANDARDS.
9. A REDUCED PRESSURE ZONE (RPZ), BACKFLOW PREVENTION ASSEMBLY (BFP) IS REQUIRED. ACCEPTABLE MODELS FEBCO 825Y, WATTS 909, WILKINS 975XL, OR CONBRACO 400.
10. FULL CIRCLE STAINLESS STEEL TAPPING SADDLE SLEEVE WITH RESILIENT SEATED TAPPING VALVE FOR PROPOSED WATER AND FORCE MAIN CONNECTIONS TO BE USED.
11. WATER TAPPING SADDLES TO BE DOUBLE STRAP BRASS OR DUCTILE IRON OR EPOXY COATED STEEL WITH STAINLESS STEEL HARDWARE. TIE STRAPS AND BOLTS SHALL BE CORROSION RESISTANT ALLOY STEEL.
12. ALL FIRE FLOW LINES WILL RUN SEPARATE FROM THE DOMESTIC WATER MAINS AND HAVE A FIRE DETECTION METER AND A FIRE LINE BACKFLOW PREVENTION ASSEMBLY (BFP) ASSEMBLY SHALL INCLUDE A LOW FLOW BY-PASS METER W/ RPZ.
13. MANHOLE FRAMES & COVERS, AND CO CAPS ARE TO BE WATER TIGHT, IF NEEDED EXISTING MANHOLE TOPS ARE TO BE ADJUSTED TO PROPOSED GRADE.
14. INFLOW PROTECTION DISHES ARE REQUIRED FOR ALL MANHOLES; STAINLESS STEEL IN PAVED AREAS, ABS IN UNPAVED AREAS.
15. A CLEANOUT SHALL BE PROVIDED AT THE ROW/PL FOR ALL SERVICE CONNECTIONS.
16. THE FGUA SHALL OWN AND MAINTAIN ALL WATER SERVICE LINES UP TO THE POINT OF CONNECTION. ALL WATER MAINS SHALL BE A MINIMUM OF FOUR INCHES (4") IN DIAMETER AND ALL SUCH WATER MAINS SHALL BE CONSTRUCTED IN DEDICATED RIGHTS-OF-WAY OR APPROPRIATE UTILITY EASEMENTS DEDICATED TO THE FGUA.
17. ALL SEWER FORCE MAIN LINES AND GRAVITY SERVICE LINES ACCEPTED MUST BE AT LEAST FOUR INCHES (4") IN DIAMETER AND EIGHT INCHES (8"), RESPECTIVELY, EXCEPT LATERAL SEWER SERVICE LINES WHICH SHALL BE NO LESS THAN SIX INCHES (6") IN DIAMETER AND ACCEPTED BY THE FGUA TO A CLEANOUT LOCATED ON THE EDGE OF THE RIGHT-OF-WAY OR AN EASEMENT.
18. A BALL CHECK VALVE FOLLOWED BY A PLUG VALVE WILL BE INSTALLED ON FORCE MAINS LOCATED IN PUE OR R/W AT THE PROPERTY LINE DISCHARGING FROM PRIVATELY OWNED LIFT STATIONS CONNECTING INTO THE UTILITY MAINS.
19. CONNECTION TO EXISTING MANHOLES AND/OR LIFT STATIONS SHALL BE BY CORE BORE WITH KOR-N-SEAL CONNECTORS.
20. ALL CONNECTIONS WILL BE MADE AS HOT TAPS. ANY EXCEPTIONS TO THIS WILL REQUIRE FGUA APPROVAL. ALL HOT TAP CONNECTIONS SHALL BE PERFORMED BY A LICENSED, UNDERGROUND CONTRACTOR, WITH ADVANCE NOTIFICATION (72 HOURS) TO THE FGUA INSPECTOR.
21. BACKFLOW PREVENTION DEVICES SHALL BE PRIVATELY OWNED AND MAINTAINED. BFPDS SHOULD BE LOCATED OUTSIDE OF ANY FGUA EASEMENT OR ROW/PUBLIC EASEMENT. IT IS THE SOLE RESPONSIBILITY OF THE PROPERTY OWNER TO MAINTAIN THE DEVICE AND PROVIDE ANNUAL TESTING CERTIFICATIONS TO THE FGUA.

THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY BOWMAN CONSULTING, INC. SHALL BE WITHOUT LIABILITY TO BOWMAN CONSULTING.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Certificate of Authorization License No. 30462
900 SE 3RD AVE, SUITE 300
Fort Lauderdale, FL 33316
Phone: (954) 884-5253
www.bowman.com
© 2025 Bowman Consulting Group Ltd

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/02/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/18/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FGUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENTS
7	12/11/25	COUNTY AND FGUA COMMENTS

FOR PERMIT

CURRENT DESIGN NOTE APPLIED	2021-005
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	2/18/2026
DRAWN BY	R.M.

SHEET
UTILITY DETAILS

SHEET NUMBER
PS-1.4

Chick-fil-A Lehigh Acres

STORMWATER MANAGEMENT REPORT

1110 Homestead Rd. N., Lehigh Acres, FL.
Sec 31 // Twp 44 // Rge 27E
Parcel No.: 31-44-27-05-00006.30B0
Existing Use: Bank
Proposed Use: Fast food w/ drive-thru

Issued: November 2025

Prepared by:
Bowman
900 SE 3rd Ave, Suite 300
Ft. Lauderdale, FL, 33316

Certificate of Authorization No. 30462

WILLIAM PFEFFER
LICENSE
No. 73058
STATE OF FLORIDA
PROFESSIONAL ENGINEER

William Pfeffer, P.E.
Florida License No. 73058

2/17/26

Contents

Project Existing Conditions 3

Project Proposed Conditions 4

Vertical Datum 5

Water Quality Calculations 5

Modeling & Results 6

Tailwater 7

Total Daily Maximum Load Analysis 7

Recovery 8

Ashlar Ave Drainage 8

Summary 9

APPENDIX A 10

Maps and Figures 10

APPENDIX B 116

Excerpts from existing Permit #36-07214-P Application 080709-3 117

APPENDIX C 128

Total Daily Maximum Load Analysis Report 129

APPENDIX D 26

Drainage Calculations 27

Proposed ICPR Nodal Map 36

Slug Load Recovery ICPR Nodal Map 48

Project Existing Conditions

The subject property with Parcel No.: 31-44-27-05-00006.30B0 is located at 1110 Homestead Rd. N., Lehigh Acres, within Lee County, Florida (Figure 1). The site is bounded by Homestead Road to the south and by Ashlar Ave. to the east and north, with an existing commercial development just west abutting the site.



The overall site is +/-1.82 acres and has an existing +/- 5,378 SF bank with drive-thru. Per ECWCD, which has since been disbanded and incorporated within SFWMD, the site is within the regional drainage boundary. According to documents found, the site is likely incorporated within Basin 400 (controlled discharge via S-SF-1 structure). The existing master system consists of a mixture of both open (swale) areas and closed (piped) systems, which directly connects to a finger canal of the Yellowtail system, specifically Canal 47-31-9, according to the narrative within Application 080709-3 (associated Permit 36-07214-P). Runoff collected by overall master system is likely attenuated within the Live oak dry pond and water quality

within this basin is compensated within the North Basin and Live Oak dry pond, according to Application 080709-3 (associated Permit 36-07214-P). Refer to Appendix B for supporting documentation from the existing permit verifying that the proposed project is within Basin 400's limits.

The site was constructed in approximately in 1996-1997 according to property records with 2 small retention ponds to the north and south. Runoff is collected via flumes to allow sheet flow into the existing systems. The existing development ranges from +/-25.0 to 26.0' NAVD within the existing retention areas and from 26.4 to 28.40' NAVD within the existing development.

There is an existing stormwater inlet at the southern corner of the property, at the intersection of Homestead Road and Ashlar Ave. This inlet and associated MES collect discharge from the site once the top of bank for the shallow retention pond are overtopped at about 25.9 to 26' NAVD.

The site is within FEMA flood zone X and does not have an associated base flood elevation (panel 12071C0475F, effective 8/28/2008). Geotechnical exploration was conducted on-site, and average water table elevation was determined to be at 21.0' NAVD. Based on the geotechnical report, the site soils exhibit characteristics consistent with Hydrologic Soil Groups A and B. For curve number calculations, Soil Group B was selected for pervious areas to ensure a more conservative estimate of runoff potential. Field infiltration tests were performed at a depth of 10' below the existing ground surface where the hole was filled with water and maintained at a constant level to allow the soil to remain saturated. The test concluded with a saturated hydraulic conductivity rate of 5.4 and 5.5 feet per day. This value was averaged to be 5.45 feet per day, which were averaged to 5.45 feet per day. Applying a safety factor of 2, the adjusted hydraulic conductivity used in both the ICPR and recovery models was 2.725 feet per day.

Project Proposed Conditions

The applicant is proposing to demolish the existing site to construct a new +/- 5,613 SF fast-food restaurant with dual lane drive-thru and associated utilities, parking, and drainage infrastructure. The proposed on-site stormwater management system consists of a series of interconnected catch basins with conveyance pipes with associated retention systems which will ultimately discharge into the existing system within homestead road at the intersection of Ashlar.

Since the proposed project is required to construct a right turn lane a portion of the site will be donated therefore the total site area below reflects this site layout change. See below table for land use breakdown for the proposed site.

Table 1: Land Use Breakdown of Existing/ Proposed Conditions

Existing			**Proposed**		**Difference**
Pervious	**34,415 sf.**	**0.79 ac.**	**25,390 sf.**	**0.58 ac.**	**-10.99%**
Impervious	**45,057 sf.**	**1.03 ac.**	**54,082 sf.**	**1.24 ac.**	**+10.99%**
Roof	*5,378 sf.*	*0.12 ac.*	*5,614 sf.*	*0.13 ac.*	*+0.54%*
Pavement	*39,679 sf.*	*0.91 ac.*	*48,468 sf.*	*1.11ac.*	
Total Site	**79,472 sf**	**1.82 ac.**	**79,472 sf**	**1.82 ac.***	**--**
**includes 1,034 sf (0.02 ac) of ROW to be donated*					

Table 2: Proposed Conditions Composite Curve Number			
	Area	CN	Weighted CN
Building	0.13	98	13
Total Impervious	1.11	98	109
Green Space	0.58	65	38
Total Calculated Area	1.82		87

Vertical Datum

The elevations in this report and the construction plans are based on the North American Vertical Datum of 1988 (NAVD). Conversion from NAVD 88 to National Vertical Datum 1929 (NGVD 29) = (+) 1.18 feet.

Water Quality Calculations

The proposed water quality treatment volume is based on the requirements located within the SFWMD ERP Applicants Handbook II, effective 6/28/2024, for commercial developments utilizing retention systems.

According to the Applicants handbook, the 25-year, 3-day storm event is to be used when computing off-site discharge, using the designated discharge limit for the site's watershed. Lastly the parking lots, the 5-year, 1-day storm event should be utilized for lowest grate inlet. The retention criteria for commercial projects has been revised so that commercial projects are required to provide minimum of 20% nutrient load reduction as required by the Applicants Handbook Volume I. Lastly, dry retention/detention areas must recover the retained/detained volume within 72 hours.

Treatment volume requirements for water quality were evaluated based on the governing value of either 1" times the entirety of the basin area, 2.5" times the percent impervious area over the site, or if FDEP Nutrient Analysis is required. Using the land use breakdown provided in Table 1, the percent impervious area was calculated, and both 1" times the entirety of the site and the 2.5" impervious area-based volume was computed. These calculations are summarized in Tables 2 and 3.

Table 2: Percent Impervious (%)	
Site=Area-Lake-Wetland-Building	1.69
Site - Pervious	1.11
Impervious/Site	65.68%

Table 3: Volume Calculations	Acre-Ft
1" over Entire Site	0.15
2.5" x %impervious	0.25
Nutrient Analysis	0.07
Impared Waters Adjustment	50% Additional
50% Reduction for Retention	50% Reduction
Total Treatment Required	**0.19**

The proposed SC-800 underground storage chambers are capable of providing +/-0.33 ac-ft of storage with an additional 0.283 ac-ft being provided within the retention ponds on-site. Since the seasonal high-water table is at approximately 21.0' NAVD and the bottom of stone is at 20.60' NAVD, the storage provided by the portion of the chambers under the SHWT is subtracted and thus a total of 0.30 ac-ft of attenuation

and treatment is provided. The required water quality based on the governing factor of 2.5 percent impervious is 0.25 ac-ft. Since the project discharges into impaired waters an additional 50% treatment was provided. The water quality was then reduced due to the use of retention systems by 50% resulting in a total required volume of 0.19 ac-ft.

TOTAL STORAGE	
VOLUME	STAGE
0.00 AF	EL 21.00
0.13 AF	EL 21.62
0.24 AF	EL 22.23
0.33 AF	EL 22.85
0.39 AF	EL 23.47
0.44 AF	EL 24.08
0.50 AF	EL 24.70
0.58 AF	EL 25.32
0.71 AF	EL 25.93
1.03 AF	EL 26.55
1.57 AF	EL 27.17
2.33 AF	EL 27.78
3.30 AF	EL 28.40

A 3" bleeder is provided at an elevation of 25.05 ft. NAVD providing 0.55 ac-ft of volume which is more than the required 0.19 ac-ft of volume. An overflow weir will be set above the 25yr-72hr storm event at an elevation of 25.80 ft.

Modeling & Results

The post conditions design for the site was conducted using ICPR software. The finish floor elevation was set by modeling 100-year, 72-hour zero discharge storm event.

The proposed site will utilize the underground storage chambers for initial water quality treatment and attenuation via elevated manifolds and interconnection with the proposed onsite retention area. Similar to existing conditions runoff will attenuate and/or percolate into the ground prior to discharging through the sites proposed 3" bleeder and overtopping the on-site berm and entering the Master system previously permitted as part of the ECWCD.

Storm Event	**Type**	**Rainfall (inches)**	**POST (CFS)**	**Max Stage (ft)**	**Min. Design Elev. (ft)**
5-year – 24 hour	Parking Lot	5.5	0	25.31	25.35
10 year – 24 hour	N/A	6.50	0	25.57	N/A
25 year – 72 hour	Berm	11.00	0.29	26.72	26.75
100 year – 72 hour Zero Discharge	FFE	14.00	0	27.38	27.40

The proposed site is located within the East County Water Control District. As such, the site is subject to a maximum allowable discharge rate based on its proportional area within the basin. The applicable discharge limit for this watershed is 30 cfs per square mile.

The peak allowable discharge rate is calculated as follows:

$$Peak\ discharge\ rate = 1.82\ acres\ x\ \frac{30\ cfs}{Square\ Mile}\ x\ \frac{1\ Square\ Mile}{640\ acres} = \mathbf{0.09}\ \frac{\boldsymbol{Ft^3}}{\boldsymbol{Second}}$$

Accordingly, the proposed project will be limited to the minimum allowable orifice size, which is a 3-inch bleeder, with the top of grate set at or above the 25-year, 72-hour design storm stage.

Tailwater

The proposed discharge point for the stormwater system is the modified swale located within the Ashlar Avenue right-of-way. This swale has been engineered to accommodate the peak flow associated with the 10-year, 24-hour design storm event, in accordance with the criteria outlined by Lee County and FDOT Drainage Design Guide Section 3.2.6.1. The tailwater condition for hydraulic modeling begins at the lowest elevation within the swale system, which is 23.74 ft. and rises to a maximum elevation of 25.23 ft. under the 10-year storm event.

Total Daily Maximum Load Analysis

The project site is located in Lehigh Acres part of District SD Group Number 3 that discharges to the Caloosahatchee River. Since the site ultimately discharges to an impaired waterway for nitrogen and phosphorus a nutrient analysis was conducted utilizing BMP Trains 2020 program, a Total Daily Maximum Load analysis was evaluated to determine pre and post development impacts within the project boundary. Based on the associated environmental assessment, the existing conditions have been classified primarily as low-intensity commercial. Once the project is completed, the land use will remain the same as low intensity commercial. In the predevelopment conditions water sheet flows from the building and pavement to roadway flumes that direct stormwater into grassed areas. In the post conditions the site will utilize catch basins to collect runoff from the roadway, sidewalk, and building. Therefore, in the existing conditions the site does not have any directly connected impervious area and in the proposed conditions the roadway, sidewalks, and building is considered to be directly connected. Refer to Appendix A for a DCIA exhibit for the proposed conditions and DCIA calculations below.

Existing Conditions DCIA and Non-DCIA Curve Number Calculations

$$DCIA = BUILDING + SIDEWALK + ROADWAY$$

$$DCIA = 0.00\ ac + 0.00\ ac = \mathbf{0.00\ ac}$$

$$DCIA\ \% = \frac{DCIA}{Total\ Site} * 100$$

$$DCIA\ \% = \frac{0.00\ ac}{1.82\ ac} * 100 = \mathbf{0.00\%}$$

Existing Conditions Non-DCIA Curve Number			
	Area	CN	Weighted CN
Building	0.12	98	12
Total Impervious	0.91	98	89
Green Space	0.79	65	51
Total Calculated Area	1.82		84

Proposed Conditions DCIA and Non-DCIA Curve Number Calculations

$$DCIA = BUILDING + SIDEWALK + ROADWAY$$

$$DCIA = 0.13\ ac + 1.11\ ac = \mathbf{1.24\ ac}$$

$$DCIA\ \% = \frac{DCIA}{Total\ Site} * 100$$

$$DCIA\ \% = \frac{1.24\ ac}{1.82\ ac} * 100 = \mathbf{68.13\%}$$

Proposed Conditions Non-DCIA Curve Number			
	Area	CN	Weighted CN
Green Space	0.58	65	38
Total Calculated Area	0.58		65

Pre-development conditions provide an annual discharge of 1.92 Kg/Yr of nitrogen and 0.32 Kg/Yr of phosphorus. After the site is redeveloped, the annual runoff concentration increases to 6.47 Kg/yr of nitrogen and 1.077 Kg/Yr of phosphorus. Utilizing retention via a combination of underground chambers and retention areas the site contains water until the bleeder elevation of 25.05 ft NAVD providing a total retention volume of 0.55 ac-ft to satisfy nutrients. Utilizing 3.62 inches of retention in BMPtrains to model the provided 0.55 ac-ft of retention the nutrients are reduced below existing conditions to a total value of 0.18 kg/yr nitrogen and 0.029 kg/yr phosphorus. Refer to Appendix B for BMPTrains input and calculations report.

Recovery

A recovery model was created in ICPR used to model recovery within the proposed system using a factor of safety of at least 2 applied to the infiltration rate to ensure that the site recovers within 12 days and that the retention design recovers to retention bottom from the bleeder elevation within 72 hours.. The retention systems were modeled with initial stage set at the 25 year – 72 hour stage of the design model. From the 25 year – 72 hour stage at an elevation of 26.72' NAVD the site returns to the bleeder elevation of 25.05' NAVD at hour 26.00. From the bleeder elevation of 25.05' NAVD the retention system was able to recover to dry conditions at elevation 22.00' NAVD within 14 hours at hour 40.00 recovering, while the SC-800 underground chamber system can recover back down to the control elevation of 21.00' NAVD within +/- 22 hours at hour 47.75. The total time for the entire site to return back to control from the 25 year – 72 hour stage was 47.75 hours or 1.99 days which is less that the required 12 days.

Ashlar Ave Drainage

In addition to the proposed underground storage chambers and retention ponds, the development also includes two swales designed to provide supplemental stormwater storage and improve runoff management from the adjacent Ashlar Avenue. According to the ALTA survey dated 6/18/25, approximately 7,690 square feet or 0.18 acres of Ashlar Avenue—roughly half the roadway from the centerline west—drains toward the subject property. Currently, there is little to no functional swale volume north of the existing driveway on Ashlar Avenue, and only a shallow swale south of the driveway, measuring approximately 1 foot deep with a bottom width of 3 to 6 inches providing a total of 561.23 cubic feet of

storage. Refer to existing swale calculations in Appendix D on page 27 of the report for swale calculations. As a result, the existing swale system is degraded and serves primarily as a conveyance path into the Homestead Road drainage system via a mitered end section, offering minimal to no retention capacity.

The proposed improvements will reconstruct and expand the swales to meet Lee County standards for side slopes, depth, and bottom width. The northern swale will provide approximately 1,755 cubic feet of storage, while the southern swale will provide approximately 1,515 cubic feet, for a combined volume of 3,270 cubic feet, capable of storing approximately 5.1 inches of rainfall prior to accounting for any conveyance. This swale has been engineered to accommodate the peak flow associated with the 10-year, 24-hour design storm event. These improvements will significantly enhance off-site drainage conditions by providing dedicated storage that the existing system currently lacks. The swales will remain connected to the Homestead Road drainage system, maintaining existing flow paths while increasing retention capacity.

Summary

The applicant is proposing a +/-5,613 SF fast food restaurant with drive-thru and associated parking, utilities, and drainage infrastructure. The on-site stormwater system consists of a series of interconnected catch basins with an on-line retention system that consists of a small on-site retention pond and underground storage chambers. The chambers and on-site retention areas are capable of providing the required treatment volume and attenuation prior to discharging off-site. The site proposes to construct a bleeder set at an elevation of 25.05' and an overflow weir 6.33' wide with min. elevation 26.72' NAVD.

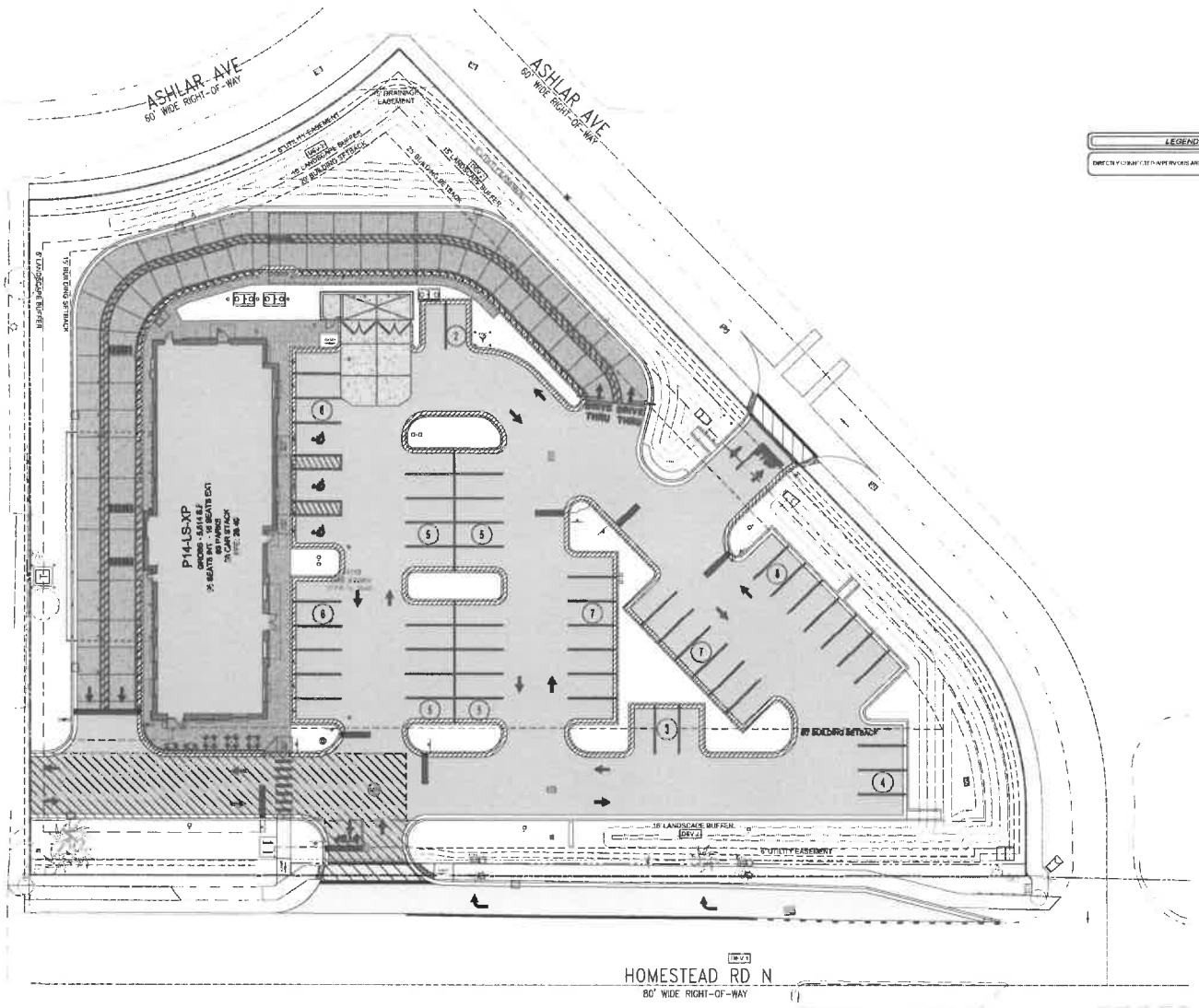
As proposed, the development will provide the required water quality and attenuation requirements on-site prior to discharging off-site. Due to the sites watershed the minimum 3" bleeder is required and will not cause adverse impacts to surrounding developments and systems. The site will provide better runoff and stormwater control while providing ample storage and water quality treatment.

APPENDIX A

Maps and Figures

GENERAL SITE PLAN NOTES

DEVIATIONS



LEGEND

DIRECTLY CONNECTED IMPERVIOUS AREA

GRAPHIC SCALE

20 0 10 20 40

(IN FEET)
1 inch = 20 ft.

811

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Bowman

Seal

WILLIAM PFEFFER, P.E.
LICENSE NO. 73058

CHICK-FIL-A
LEHIGH ACRES FSU

1110 HOMESTEAD RD N
LEHIGH ACRES, FL 33936
LEE COUNTY

FSU# 05587

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	04/22/25	RFI RESPONSES
2	04/17/25	DRAINAGE MODIFICATIONS
3	07/11/25	COUNTY COMMENTS
4	09/17/25	COUNTY COMMENTS
5	10/13/25	FOUA COMMENTS
6	11/04/25	COUNTY AND WMD COMMENT

CURRENT DESIGN NOTE APPLIED	2021 006
PROJECT #	010014-01-211
PRINTED FOR	PERMIT
DATE	11/2/2025
DRAWN BY	R.M.

FOR PERMIT

SHEET
DCIA EXHIBIT

SHEET NUMBER
EXH

National Flood Hazard Layer FIRMette

FEMA



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) Zone A, V, A99
- With BFE or Depth Zone AE, AO, AH, VE, AR
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard. Area of 1% annual chance flood with average depth less than one foot or with drainag areas of less than one square mile Zone
- Future Conditions 1% Annual Chance Flood Hazard Zone
- Area with Reduced Flood Risk due to Levee. See Notes. Zone
- Area with Flood Risk due to Levee Zone D

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard Zone
- Effective LOMRs
- Area of Undetermined Flood Hazard Zone

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- 20.2 Cross Sections with 1% Annual Chance
- 17.5 Water Surface Elevation
- Coastal Transect
- 513 Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

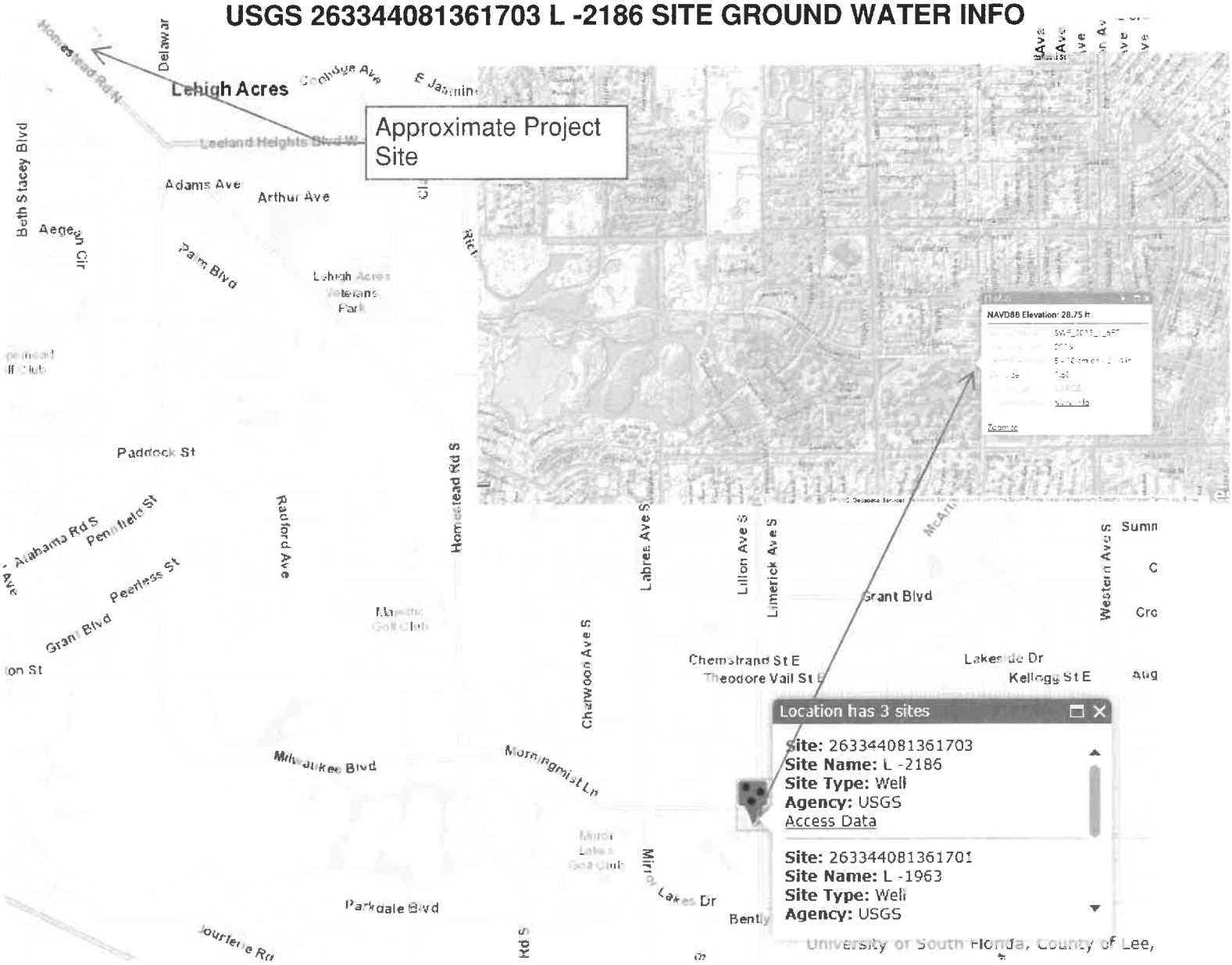
The pin displayed on the map is an approximate point selected by the user and does not represe an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 10/12/2023 at 4:23 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

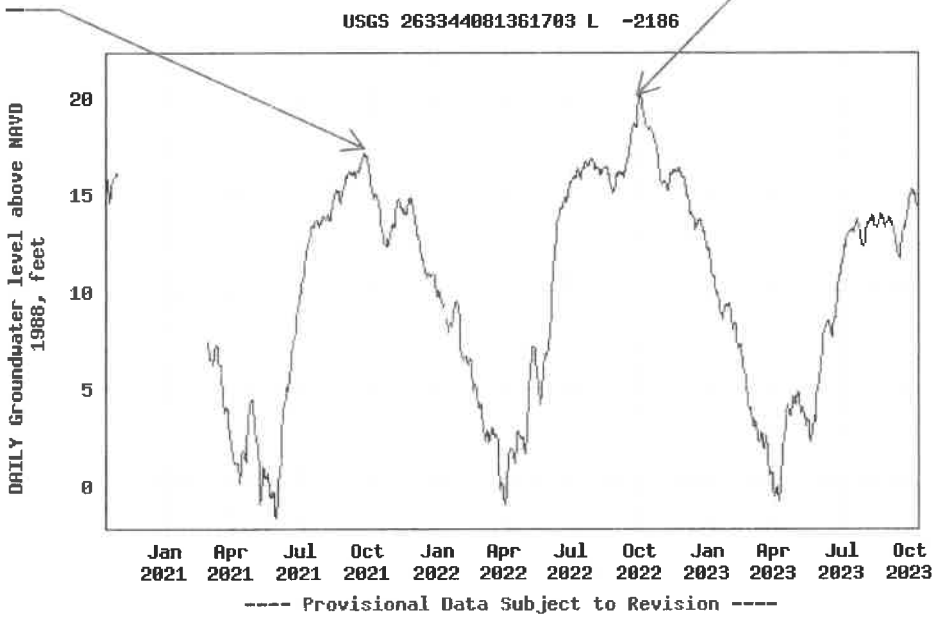
USGS 263344081361703 L -2186 SITE GROUND WATER INFO



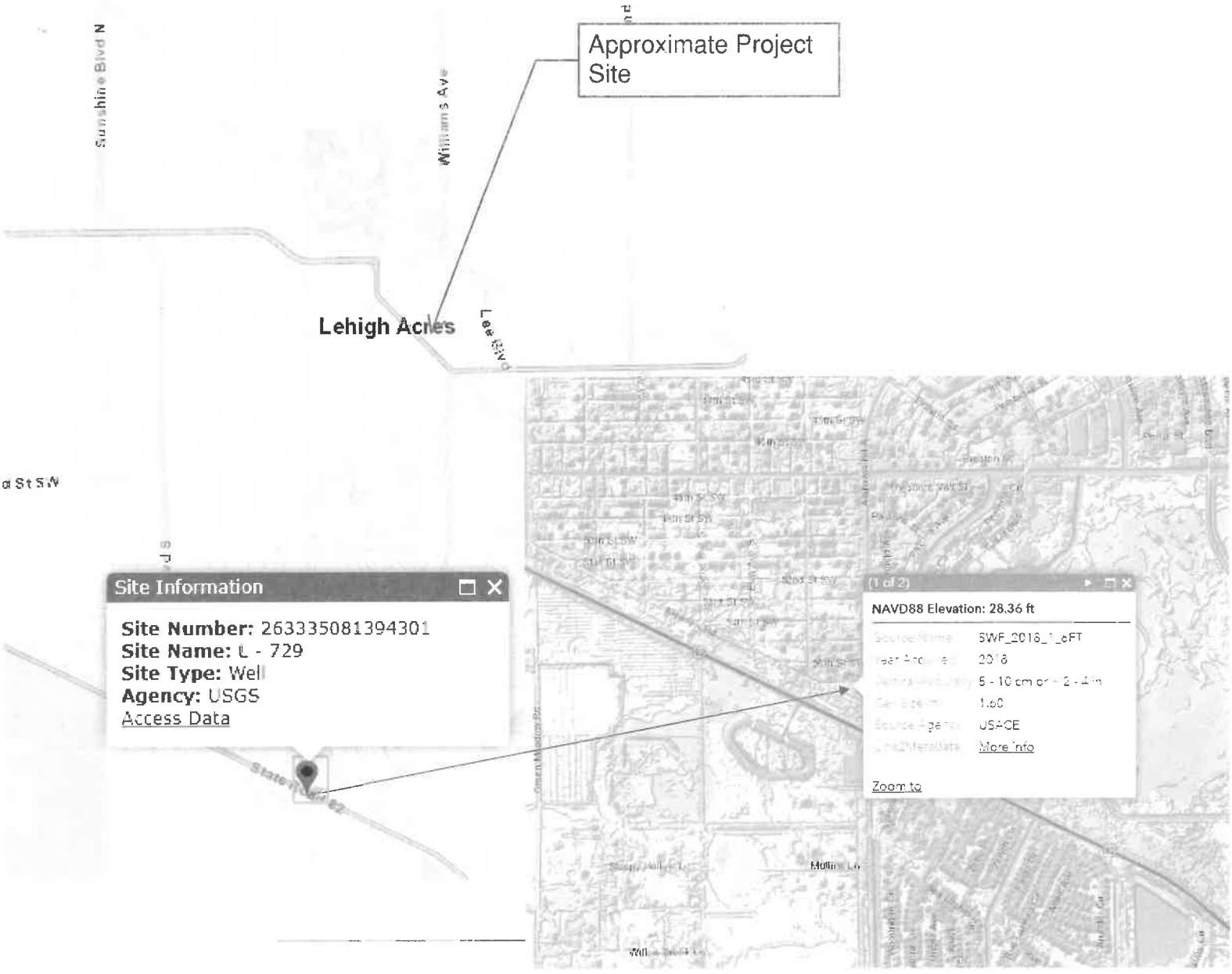
Groundwater level above NAVD 1988, feet

Typical Seasonal High Water Table

Seasonal High Water during Hurricane Ian



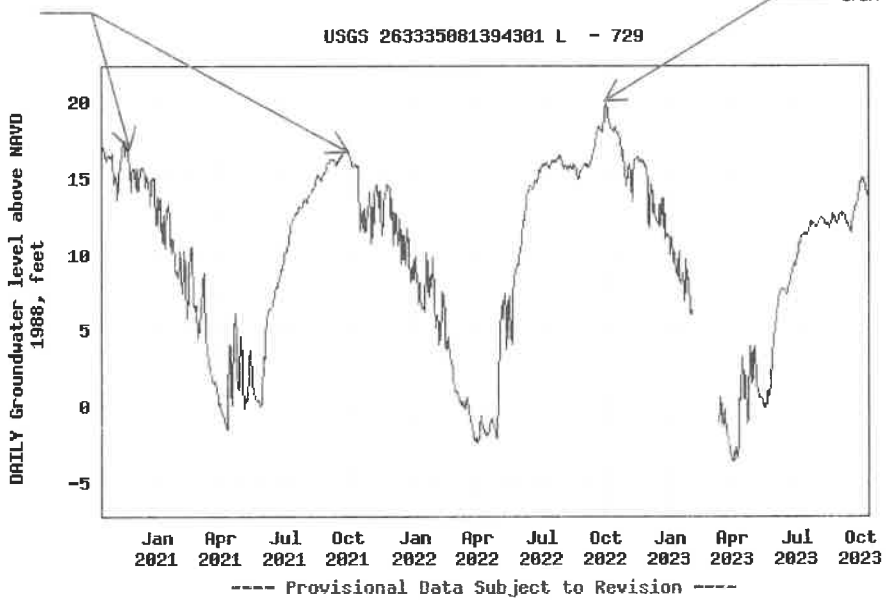
USGS 263335081394301 L - 729 SITE GROUND WATER INFO

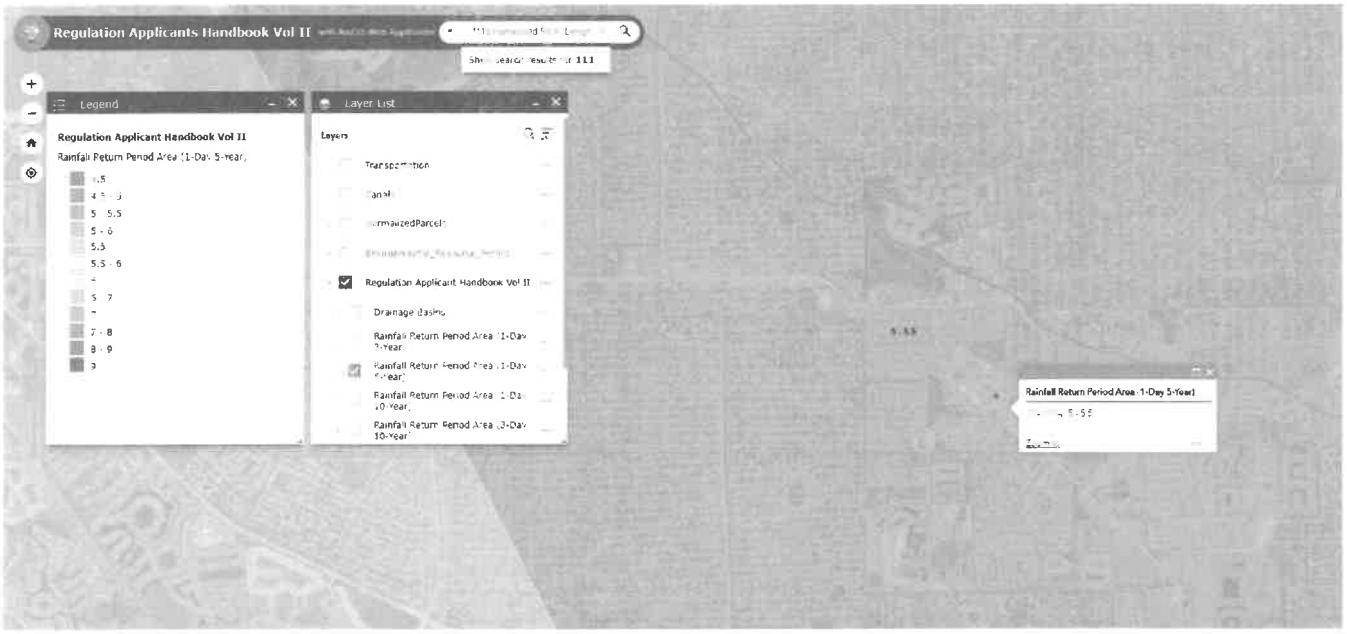


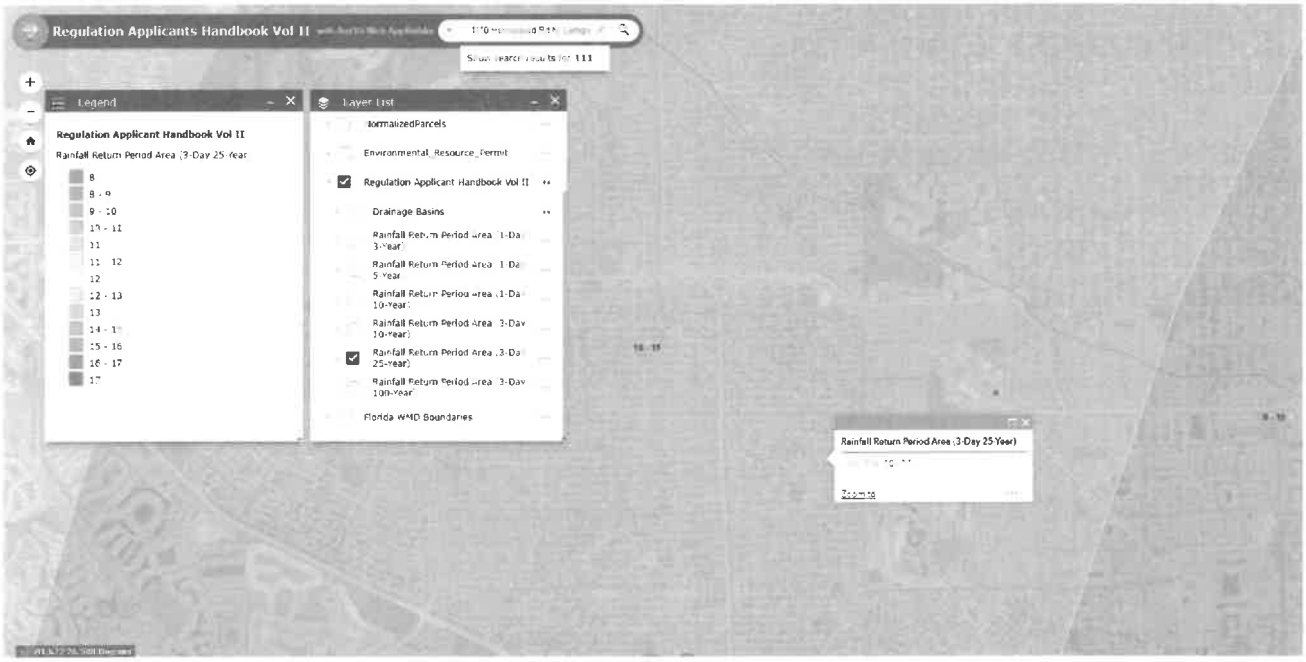
Groundwater level above NAVD 1988, feet

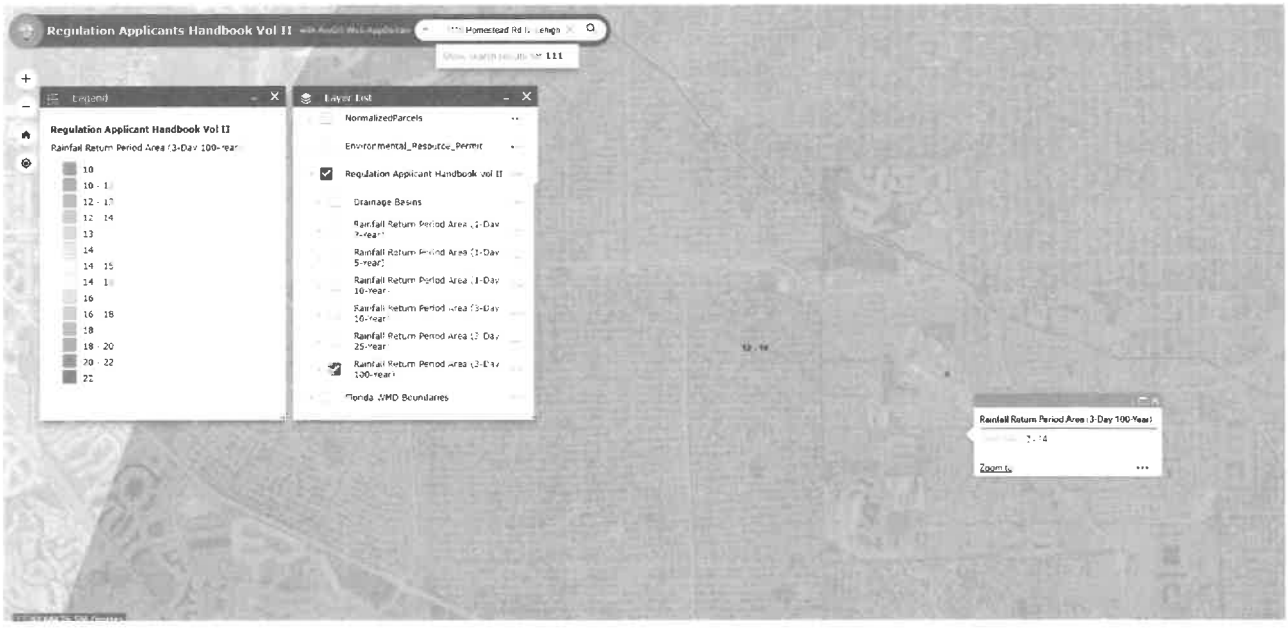
Typical Seasonal High Water Table

Seasonal High Water during Hurricane Ian




Regulation Applicants Handbook Vol II
Legend
Regulation Applicant Handbook Vol II
Rainfall Return Period Area (1-Day 5-Year)
Layer List
Layers
Regulation Applicant Handbook Vol II
Drainage Basins
Rainfall Return Period Area (1-Day 10-Year)
Rainfall Return Period Area (3-Day 10-Year)
Rainfall Return Period Area (1-Day 5-Year)


Regulation Applicants Handbook Vol II
Legend
Regulation Applicant Handbook Vol II
Rainfall Return Period Area (3-Day 25-Year)
Layer List
NormalizedParcels
Environmental_Resource_Permit
Regulation Applicant Handbook Vol II
Drainage Basins
Rainfall Return Period Area (3-Day 10-Year)
Rainfall Return Period Area (3-Day 25-Year)
Rainfall Return Period Area (3-Day 100-Year)
Florida WMD Boundaries
Rainfall Return Period Area (3-Day 25-Year)
Zoom to


Regulation Applicants Handbook Vol II
Legend
Regulation Applicant Handbook Vol II
Rainfall Return Period Area (3-Day 100-Year)
Layer List
NormalizedParcels
Environmental_Resource_Permit
Regulation Applicant Handbook Vol II
Drainage Basins
Rainfall Return Period Area (3-Day 25-Year)
Rainfall Return Period Area (3-Day 100-Year)
Florida WMD Boundaries
Rainfall Return Period Area (3-Day 100-Year)
Zoom to

Bowman

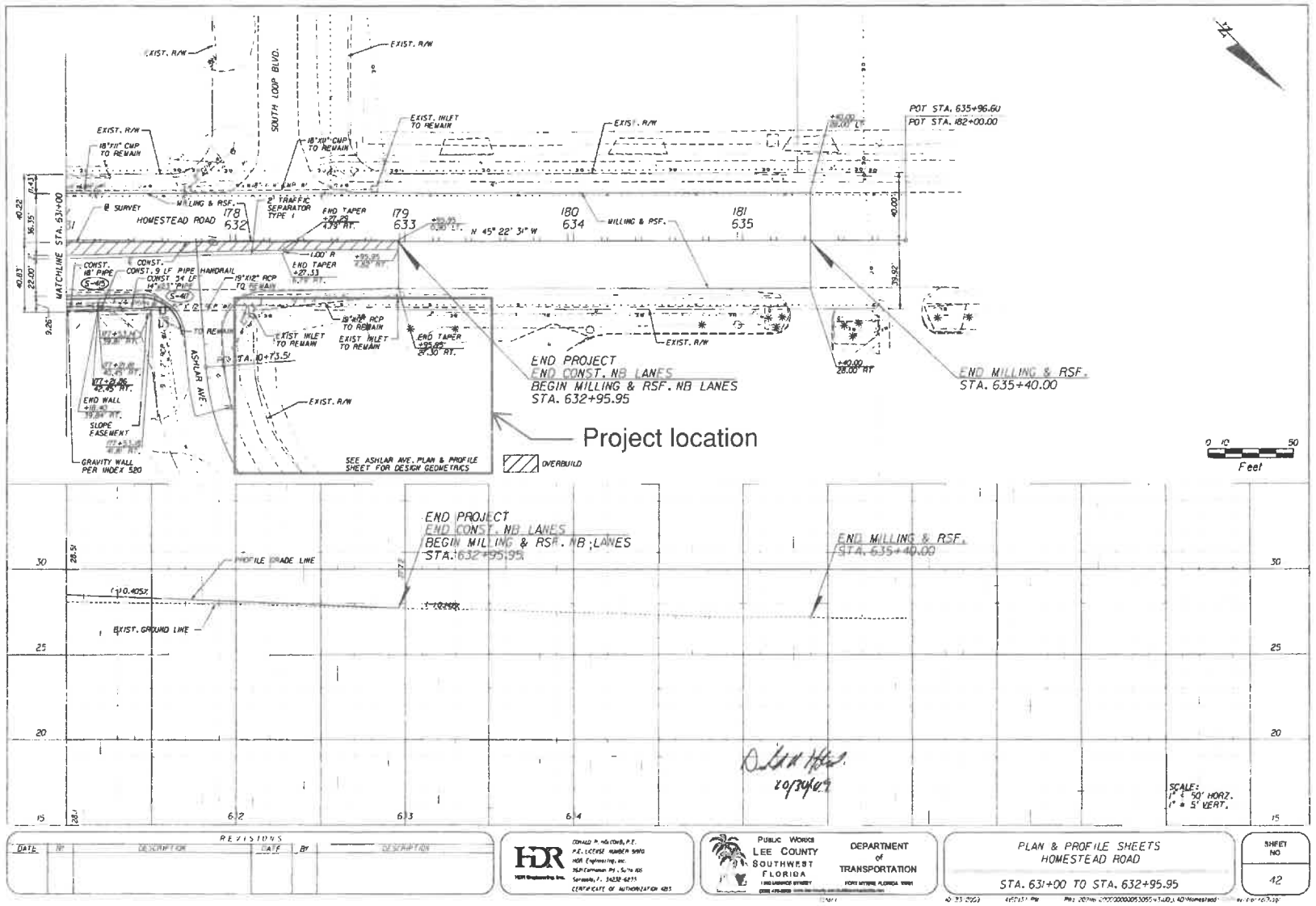
APPENDIX B

Excerpts from existing Permit #36-07214-P Application 080709-3

400 Basin (400-Series)

The 400 Basin consists of roadway drainage between Sta. 625+50 to the end of the project at Sta. 630+50, including portions of both Leeland Heights Blvd and Alabama Road near the intersection with Homestead. This existing system consists of a mixture of both open, swaled areas and closed (piped) systems, which directly connects to a finger canal of the Yellowtail system, specifically Canal 47-31-9. The proposed improvements consist mostly of some roadway re-alignments for safety and the transition from 3 to 4-lane roadway. The proposed drainage piping connects to the existing 30-inch pipe near Sta. 628+50, so no new direct to canal connection is required.

Stormwater quality for the proposed new impervious areas within this basin in compensated within the North Basin and the Liveoak Dry Pond. Similarly, stormwater attenuation is also compensated within the Liveoak dry pond.



REVISIONS

DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION

HDR

PUBLIC WORKS
LEE COUNTY
SOUTHWEST
FLORIDA

DEPARTMENT
of
TRANSPORTATION

PLAN & PROFILE SHEETS
HOMESTEAD ROAD

STA. 631+00 TO STA. 632+95.95

SHEET NO
42

Exhibit No. 2.0
Application No. 080709-3
Page 41 of 52

400 BASIN DESIGN DOCUMENTATION

FLORIDA DEPARTMENT OF TRANSPORTATION
STORM DRAIN TABULATION FORM (8/00)

Financial Project ID No. | County: LEE | UNITS: ( ) Metric [SI] (X) US customary | Revised by: MAR Date: 12.28.07
Description: HOMESTEAD ROAD | Checked by: Date:
HDR Project No.

LOCATION OF UPPER END STATION	DISTANCE (ft)	SIDE	UPPER STRUCTURE NUMBER	LOWER STRUCTURE NUMBER	INLET TYPE	TYPE OF STRUCTURE	TYPE OF LINE	LENGTH (ft)	Drainage Area (ac) C= 0.95 C= 0.20 C= 0.60 Increment	SUB Total	SUB Total (CA)	Tc (min)	Time in Section (min)	Intensity (in/hr)	TOTAL C x A	TOTAL RUNOFF (cfs)	Manhole, Inlet & Gutter Elev. (ft)	HYD. GRADIENT Crown EL. / Flow Line EL. Upper	Lower	ΔFALL (feet)	Diam. (in.)	HYD. GRAD. Physical Min. Phys. Slope (%)	Velocity (fps)	REMARKS	EDGE OF PAVEMENT, GRATE OR LID ELEVATION	CHECKS
																								ZONE or COUNTY: LEE		
																								FREQUENCY: 10-YEAR		
																								NOTES:		
																								Manning's n = 0.012		
																								USING Tw= 22.7		
ALABAMA			S-402		CI	P 8	S	39.3	0.21	0.21	0.20						25.8	25.8	0.01		0.02				INLET - HGL: 1.77	K= 0.00
018+13.00	17.80	RT		S-401					0.01	0.01	0.00	10.0	0.8	7.45	0.20	1.50	27.6	24.5	24.3	0.20	18	0.51	0.85		27.76	Partial Flow? NO	LOSS = 0.00
											0.00							23.0	22.8			0.15				COVER = 3.05	Calc d/D
ALABAMA			S-400		DBI	MOD C	S	0.0	0.03	0.03	0.03						25.8	25.8	0.00		0.01				INLET - HGL:	K= 0.00
016+28.24	22.25	LT		S-401					0.10	0.10	0.02	10.0	0.0				27.2	22.8	22.8	0.00				PARTIAL	27.22	Partial Flow?	LOSS = 0.00
											0.00							22.8	22.8							COVER = 4.42	Calc d/D
ALABAMA			S-401		CI	J 8	M	92.8	0.17	0.41	0.39						25.8	25.7	0.06		0.07				INLET - HGL: 1.83	K= 0.00
015+28.24	18.39	LT		S-403					0.00	0.11	0.02	10.8	0.9	7.27	0.41	2.99	27.6	24.3	24.0	0.30	18	0.32	1.69		27.81	Partial Flow? NO	LOSS = 0.00
											0.00							22.8	22.5			0.15				COVER = 3.30	Calc d/D
ALABAMA			S-403		DBI	MOD C	M	117.6	0.36	0.77	0.73						25.7	25.4	0.26		0.22				INLET - HGL: 1.29	K= 0.00
017+20.00	32.26	LT		S-404					0.03	0.14	0.03	11.7	0.6	7.08	0.76	5.38	27.0	24.0	23.8	0.20	18	0.17	3.04		27.00	Partial Flow? NO	LOSS = 0.00
											0.00							22.5	22.3			0.15				COVER = 3.00	Calc d/D
ALABAMA			S-404		MH	P 8	M	78.8	0.00	0.77	0.73						25.4	25.3	0.17		0.21				INLET - HGL: 3.70	K= 0.00
016+28.00	33.00	LT		S-405					0.00	0.14	0.03	12.3	0.4	6.96	0.76	5.29	29.2	23.6	23.5	0.30	18	0.38	2.99		29.15	Partial Flow? NO	LOSS = 0.00
											0.00							22.3	22.0			0.15				COVER = 5.35	Calc d/D
ALABAMA			S-406		CI	P 6	S	55.8	0.42	0.42	0.40						25.3	25.3	0.04		0.07				INLET - HGL: 1.85	K= 0.00
018+97.60	17.75	RT		S-405					0.02	0.02	0.00	10.0	0.5	7.45	0.40	3.00	27.2	24.0	23.5	0.50	18	0.90	1.70		27.38	Partial Flow? NO	LOSS = 0.00
											0.00							22.5	22.0			0.15				COVER = 3.17	Calc d/D
ALABAMA			S-405		DBI	MOD C	M	133.5	0.00	1.19	1.13						25.3	24.8	0.68		0.51				INLET - HGL: 2.14	K= 0.00
018+97.60	38.02	LT		S-407					0.10	0.26	0.05	12.7	0.5	6.89	1.18	8.15	27.4	23.5	23.3	0.20	18	0.15	4.61		27.42	Partial Flow? NO	LOSS = 0.00
											0.00							22.0	21.8			0.15				COVER = 3.92	Calc d/D
HOMESTEAD			S-407		DBI	MOD C	M	160.1	0.00	1.19	1.13						24.6	23.7	0.90		0.56				INLET - HGL: 2.93	K= 0.00
626+40.00	39.78	LT		S-410					0.36	0.64	0.13	13.2	0.6	6.80	1.26	8.55	26.9	23.3	22.9	0.40	18	0.25	4.84		26.93	Partial Flow? NO	LOSS = 0.00
											0.00							21.8	21.4			0.15				COVER = 3.63	Calc d/D
HOMESTEAD			S-409		DBI	MOD C	S	0.0	0.01	0.01	0.01						24.1	24.1	0.00		0.01				INLET - HGL:	K= 0.00
626+40.00	39.79	RT		S-408					0.10	0.10	0.02	10.0	0.0				27.3	22.0	22.0	0.00				PARTIAL	27.27	Partial Flow?	LOSS = 0.00
										0.00	0.00							22.0	22.0							COVER = 5.27	Calc d/D
HOMESTEAD			S-408		CI	J 5	S	160.0	0.09	0.10	0.10						24.1	23.9	0.20		0.12				INLET - HGL: 4.10	V(norm)= 2.26	K= 0.00
626+40.00	35.25	RT		S-411					0.00	0.10	0.02	10.0	1.2	7.45	0.12	0.86	28.2	23.5	23.1	0.40	18	0.25	2.26		28.36	Partial Flow? YES	d/D = 0.25	LOSS = 0.00
											0.00							22.0	21.6			0.15				COVER = 4.65	Qfull = 5.71	Calc d/D 0.25
HOMESTEAD			S-417		DBI	JUNC. B	S	36.5	0.50	0.50	0.48						25.8	25.1	0.17		0.47		INCLUDES 0.84 AC EXISTING PIP		INLET - HGL: 1.48	V(norm)= 4.38	K= 0.00
631+56.41	39.86	RT		S-415					1.24	1.24	0.25	15.0	0.1	6.49	0.72	4.69	27.3	26.3	26.2	0.16	14X23	0.44	4.38	CONNECT TO EXISTING PIPE	27.29	Partial Flow? YES	d/D = 0.56	LOSS = 0.00
											0.00							25.2	25.0			0.15		ADD JUNCTION BOX LID		COVER = 0.96	Qfull = 7.58	Calc d/D 0.56
HOMESTEAD			S-415		DBI	MOD C	S	220.1	0.35	0.85	0.81						25.1	24.3	0.80		0.38				INLET - HGL: 2.01	K= 0.00
631+20.00	36.83	RT		S-413					0.05	1.29	0.26	15.1	0.9	6.47	1.07	6.90	27.1	23.5	23.1	0.40	18	0.18	3.90		27.13	Partial Flow? NO	LOSS = 0.00
											0.00							22.0	21.6			0.15				COVER = 3.63	Calc d/D
HOMESTEAD			S-413		DBI	MOD C	S	100.1	0.09	0.94	0.89						24.3	23.9	0.46		0.46		BACK OF SIDEWALK Type C mod.		INLET - HGL: 2.61	K= 0.00
629+00.00	39.78	RT		S-411					0.39	1.68	0.34	16.0	0.4	6.33	1.23	7.79	26.9	23.1	22.9	0.20	18	0.20	4.41		26.93	Partial Flow? NO	LOSS = 0.00
											0.00							21.6	21.4			0.15				COVER = 3.63	Calc d/D
HOMESTEAD			S-411		CI	P 6	S	70.5	0.50	1.54	1.46						23.9	23.7	0.15		0.22				INLET - HGL: 3.65	K= 0.00
628+00.00	35.25	RT		S-410					0.02	1.80	0.36	16.4	0.3	6.28	1.82	11.44	27.5	23.4	23.2	0.20	24	0.28	3.64		27.72	Partial Flow? NO	LOSS = 0.00
											0.00							21.4	21.2			0.10				COVER = 4.11	Calc d/D
HOMESTEAD			S-410		CI	J 6	M	52.7	0.65	3.36	3.21						23.7	23.2	0.46		0.68				INLET - HGL: 3.37	K= 0.00
628+00.00	35.25	LT		S-412					0.02	2.46	0.49	16.7	0.1	6.23	3.70	23.08	27.1	23.2	23.1	0.10	24	0.19	7.35		27.28	Partial Flow? NO	LOSS = 0.00
											0.00							21.2	21.1			0.10				COVER = 3.67	Calc d/D
HOMESTEAD			EXIST. INLET		DBI	S	P8	55.8	0.23	1.29	0.95	16.0	0.0	6.05	1.16	7.01	26.6	25.6	25.5	0.05	12x18	0.83	4.39	INCLUDES 1.38 AC FROM MALL	26.63	INLET - HGL: 3.07	K= 0.00
631+00.24	39.87	LT		S-414							0.00							24.5	24.4			0.03		CONNECT TO EXISTING PIPE		Partial Flow? NO	LOSS = 0.00
																										COVER = 1.16	Calc d/D
HOMESTEAD			S-414		DBI	MOD C	S	191.6	0.51	1.68	1.60						25.5	23.2	0.50		0.26				INLET - HGL: 0.23	V(norm)= 3.30	K= 0.00
630+44.00	34.46	LT		S-412					0.22	0.46	0.09	18.0	1.0	6.05	1.69	10.21	25.7	25.5	25.0	0.50	18	0.26	3.30		25.73	Partial Flow? YES	d/D = 1.00	LOSS = 0.00
											0.00							24.0	23.5			0.15				COVER = 0.23	Qfull = 5.63	Calc d/D 1
HOMESTEAD			S-412		MH	J 8	M	7.0	0.00	5.05	4.81						23.2	23.1	0.11		1.54		CONNECT TO EXISTING BOX		INLET - HGL: 4.67	V(norm)= 11.26	K= 0.00
									0.00	2.91	0.58	19.0	0.0	5.93	5.39	31.94	27.9	23.6	23.5		30	1.42	11.26		27.91	Partial Flow? YES	d/D = 0.55	LOSS = 0.00

Bowman

APPENDIX C

Total Daily Maximum Load Analysis Report

910 SE 17th Street, 3rd Floor, Ft. Lauderdale, Florida 33316
bowman.com

Complete Report (not including cost) Ver 4.3.5

Project: Chick-Fil-A
Date: 11/3/2025 1:55:52 PM

Site and Catchment Information

Analysis: Net Improvement

Catchment Name	Chick-Fil-A
Rainfall Zone	Florida Zone 4
Annual Mean Rainfall	53.00

Pre-Condition Landuse Information

Landuse	Low-Intensity Commercial: TN=1.13 TP=0.188
Area (acres)	1.82
Rational Coefficient (0-1)	0.17
Non DCIA Curve Number	84.00
DCIA Percent (0-100)	0.00
Nitrogen EMC (mg/l)	1.130
Phosphorus EMC (mg/l)	0.188
Runoff Volume (ac-ft/yr)	1.379
Groundwater N (kg/yr)	0.000
Groundwater P (kg/yr)	0.000
Nitrogen Loading (kg/yr)	1.922
Phosphorus Loading (kg/yr)	0.320

Post-Condition Landuse Information

Landuse	Low-Intensity Commercial: TN=1.13 TP=0.188
Area (acres)	1.82
Rational Coefficient (0-1)	0.58
Non DCIA Curve Number	65.00
DCIA Percent (0-100)	68.13
Wet Pond Area (ac)	0.00
Nitrogen EMC (mg/l)	1.130
Phosphorus EMC (mg/l)	0.188
Runoff Volume (ac-ft/yr)	4.644
Groundwater N (kg/yr)	0.000

Groundwater P (kg/yr)	0.000
Nitrogen Loading (kg/yr)	6.471
Phosphorus Loading (kg/yr)	1.077

Catchment Number: 1 Name: Chick-Fil-A

Project: Chick-Fil-A
Date: 11/3/2025

Retention Design

Retention Depth (in)	3.620
Retention Volume (ac-ft)	0.549

Watershed Characteristics

Catchment Area (acres)	1.82
Contributing Area (acres)	1.820
Non-DCIA Curve Number	65.00
DCIA Percent	68.13
Rainfall Zone	Florida Zone 4
Rainfall (in)	53.00

Surface Water Discharge

Required TN Treatment Efficiency (%)	70
Provided TN Treatment Efficiency (%)	97
Required TP Treatment Efficiency (%)	70
Provided TP Treatment Efficiency (%)	97

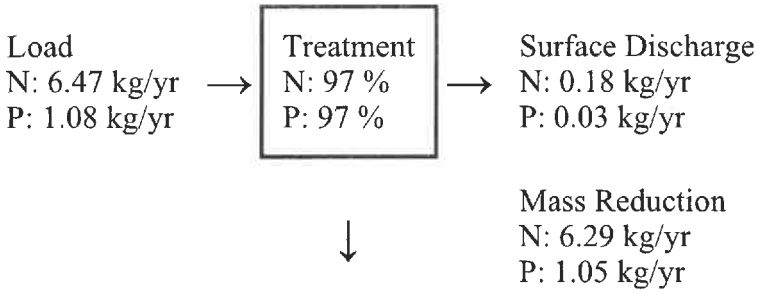
Media Mix Information

Type of Media Mix	Not Specified
Media N Reduction (%)	
Media P Reduction (%)	

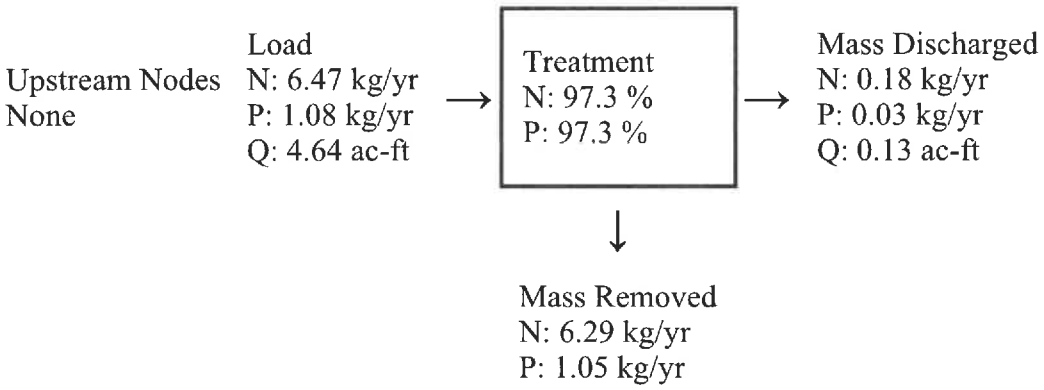
Groundwater Discharge (Stand-Alone)

Treatment Rate (MG/yr)	0.000
TN Mass Load (kg/yr)	6.294
TN Concentration (mg/L)	0.000
TP Mass Load (kg/yr)	1.047
TP Concentration (mg/L)	0.000

Load Diagram for Retention (stand-alone)



Load Diagram for Retention (As Used In Routing)



Summary Treatment Report Version: 4.3.5

Project: Chick-Fil-A

Analysis Type: Net Improvement

Date:11/3/2025

BMP Types:

Catchment 1 - (Chick-Fil-A)
Retention
Based on % removal values to the nearest percent

Routing Summary
Catchment 1 Routed to Outlet

Total nitrogen target removal met? **Yes**
Total phosphorus target removal met? **Yes**

Summary Report

Nitrogen

Surface Water Discharge

Total N pre load	1.92 kg/yr
Total N post load	6.47 kg/yr

Target N load reduction	70 %	
Target N discharge load	1.92 kg/yr	
Percent N load reduction	97 %	
Provided N discharge load	.18 kg/yr	.39 lb/yr
Provided N load removed	6.29 kg/yr	13.88 lb/yr

Phosphorus

Surface Water Discharge

Total P pre load	.32 kg/yr	
Total P post load	1.077 kg/yr	
Target P load reduction	70 %	
Target P discharge load	.32 kg/yr	
Percent P load reduction	97 %	
Provided P discharge load	.029 kg/yr	.06 lb/yr
Provided P load removed	1.047 kg/yr	2.309 lb/yr

Bowman

APPENDIX D

Drainage Calculations

Bowman

Date: 6/13/2025
Project: CFA LEHIGH ACRES
Project No: 100114

900 SE 3RD AVE, Suite 300
Ft. Lauderdale, FL 33316
Tel: 954-686-1562

Calculated By: SGS
Checked By: BP

DESIGN CRITERIA

October Water Elevation	21.00	NAVD
FEMA Elevation		NAVD

LAND USE SUMMARY - AFFECTED AREA

	Existing			Proposed		
Areas:	Square Ft.	Acres	Percent	Square Ft.	Acres	Percent
Lake	0	0.00	0.0%	0	0.00	0.0%
Building	5,378	0.12	6.6%	5,614	0.13	7.14%
Paved and Sidewalk	39,679	0.91	50.0%	48,468	1.11	60.99%
Pervious	34,415	0.79	43.4%	25,390	0.58	31.87%
Total Area:	79,472	1.82	100.0%	79,472	1.82	100.00%

EXISTING PONDS ELEVATION	AREA	STAGE AREA NODE ACRE
24.74	163.64	0.003757
25	990.04	0.022728
25.5	3824.24	0.087792
26	7728.45	0.177421

PROPOSED RETENTION PONDS	STAGE AREA NODE SF	STAGE AREA NODE ACRE	STAGE VOLUME AC-FT
22	507.4	0.011648	0
23	1148.35	0.026362	0.032
24	2156.963	0.049517	0.095
25	4169.566	0.09572	0.215
26	7575.588	0.173912	0.437

EXISTING POND SOUTH

ELEVATION	AREA	INC. DEPTH	INC. VOL (CF)	CUM. VOL (CF)	INC. VOL (AC)
24.74	163.64	0	0	0	0.000
25	847.13	0.26	130.39	130.39	0.003
25.5	1972.77	0.5	704.98	835.37	0.019
26	3038.07	0.5	1252.71	2088.08	0.048

EXISTING SOUTH SWALE

ELEVATION	AREA	INC. DEPTH	INC. VOL (CF)	CUM. VOL (CF)	INC. VOL (AC)
25.5	137.78	0	0	0	0.000
26	1438.44	0.5	394.06	394.06	0.009

EXISTING POND NORTH

ELEVATION	AREA	INC. DEPTH	INC. VOL (CF)	CUM. VOL (CF)	INC. VOL (AC)
25	142.91	0	0	0	0.000
25.5	1665.88	0.5	542	542	0.012
26	2463.88	0.5	1032.44	1574.44	0.036

EXISTING NORTH/EAST SWALE

ELEVATION	AREA	INC. DEPTH	INC. VOL (CF)	CUM. VOL (CF)	INC. VOL (AC)
25.5	47.81	0	0	0	0.000
25.9	788.06	0.4	167.17	167.17	0.004

PROJECT INFORMATION	
ENGINEERED PRODUCT MANAGER	KARL GUTMANN 239-994-6478 KARL.GUTMANN@ADSPIPE.COM
ADS SALES REP	RICK PFAFFENDORF 813-727-2545 RICK.PFAFFENDORF@ADSPIPE.COM
PROJECT NO.	S450292
PROJECT SITE COORDINATOR:	JAKOB LAMBERT 941-290-7120 JAKOB.LAMBERT@ADSPIPE.COM

Advanced Drainage Systems, Inc.

SiteAssist™
FOR STORMTECH
INSTALLATION INSTRUCTIONS
VISIT OUR APP

CHICK-FIL-A STORE #05587

LEHIGH ACRES, FL, USA

SC-800 STORMTECH CHAMBER SPECIFICATIONS

1. CHAMBERS SHALL BE STORMTECH SC-800.
2. CHAMBERS SHALL BE ARCH-SHAPED AND SHALL BE MANUFACTURED FROM VIRGIN, IMPACT-MODIFIED POLYPROPYLENE COPOLYMERS.
3. CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
4. CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNOBSTRUCTED INTERNAL SPACE WITH NO INTERNAL SUPPORTS THAT WOULD IMPEDE FLOW OR LIMIT ACCESS FOR INSPECTION.
5. THE STRUCTURAL DESIGN OF THE CHAMBERS, THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET FOR: 1) LONG-DURATION DEAD LOADS AND 2) SHORT-DURATION LIVE LOADS, BASED ON THE AASHTO DESIGN TRUCK WITH CONSIDERATION FOR IMPACT AND MULTIPLE VEHICLE PRESENCES.
6. CHAMBERS SHALL BE DESIGNED AND ALLOWABLE LOAD CONFIGURATIONS DETERMINED IN ACCORDANCE WITH ASTM F2787, "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS". LOAD CONFIGURATIONS SHALL INCLUDE: 1) INSTANTANEOUS (<1 MIN) AASHTO DESIGN TRUCK LIVE LOAD ON MINIMUM COVER 2) MAXIMUM PERMANENT (75-YR) COVER LOAD AND 3) ALLOWABLE COVER WITH PARKED (1-WEEK) AASHTO DESIGN TRUCK.
7. REQUIREMENTS FOR HANDLING AND INSTALLATION:
   - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
   - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 2".
   - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT SHALL BE GREATER THAN OR EQUAL TO 550 LBS/FT/%. THE ASC IS DEFINED IN SECTION 6.2.8 OF ASTM F2418. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.
8. ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. UPON REQUEST BY THE SITE DESIGN ENGINEER OR OWNER, THE CHAMBER MANUFACTURER SHALL SUBMIT A STRUCTURAL EVALUATION FOR APPROVAL BEFORE DELIVERING CHAMBERS TO THE PROJECT SITE AS FOLLOWS:
   - THE STRUCTURAL EVALUATION SHALL BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER.
   - THE STRUCTURAL EVALUATION SHALL DEMONSTRATE THAT THE SAFETY FACTORS ARE GREATER THAN OR EQUAL TO 1.95 FOR DEAD LOAD AND 1.75 FOR LIVE LOAD, THE MINIMUM REQUIRED BY ASTM F2787 AND BY SECTIONS 3 AND 12.12 OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS FOR THERMOPLASTIC PIPE.
   - THE TEST DERIVED CREEP MODULUS AS SPECIFIED IN ASTM F2418 SHALL BE USED FOR PERMANENT DEAD LOAD DESIGN EXCEPT THAT IT SHALL BE THE 75-YEAR MODULUS USED FOR DESIGN.
9. CHAMBERS AND END CAPS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY.
10. MANIFOLD SIZE TO BE DETERMINED BY SITE DESIGN ENGINEER. SEE TECH NOTE #6.32 FOR MANIFOLD SIZING GUIDANCE. DUE TO THE ADAPTATION OF THIS CHAMBER SYSTEM TO SPECIFIC SITE AND DESIGN CONSTRAINTS, IT MAY BE NECESSARY TO CUT AND COUPLE ADDITIONAL PIPE TO STANDARD MANIFOLD COMPONENTS IN THE FIELD.
11. ADS DOES NOT DESIGN OR PROVIDE MEMBRANE LINER SYSTEMS. TO MINIMIZE THE LEAKAGE POTENTIAL OF LINER SYSTEMS, THE MEMBRANE LINER SYSTEM SHOULD BE DESIGNED BY A KNOWLEDGEABLE GEOTEXTILE PROFESSIONAL AND INSTALLED BY A QUALIFIED CONTRACTOR.

IMPORTANT - NOTES FOR THE BIDDING AND INSTALLATION OF THE SC-800 SYSTEM

1. STORMTECH SC-800 CHAMBERS SHALL NOT BE INSTALLED UNTIL THE MANUFACTURER'S REPRESENTATIVE HAS COMPLETED A PRE-CONSTRUCTION MEETING WITH THE INSTALLERS.
2. STORMTECH SC-800 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/SC-800/DC-780 CONSTRUCTION GUIDE".
3. CHAMBERS ARE NOT TO BE BACKFILLED WITH A DOZER OR AN EXCAVATOR SITUATED OVER THE CHAMBERS. STORMTECH RECOMMENDS 3 BACKFILL METHODS:
   - STONESHOOTER LOCATED OFF THE CHAMBER BED.
   - BACKFILL AS ROWS ARE BUILT USING AN EXCAVATOR ON THE FOUNDATION STONE OR SUBGRADE.
   - BACKFILL FROM OUTSIDE THE EXCAVATION USING A LONG BOOM HOE OR EXCAVATOR.
4. THE FOUNDATION STONE SHALL BE LEVELED AND COMPACTED PRIOR TO PLACING CHAMBERS.
5. JOINTS BETWEEN CHAMBERS SHALL BE PROPERLY SEATED PRIOR TO PLACING STONE.
6. MAINTAIN MINIMUM - 6" (150 mm) SPACING BETWEEN THE CHAMBER ROWS.
7. EMBEDMENT STONE SURROUNDING CHAMBERS MUST BE A CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE: AASHTO M43 #3, 357, 4, 467, 5, 56, OR 57.
8. THE CONTRACTOR MUST REPORT ANY DISCREPANCIES WITH CHAMBER FOUNDATION MATERIALS BEARING CAPACITIES TO THE SITE DESIGN ENGINEER.
9. ADS RECOMMENDS THE USE OF "FLEXSTORM CATCH IT" INSERTS DURING CONSTRUCTION FOR ALL INLETS TO PROTECT THE SUBSURFACE STORMWATER MANAGEMENT SYSTEM FROM CONSTRUCTION SITE RUNOFF.

NOTES FOR CONSTRUCTION EQUIPMENT

1. STORMTECH SC-800 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/SC-800/DC-780 CONSTRUCTION GUIDE".
2. THE USE OF CONSTRUCTION EQUIPMENT OVER SC-800 CHAMBERS IS LIMITED:
   - NO EQUIPMENT IS ALLOWED ON BARE CHAMBERS.
   - NO RUBBER TIRED LOADERS, DUMP TRUCKS, OR EXCAVATORS ARE ALLOWED UNTIL PROPER FILL DEPTHS ARE REACHED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/SC-800/DC-780 CONSTRUCTION GUIDE".
   - WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT CAN BE FOUND IN THE "STORMTECH SC-310/SC-740/SC-800/DC-780 CONSTRUCTION GUIDE".
3. FULL 36" (900 mm) OF STABILIZED COVER MATERIALS OVER THE CHAMBERS IS REQUIRED FOR DUMP TRUCK TRAVEL OR DUMPING.

USE OF A DOZER TO PUSH EMBEDMENT STONE BETWEEN THE ROWS OF CHAMBERS MAY CAUSE DAMAGE TO THE CHAMBERS AND IS NOT AN ACCEPTABLE BACKFILL METHOD. ANY CHAMBERS DAMAGED BY THE "DUMP AND PUSH" METHOD ARE NOT COVERED UNDER THE STORMTECH STANDARD WARRANTY.

CONTACT STORMTECH AT 1-800-821-6710 WITH ANY QUESTIONS ON INSTALLATION REQUIREMENTS OR WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT.

©2025 ADS, INC

PROPOSED LAYOUT

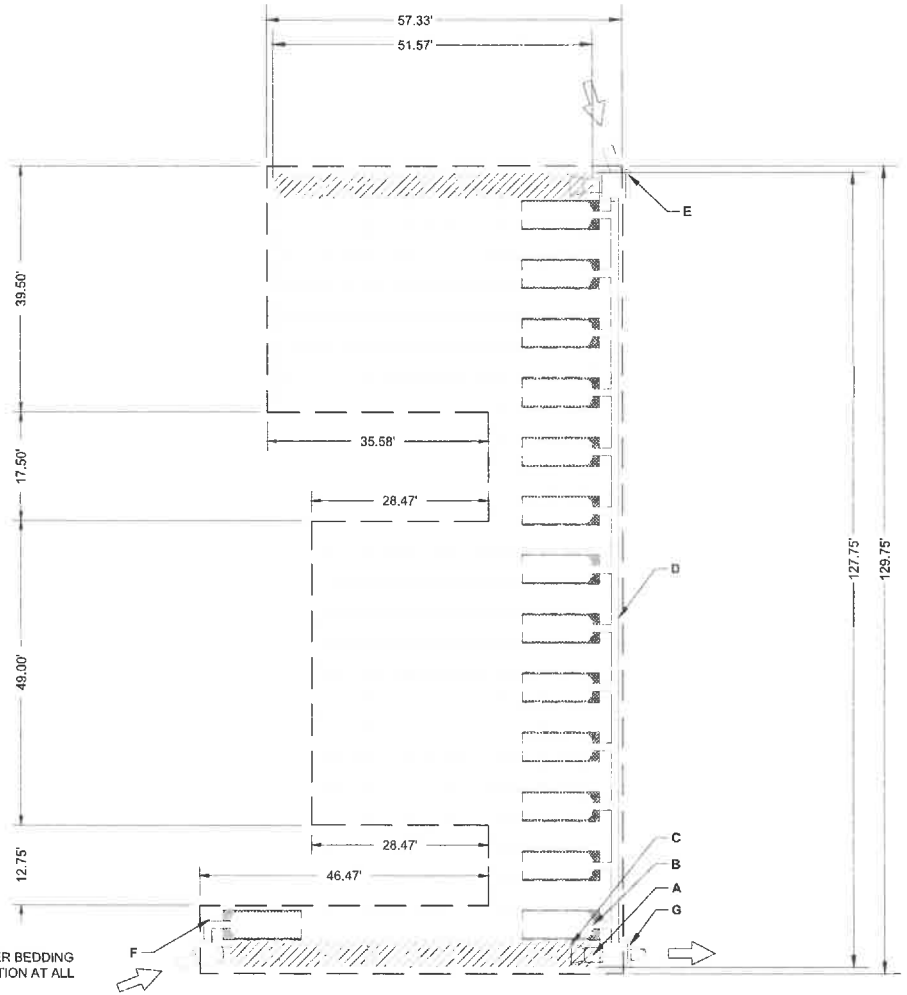
146	STORMTECH SC-800 CHAMBERS
54	STORMTECH SC-800 END CAPS
6	STONE ABOVE (in)
9	STONE BELOW (in)
40	% STONE VOID
12929	**INSTALLED SYSTEM VOLUME (CF) ABOVE INVERT 21.00 (PERIMETER STONE INCLUDED)**
6,134	SYSTEM AREA (ft²)
524	SYSTEM PERIMETER (ft)

PROPOSED ELEVATIONS

31.92	MAXIMUM ALLOWABLE GRADE (TOP OF PAVEMENT/UNPAVED)
25.67	MINIMUM ALLOWABLE GRADE (UNPAVED WITH TRAFFIC)
25.17	MINIMUM ALLOWABLE GRADE (UNPAVED NO TRAFFIC)
25.17	MINIMUM ALLOWABLE GRADE (BASE OF FLEXIBLE PAVEMENT)
25.17	MINIMUM ALLOWABLE GRADE (TOP OF RIGID PAVEMENT)
24.42	TOP OF STONE
23.92	TOP OF SC-800 CHAMBER
22.37	12" TOP MANIFOLD/CONNECTION INVERT
21.36	24" ISOLATOR ROW PLUS CONNECTION INVERT
21.17	BOTTOM OF SC-800 CHAMBER
20.42	BOTTOM OF STONE

*INVERT ABOVE BASE OF CHAMBER

PART TYPE	ITEM ON LAYOUT	DESCRIPTION	INVERT*	MAX FLOW
PRE-CORED END CAP	A	24" BOTTOM PRE-CORED END CAP, PART#: SC800EPE24BPC / TYP OF ALL 24" BOTTOM CONNECTIONS AND ISOLATOR PLUS ROWS	2.30"	
PRE-CORED END CAP	B	12" TOP PRE-CORED END CAP, PART#: SC800EPE12TPC / TYP OF ALL 12" TOP CONNECTIONS	14.40"	
FLAMP	C	INSTALL FLAMP ON 24" ACCESS PIPE / PART#: SC74024RAMP (TYP 2 PLACES)		
MANIFOLD	D	12" x 12" TOP MANIFOLD, ADS N-12	14.40"	4.6 CFS IN
CONCRETE STRUCTURE	E	(DESIGN BY ENGINEER / PROVIDED BY OTHERS)		
CONCRETE STRUCTURE	F	(DESIGN BY ENGINEER / PROVIDED BY OTHERS)		2.3 CFS IN
CONCRETE STRUCTURE	G	(DESIGN BY ENGINEER / PROVIDED BY OTHERS)		2.0 CFS OUT



ISOLATOR ROW PLUS (SEE DETAIL)

PLACE MINIMUM 12.50' OF ADSPLUS625 WOVEN GEOTEXTILE OVER BEDDING STONE AND UNDERNEATH CHAMBER FEET FOR SCOUR PROTECTION AT ALL CHAMBER INLET ROWS

BED LIMITS

CHICK-FIL-A STORE #05587
LEHIGH ACRES, FL, USA

DATE: 04/02/2025 | SVO
PROJECT #: S450292 | CHECKED: XXX

DATE	DRW	CHK	DESCRIPTION
04/09/25	AMC	---	REVISED AS PER EPM NOTES

THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS/STORMTECH UNDER THE DIRECTION OF THE PROJECT ENGINEER OF RECORD (EOR) OR OTHER PROJECT REPRESENTATIVE. THIS DRAWING IS NOT INTENDED FOR USE IN BIDDING OR CONSTRUCTION WITHOUT THE EOR'S PRIOR APPROVAL. EOR SHALL REVIEW THIS DRAWING PRIOR TO BIDDING AND/OR CONSTRUCTION. IT IS THE ULTIMATE RESPONSIBILITY OF THE EOR TO ENSURE THAT THE PRODUCT(S) DEPICTED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE

ADS
4640 TRUEMAN BLVD
HILLIARD, OH 43026
1-800-733-7473

StormTech®
Chamber System

1-800-821-6710 | WWW.STORMTECH.COM

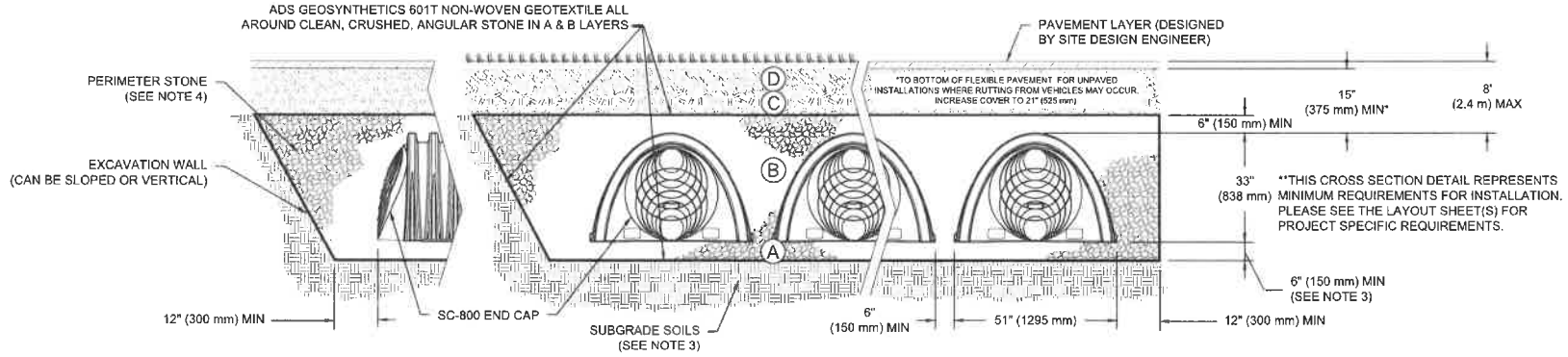
0 20 40

SHEET
2 OF 5

ACCEPTABLE FILL MATERIALS: STORMTECH SC-800 CHAMBER SYSTEMS

MATERIAL LOCATION		DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	**FINAL FILL:** FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER.	ANY SOIL/ROCK MATERIALS, NATIVE SOILS, OR PER ENGINEER'S PLANS. CHECK PLANS FOR PAVEMENT SUBGRADE REQUIREMENTS.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
C	**INITIAL FILL:** FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 15" (375 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	GRANULAR WELL-GRADED SOIL/AGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE. MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER.	AASHTO M145[1] A-1, A-2-4, A-3 OR AASHTO M43[1] 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 12" (300 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 6" (150 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. ROLLER GROSS VEHICLE WEIGHT NOT TO EXCEED 12,000 lbs (53 kN). DYNAMIC FORCE NOT TO EXCEED 20,000 lbs (89 kN).
B	**EMBEDMENT STONE:** FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE.	CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE[5]	AASHTO M43[1] 3, 357, 4, 467, 5, 56, 57	NO COMPACTION REQUIRED.
A	**FOUNDATION STONE:** FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE[5]	AASHTO M43[1] 3, 357, 4, 467, 5, 56, 57	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE.[2,3]

PLEASE NOTE:
1. THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
2. STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (150 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
3. WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.
4. ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.
5. WHERE RECYCLED CONCRETE AGGREGATE IS USED IN LAYERS 'A' OR 'B' THE MATERIAL SHOULD ALSO MEET THE ACCEPTABILITY CRITERIA OUTLINED IN TECHNICAL NOTE 6.20 "RECYCLED CONCRETE STRUCTURAL BACKFILL".



NOTES:

1. CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
2. SC-800 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS"
3. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS. REFERENCE STORMTECH DESIGN MANUAL FOR BEARING CAPACITY GUIDANCE.
4. PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
5. REQUIREMENTS FOR HANDLING AND INSTALLATION:
   - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
   - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 2".
   - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN SECTION 6.2.8 OF ASTM F2418 SHALL BE GREATER THAN OR EQUAL TO 550 LBS/FT/%. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.

CHICK-FIL-A STORE #05587
LEHIGH ACRES, FL, USA
DATE: 04/02/2025 | SVO
PROJECT #: 5450292 | CHECKED: XXX

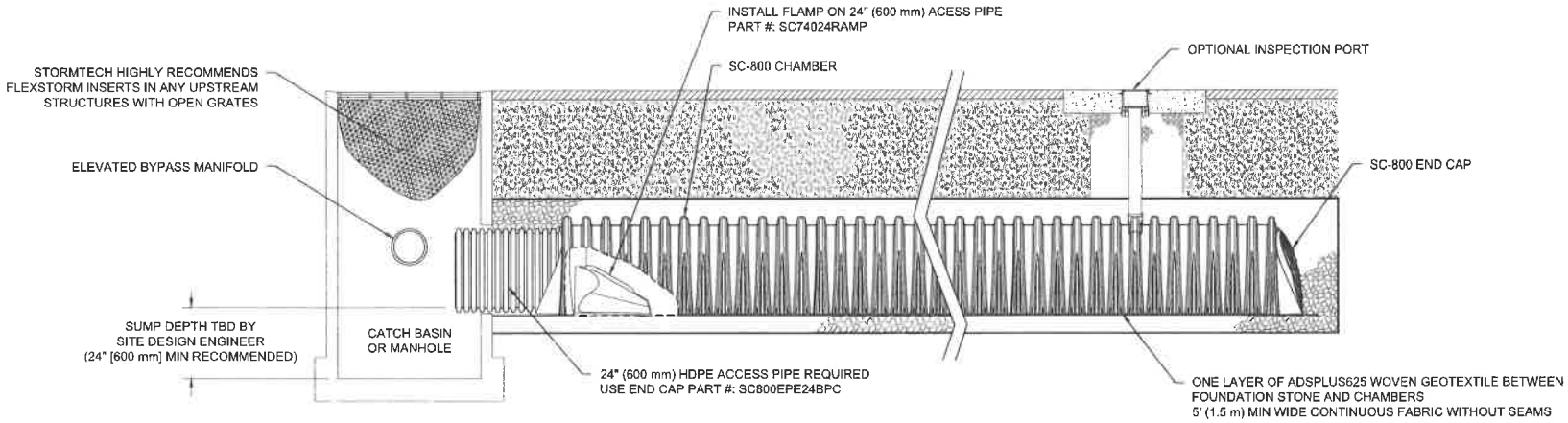
DATE	DRW	CHK	DESCRIPTION
04/09/25	AMC	--	REVISED AS PER EPM NOTES.

StormTech®
Chamber System
1-800-821-6710 | WWW.STORMTECH.COM

ADS
4640 TRUEMAN BLVD
HILLIARD, OH 43026
1-800-733-7473

SHEET
3 OF 5

THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS/STORMTECH UNDER THE DIRECTION OF THE PROJECT ENGINEER. THE DRAWING IS NOT INTENDED FOR USE IN BIDDING OR CONSTRUCTION WITHOUT THE ENGINEER'S AND OWNER'S APPROVAL. ENGINEER SHALL REVIEW THIS DRAWING PRIOR TO BIDDING AND/OR CONSTRUCTION. IT IS THE ULTIMATE RESPONSIBILITY OF THE EOR TO ENSURE THAT THE PRODUCT(S) SPECIFIED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE CODES AND REGULATIONS FOR THE PROJECT. ADS ASSUMES NO RESPONSIBILITY FOR ENGINEERING DESIGN, ADEQUACY OR PERFORMANCE OF THE STORMWATER MANAGEMENT SYSTEM OR OTHER ENGINEERED SYSTEMS ASSOCIATED WITH THE PROJECT. THIS DRAWING IS THE PROPERTY OF ADS AND IS TO BE USED FOR REFERENCE AND INFORMATION ONLY IN THE DESIGN AND CONSTRUCTION OF THE PROJECT. IT CANNOT BE ASSIGNED TO ANY THIRD PARTY OR REPRODUCED IN ANY FORM WITHOUT WRITTEN PERMISSION FROM ADS.



SC-800 ISOLATOR ROW PLUS DETAIL

NTS

INSPECTION & MAINTENANCE

STEP 1) INSPECT ISOLATOR ROW PLUS FOR SEDIMENT
- A. INSPECTION PORTS (IF PRESENT)
  - A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
  - A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
  - A.3. USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
  - A.4. LOWER A CAMERA INTO ISOLATOR ROW PLUS FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
  - A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- B. ALL ISOLATOR PLUS ROWS
  - B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW PLUS
  - B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW PLUS THROUGH OUTLET PIPE
    - i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
    - ii) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
  - B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.

STEP 2) CLEAN OUT ISOLATOR ROW PLUS USING THE JETVAC PROCESS
- A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45" (1.1 m) OR MORE IS PREFERRED
- B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
- C. VACUUM STRUCTURE SUMP AS REQUIRED

STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.

STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

NOTES

1. INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
2. CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.

CHICK-FIL-A STORE #05587
LEHIGH ACRES, FL, USA

DATE: 04/02/2025 | SVO
PROJECT #: S450292 | CHECKED: XXX

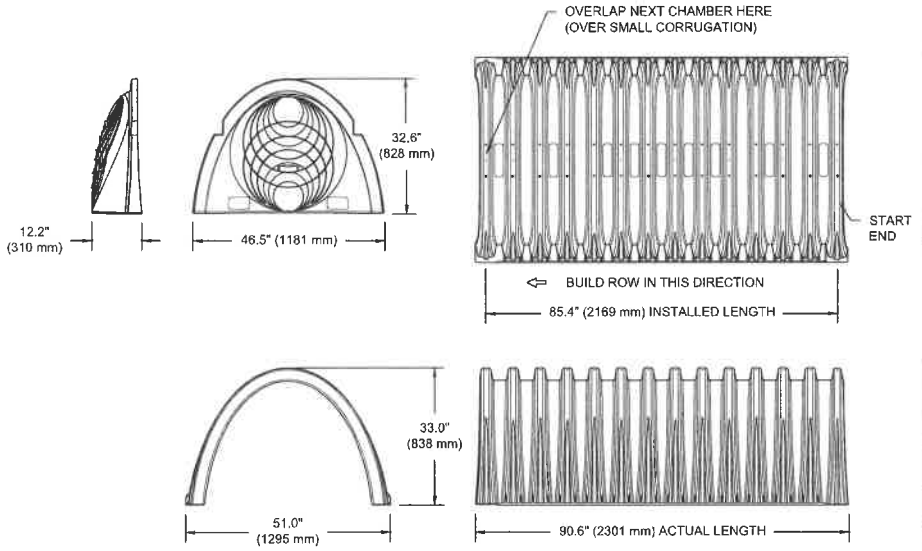
DATE	DRW	CHK	DESCRIPTION
04/09/25	AMC	---	REVISED AS PER EPM NOTES

THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS STORMTECH UNDER THE DIRECTION OF THE PROJECT ENGINEER OF RECORD (EOR) OR OTHER PROJECT REPRESENTATIVE. THIS DRAWING IS NOT INTENDED FOR USE IN BIDDING OR CONSTRUCTION WITHOUT THE EOR'S PRIOR APPROVAL. EOR SHALL REVIEW THIS DRAWING PRIOR TO BIDDING AND/OR CONSTRUCTION. IT IS THE ULTIMATE RESPONSIBILITY OF THE EOR TO ENSURE THAT THE PRODUCT(S) DEPICTED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE LAWS, REGULATIONS AND PROJECT REQUIREMENTS.

StormTech®
Chamber System
1-800-821-6710 | WWW.STORMTECH.COM
4640 TRUEMAN BLVD
HILLIARD, OH 43026
1-800-733-7473

SC-800 TECHNICAL SPECIFICATION

NTS



NOMINAL CHAMBER SPECIFICATIONS

SIZE (W X H X INSTALLED LENGTH)	51.0" X 33.0" X 85.4"	(1295 mm X 838 mm X 2169 mm)
CHAMBER STORAGE	50.6 CUBIC FEET	(1.43 m³)
MINIMUM INSTALLED STORAGE*	81.0 CUBIC FEET	(2.29 m³)
WEIGHT	81.8 lbs.	(37.1 kg)

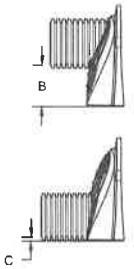
NOMINAL END CAP SPECIFICATIONS

SIZE (W X H X INSTALLED LENGTH)	46.5" X 32.6" X 10.5"	(1181 mm X 828 mm X 267 mm)
END CAP STORAGE	3.4 CUBIC FEET	(0.09 m³)
MINIMUM INSTALLED STORAGE**	15.4 CUBIC FEET	(0.43 m³)
WEIGHT	15.7 lbs.	(7.1 kg)

* ASSUMES 6" (152 mm) STONE ABOVE, BELOW, AND BETWEEN CHAMBERS
**ASSUMES 6" (152 mm) STONE ABOVE AND BELOW END CAPS, 6" (152 mm) BETWEEN ROWS, 12" (305 mm) BEYOND END CAPS

PRE-CORED HOLES AT BOTTOM OF END CAP FOR PART NUMBERS ENDING WITH "BPC"
PRE-CORED HOLES AT TOP OF END CAP FOR PART NUMBERS ENDING WITH "TPC"

PART #	STUB	B	C
SC800EPE06TPC	6" (150 mm)	21.4" (544 mm)	---
SC800EPE06BPC		---	0.9" (23 mm)
SC800EPE08TPC	8" (200 mm)	19.2" (488 mm)	---
SC800EPE08BPC		---	1.0" (25 mm)
SC800EPE10TPC	10" (250 mm)	17.0" (432 mm)	---
SC800EPE10BPC		---	1.2" (30 mm)
SC800EPE12TPC	12" (300 mm)	14.4" (366 mm)	---
SC800EPE12BPC		---	1.6" (41 mm)
SC800EPE15TPC	15" (375 mm)	11.3" (287 mm)	---
SC800EPE15BPC		---	1.7" (43 mm)
SC800EPE18TPC	18" (450 mm)	8.0" (203 mm)	---
SC800EPE18BPC		---	2.0" (51 mm)
SC800EPE24BPC	24" (600 mm)	---	2.3" (58 mm)
SC800EPE	NONE	SOLID END CAP	



NOTE: ALL DIMENSIONS ARE NOMINAL

CHICK-FIL-A STORE #05587
LEHIGH ACRES, FL, USA
DATE: 04/02/2025 | SVO
PROJECT #: S450292 | CHECKED: XXX

DATE	DRW	CHK	DESCRIPTION
04/09/25	AMC	—	REVISED AS PER EPM NOTES.

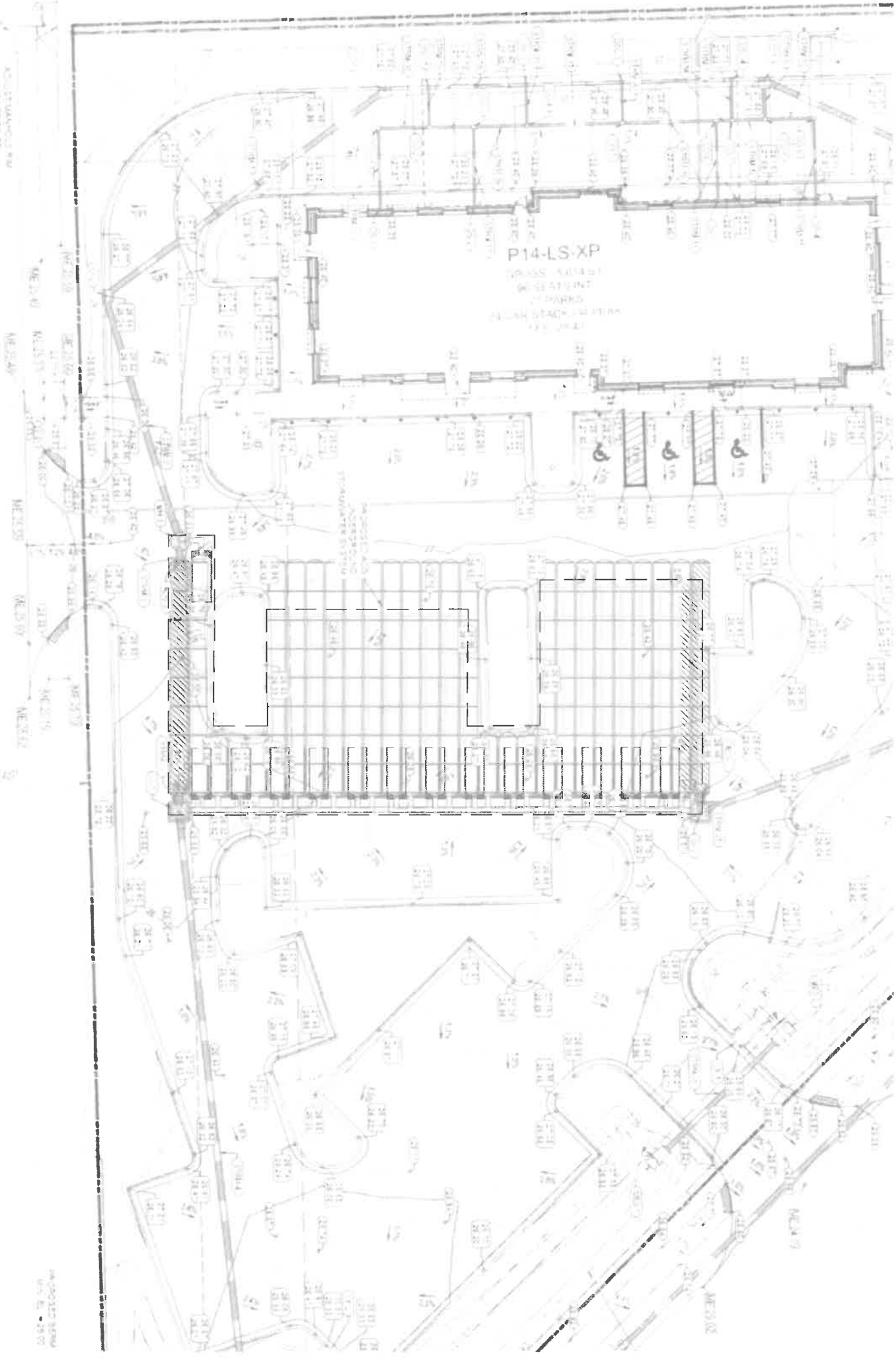
StormTech®
Chamber System

ADS
4640 TRUEMAN BLVD
HILLIARD, OH 43026
1-800-733-7473

1-800-821-6710 | WWW.STORMTECH.COM

THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS BY OTHERS. ADS ASSUMES NO RESPONSIBILITY FOR THE ACCURACY OF THE INFORMATION SHOWN ON THIS DRAWING. THIS DRAWING IS NOT INTENDED FOR USE IN BIDDING OR CONSTRUCTION UNTIL IT HAS BEEN REVIEWED BY THE ENGINEER OF RECORD (EOR) OR OTHER PROJECT REPRESENTATIVE. THIS DRAWING IS THE PROPERTY OF ADS AND SHALL NOT BE REPRODUCED IN ANY FORM WITHOUT PRIOR WRITTEN CONSENT FROM ADS. ADS RESERVES ALL RIGHTS UNDER COPYRIGHT LAWS.

THE ENGINEER OF RECORD IS RESPONSIBLE FOR THE DESIGN OF THE PROJECT, INCLUDING THE DETERMINATION OF THE NUMBER AND LAYOUT OF CHAMBERS AND END CAPS BASED ON SITE SPECIFIC LOADS, FOUNDATION CONDITIONS, GROUNDWATER CONDITIONS, AND ALL OTHER SITE CONDITIONS AFFECTING THE STORMTECH SYSTEM. THE ENGINEER OF RECORD IS RESPONSIBLE FOR REVIEWING THE SUBMITTED SHOP DRAWINGS TO ENSURE THAT THE PROJECT REQUIREMENTS ARE MET. THE ENGINEER OF RECORD IS RESPONSIBLE FOR PREPARING AND STAMPING THE FINAL CONSTRUCTION DRAWINGS FOR THE PROJECT. IT IS THE ULTIMATE RESPONSIBILITY OF THE EOR TO ENSURE THAT THE PRODUCT(S) DEPICTED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE


P14-LS-XP

Project: REV1 Chick - Fil- A Store 05587, FL

Chamber Model -	SC-800	
Units -	Imperial	
Number of Chambers -	146	
Number of End Caps -	54	
Voids in the stone (porosity) -	40	%
Base of Stone Elevation -	20.42	ft
Amount of Stone Above Chambers -	6	in
Amount of Stone Below Chambers -	9	in
Area of system -	6134	sf Min. Area - 5418 sf min. area

Include Perimeter Stone in Calculations

Click for Stage Area Data

Click to Invert Stage Area Data

Click Here for Metric

Interpolation Tools

Click to Define Top and/or Bottom of storage

Click to Define Target Volume & Reference Elevation

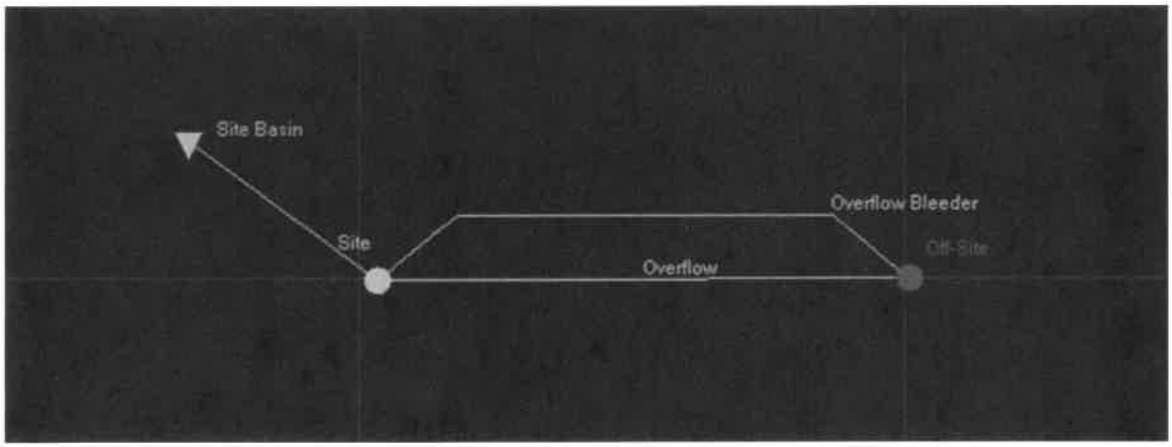
StormTech SC-800 Cumulative Storage Volumes

Height of System (inches)	Incremental Single Chamber (cubic feet)	Incremental Single End Cap (cubic feet)	Incremental Chambers (cubic feet)	Incremental End Cap (cubic feet)	Incremental Stone (cubic feet)	Incremental Ch, EC and Stone (cubic feet)	Cumulative System (cubic feet)	Elevation (feet)
48	0.00	0.00	0.00	0.00	204.47	204.47	14359.87	24.42
47	0.00	0.00	0.00	0.00	204.47	204.47	14155.40	24.34
46	0.00	0.00	0.00	0.00	204.47	204.47	13950.93	24.25
45	0.00	0.00	0.00	0.00	204.47	204.47	13746.47	24.17
44	0.00	0.00	0.00	0.00	204.47	204.47	13542.00	24.09
43	0.00	0.00	0.00	0.00	204.47	204.47	13337.53	24.00
42	0.07	0.00	10.87	0.02	200.11	211.00	13133.07	23.92
41	0.20	0.00	28.73	0.19	192.90	221.82	12922.06	23.84
40	0.29	0.01	41.66	0.33	187.67	229.66	12700.25	23.75
39	0.51	0.01	73.73	0.47	174.79	248.99	12470.58	23.67
38	0.74	0.01	108.13	0.74	160.92	269.79	12221.59	23.59
37	0.89	0.02	130.27	1.31	151.84	283.41	11951.80	23.50
36	1.01	0.03	148.17	1.82	144.47	294.46	11668.39	23.42
35	1.12	0.04	163.54	2.34	138.12	303.99	11373.93	23.34
34	1.21	0.05	177.05	2.85	132.50	312.41	11069.94	23.25
33	1.30	0.06	189.27	3.36	127.42	320.04	10757.52	23.17
32	1.37	0.07	200.35	3.80	122.81	326.96	10437.48	23.09
31	1.44	0.08	210.47	4.22	118.59	333.28	10110.52	23.00
30	1.51	0.09	219.83	4.63	114.68	339.14	9777.24	22.92
29	1.57	0.09	228.55	5.07	111.02	344.64	9438.10	22.84
28	1.62	0.10	236.69	5.52	107.58	349.79	9093.47	22.75
27	1.67	0.11	244.31	5.95	104.36	354.62	8743.67	22.67
26	1.72	0.12	251.47	6.35	101.34	359.16	8389.05	22.59
25	1.77	0.12	258.21	6.69	98.51	363.41	8029.90	22.50
24	1.81	0.13	264.56	6.93	95.87	367.36	7666.49	22.42
23	1.85	0.14	270.55	7.36	93.30	371.21	7299.13	22.34
22	1.89	0.14	276.20	7.72	90.90	374.82	6927.92	22.25
21	1.93	0.15	281.54	7.83	88.72	378.09	6553.10	22.17
20	1.96	0.15	286.59	8.05	86.61	381.25	6175.00	22.09
19	2.00	0.16	291.37	8.42	84.55	384.34	5793.75	22.00
18	2.03	0.16	295.89	8.84	82.58	387.30	5409.41	21.92
17	2.06	0.17	300.16	9.05	80.78	389.99	5022.11	21.84
16	2.08	0.17	304.19	9.25	79.09	392.53	4632.12	21.75
15	2.11	0.18	308.01	9.48	77.47	394.96	4239.59	21.67
14	2.13	0.17	311.61	9.36	76.08	397.05	3844.63	21.59
13	2.16	0.18	315.01	9.80	74.54	399.35	3447.58	21.50
12	2.18	0.18	318.21	9.91	73.22	401.34	3048.23	21.42
11	2.20	0.19	321.23	10.03	71.96	403.22	2646.89	21.34
10	2.22	0.13	324.52	7.14	71.80	403.47	2243.67	21.25
9	0.00	0.00	0.00	0.00	204.47	204.47	1840.20	21.17
8	0.00	0.00	0.00	0.00	204.47	204.47	1635.73	21.09
7	0.00	0.00	0.00	0.00	204.47	204.47	1431.27	21.00
6	0.00	0.00	0.00	0.00	204.47	204.47	1226.80	20.92
5	0.00	0.00	0.00	0.00	204.47	204.47	1022.33	20.84
4	0.00	0.00	0.00	0.00	204.47	204.47	817.87	20.75
3	0.00	0.00	0.00	0.00	204.47	204.47	613.40	20.67
2	0.00	0.00	0.00	0.00	204.47	204.47	408.93	20.59
1	0.00	0.00	0.00	0.00	204.47	204.47	204.47	20.50

Volume above elevation 21 = 12928.6

Bowman

Proposed ICPR Nodal Map



910 SE 17th Street, 3rd Floor, Ft. Lauderdale, Florida 33316
bowman.com

Simple Basin: Site Basin

Scenario:	00-POST
Node:	Site
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	10.0000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH256
Peaking Factor:	256.0
Area:	1.8200 ac
Curve Number:	87.0
Ia/S:	0.00
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Node: Off-Site

Scenario:	00-POST
Type:	Time/Stage
Base Flow:	0.00 cfs
Initial Stage:	23.74 ft
Warning Stage:	0.00 ft
Alert Stage:	0.00 ft
Boundary Stage:	

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	23.74
0	0	0	24.0000	25.23
0	0	0	360.0000	23.74

Comment:

Node: Site

Scenario:	00-POST
Type:	Stage/Volume
Base Flow:	0.00 cfs
Initial Stage:	21.00 ft
Warning Stage:	0.00 ft
Alert Stage:	0.00 ft

Stage [ft]	Volume [ac-ft]	Volume [ft3]
21.00	0.00	0
21.62	0.13	5663
22.23	0.24	10454
22.85	0.33	14375
23.47	0.39	16988
24.08	0.44	19166
24.70	0.50	21780
25.32	0.58	25265
25.93	0.71	30928
26.55	1.03	44867
27.17	1.57	68389
27.78	2.33	101495
28.40	3.30	143748

Comment:

Weir Link: Overflow

Scenario:	00-POST	**Bottom Clip**	
From Node:	Site	Default:	0.00 ft
To Node:	Off-Site	Op Table:	
Link Count:	1	Ref Node:	
Flow Direction:	None	**Top Clip**	
Damping:	0.0000 ft	Default:	0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:	
Geometry Type:	Rectangular	Ref Node:	
Invert:	26.72 ft	**Discharge Coefficients**	
Control Elevation:	26.72 ft	Weir Default:	2.800
Max Depth:	99.00 ft	Weir Table:	
Max Width:	6.33 ft	Orifice Default:	0.600
Fillet:	0.00 ft	Orifice Table:	

Comment:

Weir Link: Overflow Bleeder

Scenario:	00-POST	**Bottom Clip**	
From Node:	Site	Default:	0.00 ft
To Node:	Off-Site	Op Table:	
Link Count:	1	Ref Node:	
Flow Direction:	None	**Top Clip**	
Damping:	0.0000 ft	Default:	0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:	
Geometry Type:	Circular	Ref Node:	
Invert:	25.05 ft	**Discharge Coefficients**	
Control Elevation:	25.05 ft	Weir Default:	2.800
Max Depth:	0.25 ft	Weir Table:	

Orifice Default: 0.600
Orifice Table:

Comment:

Simulation: 005YR-1DY

Scenario: 00-POST
Run Date/Time: 11/3/2025 2:40:57 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	75.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:

Unit Hydrograph Folder:		Curve Number Set:	1
		Green-Ampt Set:	
		Vertical Layers Set:	
		Impervious Set:	1
		Roughness Set:	
		Crop Coef Set:	
		Fillable Porosity Set:	
		Conductivity Set:	
		Leakage Set:	

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6	ET for Manual Basins:	False
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	OF Region Rain Opt:	Global
		Rainfall Name:	~FLMOD
Edge Length Option:	Automatic	Rainfall Amount:	5.50 in
		Storm Duration:	24.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 010YR-1DY

Scenario:	00-POST
Run Date/Time:	11/3/2025 2:40:00 PM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	30.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: 1
Green-Ampt Set:
Vertical Layers Set:
Impervious Set: 1
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft
Edge Length Option: Automatic

IA Recovery Time: 24.0000 hr
ET for Manual Basins: False
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
OF Region Rain Opt: Global
Rainfall Name: ~FLMOD
Rainfall Amount: 6.50 in
Storm Duration: 24.0000 hr

Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 025YR-3DY

Scenario: 00-POST
Run Date/Time: 11/3/2025 2:40:14 PM
Program Version: StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	75.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Lookup Tables

Rainfall Folder:		Boundary Stage Set:	
Reference ET Folder:		Extern Hydrograph Set:	
Unit Hydrograph Folder:		Curve Number Set:	1
		Green-Ampt Set:	
		Vertical Layers Set:	
		Impervious Set:	1
		Roughness Set:	
		Crop Coef Set:	
		Fillable Porosity Set:	
		Conductivity Set:	
		Leakage Set:	

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6	ET for Manual Basins:	False
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	OF Region Rain Opt:	Global
		Rainfall Name:	~SFWMD-72
Edge Length Option:	Automatic	Rainfall Amount:	11.00 in
		Storm Duration:	72.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 100YR-3DY

Scenario:	00-POST
Run Date/Time:	11/3/2025 2:41:34 PM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	75.0000

Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]

Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: 1
Green-Ampt Set:
Vertical Layers Set:
Impervious Set: 1
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr
ET for Manual Basins: False
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
OF Region Rain Opt: Global
Rainfall Name: ~SFWMD-72

Edge Length Option:	Automatic	Rainfall Amount:	14.00 in
		Storm Duration:	72.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simple Basin Runoff Summary [00-POST]

Basin Name	Sim Name	Max Flow [cfs]	Time to Max Flow [hrs]	Total Rainfall [in]	Total Runoff [in]	Area [ac]
Site Basin	005YR-1DY	5.09	12.0500	5.50	4.04	1.8200
Site Basin	010YR-1DY	6.24	12.0500	6.50	4.99	1.8200
Site Basin	025YR-3DY	8.56	60.0167	11.00	9.38	1.8200
Site Basin	100YR-3DY	11.03	60.0167	14.00	12.34	1.8200

Node Max Conditions [00-POST]

Node Name	Sim Name	Warning Stage [ft]	Alert Stage [ft]	Max Stage [ft]	Min/Max Delta Stage [ft]	Max Total Inflow [cfs]	Max Total Outflow [cfs]	Max Surface Area [ft2]
Off-Site	005YR-1DY	0.00	0.00	25.23	0.0005	0.07	0.00	0
Off-Site	010YR-1DY	0.00	0.00	25.23	0.0005	0.15	0.00	0
Off-Site	025YR-3DY	0.00	0.00	25.23	0.0005	0.30	0.04	0
Off-Site	100YR-3DY	0.00	0.00	25.23	0.0005	0.00	0.00	0

Node Max Conditions [00-POST]

Node Name	Sim Name	Warning Stage [ft]	Alert Stage [ft]	Max Stage [ft]	Min/Max Delta Stage [ft]	Max Total Inflow [cfs]	Max Total Outflow [cfs]	Max Surface Area [ft2]
Site	005YR-1DY	0.00	0.00	25.31	0.0010	5.09	0.07	9773
Site	010YR-1DY	0.00	0.00	25.57	0.0010	6.24	0.15	10911
Site	025YR-3DY	0.00	0.00	26.72	0.0010	8.56	0.30	34649
Site	100YR-3DY	0.00	0.00	27.38	0.0010	11.03	0.00	51408

MODEL STAGE RESULTS

Link Min/Max Conditions [00-POST]

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
Overflow	005YR-1DY	0.00	0.00	0.00	0.00	0.00	0.00
Overflow	010YR-1DY	0.00	0.00	0.00	0.00	0.00	0.00
Overflow	025YR-3DY	0.00	0.00	0.00	0.00	0.00	0.00
Overflow	100YR-3DY	0.00	0.00	0.00	0.00	0.00	0.00

Link Min/Max Conditions [00-POST]

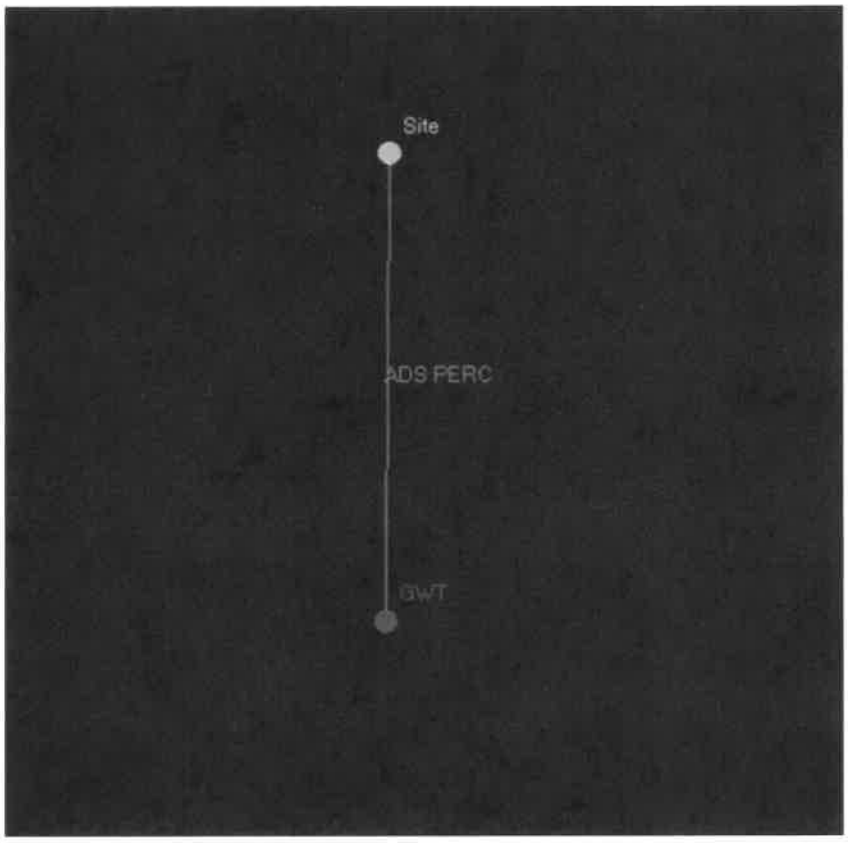
Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
Overflow	005YR-1DY	0.07	0.00	0.00	0.00	0.00	0.00

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
Bleeder							
Overflow Bleeder	010YR-1DY	0.15	0.00	0.00	0.00	0.00	0.00
Overflow Bleeder	025YR-3DY	0.29	-0.04	0.00	0.00	0.00	0.00
Overflow Bleeder	100YR-3DY	0.00	0.00	0.00	0.00	0.00	0.00

25YR-72HR
DISCHARGE RATE
*MIN. 3" BLEEDER
USED

Bowman

Slug Load Recovery ICPR Nodal Map



Node: GWT

Scenario: 00-POST
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 21.00 ft
Warning Stage: 22.00 ft
Alert Stage: 0.00 ft
Boundary Stage:

Comment:

Node: Site

Scenario: 00-POST
Type: Stage/Volume
Base Flow: 0.00 cfs
Initial Stage: 26.72 ft
Warning Stage: 0.00 ft
Alert Stage: 0.00 ft

Stage [ft]	Volume [ac-ft]	Volume [ft3]
21.00	0.00	0
21.62	0.13	5663
22.23	0.24	10454
22.85	0.33	14375
23.47	0.39	16988
24.08	0.44	19166
24.70	0.50	21780
25.32	0.58	25265
25.93	0.71	30928
26.55	1.03	44867
27.17	1.57	68389
27.78	2.33	101495
28.40	3.30	143748

Comment:

Percolation Link: ADS PERC

Scenario: 00-POST
From Node: Site
To Node: GWT
Link Count: 1
Flow Direction: Both
Aquifer Base Elevation: 10.00 ft
Water Table Elevation: 21.00 ft
Surface Area Option: Use 1st Point in Stage/Area Table
Vertical Flow Termination: Constant Rate
Constant Rate: 2.7 fpd

Annual Recharge Rate:	0 ipy
Horizontal Conductivity:	2.725 fpd
Vertical Conductivity:	2.725 fpd
Fillable Porosity:	0.200
Layer Thickness:	0.00 ft

Comment:

Simulation: Recovery

Scenario:	00-POST
Run Date/Time:	11/3/2025 2:59:02 PM
Program Version:	StormWise 4.08.03

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	75.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Lookup Tables

Rainfall Folder:		Boundary Stage Set:	
Reference ET Folder:		Extern Hydrograph Set:	
Unit Hydrograph Folder:		Curve Number Set:	1
		Green-Ampt Set:	
		Vertical Layers Set:	
		Impervious Set:	1
		Roughness Set:	
		Crop Coef Set:	
		Fillable Porosity Set:	
		Conductivity Set:	
		Leakage Set:	

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6	ET for Manual Basins:	False
Over-Relax Weight Fact:	0.5 dec	Ia/S:	0.20 dec
dZ Tolerance:	0.0010 ft		
Max dZ:	1.0000 ft	Smp/Man Basin Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	OF Region Rain Opt:	Global
		Rainfall Name:	~SFWMD-72
Edge Length Option:	Automatic	Rainfall Amount:	0.00 in
		Storm Duration:	72.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Rate [cfs]
Recovery	Site	0.0000	26.72	0.00
Recovery	Site	0.2511	26.71	0.31
Recovery	Site	0.5050	26.70	0.31
Recovery	Site	0.7527	26.70	0.31
Recovery	Site	1.0027	26.69	0.31
Recovery	Site	1.2527	26.68	0.31
Recovery	Site	1.5027	26.67	0.31
Recovery	Site	1.7527	26.66	0.31
Recovery	Site	2.0027	26.65	0.31
Recovery	Site	2.2527	26.65	0.31
Recovery	Site	2.5027	26.64	0.31
Recovery	Site	2.7527	26.63	0.31
Recovery	Site	3.0027	26.62	0.31
Recovery	Site	3.2527	26.61	0.31
Recovery	Site	3.5027	26.60	0.31
Recovery	Site	3.7527	26.59	0.31
Recovery	Site	4.0027	26.58	0.31
Recovery	Site	4.2527	26.58	0.31
Recovery	Site	4.5027	26.57	0.31
Recovery	Site	4.7527	26.56	0.31
Recovery	Site	5.0027	26.55	0.31
Recovery	Site	5.2527	26.54	0.31
Recovery	Site	5.5027	26.53	0.31
Recovery	Site	5.7527	26.52	0.31
Recovery	Site	6.0027	26.51	0.31
Recovery	Site	6.2527	26.50	0.31
Recovery	Site	6.5027	26.49	0.31
Recovery	Site	6.7527	26.48	0.31
Recovery	Site	7.0027	26.47	0.31
Recovery	Site	7.2527	26.46	0.31
Recovery	Site	7.5027	26.45	0.31
Recovery	Site	7.7527	26.44	0.31
Recovery	Site	8.0027	26.43	0.31
Recovery	Site	8.2527	26.42	0.31
Recovery	Site	8.5027	26.41	0.31
Recovery	Site	8.7527	26.40	0.31
Recovery	Site	9.0027	26.39	0.31
Recovery	Site	9.2527	26.38	0.31
Recovery	Site	9.5027	26.37	0.31
Recovery	Site	9.7527	26.36	0.31
Recovery	Site	10.0027	26.35	0.31
Recovery	Site	10.2527	26.34	0.31

25YR-72HR MAX STAGE

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Rate [cfs]
Recovery	Site	10.5027	26.33	0.31
Recovery	Site	10.7527	26.32	0.31
Recovery	Site	11.0027	26.31	0.31
Recovery	Site	11.2527	26.29	0.31
Recovery	Site	11.5027	26.28	0.31
Recovery	Site	11.7527	26.27	0.31
Recovery	Site	12.0027	26.26	0.31
Recovery	Site	12.2527	26.25	0.31
Recovery	Site	12.5027	26.24	0.31
Recovery	Site	12.7527	26.22	0.31
Recovery	Site	13.0027	26.21	0.31
Recovery	Site	13.2527	26.20	0.31
Recovery	Site	13.5027	26.19	0.31
Recovery	Site	13.7527	26.17	0.31
Recovery	Site	14.0027	26.16	0.31
Recovery	Site	14.2527	26.15	0.31
Recovery	Site	14.5027	26.13	0.31
Recovery	Site	14.7527	26.12	0.31
Recovery	Site	15.0027	26.11	0.31
Recovery	Site	15.2527	26.09	0.31
Recovery	Site	15.5027	26.08	0.31
Recovery	Site	15.7527	26.06	0.31
Recovery	Site	16.0027	26.05	0.31
Recovery	Site	16.2527	26.03	0.31
Recovery	Site	16.5027	26.02	0.31
Recovery	Site	16.7527	26.00	0.31
Recovery	Site	17.0027	25.99	0.31
Recovery	Site	17.2527	25.97	0.31
Recovery	Site	17.5027	25.95	0.31
Recovery	Site	17.7527	25.94	0.31
Recovery	Site	18.0027	25.92	0.31
Recovery	Site	18.2527	25.90	0.31
Recovery	Site	18.5027	25.88	0.31
Recovery	Site	18.7527	25.86	0.31
Recovery	Site	19.0027	25.85	0.31
Recovery	Site	19.2527	25.83	0.31
Recovery	Site	19.5027	25.81	0.31
Recovery	Site	19.7527	25.79	0.31
Recovery	Site	20.0027	25.77	0.31
Recovery	Site	20.2527	25.75	0.31
Recovery	Site	20.5027	25.73	0.31
Recovery	Site	20.7527	25.70	0.31

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Rate [cfs]
Recovery	Site	21.0027	25.68	0.31
Recovery	Site	21.2527	25.66	0.31
Recovery	Site	21.5027	25.64	0.31
Recovery	Site	21.7527	25.61	0.31
Recovery	Site	22.0027	25.59	0.31
Recovery	Site	22.2527	25.56	0.31
Recovery	Site	22.5027	25.54	0.31
Recovery	Site	22.7527	25.51	0.31
Recovery	Site	23.0027	25.48	0.31
Recovery	Site	23.2527	25.45	0.31
Recovery	Site	23.5011	25.42	0.31
Recovery	Site	23.7561	25.39	0.31
Recovery	Site	24.0026	25.36	0.31
Recovery	Site	24.2515	25.32	0.31
Recovery	Site	24.5037	25.28	0.31
Recovery	Site	24.7540	25.24	0.31
Recovery	Site	25.0038	25.21	0.31
Recovery	Site	25.2531	25.17	0.31
Recovery	Site	25.5007	25.12	0.31
Recovery	Site	25.7542	25.08	0.31
Recovery	Site	26.0015	25.04	0.31
Recovery	Site	26.2509	24.99	0.31
Recovery	Site	26.5007	24.95	0.31
Recovery	Site	26.7529	24.90	0.31
Recovery	Site	27.0036	24.85	0.31
Recovery	Site	27.2522	24.80	0.31
Recovery	Site	27.5017	24.75	0.31
Recovery	Site	27.7501	24.69	0.31
Recovery	Site	28.0035	24.63	0.31
Recovery	Site	28.2525	24.58	0.31
Recovery	Site	28.5013	24.52	0.31
Recovery	Site	28.7509	24.46	0.31
Recovery	Site	29.0026	24.39	0.31
Recovery	Site	29.2514	24.33	0.31
Recovery	Site	29.5018	24.26	0.31
Recovery	Site	29.7526	24.20	0.31
Recovery	Site	30.0017	24.13	0.31
Recovery	Site	30.2509	24.06	0.31
Recovery	Site	30.5025	23.99	0.31
Recovery	Site	30.7521	23.92	0.31
Recovery	Site	31.0002	23.84	0.31
Recovery	Site	31.2528	23.77	0.31

TIME TO RECOVER TO BLEEDER ELEVATION

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Rate [cfs]
Recovery	Site	31.5012	23.70	0.31
Recovery	Site	31.7504	23.63	0.31
Recovery	Site	32.0020	23.56	0.31
Recovery	Site	32.2520	23.49	0.31
Recovery	Site	32.5011	23.42	0.31
Recovery	Site	32.7530	23.35	0.31
Recovery	Site	33.0010	23.28	0.31
Recovery	Site	33.2504	23.22	0.31
Recovery	Site	33.5006	23.16	0.31
Recovery	Site	33.7531	23.10	0.31
Recovery	Site	34.0017	23.04	0.31
Recovery	Site	34.2539	22.98	0.31
Recovery	Site	34.5030	22.93	0.31
Recovery	Site	34.7535	22.88	0.31
Recovery	Site	35.0046	22.82	0.31
Recovery	Site	35.2548	22.77	0.31
Recovery	Site	35.5043	22.72	0.31
Recovery	Site	35.7512	22.67	0.31
Recovery	Site	36.0012	22.63	0.31
Recovery	Site	36.2518	22.58	0.31
Recovery	Site	36.5019	22.53	0.31
Recovery	Site	36.7522	22.49	0.31
Recovery	Site	37.0023	22.45	0.31
Recovery	Site	37.2526	22.40	0.31
Recovery	Site	37.5042	22.36	0.31
Recovery	Site	37.7502	22.32	0.31
Recovery	Site	38.0051	22.28	0.31
Recovery	Site	38.2561	22.24	0.31
Recovery	Site	38.5011	22.20	0.31
Recovery	Site	38.7524	22.16	0.31
Recovery	Site	39.0013	22.13	0.31
Recovery	Site	39.2510	22.09	0.31
Recovery	Site	39.5010	22.05	0.31
Recovery	Site	39.7512	22.01	0.31
Recovery	Site	40.0011	21.98	0.31
Recovery	Site	40.2506	21.94	0.31
Recovery	Site	40.5061	21.91	0.31
Recovery	Site	40.7521	21.87	0.31
Recovery	Site	41.0042	21.84	0.31
Recovery	Site	41.2508	21.80	0.31
Recovery	Site	41.5034	21.77	0.31
Recovery	Site	41.7559	21.73	0.31

TIME FOR RETETNION AREAS TO RECOVER TO BOTTOM

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Rate [cfs]
Recovery	Site	42.0007	21.70	0.31
Recovery	Site	42.2562	21.67	0.31
Recovery	Site	42.5015	21.63	0.31
Recovery	Site	42.7507	21.60	0.31
Recovery	Site	43.0007	21.57	0.31
Recovery	Site	43.2548	21.54	0.31
Recovery	Site	43.5007	21.51	0.31
Recovery	Site	43.7557	21.47	0.31
Recovery	Site	44.0057	21.44	0.31
Recovery	Site	44.2540	21.41	0.31
Recovery	Site	44.5073	21.38	0.31
Recovery	Site	44.7540	21.35	0.31
Recovery	Site	45.0040	21.32	0.31
Recovery	Site	45.2532	21.29	0.31
Recovery	Site	45.5065	21.26	0.31
Recovery	Site	45.7523	21.23	0.31
Recovery	Site	46.0023	21.20	0.31
Recovery	Site	46.2523	21.17	0.31
Recovery	Site	46.5023	21.14	0.31
Recovery	Site	46.7523	21.11	0.31
Recovery	Site	47.0023	21.08	0.31
Recovery	Site	47.2523	21.05	0.31
Recovery	Site	47.5023	21.02	0.31
Recovery	Site	47.7523	21.00	0.00
Recovery	Site	48.0023	21.00	0.00
Recovery	Site	48.2523	21.00	0.00
Recovery	Site	48.5023	21.00	0.00
Recovery	Site	48.7523	21.00	0.00
Recovery	Site	49.0023	21.00	0.00
Recovery	Site	49.2523	21.00	0.00
Recovery	Site	49.5023	21.00	0.00
Recovery	Site	49.7523	21.00	0.00
Recovery	Site	50.0023	21.00	0.00
Recovery	Site	50.2523	21.00	0.00
Recovery	Site	50.5023	21.00	0.00
Recovery	Site	50.7523	21.00	0.00
Recovery	Site	51.0023	21.00	0.00
Recovery	Site	51.2523	21.00	0.00
Recovery	Site	51.5023	21.00	0.00
Recovery	Site	51.7523	21.00	0.00
Recovery	Site	52.0023	21.00	0.00
Recovery	Site	52.2523	21.00	0.00

TIME FOR UNDERGROUND CHAMBERS AND SITE TO RECOVER TO CONTROL ELEVATION

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Rate [cfs]
Recovery	Site	52.5023	21.00	0.00
Recovery	Site	52.7523	21.00	0.00
Recovery	Site	53.0023	21.00	0.00
Recovery	Site	53.2523	21.00	0.00
Recovery	Site	53.5023	21.00	0.00
Recovery	Site	53.7523	21.00	0.00
Recovery	Site	54.0023	21.00	0.00
Recovery	Site	54.2523	21.00	0.00
Recovery	Site	54.5023	21.00	0.00
Recovery	Site	54.7523	21.00	0.00
Recovery	Site	55.0023	21.00	0.00
Recovery	Site	55.2523	21.00	0.00
Recovery	Site	55.5023	21.00	0.00
Recovery	Site	55.7523	21.00	0.00
Recovery	Site	56.0023	21.00	0.00
Recovery	Site	56.2523	21.00	0.00
Recovery	Site	56.5023	21.00	0.00
Recovery	Site	56.7523	21.00	0.00
Recovery	Site	57.0023	21.00	0.00
Recovery	Site	57.2523	21.00	0.00
Recovery	Site	57.5023	21.00	0.00
Recovery	Site	57.7523	21.00	0.00
Recovery	Site	58.0023	21.00	0.00
Recovery	Site	58.2523	21.00	0.00
Recovery	Site	58.5023	21.00	0.00
Recovery	Site	58.7523	21.00	0.00
Recovery	Site	59.0023	21.00	0.00
Recovery	Site	59.2523	21.00	0.00
Recovery	Site	59.5023	21.00	0.00
Recovery	Site	59.7523	21.00	0.00
Recovery	Site	60.0023	21.00	0.00
Recovery	Site	60.2523	21.00	0.00
Recovery	Site	60.5023	21.00	0.00
Recovery	Site	60.7523	21.00	0.00
Recovery	Site	61.0023	21.00	0.00
Recovery	Site	61.2523	21.00	0.00
Recovery	Site	61.5023	21.00	0.00
Recovery	Site	61.7523	21.00	0.00
Recovery	Site	62.0023	21.00	0.00
Recovery	Site	62.2523	21.00	0.00
Recovery	Site	62.5023	21.00	0.00
Recovery	Site	62.7523	21.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Rate [cfs]
Recovery	Site	63.0023	21.00	0.00
Recovery	Site	63.2523	21.00	0.00
Recovery	Site	63.5023	21.00	0.00
Recovery	Site	63.7523	21.00	0.00
Recovery	Site	64.0023	21.00	0.00
Recovery	Site	64.2523	21.00	0.00
Recovery	Site	64.5023	21.00	0.00
Recovery	Site	64.7523	21.00	0.00
Recovery	Site	65.0023	21.00	0.00
Recovery	Site	65.2523	21.00	0.00
Recovery	Site	65.5023	21.00	0.00
Recovery	Site	65.7523	21.00	0.00
Recovery	Site	66.0023	21.00	0.00
Recovery	Site	66.2523	21.00	0.00
Recovery	Site	66.5023	21.00	0.00
Recovery	Site	66.7523	21.00	0.00
Recovery	Site	67.0023	21.00	0.00
Recovery	Site	67.2523	21.00	0.00
Recovery	Site	67.5023	21.00	0.00
Recovery	Site	67.7523	21.00	0.00
Recovery	Site	68.0023	21.00	0.00
Recovery	Site	68.2523	21.00	0.00
Recovery	Site	68.5023	21.00	0.00
Recovery	Site	68.7523	21.00	0.00
Recovery	Site	69.0023	21.00	0.00
Recovery	Site	69.2523	21.00	0.00
Recovery	Site	69.5023	21.00	0.00
Recovery	Site	69.7523	21.00	0.00
Recovery	Site	70.0023	21.00	0.00
Recovery	Site	70.2523	21.00	0.00
Recovery	Site	70.5023	21.00	0.00
Recovery	Site	70.7523	21.00	0.00
Recovery	Site	71.0023	21.00	0.00
Recovery	Site	71.2523	21.00	0.00
Recovery	Site	71.5023	21.00	0.00
Recovery	Site	71.7523	21.00	0.00
Recovery	Site	72.0023	21.00	0.00
Recovery	Site	72.2523	21.00	0.00
Recovery	Site	72.5023	21.00	0.00
Recovery	Site	72.7523	21.00	0.00
Recovery	Site	73.0023	21.00	0.00
Recovery	Site	73.2523	21.00	0.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]	Total Outflow Rate [cfs]
Recovery	Site	73.5023	21.00	0.00
Recovery	Site	73.7523	21.00	0.00
Recovery	Site	74.0023	21.00	0.00
Recovery	Site	74.2523	21.00	0.00
Recovery	Site	74.5023	21.00	0.00
Recovery	Site	74.7523	21.00	0.00
Recovery	Site	75.0023	21.00	0.00

PART IV #26-40

Recommended Action

Engineering Review:

By: _____ 3/4/2026 **Approved** **X** **Denied** ___

District Field Review:

By: _____ **Approved** ___ **Denied** ___

District Manager: _____ **Date**: _____

Comments:

The referenced permit application for Chick-Fil-A at 1110 Homestead Rd N, in Lehigh Acres, submitted by Bowman Consulting Group Ltd, has been reviewed and is recommended for approval with the following stipulations/comments:

1. Grading must be closely monitored during construction in the South corner where a 6.33' long overflow spillway at elevation 26.72' will be provided. The grading must allow for water to exit the proposed dry retention pond and flow into the existing grate inlet with rim elevation 25.95' without overflowing onto the sidewalk and Homestead Rd.

2. It is the Owner's/Applicant's responsibility to maintain erosion control, especially in the South corner where a 23 LF, 3" PVC bleeder pipe is going to be installed, to prevent the exceptionally long pipe from being clogged with soil and debris, so the pipe can function as intended.

3. The permittee shall submit signed and sealed record drawings within thirty (30) days of project completion. The drawings must include sufficient detail to demonstrate that the permitted facilities have been constructed in substantial compliance with LA-MSID–approved plans and design.

4. Prior to commencing construction, the permittee shall provide LA-MSID with copies of all applicable permits from other governing agencies.

FLORIDA DEPARTMENT OF
Environmental Protection

Bob Martinez Center
2600 Blair Stone Road
Tallahassee, Florida 32399-2400

Ron DeSantis
Governor

Jay Collins
Lt. Governor

Alexis A. Lambert
Secretary

SELF-CERTIFICATION FOR A STORMWATER MANAGEMENT SYSTEM IN UPLANDS SERVING LESS THAN 10 ACRES OF TOTAL PROJECT AREA AND LESS THAN 2 ACRES OF IMPERVIOUS SURFACES

Owner(s)/Permittee(s):	Lemonade MM Lehigh Acres LLC
File No:	TBD
File Name:	TBD
Site Address:	1110 Homestead Rd N Lehigh Acres FL - 33936 6035
County:	Lee
Latitude:	26° 36' 17.5118"
Longitude:	-81° 38' 49.2139"
Total Project Area:	1.82
Total Impervious Surface Area:	1.24
Approximate Date of Commencement of Construction:	02/14/2026
Registered Florida Professional:	William Barbaro, P.E.
License No.:	64761
Company:	Bowman Consulting Group Ltd.

Date: TBD: DATE OF SUBMISSION

William Barbaro, P.E. certified through the Department's Enterprise Self-Service Application portal that the project described above was designed by the above-named Florida registered professional to meet the following requirements:

(a)The total project area involves less than 10 acres and less than 2 acres of impervious surface;
(b)Activities will not impact wetlands or other surface waters;
(c)Activities are not conducted in, on, or over wetlands or other surface waters;
(d)Drainage facilities will not include pipes having diameters greater than 24 inches, or the hydraulic equivalent, and will not use pumps in any manner;
(e)The project is not part of a larger common plan, development, or sale; and
(f)The project does not:

Home (/DepEssa)

Thank You!

You have completed the process for the Certification of Qualification to use a General Permit for a Stormwater Management System in uplands serving less than 10 acres total project area and less than 2 acres impervious under s. 403.814(12), F.S..

An email will be sent to you confirming your self-certification.

If you have any questions, please contact either:
the FDEP South District at FTMERP_Compliance@FloridaDEP.gov (mailto:FTMERP_Compliance@FloridaDEP.gov)
or
The South Florida Water Management District at selfcert@sfwmd.gov (mailto:selfcert@sfwmd.gov) .

Thank you for using the FDEP Business Portal.
We would appreciate your feedback. Please take our survey. (https://www.surveygizmo.com/s3/4857537/DEP-Business-Portal-Survey&cert=20&env=PROD)

Done

© Copyright 2026 Florida Department of Environmental Protection (/)
- Accessibility Information (https://floridadep.gov/accessibility-information)
- Disclaimer (https://floridadep.gov/disclaimer)
- Privacy Statement (https://floridadep.gov/privacy-statement)

Under penalty of perjury, I declare that I have read the foregoing and the facts alleged are true to the best of my knowledge and belief.

Agent to Property Owner

Property Owner
Andres Mizrahi – Authorized Agent

Print Name

************NOTE: NOTARY PUBLIC IS NOT REQUIRED FOR ADMINISTRATIVE APPROVALS************
ALL OTHER APPLICATION TYPES MUST BE NOTARIZED

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was sworn to (or affirmed) and subscribed before me by means of ☐ physical presence or ☐ online notarization, on _____ (date) by _____ (name of person providing oath or affirmation), who is personally known to me or who has produced _____ (type of identification) as identification.

STAMP/SEAL

Signature of Notary Public

PART IV #26-40

Recommended Action

Engineering Review:

By: _____ 3/4/2026 **Approved** **X** **Denied** ___

District Field Review:

By: _____ **Approved** ___ **Denied** ___

District Manager: _____ **Date**: _____

Comments:

The referenced permit application for Chick-Fil-A at 1110 Homestead Rd N, in Lehigh Acres, submitted by Bowman Consulting Group Ltd, has been reviewed and is recommended for approval with the following stipulations/comments:

1. Grading must be closely monitored during construction in the South corner where a 6.33' long overflow spillway at elevation 26.72' will be provided. The grading must allow for water to exit the proposed dry retention pond and flow into the existing grate inlet with rim elevation 25.95' without overflowing onto the sidewalk and Homestead Rd.

2. It is the Owner's/Applicant's responsibility to maintain erosion control, especially in the South corner where a 23 LF, 3" PVC bleeder pipe is going to be installed, to prevent the exceptionally long pipe from being clogged with soil and debris, so the pipe can function as intended.

3. The permittee shall submit signed and sealed record drawings within thirty (30) days of project completion. The drawings must include sufficient detail to demonstrate that the permitted facilities have been constructed in substantial compliance with LA-MSID–approved plans and design.

4. Prior to commencing construction, the permittee shall provide LA-MSID with copies of all applicable permits from other governing agencies.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: PER #26-41 Ray Ave Gas Station - 602 Meadow Road

Meeting Date: March 16, 2026

Agenda Item # 2J

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve PER #26-41 Ray Ave Gas Station - 602 Meadow Road, with any listed stipulations.

What the action accomplishes: Allows Celestial Liberty Dreams, LLC to build a convenience store and gas station at 602 Meadow Road.

Fiscal Impact: Permit Fees: $1,500.00

Staff comments: Approved according to the plans signed/sealed February 17, 2026.

Staff review and recommended approval:

[x] **MANAGER** [ ] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

LEHIGH ACRES MUNICIPAL SERVICE IMPROVEMENT DISTRICT

Agenda Packet Item

Yellow Sheet # **AIS** **Agenda Item** **2J**

☐	Audit	_____	☐	Invoices	_____
☐	Advertisement	_____	☐	Job Description	_____
☐	Application	_____	☐	Legislation	_____
☐	Appraisal	_____	☐	Loan Documents	_____
☐	Bid	_____	☐	Maint. Plan	_____
☐	Bid Specifications	_____	☐	Map	_____
☐	Bid Tabulations	_____	☐	Memorandum	_____
☐	Budget	_____	☐	Minutes	_____
☐	Budget Backup	_____	☐	News Article	_____
☐	Budget Amend.	_____	☐	Oath of Office	_____
☐	Canal Wid. Plan	_____	☐	Old Board Packages	_____
☐	C.I.P.	_____	☐	Org. Structure	_____
☐	Cap Replacement	_____	☐	Out Parcel Info	_____
☐	Change Order	_____	☐	Ownership Docs	_____
☐	Conf. Agenda	_____	☐	Permit Application	_____
☐	Contract-Redline	_____	☐	Permit, Final	_____
☐	Contract-Clean	_____	☐	Pictures	_____
☐	Corr./ Attorneys	_____	☐	Plans, Site	_____
☐	Corr./Engineers	_____	☐	Plat	_____
☐	Correspondence	_____	☐	P & P-Redline	_____
☐	Corr./Staff	_____	☐	P & P-Clean	_____
☐	Cost Comparison	_____	☐	Pros/Cons	_____
☐	Easement	_____	☐	Resolution	_____
☐	Eng. Report	_____	☐	Statute, Florida	_____
☐	Facility Inventory	_____	☐	Strategic Plan	_____
☐	Floor Plan	_____	☐	Study	_____
☐	Gant Chart	_____	☐	Survey	_____
☐	Goals	_____	☐	Travel Form	_____
☐	Gant Chart	_____	☐	Survey	_____
☐	Goals	_____	☐	Travel Form	_____

LEHIGH ACRES - MUNICIPAL SERVICES IMPROVEMENT DISTRICT PERMIT

THIS PERMIT is granted this 16th day of March, 2026, by Lehigh Acres Municipal Services Improvement District (hereinafter referred to as "DISTRICT"), 601 East County Lane, Lehigh Acres, Florida 33936, to Celestial Liberty Dreams, LLC (Binoy Shah), 14137 Blue Bay Circle, Fort Myers, FL 33913, hereinafter referred to as the "Permittee") as a non-exclusive permit for a proposed commercial development (gas station/convenience store) on (0.80 +/- acres), located at 602 Meadow Road, with an on-site detention system and a stormwater outfall to the existing road ditch along Meadow Road, in Lehigh Acres, FL.

W I T N E S S E T H:

1. In the event the DISTRICT is not the fee title owner of the real property (the "Property") to which this Permit is applicable, the Permittee agrees, prior to commencement, to obtain all necessary consents from the fee title owners of the Property.

2. This Permit does not constitute a waiver of the necessity by the Permittee to obtain such other necessary and appropriate permits required by other governmental bodies or agencies; therefore the Permittee, prior to commencement, is required to obtain any and all other applicable federal, state, and local permits required in connection with Permittee's use of the Property and agrees that at all times it will comply with the requirements of all federal, state and local laws, ordinances, rules and regulations applicable or pertaining to the use of the Property by the Permittee.

3. Permittee understands and agrees that the use of the Property pursuant to this Permit is subordinate to the rights and interests of the DISTRICT and to the extent applicable that of the fee title owner of the Property. Further, Permittee does hereby stipulate that the Permittee is not relying upon any representations whatsoever by the DISTRICT regarding the DISTRICT'S right, title, interest, or ownership as to the Property for which this Permit is applicable.

4. The Permittee, by accepting this permit, specifically agrees to allow authorized District Personnel, upon presentation of credentials or other documents as may be required by law, access to the premises, at reasonable times, where the permitted activity is located or conducted for the purpose of:
    a. Having access to and copying any records that must be kept under the conditions of the permit:
    b. Reviewing the facility, equipment, practices, or operations regulated or required under this permit; and
    c. Sampling or monitoring any substances or parameters at any location reasonably necessary to assure compliance with this permit or District rules.

5. The DISTRICT specifically reserves the right to maintain its facilities located over, under or upon the Property; to make improvements; add additional facilities; maintain, construct or alter its roads; maintain any facilities, devices, or improvements on the Property which aid in or are necessary to the DISTRICT'S operation; and the right to enter upon the Property at all times for such purposes. Permittee understands that in the exercise of such rights and interest, the DISTRICT may require Permittee, at no cost to DISTRICT, to relocate, alter, or remove the Permittee's facilities and equipment or other improvements made by Permittee pursuant to this Permit which interfere with or prevent the DISTRICT, in the DISTRICT'S opinion, from properly and faithfully constructing, improving and maintaining its facilities. If the Permittee should fail to relocate, alter, or remove the Permittee's facilities, equipment or other improvements made by the Permittee pursuant to this Permit within a reasonable time period following receipt of notice from the DISTRICT requesting same, then in that event the DISTRICT shall have the right to enter upon the lands and make such relocation, alterations or removal of the Permittee's facilities, equipment/or other improvements at no cost to DISTRICT.

6. Permittee agrees that it will not use the Property in any manner which interferes with the DISTRICT'S use of the Property or causes a hazardous condition to exist.

7. The DISTRICT assumes no responsibility for the ownership, operation and/or maintenance of the Permittee's facilities as permitted herein.

8. The Permittee agrees to backfill any excavation it makes within the Property and to repair or replace (i) any below-grade authorized facilities and features located therein, including but not limited to: water, sewer, stormwater or irrigation facilities, structures or appurtenances and (ii) as to at or above-grade facilities and features, this requirement shall only be applicable to grasses and sod, sidewalks, and roadway improvements (the term roadway improvements shall include but not be limited to curbing, roadway base, sub grade and asphaltic or concrete surfaces) which are removed or damaged as a direct result of said excavation. Such repair or replacement shall substantially restore these features to their condition as it existed immediately prior to the event precipitating the excavation.

9. All below-grade Permittee installations, including water and sewer lines, whether sub aqueous or not, together with appurtenant facilities, shall be located underground and have a minimum ground cover of thirty inches (30") below original design profile grade, including District waterways and roadways.

10. All Permittee activity related to sub aqueous utility, water and/or sewer lines, including appurtenant facilities, shall be conducted in such fashion that the primary functional purpose of the canal or lake body facility, specifically flood control, within which the activity is taking place is not adversely affected. More specifically, sub aqueous construction shall conform to the criteria given in the Lehigh Acres Municipal Services Improvement District (Lehigh Acres – MSID) Administrative Policy and Procedures.

11. Permittee shall, at no expense to DISTRICT and within a reasonable time following notice, adjust the positions and elevations of its facilities as may be required in connection with future improvements to, or construction of, works of the DISTRICT.

12. Permittee does hereby indemnify and hold harmless the DISTRICT, its Board of Commissioners, officers, personnel, and it's Engineers against any claims, losses, damages (including consequential), expenses, or legal fees that might arise out or result from the implementation of the proposed project by the Permittee or that may be caused by a failure of DISTRICT facilities or property.

13. The Permittee shall be obligated at the time of submission of its Permit application, to provide a detail of any other existing improvements located over, under, upon or through the Property and the Permittee's accompanying plans and specifications shall clearly indicate how the Permittee will comply with required clearances between the requested permitted activity and existing improvements located over, under and upon the Property, including compliance with other permitted users duly adopted and published standards.

14. Permittee shall reimburse the DISTRICT for its legal, engineering and other expenses incurred as a result of the implementation of the proposed project.

15. If Permittee should violate any of the terms or conditions of the Permit and shall not correct or remedy same within ten (10) days of receiving written notice of said violation from the DISTRICT, then in that event, the DISTRICT may, at its option, revoke, cancel and terminate this Permit.

16. This Permit may not be assigned by the Permittee without the prior written approval of the DISTRICT.

17. Unless otherwise adjusted, modified or terminated as set forth hereinabove, including the Conditions attached hereto, this Permit shall continue in full force and effect so long as Permittee complies with the terms of this Permit.

18. In any litigation, including breach, enforcement or interpretation arising out of this Permit, the prevailing party to this Permit shall be entitled to recover reasonable attorney's fees, costs and expenses related to said action from the non-prevailing party.

19. That attached hereto are Supplemental Conditions to this Permit which are incorporated herein and made a part hereof.

LEHIGH ACRES – MUNICIPAL SERVICES IMPROVEMENT DISTRICT

BY: _____

NAME TYPED: David E. Lindsay

TITLE: District Manager

SUPPLEMENTAL CONDITIONS TO PERMIT NO. 26-41

A. GENERAL CONDITIONS

1. Notification shall be given by the Permittee to the District Engineer, AIM Engineering, at 239-332-4569 forty-eight (48) hours prior to commencement of the permitted activity. The District Engineer shall establish points of construction, if applicable, which will require inspection by the District Engineer prior to construction of the permitted activity. When the Permittee considers the permitted activity complete, the Permittee shall notify the District Engineer and schedule an on-site final inspection which shall be held in the presence of a representative of the Permittee and the District Engineer.

2. The permitted activity shall be implemented, constructed and/or installed in full accordance with the approved plans and specifications within six (6) full calendar years from the date of permit approval. Deviations from the approved plans or specifications shall be coordinated with and approved by the District Engineer prior to implementation of any such deviation.

3. When working in a District road right-of-way, not more than one-half (½) of the road or street shall be closed and traffic shall be controlled so as to provide for the safety and minimum hindrance to the public. All traffic control operations shall conform to the most current issue of the Florida Department of Transportation publication "Manual on Traffic Controls and safe Practices for Street and Highway Construction, Maintenance and Utility Operations."

4. The District Engineer shall be the final authority as to the quality and quantity of the material and works required to satisfy the terms and conditions of this Permit as they relate to District improvements and facilities.

5. Upon completion of the permitted activity and after its final review and acceptance by the District Engineer THE PERMITTEE SHALL DELIVER TO THE DISTRICT OFFICE ONE COMPLETE SET OF THE FINAL "RECORD DRAWINGS." FAILURE TO PROVIDE THE FINAL RECORD DRAWINGS MAY RESULT IN THE REVOCATION, CANCELLATION AND TERMINATION OF THIS PERMIT.

6. The above Conditions shall be continuing obligations of the Permittee and shall be complied with in the event of any subsequent maintenance, repair, replacement or modification of the permitted improvement.

Stipulations:

1). Permit #26-41 is approved according to the revised plans and calculations signed/sealed March 9th, 2026.

2). Maintenance of the entire on-site storm water management system, including the control structure and outfall pipe into the roadside ditch is the responsibility of the Permittee.

3). An "As-Built" survey showing the completed improvements (including perimeter berm, pipe inverts, finished floor elevation and outfall structure elevations) must be submitted to Lehigh Acres - MSID within 30 days following completion of the improvements.

4). Permittee shall obtain any other permits required by EPA, FDEP, SFWMD, Lee County, LC DOT, and FDOT prior to starting and submit a copy to LA-MSID.

PART I

DATE SUBMITTED: 02 - 19 - 26

PERMIT #: 26 - 41

APPLICATION FOR A PERMIT

*Note: The review of permit applications will be conducted per the Lehigh Acres Municipal Services Improvement District's Administrative Policy and Procedures

AGENT NAME: Quattrone & Associates, Inc. **PHONE #:** (239)936-5222

PROJECT NAME: Ray Avenue Gas Station

TOTAL ACREAGE: 0.80

Property Address (STRAP associated with this address will be used as the primary STRAP for this project):					
Mailing Address:	**602 Meadow Rd.**				
City:	**Lehigh Acres**	**State:**	**FL**	**Zip:**	**33973**

STRAP NUMBERS (*attach additional sheet if necessary):*

0	5	-	4	5	-	2	6	-	0	3	-	0	0	0	3	5	-	0	0	1	0
		-			-			-			-						-				
		-			-			-			-						-				
		-			-			-			-						-				
		-			-			-			-						-				

Brief description of proposed project:

Application for: Proposed construction of a convenience store to include a gas station.

Is the project proposed to be developed in phases? ☐ Yes ☒ No

Estimated # of Phases:	1
***Permit Fee:**	$ **(to be verified by staff)**

* See Administrative Policy and Procedures Sections 4.15 – 4.17 for current Permit Fee Amount.

PART II

OWNER/APPLICANT/ DEVELOPER INFORMATION

A.

Name of Applicant	Celestial Liberty Dreams, LLC - Binoy Shah						
Mailing Address:	Street:	14137 blue bay circle					
City:	Fort Myers	State:	FL			Zip:	33913
Phone No.	Area Code:	901	Number:	335-2588		Ext.	
Fax No.	Area Code:		Number:	-			
E-mail Address:	anilshah0413@gmail.com binoyshah16@gmail.com						

B. Relationship of applicant to property*:

☒ Owner* ☐ Trustee* ☐ Option holder* ☐ Lessee*

☐ Contract Purchaser* ☐ Other (indicate)* _____

C.

Name of Developer:	Unknown						
Mailing Address:	Street:						
City:		State:				Zip:	
Phone No.	Area Code:	( )	Number:	-		Ext.	
Fax No.	Area Code:	( )	Number:	-			
E-mail Address:							

D.

Name of Authorized Agent(s):	Quattrone & Associates, Inc.						
Mailing Address:	Street:	4301 Veronica Shoemaker Blvd.					
City:	Ft. Myers	State:	FL			Zip:	33916
Phone No.	Area Code:	(239)	Number:	936-5222		Ext.	
Fax No.	Area Code:	( )	Number:	-			
E-mail Address:	josh@qainc.net & permits@qainc.net						

E.

Name of Property Owner:	Same as Applicant						
Mailing Address:	Street:						
City:		State:				Zip:	
Phone No.	Area Code:	( )	Number:	-		Ext.	
Fax No.	Area Code:	( )	Number:	-			
E-mail Address:							

*A disclosure of the name of all persons or entities having an ownership interest in the property is required (attached).

PART III

DISCLOSURE OF OWNERSHIP INTEREST FORM FORM:

PRIMARY STRAP NUMBER: 05 - 45 - 26 - 03 - 00035 - 0010

1. If the property is owned in the fee simple by an **INDIVIDUAL**, tenancy by the entirety, tenancy in common, or joint tenancy, list all parties with an ownership interest as well as the percentage of such interest.

Name and Address	Percentage of Ownership

2. If the property is owned by a **CORPORATION**, list the officers and stockholders and the percentage of stock owned by each.

Name, Address, Office	Percentage of Stock

3. If the property is in the name of a **TRUSTEE**, list the beneficiaries of the trust with percentages of interest.

Name and Address	Percentage of Interest

4. If the property is in the name of a **GENERAL PARTNERSHIP OR LIMITED PARTNERSHIP**, list the names of general and limited partners.

Name and Address	Percentage of Interest
Anil Shah	50%
Binoy Shah	35%
Hitesh Patel	15%

5. If there is a **CONTACT FOR PURCHASE**, whether contingent on this application Or not, and whether a Corporation, Trustee, or Partnership, list the names of the contract purchasers below, including the officers, stockholders, beneficiaries, or partners.

Name and Address	Percentage of Interest

Date of Contract (mm/dd/yy): / /

6. If any contingency clause or contract terms involve additional parties, list all individuals or officers, if a corporation, partnership, or trust.

Name and Address	Percentage of Interest

For any changes of ownership of changes in contracts for purchase subsequent to the date of the application, but prior to the date of final certificate for compliance, a supplemental disclosure of interest must be filed.

The above is a full disclosure of all parties of interest in this application, to the best of my knowledge and belief.

Binoy Shah
SIGNATURE OF APPLICANT

Binoy Shah
PRINTED OR TYPED NAME OF APPLICANT

STATE OF FLORIDA
COUNTY OF LEE

The foregoing instrument was acknowledged before me this 23rd day of Sept. (m), 2024 (y), by Binoy Shah who is personally know to me or has produced FL DL as identification.

SANA HUSSAIN
Notary Public - State of Florida
Commission # HH 198337
My Comm. Expires Nov 14, 2025

Sana Hussain
SIGNATURE OF NOTARY PUBLIC

Sana Hussain
PRINTED NAME OF NOTARY PUBLIC

PART IV

EXPLANATION

All Permit Application sections must be completed and submitted.

Permit Application fees and submittal items (see Part VI) must be submitted with the application.

Any and all work involving the lands and/or facilities of the District shall not commence without a Board approved permit.

All set policies, procedures, technical requirements, and fee schedules can be found and reviewed in the 'Administrative Policy and Procedures Guide'. The Guide is available at the District Office located at 601 East County Lane, Lehigh Acres, FL or online at la-msid.com/.

PART V

PROFESSIONAL CONSULTANT FORM

ATTORNEY

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

ENGINEER

Name:	Josh Eisenoff, P.E.						
Mailing Address:	4301 Veronica Shoemaker Blvd.						
City:	Ft. Myers	State:	FL			Zip:	33916
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

OTHER REPRESENTATIVE/PROFESSIONAL

Name:							
Mailing Address:							
City:		State:				Zip:	
Business No.	Area Code:	( )	Number:	-		Ext.	
Other Contact:							

PART VI

SUBMITTALS

NO. OF COPIES		SUBMITTAL DOCUMENT
SUB'D	REQ'D	
		ITEMS REQUIRED FOR INITIAL SUBMITTAL
1	2	**Construction/Development Plans** signed and sealed by a Licensed Florida Professional Engineer including: 1. Location map showing the location of the proposed project in relation to adjacent streets and LA-MSID facilities. 2. Details of proposed works that fully communicate the work and/or use desired (i.e. Existing Conditions Survey, Proposed Site Plan, Paving Grading & Drainage Plan, Cross Sections, Element Details, and Erosion Control Plan sheets). 3. Sufficient survey data to determine elevations, property and ROW lines, limits of existing and proposed conditions, and notation of utilized vertical datum (NAVD/NGVD).
1	2	**Stormwater Drainage Calculations** signed and sealed by a Licensed Florida Professional Engineer including, but not limited to, computer stormwater routing modeling data (ICPR4, Cascade, etc.) to verify compliance with the Design Criteria (input summary data below): **Water Quality Volume (Required):** _____0.13_(ACRE-FT) **Water Quality Volume (Provided):** __0.13____(ACRE-FT) **Control Elevation:** __23.50__(FT-NGVD) **Minimum Road/Parking Elevation:** _**26.07**___(FT-NGVD) *(above the 10-year, 1-day design storm peak stage elevation)* **Minimum Perimeter Berm Elevation:** _**26.80**__(FT-NGVD) *(above the 25-year, 3-day design storm peak stage elevation)* **Minimum Finished Floor Elevation:** _**27.92**__(FT-NGVD) *(above the 100-year, 3-day design storm peak stage elevation and previously experienced high-water levels external to project)*
1	2	**Flash Drive / CD** containing plans in both AutoCAD (.dwg file drawn in "State Plane Coordinates") and PDF formats.
2	2	**Permit Application** (PARTS I & II)
2	2	**Disclosure of Owner Form (signed and notarized)** (PART III)
2	2	**Professional Consultant Form & Submittals** (PARTS V & VI)
	2	**Copy of Approved FDEP Notice of Intent (NOI)** for all sites one (1) acre and greater in size.
1	2	**Copy of approved SWFMD permit**, for all sites required to get a permit, **or FDEP "10/2 Self-Certification"** approval.
1	1	**Check for appropriate Permit Fees** made out to LA-MSID.

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd. Fort Myers, FL
239.936.5222 | QAINC.net | f 239.936.7228

LETTER OF TRANSMITTAL

TO: LAMSID
PHONE:
FAX:

DATE: 2/18/26
PROJECT: Ray Avenue Gas Station
JOB #: 230704
RE:

THE FOLLOWING ITEMS ARE BEING SENT VIA:

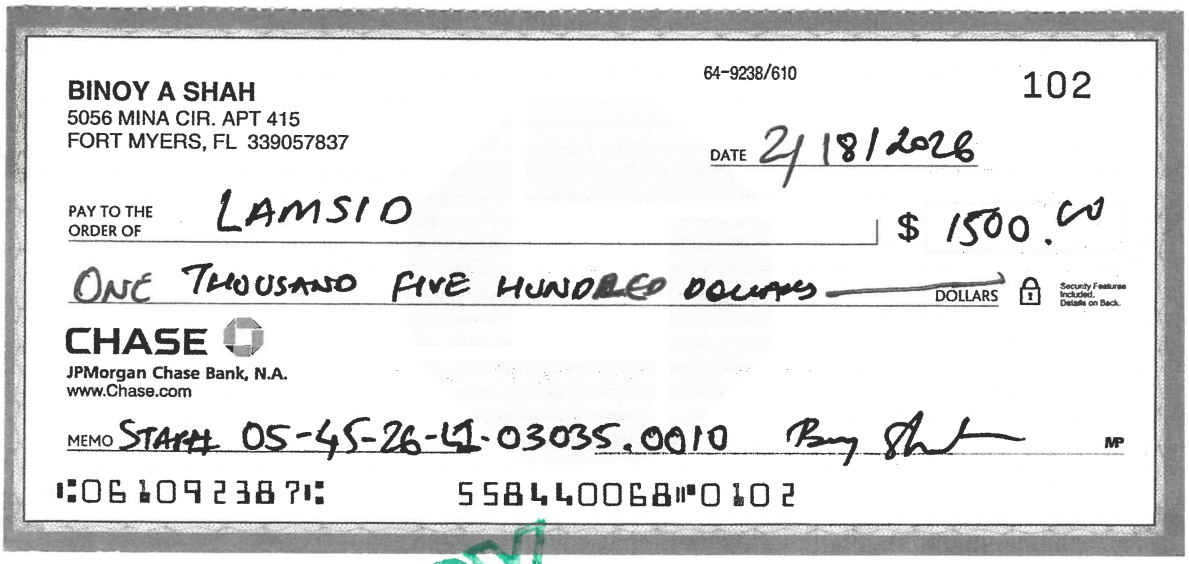
- [ ] FAX with page(s) to follow
- [ ] Hand delivery
- [x] U.S. Mail
- [ ] Federal/Airborne Express

Quantity	DESCRIPTION
2	Civil Plans
2	Stormwater Report
2	Application
2	10-2
1	Check for $1500
1	CD

REMARKS:

CC: File

FROM: Shelly Stalnos



BINOY A SHAH
5056 MINA CIR. APT 415
FORT MYERS, FL 339057837

64-9238/610

102

DATE 2/18/2026

PAY TO THE ORDER OF LAMSIO

$ 1500.00

ONE THOUSAND FIVE HUNDRED DOLLARS

DOLLARS

Security Features Included. Details on Back.

CHASE
JPMorgan Chase Bank, N.A.
www.Chase.com

MEMO STAPL 05-45-26-41-03035.0010

MP

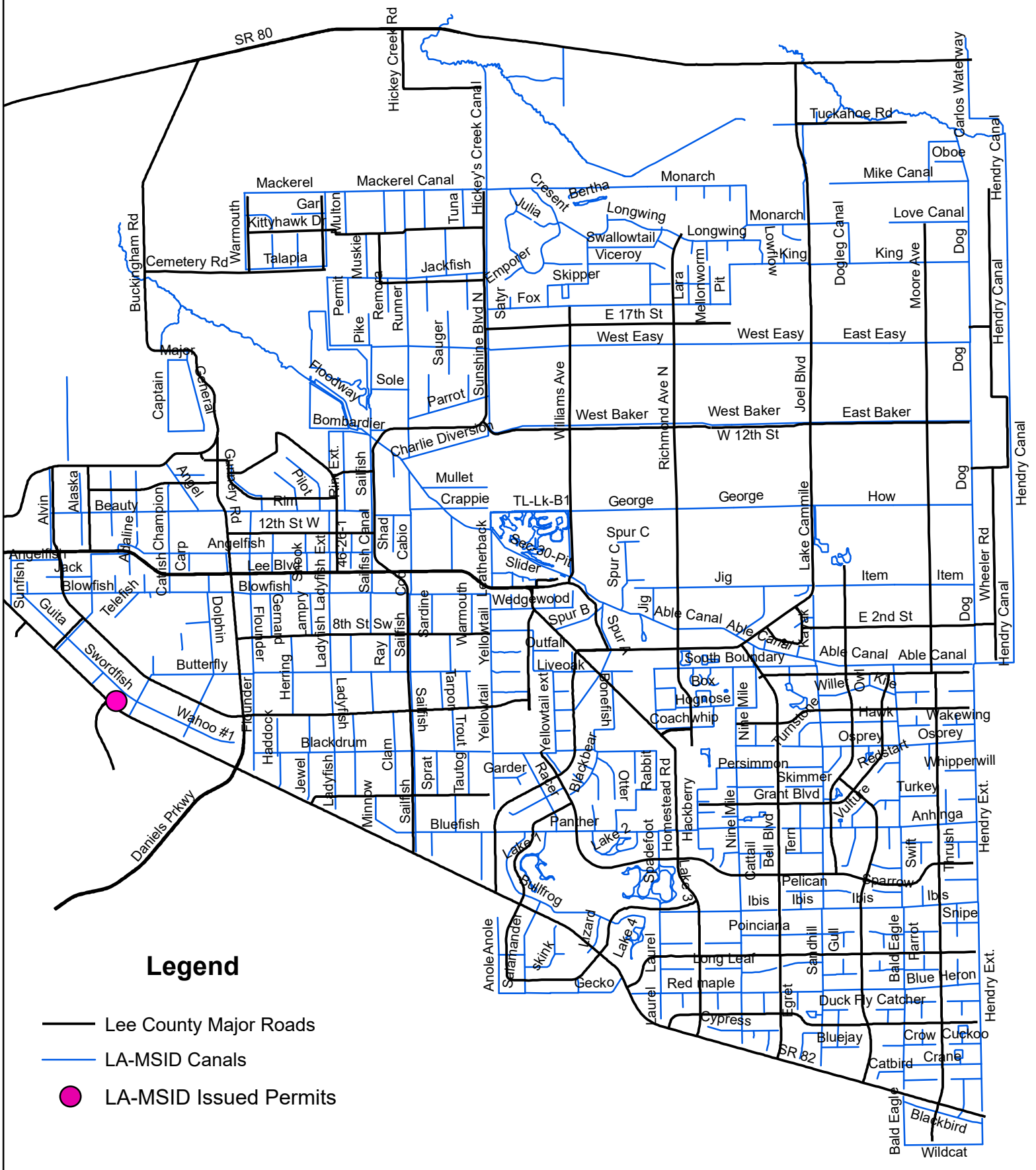
⑆061092387⑆ 5584400688⑈0102

COPY

RECEIVED
FEB 19 2026
LEHIGH ACRES
MUNICIPAL SERVICES
IMPROVEMENT DISTRICT

Permit #26-41

Permit # 2026-41



LEE COUNTY DEVELOPMENT ORDER

FOR

RAY AVENUE GAS STATION

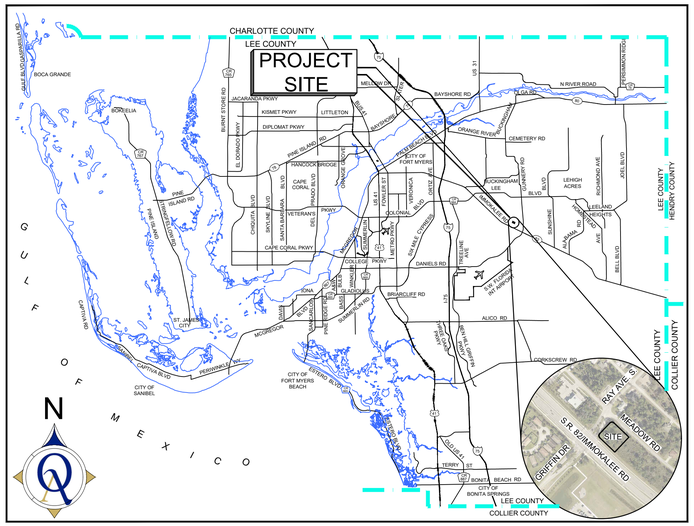
604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

REV	REVISION	DATE

OWNER / DEVELOPER:

CELESTIAL LIBERTY DREAMS, LLC.
P.O. BOX 571
ESTERO, FL 33929
LEE COUNTY

LOCATION MAP



SHEET INDEX

SITE DEVELOPMENT: SD

SD: 01 - COVER - DO
SD: 02 - EXISTING CONDITIONS & DEMOLITION PLAN
SD: 03 - AERIAL & FLUCCS PLAN
SD: 04 - EROSION CONTROL PLAN
SD: 05 - EROSION CONTROL DETAILS
SD: 06 - MASTER SITE PLAN
SD: 07 - SIGNAGE & STRIPING PLAN
SD: 08 - PAVING, GRADING & DRAINAGE PLAN
SD: 09 - PROJECT CROSS SECTIONS - A
SD: 10 - PROJECT CROSS SECTIONS - B
SD: 11 - PROJECT DETAILS - A
SD: 12 - PROJECT DETAILS - B
SD: 13 - PROJECT NOTES
~~SD: 14 - MASTER UTILITY & FIRE PROTECTION PLAN~~
~~SD: 15 - UTILITY PLAN & PROFILE~~
~~SD: 16 - FGUA UTILITIES DETAILS~~

LANDSCAPING: LS

~~LS: 01 - LANDSCAPING PLAN~~
~~LS: 02 - LANDSCAPING DETAILS~~

LIGHTING: LT

~~LT: 01 - LIGHTING PLAN~~

SITE PERMIT REQUIREMENTS		
AGENCY	STATUS	NOTES
LEE COUNTY DEVELOPMENT ORDER	REQUIRED	
LEE COUNTY PORT AUTHORITY	N/A	
FLORIDA GOVERNMENTAL UTILITY AUTHORITY	REQUIRED	
S.F.W.M.D. ENVIRONMENTAL RESOURCE	REQUIRED	
S.F.W.M.D. DEWATERING PERMIT	N/A	
S.F.W.M.D. WATER USE	REQUIRED	
DOH WATERLINE EXTENSION PERMIT	REQUIRED	
FDEP WASTE WATER EXTENSION PERMIT	REQUIRED	

LEE COUNTY, FLORIDA

SECTION 05, TOWNSHIP 45S, RANGE 26E
STRAP # 05-45-26-03-00035.0010
230704 - I

NAD83 FLORIDA STATE PLANES, WEST ZONE, US FOOT

CONTRACTOR STAKEOUT SHALL BE FROM THE APPROVED CAD FILE BY QUATTRONE AND ASSOCIATES

VERTICAL DATUM: NAVD

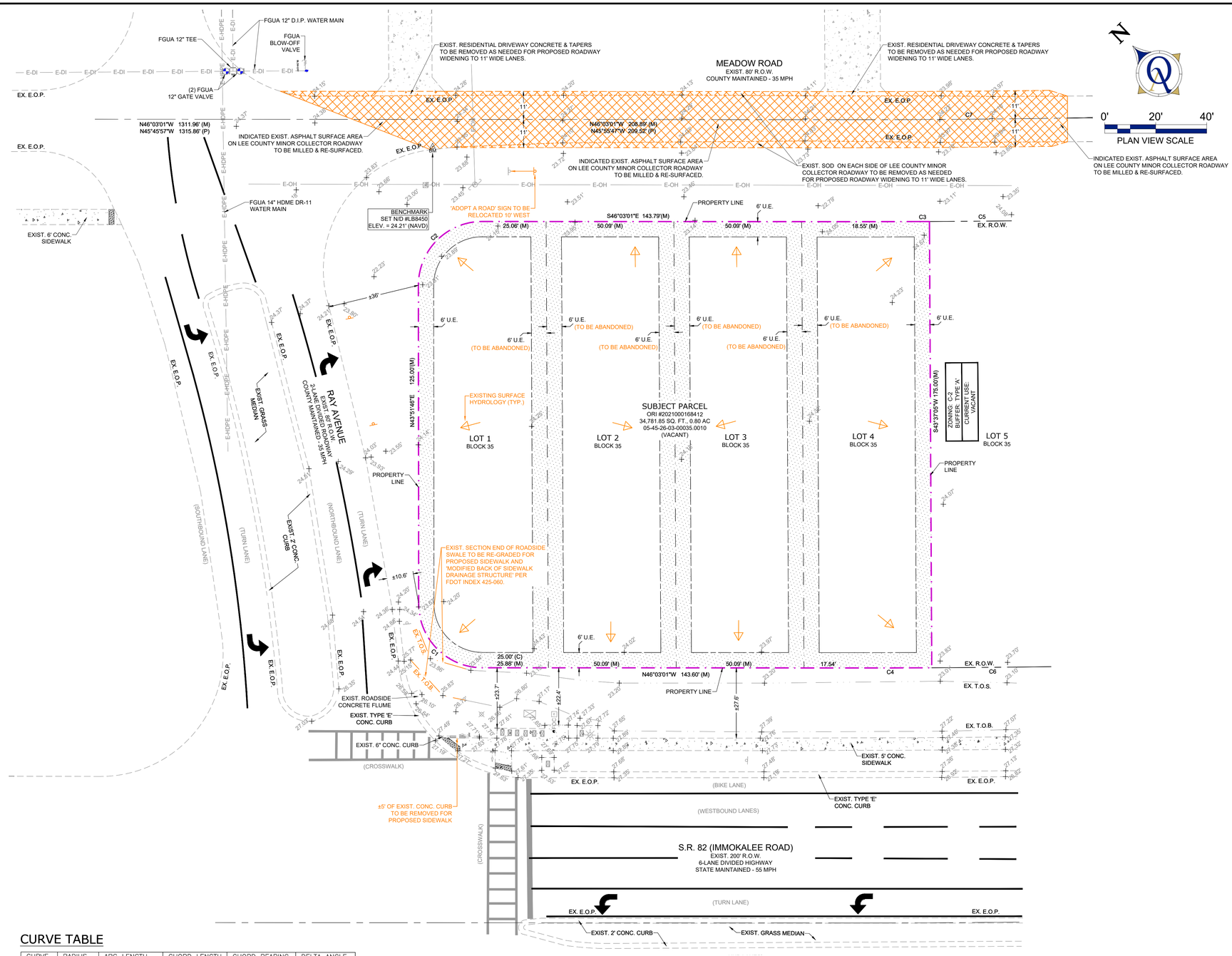
Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 VERONICA SHOEMAKER BLVD., FORT MYERS, FLORIDA 33916
| 239.936.5222 (P) | 239.936.7228 (F) | QAINC.NET | COA # 9465 |

RESERVED FOR PERMIT AGENCY APPROVAL

Joshua A. Eisenoff, State of Florida, Professional Engineer, License No. 97911. This item has been digitally signed and sealed by Joshua A. Eisenoff on the date indicated here. Signature must be verified on any electronic copies.

JOSHUA A. EISENOFF, P.E.
FL. PE #97911
DATE:2/17/2026

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CAD\DRAWINGS\01-CAD FILES\01-CAD FILES\01-CAD\230704-SD.dwg, 2/17/2026 2:39:39 PM, Joshua Eisenoff



CURVE TABLE

CURVE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE
C1(P)	25.00'	39.23'	35.33'	N01°05'38"W	89°54'46"
C2(P)	25.00'	39.31'	35.38'	N88°54'22"E	90°05'14"
C3(M)	5449.05'	31.54'	31.54'	S46°12'58"E	0°19'54"
C4(M)	5624.05'	32.55'	32.55'	N46°12'58"W	0°19'54"
C5(M)	5449.05'	1361.40'	1357.86'	S53°32'21"E	14°18'54"
C6(M)	5624.05'	1405.12'	1401.47'	S53°32'21"E	14°18'54"
C7(M)	5409.05'	1232.70'	1230.03'	S52°34'44"E	13°03'27"

LEGEND FOR EXISTING CONDITION PLAN:

EXST.FEATURES (ROADS, WALKS, ECT.) — EXST. TOPO ELEV. (NAVD)
CENTER LINE — PUBLISHED BENCH MARK
SITE RIGHT OF WAY — EXIST. FIRE HYDRANT
SITE BOUNDARY — EXIST. GATE VALVE
SITE EASEMENT — EXST. WATER METER
SITE PARCEL / LOT LINE — EXST. DRAINAGE INLET
EXST. STREET SIGN — EXST. POWER POLE
EXST. TRAFFIC LITE & POLE — EXST. LIGHT POLE
EXST. TRAFFIC LITE BOX — OVERHEAD POWER LINES
EXST. FIBER OPTIC MARKER — EXST. ELEC. PULL BOX
EXST. ELECTRIC METER

TYPICAL HATCH PATTERNS

ASPHALT/CONCRETE OR OTHERS TO BE DEMOED — EASEMENTS

TYPICAL ABBREVIATIONS

WLP	WOODEN LIGHT POLE	N/D	NAIL & DISC
CPP	CONCRETE POWER POLE	EX.	EXISTING
GW	GUY WIRE	R.O.W.	RIGHT-OF-WAY
OH	OVERHEAD UTILITIES	U.E.	UTILITY EASEMENT
CP	CONCRETE PAD	E.O.P.	EDGE OF PAVEMENT
WCS	WATER CONTROL STRUCTURE	T.O.B./T.O.S.	TOP OF BANK / TOE OF SLOPE

DEMOLITION NOTES:

1. CONTRACTOR TO PROVIDE SERVICES FOR EFFECTIVE AIR AND WATER POLLUTION CONTROLS AS REQUIRED BY LOCAL AUTHORITIES HAVING JURISDICTION.
2. DEMOLITION DEBRIS SHALL BE PROPERLY DISPOSED OF. CONTRACTOR SHALL DEVELOP A DEMOLITION MATERIALS MANAGEMENT PLAN AND MAKE ALL EFFORTS TO REUSE OR RECYCLE DEBRIS AT A COUNTY APPROVED RECYCLING FACILITY IN ACCORDANCE WITH LEE COUNTY SOLID WASTE DIVISION REQUIREMENTS.
3. IF HAZARDOUS MATERIALS ARE ENCOUNTERED DURING DEMOLITION, COMPLY WITH APPLICABLE FEDERAL, STATE, AND LOCAL REGULATIONS, LAWS AND ORDINANCES CONCERNING REMOVAL, HANDLING AND PROTECTION AGAINST EXPOSURE OR ENVIRONMENTAL POLLUTION.
4. ANY HAZARDOUS MATERIAL FOUND ON SITE MUST BE REMOVED PRIOR TO SITE CONSTRUCTION BY A LICENSED HAZARDOUS WASTE REMOVAL CONTRACTOR.
5. CONTRACTOR TO CONTACT ALL UTILITY COMPANIES 72 HOURS PRIOR TO THE START OF DEMOLITION.
6. IT IS THE CONTRACTOR'S RESPONSIBILITY TO LOCATE AND TAKE ALL POSSIBLE PRECAUTIONS TO AVOID ANY DAMAGE TO UNDERGROUND PIPELINES, TELEPHONE, CABLE TV, ELECTRIC LINES/CONDUITS AND STRUCTURES IN ADVANCE OF DEMOLITION OR CONSTRUCTION.
7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ANY CLEARING OR TREE REMOVAL PERMITS REQUIRED FOR DEMOLITION.
8. EXISTING GROUND WATER WELLS ON SITE SHALL BE PROPERLY ABANDONED BY LEE COUNTY LICENSED WATER WELL CONTRACTOR IN ACCORDANCE WITH LEE COUNTY NATURAL RESOURCES DEPARTMENT REQUIREMENTS & COMPLETE A DEMOLITION WELL PROTECTION FORM. ABANDONMENT SHALL ALSO COMPLY WITH SFWMD AND DEP PLUGGING REQUIREMENTS. WELL LOCATIONS ARE APPROXIMATE & ALL WELLS MAY NOT BE SHOWN ON THIS PLAN.
9. ANY EXISTING WATER OR SEWER INFRASTRUCTURE TO BE ABANDONED IS TO BE PLUGGED, CAPPED AND MARKED AND ABANDONED IN ACCORDANCE WITH LCU & FDEP REGULATIONS.
10. ABANDONED SEPTIC TANKS OR DRAIN FIELDS HAVE NOT BEEN FIELD LOCATED AND LOCATIONS ARE APPROXIMATE AND SHALL BE CONFIRMED BY SITE INSPECTION. SEPTIC TANKS AND DRAIN FIELDS SHALL BE REMOVED IN ACCORDANCE WITH HRS AND FDEP REQUIREMENTS.
11. A NOTICE OF COMMENCEMENT FOR DEMOLITION ACTIVITIES SHALL BE RECORDED WITH WITH LEE COUNTY.
12. ALL EXISTING CABBAGE PALM TREES SHALL BE RELOCATED ONSITE TO THE GREATEST EXTENT POSSIBLE.

VERTICAL DATUM: NAVD

THIS DRAWING AND ALL ELEVATIONS SHOWN ARE BASED ON THE NATIONAL AMERICAN VERTICAL DATUM (NAVD 88) CONVERT TO NGVD 29 ADD: 1.2

EXISTING CONDITIONS ARE BASED ON INFORMATION PROVIDED BY
K.E. TRASK, P.A. (LB8450)
12345 BLASINGIM ROAD
FT. MYERS, FL. 33966
(239) 980-1495

REV	DATE	REVISION

RAY AVENUE GAS STATION
EXISTING CONDITIONS & DEMOLITION PLAN

604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF, STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NO. 97911. THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY JOSHUA A. EISENOFF, P.E. ON THE DATE INDICATED HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED & SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES

JOSHUA A. EISENOFF, P.E.
FL. PE #97911
VALID ONLY WITH STAMPED SEAL

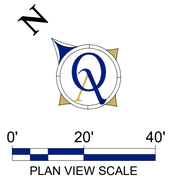
SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-02
02 OF 16

230704 - I

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CADD FILES\01-QAI CADD - VERSION I\02-PRODUCTIONS PLANS\230704-SD





FEMA FLOOD ZONES:

THIS PROPERTY IS LOCATED IN FLOOD ZONE: X
BASE FLOOD ELEVATION: X = NO BASE FLOOD ELEVATION
IN FIRM COMMUNITY: 125124 PANEL NO.: 0475 MAP #: 12071C0475F DATED: 08/28/2008

SOILS LEGEND:

42	69% WABASSO SAND, LIMESTONE SUBSTRATUM	0% - 2% SLOPES
13	31% BOCA FINE SAND, URBAN LAND COMPLEX	0% - 2% SLOPES

FLUCCS LEGEND:

411-E3	PINE FLATWOODS	100%

CURVE TABLE

CURVE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE
C1(P)	25.00'	39.23'	35.33'	N01°05'38"W	89°54'46"
C2(P)	25.00'	39.31'	35.38'	N88°54'22"E	90°05'14"
C3(M)	5449.05'	31.54'	31.54'	S46°12'58"E	0°19'54"
C4(M)	5624.05'	32.55'	32.55'	N46°12'58"W	0°19'54"
C5(M)	5449.05'	1361.40'	1357.86'	S53°32'21"E	14°18'54"
C6(M)	5624.05'	1405.12'	1401.47'	S53°32'21"E	14°18'54"
C7(M)	5409.05'	1232.70'	1230.03'	S52°34'44"E	13°03'27"

REV	REVISION	DATE

RAY AVENUE GAS STATION
AERIAL & FLUCCS PLAN

604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF, STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NO. 97911. THIS ITEM HAS BEEN DIGITALLY SIGNED & SEALED BY JOSHUA A. EISENOFF, P.E. ON THE DATE ADJACENT HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED & SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

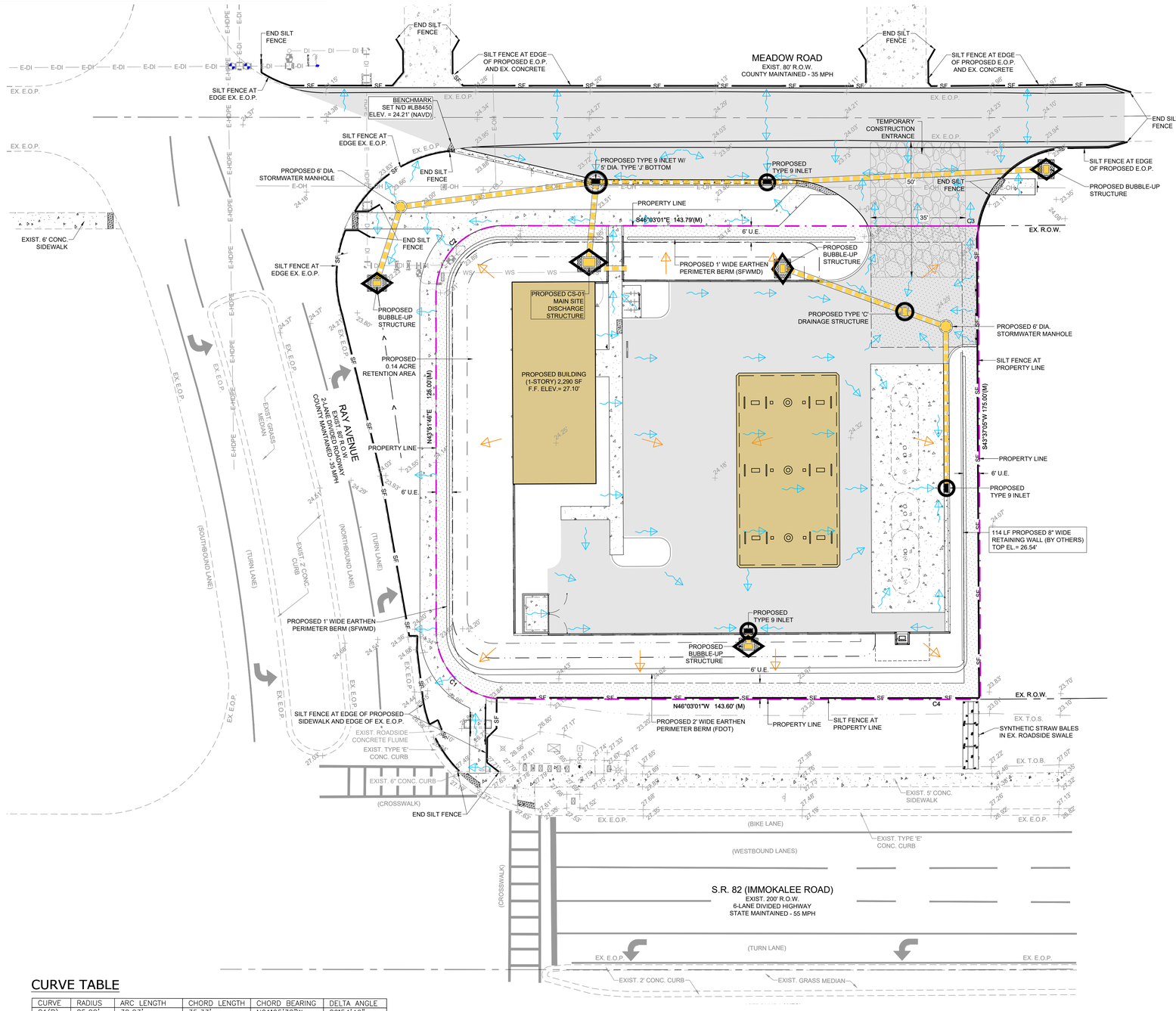
JOSHUA A. EISENOFF, P.E.
FL PE #97911
VALID ONLY WITH STAMPED SEAL

SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-03
03 OF 16

230704 - I

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CADD FILES\01-QAI CADD - VERSION 102-PRODUCTIONS PLANS\230704-SD



STORM WATER POLLUTION PREVENTION NOTES AND LEGEND:

1. PRIOR TO COMMENCEMENT OF CONSTRUCTION ACTIVITY APPROPRIATE EROSION CONTROL DEVICES SHALL BE INSTALLED TO CONTROL AND REDUCE SOIL EROSION AND SEDIMENT TRANSPORT TO OFF SITE AREAS. THE CONTRACTOR SHALL MAINTAIN THESE DEVICES THROUGHOUT THE DURATION OF CONSTRUCTION. ALL DEVICES SHALL REMAIN IN PLACE UNTIL PERMANENT EROSION CONTROL IS ESTABLISHED.
2. THE CONTRACTOR SHALL SUPPLEMENT THIS PLAN AS MAY BE REQUIRED TO CONTROL AND REDUCE SOIL EROSION BASED ON THE CONTRACTORS MEANS, METHODS AND TECHNIQUE OF CONSTRUCTION. THE CONTRACTOR SHALL ALSO FILE A NPDES NOTICE OF INTENT WITH THE EPA AND LEE COUNTY DEVELOPMENT SERVICE DIVISION AT LEAST 48 HOURS PRIOR TO THE START OF CONSTRUCTION.
3. THE CONTRACTOR SHALL CONTACT LEE COUNTY NPDES STORMWATER REVIEWER PRIOR TO ANY DEWATERING OPERATIONS.
4. ALL PROPOSED AND EXISTING INLETS SUBJECT TO SEDIMENTATION SHALL BE COVER WITH FILTER FABRIC (HELD IN PLACE BY THE GRATE).
5. THERE ARE NO APPARENT CONTRIBUTING UPSTREAM FLOWS FROM ADJACENT PROPERTIES; AND DURING CONSTRUCTION SURROUNDING FLOWS PATTERNS MUST BE MAINTAINED.

NOTE: THESE DETAILS REPRESENT TYPICAL BEST MANAGEMENT PRACTICES FOR SOIL EROSION CONTROL. THEY MAY NOT SATISFY ALL REQUIREMENTS FOR COMPLIANCE WITH REGULATORY AGENCIES AND/OR SPECIFIC PERMIT CONDITIONS.

- EX. SURFACE HYDROLOGY
- PRO. SURFACE HYDROLOGY
- INLET FABRIC PROTECTIONS
- STAKED SYNTHETIC STRAW BALE AT INLETS
- SYNTHETIC STRAW BALE
- STAKED SILT FENCE
- FLOATING TURBIDITY BARRIER

CURVE TABLE

CURVE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE
C1(P)	25.00'	39.23'	35.33'	N01°05'38"W	89°54'46"
C2(P)	25.00'	39.31'	35.38'	N88°54'22"E	90°05'14"
C3(M)	5449.05'	31.54'	31.54'	S46°12'58"E	0°19'54"
C4(M)	5624.05'	32.55'	32.55'	N46°12'58"W	0°19'54"
C5(M)	5449.05'	1361.40'	1357.86'	S53°32'21"E	14°18'54"
C6(M)	5624.05'	1405.12'	1401.47'	S53°32'21"E	14°18'54"
C7(M)	5409.05'	1232.70'	1230.03'	S52°34'44"E	13°03'27"

DATE	REVISION	REV

RAY AVENUE GAS STATION
EROSION CONTROL PLAN

604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF, STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NO. 97911. THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY JOSHUA A. EISENOFF, P.E. ON THE DATE INDICATED HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED & SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

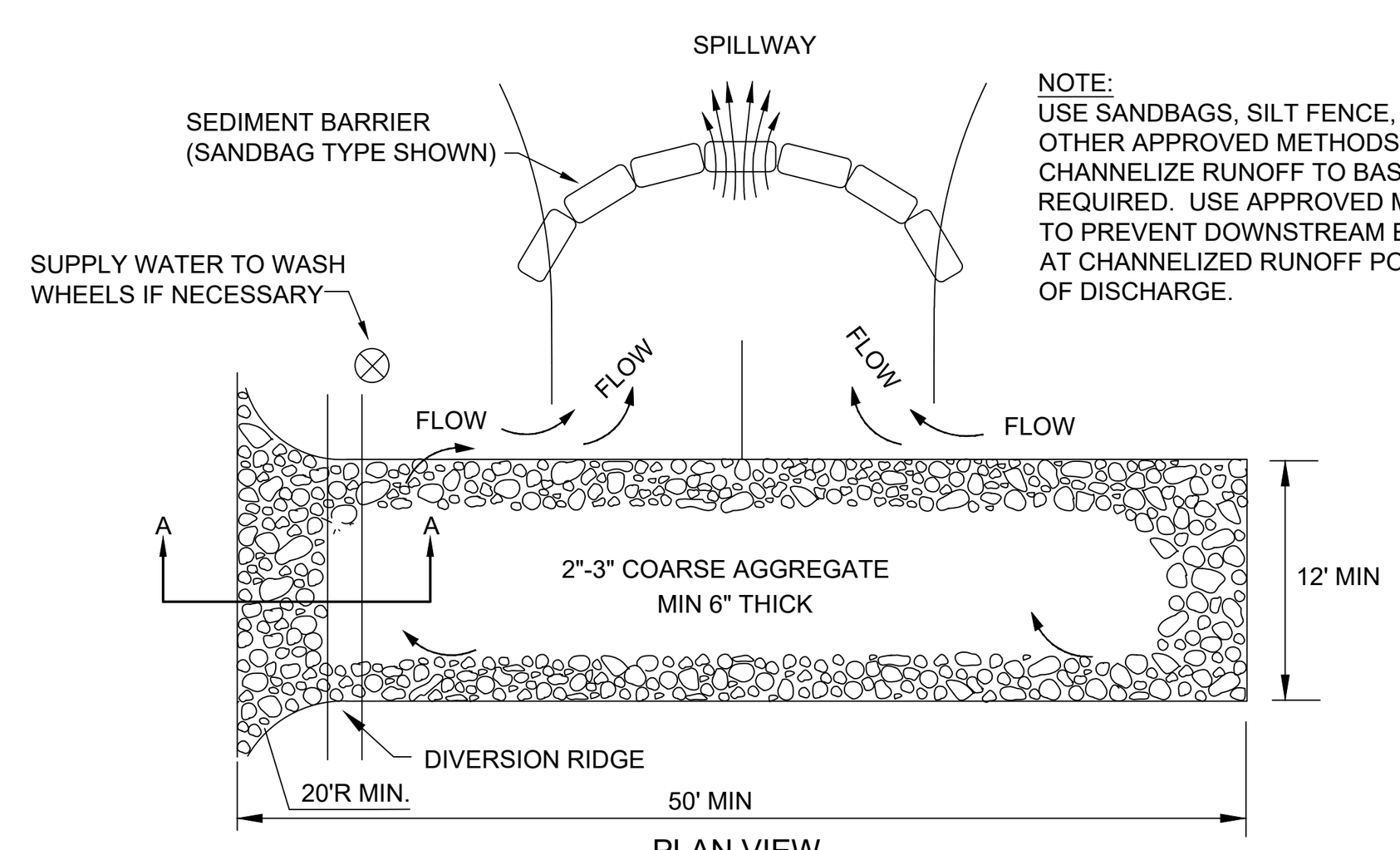
JOSHUA A. EISENOFF, P.E.
FL. PE #97911
VALID ONLY WITH STAMPED SEAL

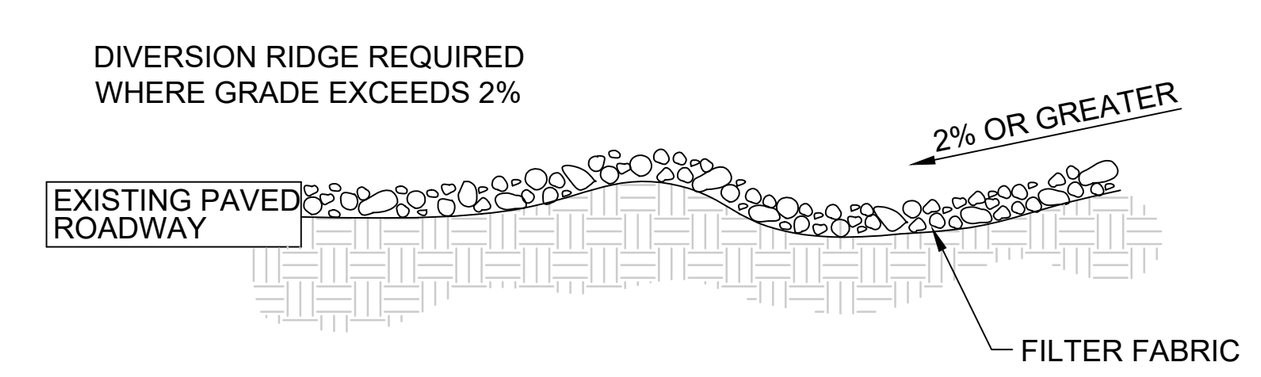
SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-04
04 OF 16

230704 - I

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CADD FILES\01-QAI CADD - VERSION I\02-PRODUCTIONS PLANS\230704-SD



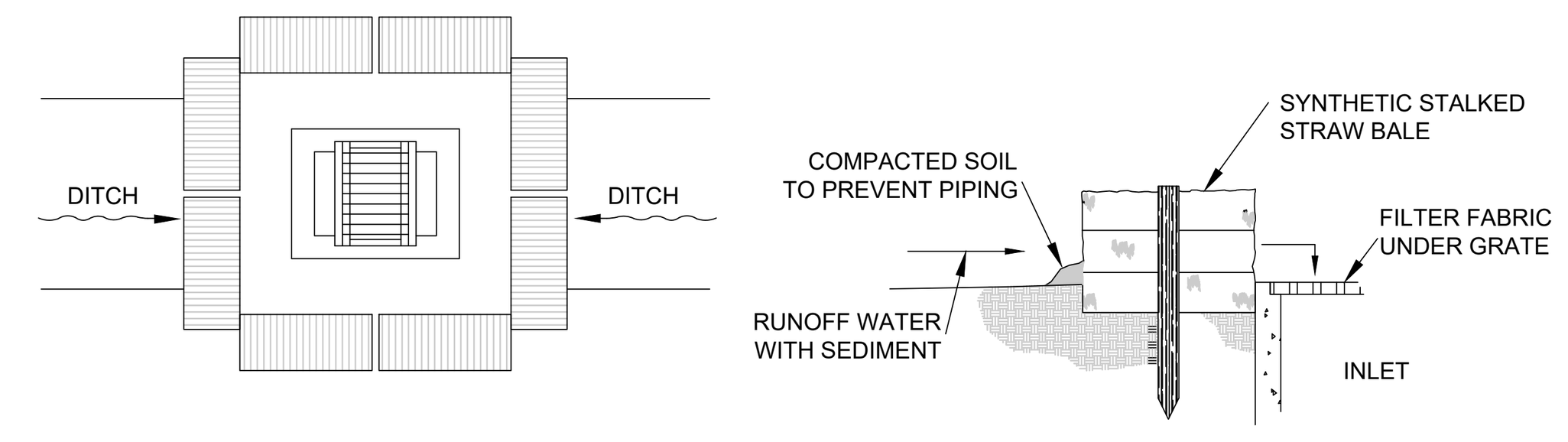


SECTION A-A

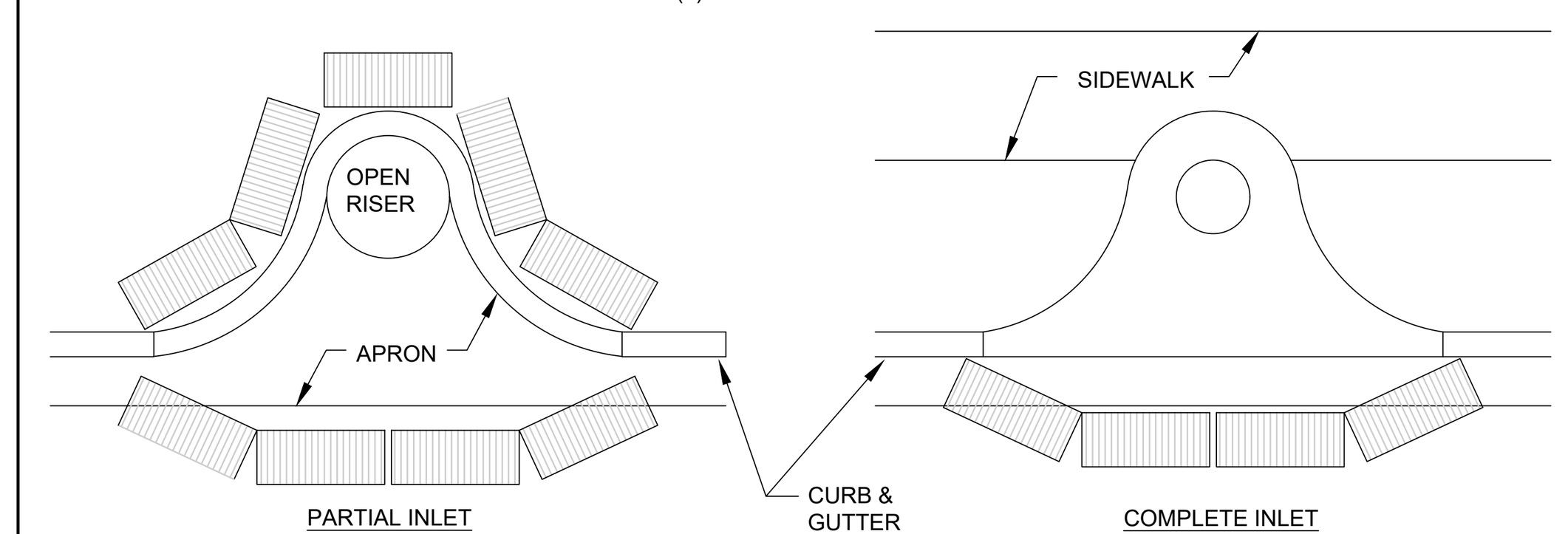
NOTES:
1. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE TOP DRESSING REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT.
2. WHEN NECESSARY, WHEELS SHALL BE CLEANED PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY.
3. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE THAT DRAINS INTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN.

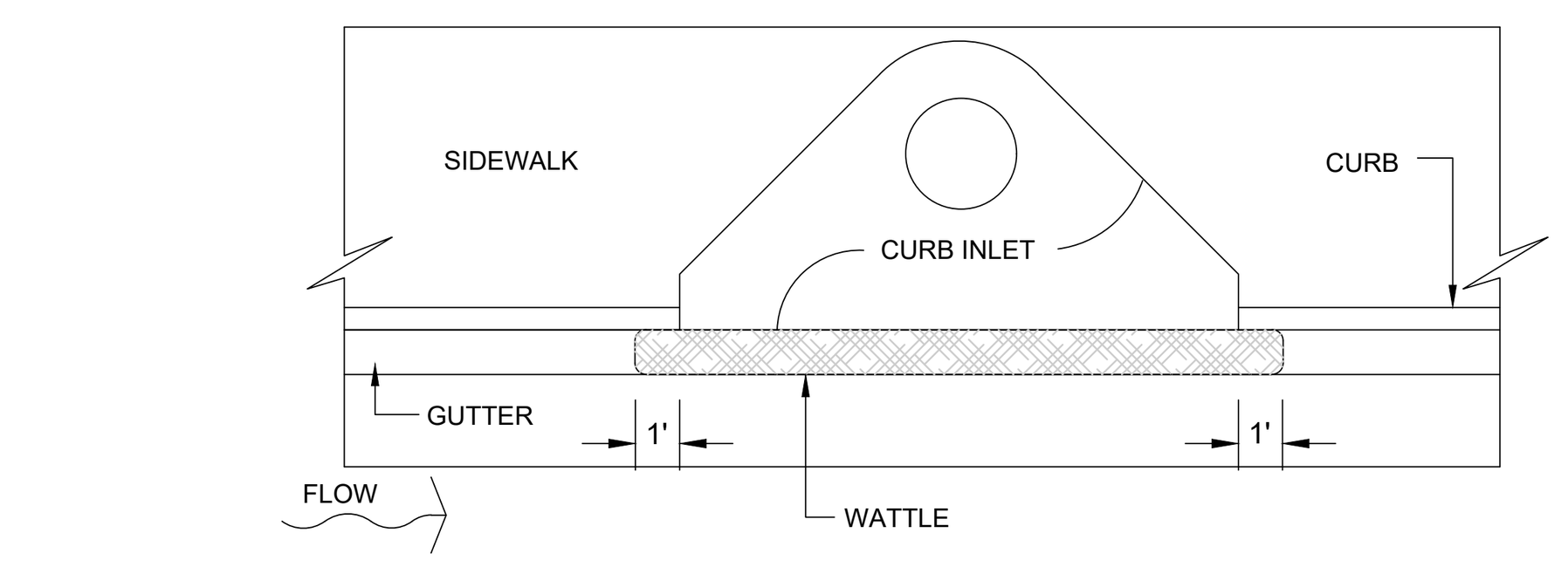
TEMPORARY CONSTRUCTION ACCESS DETAIL

N.T.S.



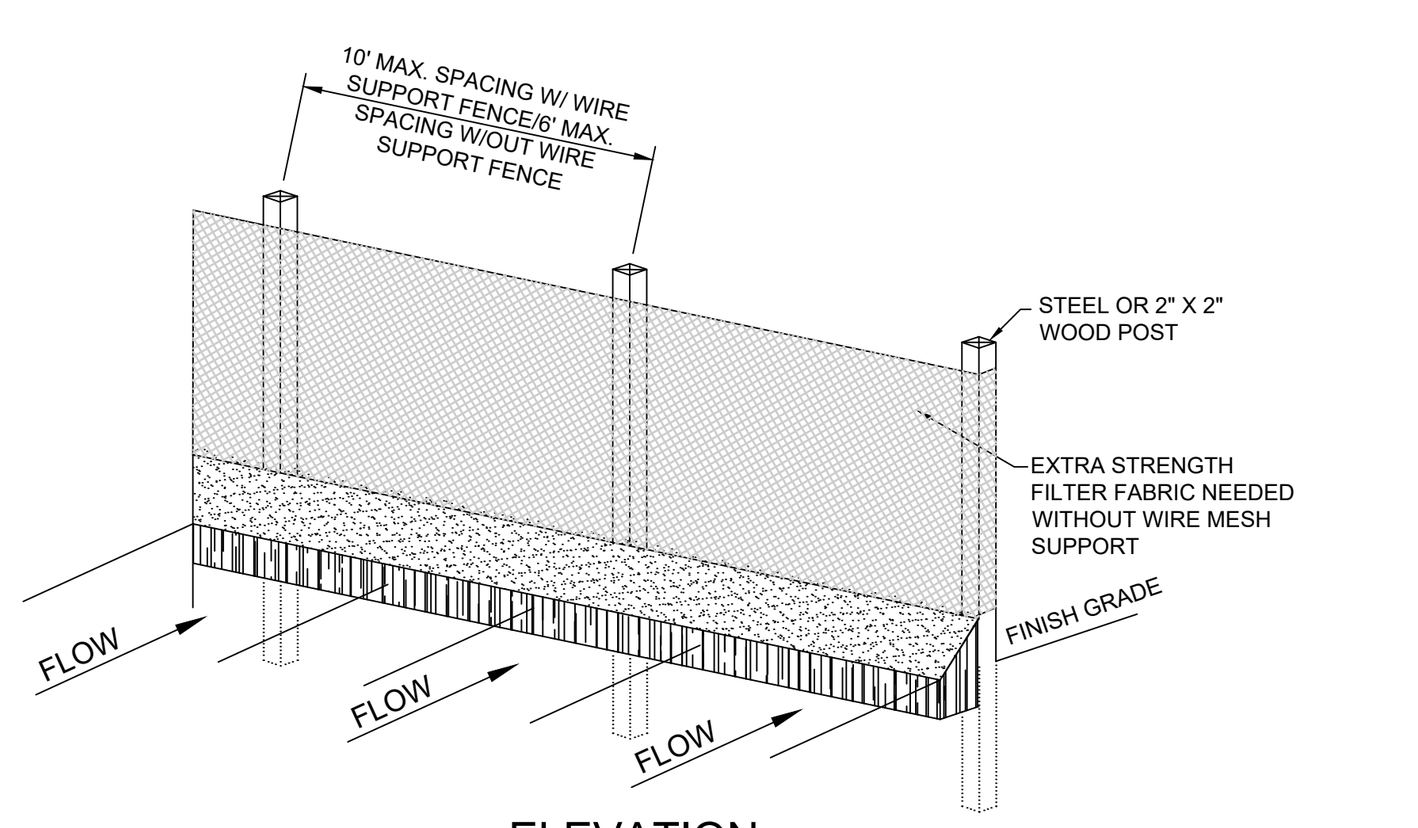
DITCH BOTTOM INLET
ANCHOR BALES WITH (2)-2"X2"X4' STAKES PER SYNTHETIC STRAW BALE.



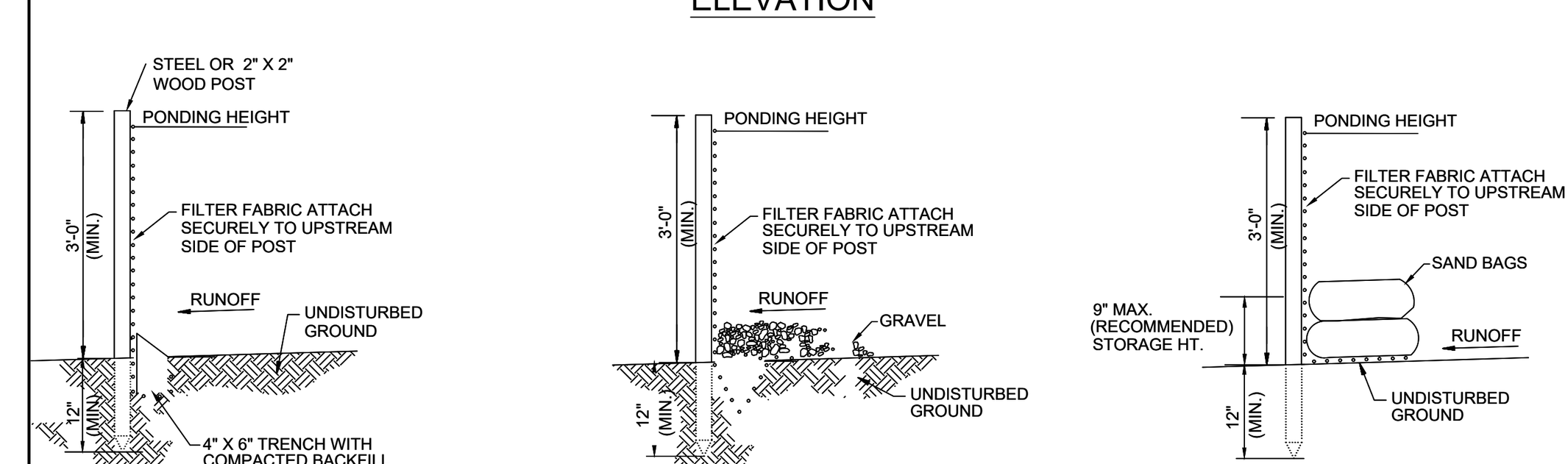


CURB INLET PROTECTION DETAIL

N.T.S.

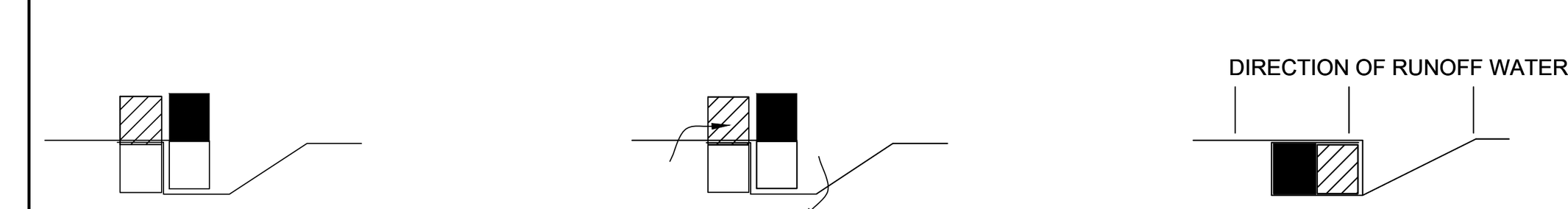


ELEVATION



STANDARD DETAIL | ALTERNATE DETAIL A | ALTERNATE DETAIL B
FOR TREE DRIP LINE OR SIDEWALKS

NOTES:
1. INSPECT AND REPAIR FENCE AFTER EACH STORM EVENT AND REMOVE SEDIMENT WHEN NECESSARY.
2. REMOVED SEDIMENT SHALL BE DEPOSITED TO AN AREA THAT WILL NOT CONTRIBUTE SEDIMENT OFF-SITE AND CAN BE PERMANENTLY STABILIZED.
3. SILT FENCE SHALL BE PLACED ON SLOPE CONTOURS TO MAXIMIZE PONDING EFFICIENCY.



PLACE THE END POST OF THE SECOND FENCE INSIDE THE END POST OF THE FIRST FENCE

ROTATE BOTH POST AT LEAST 180 DEGREES IN A CLOCKWISE DIRECTION TO CREATE A TIGHT SEAL WITH THE FABRIC MATERIAL

DRIVE BOTH POST ABOUT 10 INCHES INTO THE GROUND AND BURY FLAP

SILT FENCE DETAIL

N.T.S.

STORM WATER POLLUTION PREVENTION NOTES:

THIS PLAN HAS BEEN PREPARED TO ENSURE COMPLIANCE WITH RULES OF THE FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION, CHAPTER 17-25-FAC AND THE SOUTH FLORIDA WATER MANAGEMENT DISTRICT.

WETLANDS/BUFFERS
NO WETLANDS OR BUFFERS ARE ASSOCIATED WITH THIS PROJECT.

SWPPP INTENT
THE INTENT OF THIS SWPPP IS TO COMPLY WITH THE INTENT OF THE GENERIC PERMIT AND TO PREVENT THE RELEASE OF SOILS, TRASH, CHEMICALS, TOXINS AND OTHER POLLUTANTS, BY WATER , AIR, VEHICLE TRANSPORT OR OTHER MEANS THAT CAN IMPACT STORM WATER QUALITY. THE CONTRACTOR SHALL OBTAIN A COPY OF THE GENERIC PERMIT AND RETAIN ON-SITE FOR FUTURE REFERENCE. THE CONTRACTOR SHALL READ AND UNDERSTAND THE PERMIT, AND ENSURE THAT THE BMP'S ARE INSTALLED AND THE EXECUTION OF THE WORK IS PERFORMED TO MEET THE INTENT OF THE GENERIC PERMIT AND THE SWPPP.

POTENTIAL SOURCES OF POLLUTION
THE POTENTIAL SOURCES OF POLLUTION THAT MAY REASONABLY BE EXPECTED TO AFFECT THE QUALITY OF STORM WATER DISCHARGE ASSOCIATED WITH CONSTRUCTION ACTIVITY INCLUDE: SEDIMENT, PESTICIDES, FERTILIZER, PLASTER, CLEANING SOLVENTS, ASPHALT, CONCRETE, GLUE, ADHESIVES, PAINTS, CURING COMPOUNDS, WOOD PRESERVATIVES, HYDRAULIC OIL FLUIDS, GASOLINE, DIESEL FUEL AND KEROSENE.

GENERAL NOTES
A. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FILE "NOTICE OF INTENT TO USE GENERIC PERMIT FOR STORMWATER DISCHARGE FROM CONSTRUCTION ACTIVITIES" (DEP FORM 62-621.300(4)(B) OR LATEST VERSION) TO FDEP TO THE FOLLOWING ADDRESS OR THROUGH THE FDEP ON-LINE SYSTEM AT LEAST TWO (2) DAYS BEFORE COMMENCEMENT OF CONSTRUCTION:

NPDES STORMWATER NOTICES CENTER, MS #2510 FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION, 2600 BLAIR STONE ROAD, TALLAHASSEE, FLORIDA 32399-2400

THE CONTRACTOR SHALL SUBMIT A NOTICE OF TERMINATION (NOT) WITHIN 14 CALENDAR DAYS AFTER THE SITE HAS ACHIEVED FINAL STABILIZATION (I.E. ALL DISTURBED SOILS AT THE SITE HAVE BEEN FINAL STABILIZED), TEMPORARY BMPS HAVE BEEN REMOVED, AND STORMWATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITY FROM THE SITE AUTHORIZED BY THE PERMIT HAVE BEEN ELIMINATED.

AN ENVIRONMENTAL RESOURCE PERMIT, PERMITTED THROUGH SOUTH FLORIDA WATER MANAGEMENT DISTRICT, HAS BEEN OBTAINED FOR THIS PROJECT.
MS4 OPERATOR NAME (IF ANY): LEE COUNTY

THE CONTRACTOR SHALL PROVIDE A COPY OF THE NOI AND SUBSEQUENT NOT OR THE ACKNOWLEDGEMENT LETTERS FOR THE NOI OR NOT TO THE MS4 WITHIN 7 DAYS OF RECEIPT. THE CONTRACTOR SHALL ALSO COORDINATE WITH THE MS4 TO ENSURE THAT ALL SPECIFIC REQUIREMENTS ARE MET.

B. WHERE PRACTICAL, STORMWATER SHALL BE CONVEYED BY SWALES. SWALES SHALL BE CONSTRUCTED AS SHOWN ON PLANS.

C. EROSION CONTROL MEASURES SHALL BE EMPLOYED TO MINIMIZE TURBIDITY OF SURFACE WATERS LOCATED DOWNSTREAM OF ANY CONSTRUCTION ACTIVITY. WHILE THE VARIOUS MEASURES REQUIRED WILL BE SITE SPECIFIC, THEY SHALL BE EMPLOYED AS NEEDED IN ACCORDANCE WITH THE FOLLOWING:

I. IN GENERAL, EROSION SHALL BE CONTROLLED AT THE FURTHEST PRACTICAL UPSTREAM LOCATION.
II. NEW AND EXISTING STORMWATER INLETS AND OUTFALL STRUCTURES SHALL BE PROTECTED DURING CONSTRUCTION. PROTECTION MEASURES SHALL BE EMPLOYED IMMEDIATELY AS REQUIRED DURING THE VARIOUS STAGES OF CONSTRUCTION.
III. PERIMETER EROSION CONTROL DEVICES SHALL REMAIN IN PLACE UNTIL FINAL SITE STABILIZATION HAS BEEN ESTABLISHED

D. CLEARING AND GRUBBING OPERATIONS SHALL BE CONTROLLED SO AS TO MINIMIZE UNPROTECTED ERODIBLE AREAS EXPOSED TO WEATHER. GENERAL EROSION CONTROL BMP'S SHALL BE EMPLOYED TO MINIMIZE SOIL EROSION AND OFF-SITE SEDIMENTATION. WHILE THE VARIOUS TECHNIQUES REQUIRED WILL BE SITE AND PLAN SPECIFIC, THEY SHOULD BE EMPLOYED PRIOR TO ANY CONSTRUCTION ACTIVITY.

E. THE CONTRACTOR SHALL FURNISH, INSTALL PER THE SEQUENCE OF CONSTRUCTION, MAINTAIN AND SUBSEQUENTLY REMOVE, ALL NECESSARY TEMPORARY BMPS. THE CONTRACTOR WILL FURNISH AND INSTALL ALL NECESSARY PERMANENT BMPS.

F. THE CONTRACTOR SHALL ADJUST, ADD OR MODIFY BMPS AS NECESSARY TO COMPLY WITH THE INTENT OF THE GENERIC NPDES PERMIT AND THE SWPPP FOR NO ADDITIONAL COMPENSATION. THE CONTRACTOR SHALL CONSULT WITH THE CEC PRIOR TO ADJUSTING, ADDING OR MODIFYING BMPS THAT AFFECT THE HYDRAULICS OF THE SITE OR BEFORE ADDING BMPS NOT DETAILED IN THE SWPPP.

G. THE CONTRACTOR IS ADVISED THAT THE CONTRACT DRAWINGS ONLY INDICATE EROSION, SEDIMENT, AND TURBIDITY CONTROLS AT LOCATIONS DETERMINED IN THE DESIGN PROCESS. HOWEVER, THE CONTRACTOR IS REQUIRED TO PROVIDE ANY ADDITIONAL CONTROLS NECESSARY TO PREVENT THE POSSIBILITY OF SILTING ANY ADJACENT LOWLAND PARCEL OR RECEIVING WATER.

H. EROSION AND SEDIMENT CONTROL MEASURES ARE TO BE PLACED PRIOR TO, OR AS THE FIRST STEP IN CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH ALL EROSION CONTROL MEASURES SHOWN ON THE PLANS. THE EROSION CONTROL SYSTEM DESCRIBED WITHIN THE CONSTRUCTION DOCUMENTS SHOULD BE CONSIDERED TO REPRESENT THE MINIMUM ACCEPTABLE STANDARDS FOR THIS PROJECT. ADDITIONAL EROSION CONTROL MEASURES MAY BE REQUIRED DEPENDENT UPON THE STAGE OF CONSTRUCTION, THE SEVERITY OF THE RAINFALL EVENT AND/OR AS DEEMED NECESSARY AS A RESULT OF ON-SITE INSPECTIONS BY THE OWNER, THEIR REPRESENTATIVES, OR THE APPLICABLE JURISDICTIONAL AUTHORITIES. THESE ADDITIONAL MEASURES (IF NEEDED) SHALL BE INSTALLED AT NO ADDITIONAL COST TO THE OWNER. IT SHOULD BE NOTED THAT THE MEASURES IDENTIFIED ON THIS PLAN ARE ONLY SUGGESTED BEST MANAGEMENT PRACTICES (BMPS). THE CONTRACTOR SHALL PROVIDE POLLUTION PREVENTION AND EROSION CONTROL MEASURES AS SPECIFIED PER FDEP AND AS NECESSARY FOR EACH SPECIFIC APPLICATION. IT IS THE CONTRACTOR'S ULTIMATE RESPONSIBILITY TO ASSURE THAT THE STORMWATER DISCHARGE FROM THE SITE DOES NOT EXCEED THE TOLERANCES ESTABLISHED BY ANY OF THE APPLICABLE JURISDICTIONAL AUTHORITIES.

I. THE CONTRACTOR SHALL KEEP THE SWPPP CURRENT AT ALL TIMES. THE CONTRACTOR SHALL SIGN AND DATE ANY CHANGES TO THE SWPPP AND KEEP THEM AS ATTACHMENTS TO THE ORIGINAL PLAN. WHENEVER ANY OF THE FOLLOWING EVENTS OCCUR, THE CONTRACTOR SHALL UPDATE THE SWPPP WITHIN 7 DAYS:
I. THERE IS A CHANGE IN DESIGN, CONSTRUCTION OPERATION OR MAINTENANCE THAT HAS A SIGNIFICANT EFFECT ON THE DISCHARGE FROM THE PROJECT
II. THERE IS A NEW DISCHARGE POINT OUR OUTFALL
III. THERE IS A CHANGE IN THE LOCATION OF A DISCHARGE POINT OF OUTFALL
IV. AN INSPECTION REVEALS THAT BMPS ARE INEFFECTIVE AT ELIMINATING OR MINIMIZING POLLUTANTS IN THE STORMWATER DISCHARGED FROM THE SITE.
V. THERE IS A NEW SUBCONTRACTOR IMPLEMENTING ANY PORTION OF THE SWPPP
VI. A RELEASE CONTAINING A HAZARDOUS SUBSTANCE IN AN AMOUNT EQUAL TO OR GREATER THAN A REPORTABLE QUANTITY OCCURS DURING A 24-HOUR PERIOD

J. THE CONTRACTOR SHALL ENSURE THAT THE CONTRACTOR AND ALL SUBCONTRACTORS RESPONSIBLE FOR IMPLEMENTING SWPPP CONTROL MEASURES FILL OUT THE CONTRACTOR / SUBCONTRACTOR CERTIFICATION TABLE INCLUDED IN THIS SWPPP.

K. THE CONTRACTOR SHALL COMPLETE THE CONSTRUCTION SEQUENCE TABLE INCLUDING IN THIS SWPPP PRIOR TO PROCEEDING WITH THE INSTALLATION OF BMPS AND PRIOR TO GROUND DISTURBING ACTIVITIES. THE CONTRACTOR SHALL COMPLETE THE TABLE WITH ANTICIPATED DATES IN WHICH THE BMP WILL BE UTILIZED OR THE ACTIVITY WILL OCCUR.

STABILIZATION
A. STABILIZATION MEASURES SHALL BE INITIATED IMMEDIATELY IN PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED AND WILL REMAIN UNDISTURBED FOR 7 DAYS OR MORE. STABILIZE BY COVERING WITH ADEQUATE AMOUNTS OF MULCH OVER SEED AND PERIODICALLY WATER TO PROMOTE AND MAINTAIN GROWTH OF THE TEMPORARY GROUNDCOVER, OR BY THE USE OF AN APPROPRIATE ALTERNATIVE BMP.

B. PERMANENT SOIL EROSION CONTROL MEASURES FOR ALL SLOPES, CHANNELS, DITCHES OR ANY DISTURBED LAND AREAS SHALL BE COMPLETED IMMEDIATELY AFTER FINAL GRADING. WHEN IT IS NOT POSSIBLE TO PERMANENTLY PROTECT A DISTURBED AREA IMMEDIATELY AFTER GRADING OPERATIONS, TEMPORARY EROSION CONTROL MEASURES SHALL BE INSTALLED. ALL TEMPORARY PROTECTION SHALL BE MAINTAINED UNTIL PERMANENT MEASURES ARE IN PLACE AND ESTABLISHED.

C. ALL GRASS SLOPES CONSTRUCTED STEEPER THAN 2H:1V SHALL BE SODDED IMMEDIATELY AFTER FINAL GRADE IS ESTABLISHED.

DUST CONTROL
A. BARE EARTH AREAS SHALL BE WATERED DURING CONSTRUCTION AS NECESSARY TO MINIMIZE THE TRANSPORT OF FUGITIVE DUST. IN NO CASE SHALL FUGITIVE DUST BE ALLOWED TO LEAVE THE SITE UNDER CONSTRUCTION.

B. AS REQUIRED AFTER COMPLETION OF CONSTRUCTION, BARE EARTH AREAS SHALL BE VEGETATED.

C. AT ANY TIME BOTH DURING AND AFTER SITE CONSTRUCTION THAT WATERING AND/OR VEGETATION ARE NOT EFFECTIVE IN CONTROLLING WIND EROSION AND/OR TRANSPORT OF FUGITIVE DUST, OTHER METHODS AS ARE NECESSARY FOR SUCH CONTROL SHALL BE EMPLOYED. THESE METHODS MAY INCLUDE ERECTION OF DUST CONTROL FENCES. IF REQUIRED, DUST CONTROL FENCES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE DETAIL FOR A SILT FENCE EXCEPT THE MINIMUM HEIGHT SHALL BE 4 FEET.

WASTE MANAGEMENT
A. THE CONTRACTOR SHALL ENSURE THAT ALL WASTE AND DEBRIS ARE MANAGED DAILY SUCH THAT THEY WILL NOT IMPACT STORMWATER OR LEAVE THE PERMITTED AREA, AND DISPOSED OF PROPERLY IN ACCORDANCE WITH APPLICABLE STATE, LOCAL AND FEDERAL REGULATIONS.

B. THE CONTRACTOR SHALL ENSURE THAT ALL CHEMICALS, OILS, FUELS, HAZARDOUS WASTE, UNIVERSAL WASTE AND TOXIC SUBSTANCES ARE PROPERLY MANAGED AND DISPOSED OF IN ACCORDANCE WITH APPLICABLE STATE, LOCAL AND FEDERAL REGULATIONS. THE CONTRACTOR SHALL ENSURE THAT WASTE IS NOT DISCHARGED FROM THE SITE, AND DOES NOT IMPACT STORMWATER OR GROUNDWATER.

C. THE CONTRACTOR SHALL PROVIDE APPROPRIATE AND ADEQUATE WASHOUT FACILITIES TO ENSURE THAT CHEMICALS AND WASTE IS NOT DISCHARGED FROM THE SITE, AND DO NOT IMPACT STORMWATER OR GROUNDWATER. (E.G. CONCRETE/MASONRY WASHOUT, PAINT WASHOUT, EIFS, ETC.) THE CONTRACTOR SHALL CLEAN UP SPILLS PROMPTLY AND ENSURE THAT WASHOUT AREAS ARE PROPERLY MAINTAINED TO PROVIDE ADEQUATE VOLUME TO PREVENT OVERFLOW.

D. THE CONTRACTOR SHALL PROVIDE ADEQUATE SANITARY FACILITIES FOR SITE PERSONNEL, MAINTAIN THROUGHOUT CONSTRUCTION, AND PROVIDE FOR PROPER DISPOSAL IN ACCORDANCE WITH APPLICABLE STATE, LOCAL AND FEDERAL REGULATIONS. SANITARY FACILITIES SHALL BE PROPERLY SECURED TO PREVENT TIPPING.

E. A SPILL CONTROL AND CONTAINMENT KIT (CONTAINING FOR EXAMPLE, ABSORBENT MATERIAL SUCH AS KITTY LITTER OR SAWDUST, ACID, BASE, NEUTRALIZING AGENT, BROOMS, DUST PANS, MOPS, RAGS, GLOVES, GOGGLES, PLASTIC AND METAL TRASH CONTAINERS, ETC.) SHALL BE PROVIDED AT THE CONSTRUCTION SITE AND IT'S LOCATION(S) SHALL BE IDENTIFIED WITH LEGIBLE SIGNAGEAND SHOWN ON THE SITE MAPS.
A. THE SPILL CONTROL AND CONTAINMENT KIT SHALL BE OF SUFFICIENT QUANTITIES AND APPROPRIATE CONTENT TO CONTAIN A SPILL FROM THE LARGEST ANTICIPATED PIECE OF EQUIPMENT AND FROM THE LARGEST ANTICIPATED QUANTITIES OF PRODUCTS STORED ON THE SITE AT ANY GIVEN TIME.
B. CONTENTS SHALL BE INSPECTED DURING THE STORMWATER INSPECTION.

F. WHEN A SPILL OF REPORTABLE QUANTITIES IS DISCOVERED ON THE SITE, THE CONTRACTOR SHALL CLEAN UP ALL SPILLED MATERIALS AND DISPOSE OF IN ACCORDANCE WITH APPLICABLE LAWS AND REGULATIONS. THE CONTRACTOR SHALL NOTIFY THE APPROPRIATE AUTHORITIES IN ACCORDANCE WITH APPLICABLE LAWS AND REGULATIONS, THE OWNER AND PROJECT ENGINEER. THE CONTRACTOR SHALL RETAIN CLEANUP INFORMATION AS WELL AS DISPOSAL MANIFESTS WITH THEIR SWPPP.

MATERIALS MANAGEMENT, AND EQUIPMENT STAGING AND MAINTENANCE
A. EXCAVATED MATERIAL SHALL NOT BE DEPOSITED IN LOCATIONS WHERE IT COULD BE WASHED AWAY BY HIGH WATER OR STORM WATER RUNOFF. STOCKPILED MATERIAL SHALL BE COVERED OR ENCIRCLED WITH SEDIMENT CONTAINMENT DEVICES.

B. HEAVY CONSTRUCTION EQUIPMENT PARKING AND MAINTENANCE AREAS SHALL BE DESIGNED TO PREVENT OIL, GREASE, AND LUBRICANTS FROM ENTERING SITE DRAINAGE FEATURES INCLUDING STORMWATER COLLECTION AND TREATMENT SYSTEMS. CONTRACTORS SHALL PROVIDE BROAD DIKES OR SILT SCREENS AROUND, AND SEDIMENT SUMPS WITHIN, SUCH AREAS AS REQUIRED TO CONTAIN SPILLS OR OIL, GREASE, LUBRICANTS, OR OTHER CONTAMINANTS. CONTRACTOR SHALL HAVE AVAILABLE, AND SHALL USE, ABSORBENT FILTER PADS TO CLEAN UP SPILLS IMMEDIATELY AFTER ANY OCCURRENCE.

C. THE CONTRACTOR SHALL ENSURE THAT ALL TOXIC / HAZARDOUS SUBSTANCES AND CHEMICALS ARE PROPERLY STORED, OUT OF THE WEATHER, AND USED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. THE CONTRACTOR SHALL ENSURE THAT THESE PRODUCTS ARE STORED AND USED IN SUCH A MANNER THAT WILL NOT NEGATIVELY IMPACT STORMWATER, GROUNDWATER OR PROTECTED SPECIES.

D. THE CONTRACTOR SHALL ENSURE THAT ALL MATERIALS, EQUIPMENT, DEBRIS, WASTE, TRAILERS, AND OTHER SUPPORT RELATED ITEMS ARE CONTAINED WITHIN THE PERMITTED LIMITS OF DISTURBANCE. THE CONTRACTOR SHALL ENSURE THAT THE STORAGE AND USE OF SUCH ITEMS DOES NOT NEGATIVELY IMPACT STORMWATER OR GROUNDWATER.

OFFSITE VEHICLE TRACKING
A. THE CONTRACTOR SHALL ENSURE THAT THE CONSTRUCTION EXIT IS USED BY ALL VEHICLES AND EQUIPMENT ENTERING OR LEAVING THE JOBSITE. THE CONTRACTOR SHALL MONITOR AND MAINTAIN THE CONSTRUCTION EXIT TO ENSURE THAT NO SOILS ARE TRACKED OFFSITE BY TIRES OR TRACKS, AND THAT NO SOILS ARE SPILLED BY TRUCKS OR EQUIPMENT LEAVING THE SITE. ALL TRACKED OR SPILLED SOILS SHALL BE SHOVELED OR SWEPT FROM THE ROADWAY AND RETURNED TO THE SITE. WATER SHALL NOT BE USED TO CLEAN THE SOILS FROM THE ROADWAY UNLESS THE WATER AND SOILS ARE RECOVERED BY THE USE OF A VACUUM TRUCK OR SIMILAR DEVICE.

FERTILIZERS, HERBICIDES AND PESTICIDES
A. THE CONTRACTOR SHALL ENSURE THAT ALL FERTILIZERS, HERBICIDES, PESTICIDES AND SIMILAR PRODUCTS ARE PROPERLY STORED, OUT OF THE WEATHER, AND APPLIED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. THE CONTRACTOR SHALL ENSURE THAT THESE PRODUCTS ARE USED IN SUCH A MANNER THAT WILL NOT NEGATIVELY IMPACT STORMWATER, GROUNDWATER OR PROTECTED SPECIES.

B. NUTRIENTS SHALL BE APPLIED ONLY AT RATES NECESSARY TO ESTABLISH AND MAINTAIN VEGETATION.

INSPECTIONS AND MAINTENANCE
A. THE CONTRACTOR SHALL INSPECT BMPS (I.E. DISCHARGE LOCATIONS, CONSTRUCTION EXIT, PERIMETER CONTROLS, INLET PROTECTION, STABILIZATION, EROSION CONTROL, DOCUMENTATION, WASTE DISPOSAL AREAS, MATERIAL STORAGE AREAS, ETC.) TO ENSURE THAT BMPS ARE NOT CAUSING OR CONTRIBUTION TO VIOLATIONS OF WATER QUALITY STANDARDS OR RESULTING IN OFFSITE SEDIMENTATION; ENSURE THAT BMPS ARE INSTALLED, MAINTAINED AND OPERATING CORRECTLY AND EFFECTIVELY; ENSURE THAT BMPS ASSOCIATED WITH STORAGE AND WASTE DISPOSAL AREAS ARE BEING USED AND MAINTAINED PROPERLY; ENSURE THAT THE CONSTRUCTION EXIT IS FUNCTION PROPERLY TO PREVENT OFFSITE TRACKING OF SEDIMENT; ENSURE THAT EROSION PREVENTION MEASURES ARE MAINTAINED TO PREVENT VISIBLE EROSION OF DISTURBED AREAS AND SEDIMENTATION AT THE DISCHARGE POINTS; AND DETERMINE IF CONSTRUCTION ACTIVITIES HAVE ALTERED THE EFFECTIVENESS OF BMPS. INSPECTIONS MUST BE COMPLETED AT LEAST ONCE EVERY SEVEN (7) CALENDAR DAYS, AND WITHIN 24 HOURS AFTER A RAINSTORM OF 0.50 INCHES OR GREATER EVEN IF IT RAINS ON THE WEEKEND OR A HOLIDAY.

B. THE CONTRACTOR SHALL REPORT ALL INSPECTION FINDINGS AND CORRECTIVE ACTIONS TAKEN AS A RESULT OF THE INSPECTION USING THE STORMWATER POLLUTION PREVENTION PLAN INSPECTION REPORT FORM PROVIDED BY FDEP OR AN EQUIVALENT FORM. INSPECTION REPORTS SHALL BE SIGNED BY THE INSPECTOR AND A RESPONSIBLE AUTHORITY AS DEFINED BY THE PERMIT. INSPECTION REPORTS SHALL BE MAINTAINED WITH THE SWPPP. THE INSPECTOR MUST BE A QUALIFIED EROSION AND SEDIMENT CONTROL INSPECTOR AS DEFINED BY THE FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION.

C. ANY MAINTENANCE, REPAIR AND NECESSARY REVISIONS TO BMP ITEMS SHALL BE ADDRESSED IN A TIMELY MANNER, BUT IN NO CASE LATER THAN 7 CALENDAR DAYS FOLLOWING THE INSPECTION OR IDENTIFICATION OF THE ISSUE. UNLESS OTHERWISE SPECIFIED, ACCUMULATED SEDIMENTS SHOULD BE REMOVED BEFORE THEY REACH ONE-HALF OF THE CAPACITY OF THE CONTROL DEVICE.

ALLOWABLE NON-STORMWATER DISCHARGES
THE GENERIC PERMIT FOR STORMWATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITIES PROHIBIT MOST NON-STORMWATER DISCHARGES DURING THE CONSTRUCTION PHASE. CERTAIN DISCHARGES ARE ALLOWED BY THE PERMIT, PROVIDED APPROPRIATE BMP'S ARE UTILIZED AND THE DISCHARGE DOES NOT CAUSE OR CONTRIBUTE TO A VIOLATION OF WATER QUALITY STANDARDS. ALLOWABLE NON-STORMWATER DISCHARGES THAT OCCUR DURING CONSTRUCTION ON THIS PROJECT PER PART 3.2 OF THE GENERIC PERMIT ARE:
DISCHARGES FROM FIRE FIGHTING ACTIVITIES.
FIRE HYDRANT FLUSHINGS.
WATERS WITHOUT DETERGENTS USED TO SPRAY OFF LOOSE SOLIDS FROM VEHICLES.
WATERS USED TO CONTROL DUST.
POTABLE WATER SOURCES SUCH AS WATERLINE FLUSHINGS.
LANDSCAPE IRRIGATION AND DRAINAGE.
ROUTINE EXTERNAL BUILDING WASHDOWN PROVIDED NO DETERGENTS ARE USED.
PAVEMENT WASHWATERS THAT DO NOT CONTAIN DETERGENTS, LEAKS, SPILLS OF TOXIC OR HAZARDOUS MATERIALS.
AIR CONDITIONING CONDENSATE.
SPRING WATER.
FOUNDATION OR FOOTING DRAIN FLOWS THAT ARE NOT CONTAMINATED WITH PROCESS MATERIAL SUCH AS SOLVENTS.
NONCONTAMINATED GROUND WATER ASSOCIATED WITH DEWATERING ACTIVITIES AS DESCRIBED IN PART 3.4 OF THE GENERIC PERMIT.

RETENTION OF RECORDS
THE PERMITTEE SHALL RETAIN COPIES OF STORMWATER POLLUTION PREVENTION PLANS AND ALL REPORTS REQUIRED BY THIS PERMIT, AND RECORDS OF ALL DATA USED TO COMPLETE THE NOTICE OF INTENT TO BE COVERED BY THIS PERMIT, FOR A PERIOD OF AT LEAST THREE (3) YEARS FROM THE DATE THAT THE SITE IS FINALLY STABILIZED AND THE NOTICE OF TERMINATION (NOT) IS SUBMITTED.

NOTE:
CONTRACTOR SHALL PAY CLOSE ATTENTION WHEN CLEARING AND/OR GRADING THE SITE TO ENSURE THAT WHEN EXISTING ROOTS ARE ENCOUNTERED THEY ARE CUT OFF EVENLY WITH CLEAN SHARP PRUNING TOOLS. CONTRACTOR SHALL BE RESPONSIBLE FOR MINIMIZING THE DAMAGE OF THE EXISTING ROOT SYSTEMS.

REV	REVISION	DATE

RAY AVENUE GAS STATION
EROSION CONTROL DETAILS
604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF, STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NO. 97911; THIS ITEM HAS BEEN DIGITALLY SIGNED & SEALED BY JOSHUA A. EISENOFF, P.E. ON THE DATE INDICATED HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED & SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

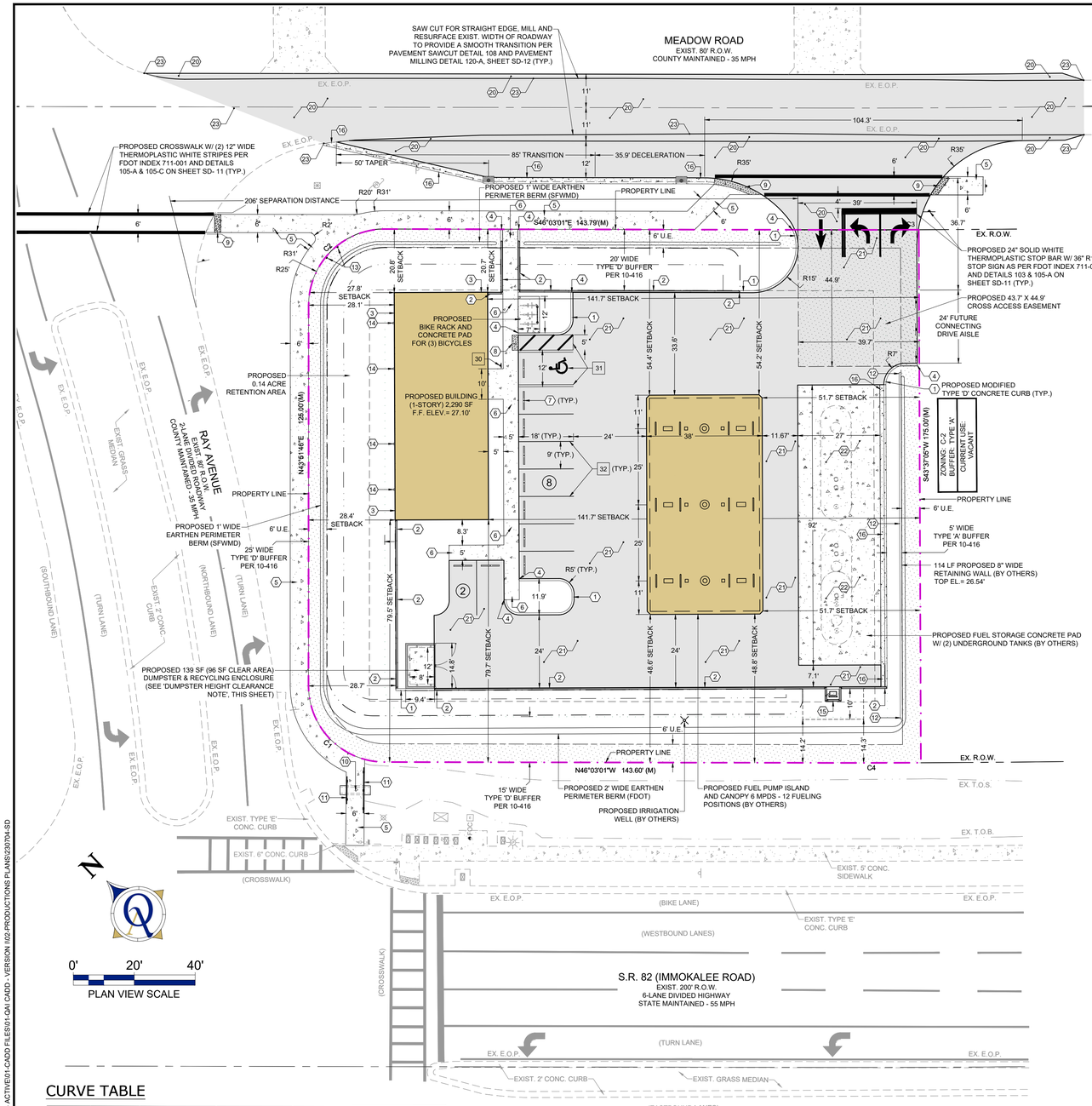
JOSHUA A. EISENOFF, P.E.
FL. PE #97911
VALID ONLY WITH STAMPED SEAL

SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-05
05 OF 16

230704 - I

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CADD FILES\01-QAI CADD - VERSION I\02-PRODUCTIONS PLANS\230704-SD



CURVE TABLE

CURVE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE
C1(P)	25.00'	39.23'	35.33'	N01°05'38"W	89°54'46"
C2(P)	25.00'	39.31'	35.38'	N88°54'22"E	90°05'14"
C3(M)	5449.05'	31.54'	31.54'	S46°12'58"E	0°19'54"
C4(M)	5624.05'	32.55'	32.55'	N46°12'58"W	0°19'54"
C5(M)	5449.05'	1361.40'	1357.86'	S53°32'21"E	14°18'54"
C6(M)	5624.05'	1405.12'	1401.47'	S53°32'21"E	14°18'54"
C7(M)	5409.05'	1232.70'	1230.03'	S52°34'44"E	13°03'27"

SITE DEVELOPMENT REQUIREMENTS:

	REQUIRED	PROVIDED
ROAD - PUBLIC	50 FT	>= 50 FT
ROAD - PRIVATE	20 FT	N/A
SIDE YARD	20 FT	>= 50 FT
REAR YARD	15 FT	15 FT
WATERBODY	25 FT	N/A
ACCESSORY STRUCTURE:		
SIDE	5 FT	N/A
REAR	5 FT	N/A
MAXIMUM LOT COVERAGE	40%	7%
MINIMUM OPEN SPACE	30%	39%
MAX BUILDING HEIGHT	35 FT **	< 35 FT
MIN. BUILDING SEPARATION	15 FT	N/A

**BUILDINGS EXCEEDING 35 FEET IN HEIGHT MUST MAINTAIN ADDITIONAL SEPARATION AND SETBACKS CONSISTENT WITH 34-2174(A) AND 34-935(E)(4)

PROPOSED PROJECT LAND USE:

TOTAL BASIN AREA	0.80	AC.	100%
BUILDING GROUND FLOOR AREA	0.11	AC.	14%
PAVEMENT AREA	0.29	AC.	36%
OTHER IMPERVIOUS	0.08	AC.	10%
PERVIOUS ACREAGE	0.32	AC.	40%
TOTAL IMPERVIOUS AREA	0.48	AC.	60%
TOTAL PERVIOUS AREA	0.32	AC.	40%
IMPERVIOUS AREA OUTSIDE OF BASIN*	0.07	AC.	N/A
TOTAL PROJECT AREA	0.80	AC.	N/A

*IMPERVIOUS AREA OUTSIDE BASIN CONSISTS OF DRIVEWAY CONNECTION WITH RIGHT TURN LANE

LEE PLAN CONSISTENCY:

THE PROPOSED PROJECT IS CONSISTENT WITH THE LEE COUNTY LAND DEVELOPMENT CODE AND THE GOALS, OBJECTIVES & POLICIES SET FORTH IN THE LEE PLAN. INCLUDING BUT NOT LIMITED TO STANDARDS AND PROVISIONS, SITE LOCATION CRITERIA, ACREAGE ALLOCATION TABLE, LAND USE CATEGORY & LAND USE MAP.

PROJECT PHASING PLAN:

THE PROJECT SHALL BE CONSTRUCTED IN ONE (1) PHASE.

PHASE 1 - ALL OFFSITE IMPROVEMENTS, ONSITE INFRASTRUCTURE TO INCLUDE UTILITIES, DRAINAGE, & PARKING LOT.

NOTES:

1. ALL EXISTING AND PROPOSED GRADES ARE N.A.V.D. OF 1988.
2. THIS PROJECT PRESENTS NO ADVERSE IMPACTS ON LOCAL GROUND OR SURFACE WATERS.
3. THIS PROJECT DOES NOT ADVERSELY IMPACT THE FLOOD PLAIN OR ANY RIVERINE AREAS.
4. THIS SITE CAN BE USED SAFELY FOR THE DEVELOPMENT AS SHOWN WITHOUT UNDUE DANGER FROM FLOOD OR ADVERSE SOIL AND/OR FOUNDATION CONDITIONS.
5. THIS SITE DOES NOT EXHIBIT ANY SALT WATER PONDING.

PUBLIC TRANSIT: LDC 10-441

DISTANCE TO NEAREST PUBLIC TRANSIT ROUTE: 2.6 MILES - STOP I.D.: 11719 @ COLONIAL BLVD. & S.R. 82

EXISTING CONDITIONS ARE BASED ON INFORMATION PROVIDED BY
K.E. TRASK, P.A. (LB8450)
12345 BLASINGIM ROAD
FT. MYERS, FL. 33966
(239) 980-1495

DUMPSTER HEIGHT CLEARANCE NOTE:

PROPOSED DUMPSTER SHALL BE CONSTRUCTED IN A LOCATION TO ALLOW FOR A MINIMUM OF 22' OF VERTICAL CLEARANCE AND NO OBSTRUCTIONS

LANDSCAPING NOTE:

ALL DISTURBED AREAS IN THE FDOT RIGHT-OF-WAY WILL BE SODDED (SEE LANDSCAPING PLANS)

ARCHITECTURAL NOTE:

ARCHITECT WILL BE REQUIRED TO DESIGN PROPOSED BUILDING TO MEET DESIGN PARAMETERS SPECIFIED IN LEE COUNTY LAND DEVELOPMENT CODE SECTION 33-1415 AND 10-620.

ADDITIONAL NOTES:

STREET NUMBER TO BE POSTED ON PROPOSED BUILDING IN A MANNER THAT IS CLEARLY VISIBLE FROM THE ACCESS POINTS PER LEE COUNTY LAND DEVELOPMENT CODE SECTION 33-1404 (1).

SITE PLAN LEGEND:

- EXST.FEATURES (ROADS, WALKS, ECT.)
- CENTER LINE
- SITE RIGHT OF WAY
- SITE BOUNDARY
- SITE EASEMENT
- SITE PARCEL / LOT LINE
- SITE BUFFER
- DRAINAGE TOB
- DRAINAGE TOE
- SD ASPHALT
- HD ASPHALT
- EXST. CONCRETE
- PROPOSED CONCRETE
- EXST. TOPO ELEV. (NAVD)
- PUBLISHED BENCH MARK
- EXST. STREET SIGN
- EXST. GATE VALVE
- EXST. WATER METER
- EXST. DRAINAGE INLET
- EXST. POWER POLE
- EXST. LIGHT POLE
- OVERHEAD POWER LINES
- EXST. ELEC. PULL BOX
- EXST. FIBER OPTIC MARKER
- EXST. ELECTRIC METER
- EXST. TRAFFIC LITE & POLE
- EXST. TRAFFIC LITE BOX

SITE KEYNOTES LEGEND

1. MODIFIED TYPE 'D' CURB, REFER TO DETAIL 104-D ON SD-11.
2. CONCRETE BLOCK RETAINING WALL W/ MODIFIED TYPE 'D' CONCRETE CURB & MINIMUM 42" HIGH GUIDERAIL (FDOT INDEX 515-080) OR FENCE, REFER TO CROSS SECTIONS ON SD-09 & SD-10.
3. COMBINATION CONCRETE BLOCK RETAINING WALL & BUILDING STEM WALL, REFER TO CROSS SECTIONS ON SD-09 & SD-10.
4. 6" NOSE DOWN OF OF TYPE 'D' CURB, REFER TO DETAIL 113-B ON SD-11.
5. 6' WIDE OFF-SITE LEE COUNTY DOT CONCRETE SIDEWALK, REFER TO DETAIL 110-H ON SD-12.
6. 5' WIDE ON-SITE STANDARD FLUSH SIDEWALK. WHERE ABUTS PARKING STALLS (TYP), REFER TO DETAILS 117-A & 118 ON SD-11.
7. WHEEL STOP (TYP), REFER TO DETAIL 101 ON SD-11.
8. CONCRETE HANDICAP RAMP - MAXIMUM SLOPE 1:12 (8.33%) (PER A.D.A. REQUIREMENTS AND FDOT INDEX NO. 522-002, CR-C. MAXIMUM CROSS SLOPE 1:50 (2.00%), REFER TO DETAIL 117-A (FLUSH SIDEWALK AT HANDICAP RAMP) ON SD-11 W/ DETECTABLE WARNING, REFER TO DETAIL 102 (DETECTABLE WARNING) ON SD-11. COLOR SHALL BE YELLOW.
9. 2' WIDE ADA DETECTABLE WARNING, REFER TO DETAIL 102 ON SD-11. COLOR SHALL BE YELLOW.
10. RETRACTABLE VEHICULAR GATE W/ KNOX BOX & DIGITAL KEYPAD ENTRY (BY OTHERS).
11. 42" HIGH STEEL PIPE GUIDERAIL PER FDOT INDEX 515-080.
12. 8" WIDE CONCRETE BLOCK RETAINING WALL (BY OTHERS)
13. 6" DIA. DIP BOLLARD PAINTED RED (FIRE HYDRANT PROTECTION)
14. ROOF DRAINS (BY OTHERS - SEE BUILDING PLANS)
15. ABOVE GROUND PIPING & VENTS TO UNDERGROUND FUEL TANKS (BY OTHERS)
16. TYPE 'F' CONCRETE CURB, REFER TO DETAIL 104-F ON SD-11.
20. LEE COUNTY DOT MINOR COLLECTOR PAVEMENT, REFER TO DETAIL 110-C ON SD-12.
21. ON-SITE STANDARD-DUTY PAVEMENT, REFER TO DETAIL 112-B ON SD-11.
22. STANDARD-DUTY CONCRETE, REFER TO DETAIL 112-E ON SD-11.
23. SAW CUT/MILL PAVEMENT TO MATCH EXISTING, REFER TO DETAIL 108 & 120-A ON SD-12.
30. HANDICAP SIGN (FTP 25 700-1-10), REFER TO DETAILS 100-A & 103 ON SD-11.
31. HANDICAP ALL WEATHER STRIPING PER FDOT INDEX NO. 711-001, REFER TO DETAIL 100-A ON SD-11.
32. 6" WIDE ALL WEATHER SOLID WHITE LINE (STANDARD PARKING STALL - TYP), REFER TO DETAIL 100-A ON SD-11.
33. 6" WIDE ALL WEATHER SOLID WHITE LINE (PARALLEL PARKING STALL), REFER TO DETAIL 100-A ON SD-11.

PARKING CALCULATIONS: LDC 34-2020

CONVENIENCE FOOD & BEVERAGE STORE W/ (6) FUEL PUMPS (GROSS FLOOR AREA = GFA)

PARKING SPACES REQUIRED: 1 SPACE / 200 SQ. FT. OF GFA (MINUS 1 SPACE / 4 FUEL PUMPS)

TOTAL BUILDING GFA:	2,290 SF
TOTAL NUMBER OF SPACES REQUIRED:	2,290/200 = 11
(MINUS 1 SPACE / 4 FUEL PUMPS = 3 SPACES)	11 - 3 = 8
TOTAL NUMBER OF HANDICAP SPACES REQUIRED	1
TOTAL NUMBER OF HANDICAP SPACES PROVIDED	1
TOTAL NUMBER OF SPACES PROVIDED *	8

BICYCLE PARKING REQUIREMENTS PER LDC 10-610(d)(3)

SPACES TOTALING FIVE PERCENT OF REQUIRED MOTOR VEHICLE SPACES.
REQUIRED: 8 X 5% = 1 BICYCLE SPACE
PROVIDED: 1 BICYCLE RACK & CONCRETE PAD FOR STORAGE OF 3 BICYCLES
* HANDICAP SPACES ARE INCLUDED IN TOTAL SPACES PROVIDED

REFUSE AND SOLID WASTE DISPOSAL FACILITIES: LDC 10-261

1. A MINIMUM OVERHEAD CLEARANCE OF 22 FEET IS REQUIRED AND A 12' WIDE UNOBSTRUCTED ACCESS OPENING MUST BE PROVIDED TO ACCOMMODATE ALL STORAGE AREAS/CONTAINERS.
2. ALL STORAGE AREAS / CONTAINERS MUST BE ADEQUATELY SHIELDED BY LANDSCAPED SCREEN OR SOLID FENCING ALONG AT LEAST THREE SIDES. USE OF CHAIN LINK FENCING TO MEET THIS REQUIREMENT IS PROHIBITED. REFER TO LEE COUNTY LCD 10-610(c)(2) FOR GUIDELINES.
3. COMMERCIAL, INDUSTRIAL & MULTIFAMILY DEVELOPMENT USING A COMPACTOR FOR GARBAGE COLLECTION MUST PROVIDE SUFFICIENT SPACE FOR THE COMPACTOR (INCLUDING RECEIVER) IN ADDITION TO SPACE REQUIRED FOR RECYCLABLE COLLECTION.
4. ENCLOSURE SETBACKS. CONTAINER SPACE ENCLOSURES USES AS PROVIDED IN ACCORD WITH LEE COUNTY LDC 10-416(d)(3) & (4). CONCRETE WALL ENCLOSURES MAY NOT BE LOCATED WITHIN A PUBLIC UTILITY OR DRAINAGE EASEMENT.

0-5,000 SF REQUIRE 60 SF (GARBAGE) & 24 SF (RECYCLABLE), TOTAL = 84 SF
5,001 - 10,000 SF REQUIRES 80 SF (GARBAGE) & 48 SF (RECYCLABLE), TOTAL = 128 SF
10,001 - 25,000 SF REQUIRES 120 SF (GARBAGE) & 96 SF (RECYCLABLE), TOTAL = 216 SF
25,000 SF + REQUIRES 216 SF FOR THE FIRST 25,000 SF PLUS 8 SF FOR EACH ADDITIONAL 1,000 SF.

BUILDING USE:2,220 SF CONVENIENCE FOOD & BEVERAGE STORE W/ (12) FUEL PUMPS
MINIMUM 60 SF AREA FOR GARBAGE + MINIMUM 24 SF FOR RECYCLABLES = 84 SF

TOTAL REQUIRED DISPOSAL AREA (SF) =	84 SF
TOTAL PROVIDED DISPOSAL AREA (SF) =	96 SF

DATE	REVISION	REV

RAY AVENUE GAS STATION
MASTER SITE PLAN

604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF, STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NO. 97911. THIS ITEM HAS BEEN DIGITALLY SIGNED & SEALED BY JOSHUA A. EISENOFF, P.E. ON THE DATE INDICATED HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

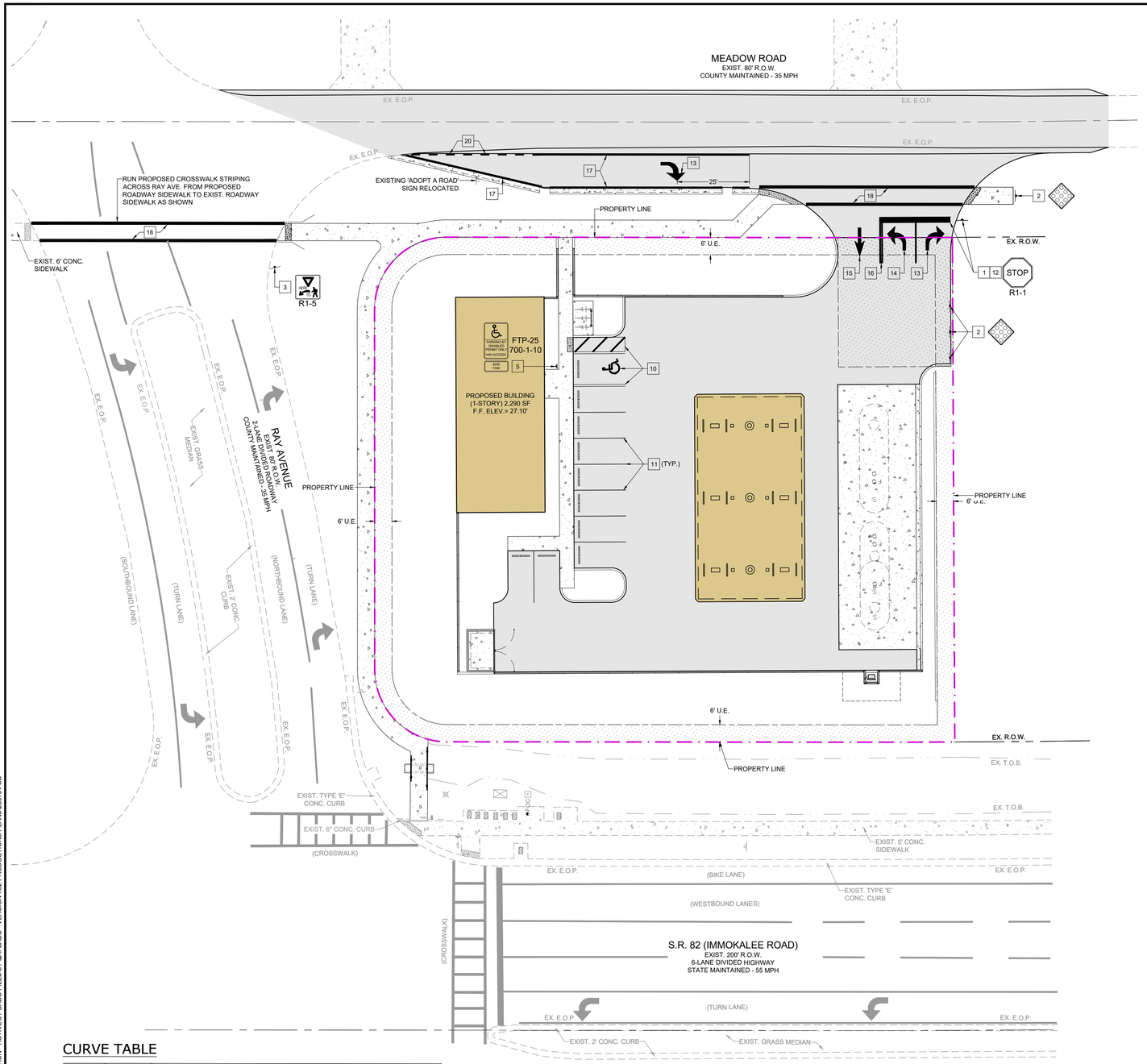
JOSHUA A. EISENOFF, P.E.
FL. PE #97911
VALID ONLY WITH STAMPED SEAL

SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-06
06 OF 16

230704 - I

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CADD FILES\01-QA\ CADD - VERSION \02-PRODUCTIONS PLANS\230704-SD



CURVE TABLE

CURVE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE
C1(P)	25.00'	39.23'	35.33'	N01°05'38"W	89°54'46"
C2(P)	25.00'	39.31'	35.38'	N88°54'22"E	90°05'14"
C3(M)	5449.05'	31.54'	31.54'	S46°12'58"E	0°19'54"
C4(M)	5624.05'	32.55'	32.55'	N46°12'58"W	0°19'54"
C5(M)	5449.05'	1361.40'	1357.86'	S53°32'21"E	14°18'54"
C6(M)	5624.05'	1405.12'	1401.47'	S53°32'21"E	14°18'54"
C7(M)	5409.05'	1232.70'	1230.03'	S52°34'44"E	13°03'27"

N

0' 20' 40'

PLAN VIEW SCALE

SIGNAGE & STRIPING KEYNOTES LEGEND

1	STOP SIGN (R1-1)
2	CASE II RED REFLECTOR SIGN (END OF ROAD / SIDEWALK)
3	YIELD HERE TO PEDESTRIANS LEFT ARROW SIGN (R1-5)
4	DO NOT ENTER SIGN (R5-1)
5	HANDICAP SIGN (FTP 25 700-1-10), REFER TO DETAIL 103 SD-11.
10	HANDICAP ALL WEATHER STRIPING PER FDOT INDEX NO. 711-001, REFER TO DETAIL 100-A ON SD-11.
11	6" WIDE ALL WEATHER SOLID WHITE LINE (STANDARD & PARALLEL PARKING STALL), REFER TO DETAIL 100-A ON SD-11.
12	24" WIDE THERMOPLASTIC SOLID WHITE PAINTED STOP BAR W/ STOP SIGN (R1-1), PER FDOT INDEX NO. 711-001, REFER TO DETAILS ON SD-11.
13	THERMOPLASTIC SOLID WHITE PAINTED RIGHT TURN ARROW, REFER TO FDOT INDEX 711-001 FOR PAVEMENT MARKINGS
14	THERMOPLASTIC SOLID WHITE PAINTED LEFT TURN ARROW, REFER TO FDOT INDEX 711-001 FOR PAVEMENT MARKINGS
15	THERMOPLASTIC SOLID WHITE PAINTED THROUGH LANE ARROW, REFER TO FDOT INDEX 711-001 FOR PAVEMENT MARKINGS
16	THERMOPLASTIC DOUBLE 6" SOLID YELLOW STRIPE, REFER TO FDOT INDEX 711-001 FOR PAVEMENT MARKINGS
17	THERMOPLASTIC 6" SOLID WHITE STRIPE, REFER TO FDOT INDEX 711-001 FOR PAVEMENT MARKINGS
18	THERMOPLASTIC 12" SOLID WHITE STRIPES SPACED 6' APART IN LEE COUNTY DOT R.O.W. AND 5' APART ON-SITE, REFER TO FDOT INDEX 711-001 FOR PAVEMENT MARKINGS
19	THERMOPLASTIC 4" SOLID WHITE STRIPE, REFER TO FDOT INDEX 711-001 FOR PAVEMENT MARKINGS
20	THERMOPLASTIC 6" SOLID WHITE DASHED STRIPE (6' O.C.), REFER TO FDOT INDEX 711-001 FOR PAVEMENT MARKINGS

SIGNS & PAVEMENT MARKING LEGEND

STOP R1-1	36" x 36" STOP SIGN (R1-1) WITH 24" SOLID WHITE STOP BAR
R1-5	30"x30" YIELD HERE TO PEDESTRIANS LEFT ARROW SIGN (R1-5)
	18"x18" CASE II RED REFLECTOR SIGN (END OF ROAD / SIDEWALK)
DO NOT ENTER	36"x36" DO NOT ENTER SIGN(R5-1)
	FDOT SIGN & POST REFER TO SIGN MOUNTING, PER FDOT INDEX 700-010-1
	12"x24" HANDICAP PARKING ONLY SIGN (FTP-25, 700-1-10)
FTP-25 700-1-10	
	WHITE THERMOPLASTIC THROUGH LANE-USE ARROW
	WHITE THERMOPLASTIC RIGHT TURN ARROW
	24" WHITE THERMOPLASTIC STOP BAR
	REFER TO FDOT INDEX 711-001

SIGNING AND PAVEMENT MARKING NOTES:

1. ALL SIGNING AND PAVEMENT MARKINGS SHALL BE IN ACCORDANCE WITH THE F.D.O.T. STANDARDS FOR ROAD AND BRIDGE CONSTRUCTION, LATEST EDITION, THE F.D.O.T. ROADWAY AND TRAFFIC DESIGN STANDARDS, LATEST EDITION, THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES AND LEE COUNTY DEPARTMENT OF TRANSPORTATION PUBLISHED STANDARDS.
2. PAVEMENT MARKINGS SHALL BE PERMANENT TRAFFIC PAINT IN ACCORDANCE WITH F.D.O.T. STANDARD SPECIFICATIONS SECTION 710.
3. MATCH EXISTING PAVEMENT MARKINGS AT EXISTING ROADS.
4. REMOVE ANY EXISTING SIGNS OR PAVEMENT MARKINGS IN CONFLICT WITH THOSE SHOWN ON PLANS.
5. ALL STOP SIGN LOCATIONS SHALL INCLUDE A 24" PAINTED WHITE STOP BAR UNLESS NOTED OTHERWISE.
6. THE CONTRACTOR SHALL COMPLY WITH THE "STATE OF FLORIDA MANUAL OF TRAFFIC CONTROL AND SAFE PRACTICES FOR STREET AND HIGHWAY CONSTRUCTION, MAINTENANCE, AND UTILITY OPERATIONS" AND WITH THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES".
7. THE CONTRACTOR SHALL FURNISH AND INSTALL STREET NAME SIGNS IN ACCORDANCE WITH LEE COUNTY DEVELOPMENT STANDARDS.
8. ALL PAVEMENT MARKINGS WITHIN THE FDOT RIGHT OF WAY SHALL BE THERMO PLASTIC
9. ALL EXISTING PAVEMENT MARKINGS THAT ARE IN CONFLICT WITH PROPOSED MARKINGS SHALL BE REMOVED BY HYDRO BLASTING.
10. THE DEVELOPMENT MUST MEET THE FDOT MINIMUM DRIVEWAY LENGTH PER OF 30' FOR SMALL COMMERCIAL DEVELOPMENTS.

SITE PLAN LEGEND:

HD ASPHALT	EXST.FEATURES (ROADS, WALKS, ECT.)
SD ASPHALT	CENTER LINE
EXST. CONCRETE	SITE RIGHT OF WAY
PROPOSED CONCRETE	SITE BOUNDARY
EASEMENT	SITE EASEMENT
SITE PARCEL / LOT LINE	SITE BUFFER

REV	REVISION	DATE

RAY AVENUE GAS STATION
SIGNAGE & STRIPING PLAN

604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF,
STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NO. 97911. THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY JOSHUA A. EISENOFF, P.E.
ON THE DATE INDICATED HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED & SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JOSHUA A. EISENOFF, P.E.
FL. PE #97911

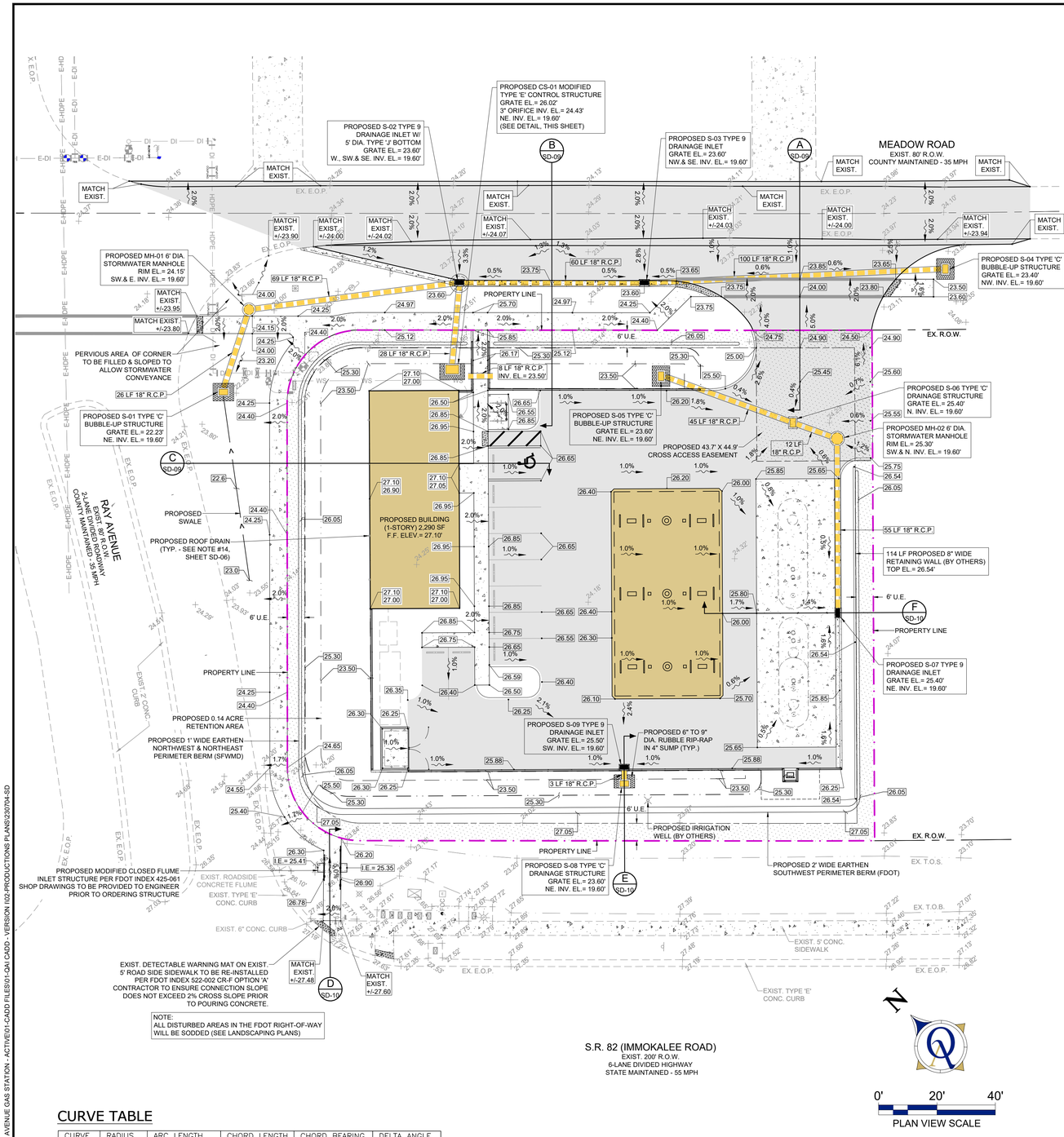
VALID ONLY WITH STAMPED SEAL

SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-07
07 OF 16

230704 - I

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CADD FILES\01-QAI CADD - VERSION I\02-PRODUCTIONS PLANS\230704-SD



CURVE TABLE

CURVE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE
C1(P)	25.00'	39.23'	35.33'	N01°05'38"W	89°54'46"
C2(P)	25.00'	39.31'	35.38'	N88°54'22"E	90°05'14"
C3(M)	5449.05'	31.54'	31.54'	S46°12'58"E	0°19'54"
C4(M)	5624.05'	32.55'	32.55'	N46°12'58"W	0°19'54"
C5(M)	5449.05'	1361.40'	1357.86'	S53°32'21"E	14°18'54"
C6(M)	5624.05'	1405.12'	1401.47'	S53°32'21"E	14°18'54"
C7(M)	5409.05'	1232.70'	1230.03'	S52°34'44"E	13°03'27"

SITE DATA:

MINIMUM PAVEMENT ELEV.
ELEV. 25.37' NAVD

MINIMUM FINISH FLOOR ELEVATION:
ELEV. 26.93' NAVD

AVERAGE EXISTING GROUND ELEVATION:
ELEV. 24.25' NAVD

ZONING:
C-2 COMMERCIAL

CONTROL ELEVATION:
ELEV. 22.50' NAVD

PERIMETER BERM ELEVATION:
NE, NW & SW BOUNDARIES: 26.05' NAVD (SFWMD)
SE BOUNDARY: 27.05' NAVD (FDOT)

STRAP NUMBERS:
05-45-26-03-00035.0010

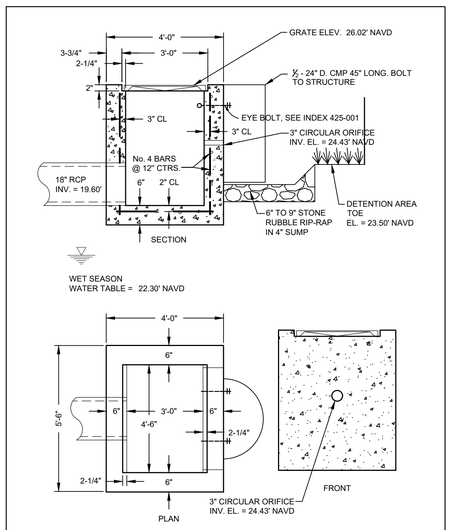
SOILS:
13 - BOCA FINE SAND, URBAN LAND COMPLEX - 31%
42 - WABASSO SAND, LIMESTONE SUBSTRATUM - 69%

FLUCCS CODE:
411-E3 (PINE FLATWOODS) - 100%

FEMA FLOOD ZONE:
THIS PROPERTY IS LOCATED FLOOD ZONE X BASE FLOOD ELEV. = NONE
IN FIRM COMMUNITY 125124 PANEL NO.0475 MAP #: 12071C0475F DATED:08/28/2008

SURFACE WATER MANAGEMENT SUMMARY TABLE

SFWMD	RAINFALL (INCES)	STAGE (FT-NAVD)	*STAGE (FT-NGVD)*
10 YEAR, 24-HOUR STORM EVENT	6.50"	25.37	*26.57*
25 YEAR, 72-HOUR PEAK STORM EVENT	11.00"	26.02	*27.22*
25 YEAR, 72-HOUR BACK-TO-BACK STORM EVENT	11.00"	26.04	*27.24*
100 YEAR, 72-HOUR STORM EVENT (ZERO DISCHARGE)	14.00"	26.93	*28.13*
CONTROL ELEVATION		22.50	*23.70*
WET SEASON WATER TABLE		22.50	*23.70*
MINIMUM PARKING LOT & ROAD CROWN ELEVATION		25.37	*26.57*
MINIMUM PERIMETER BERM ELEVATION		26.04	*27.24*
MINIMUM FINISH FLOOR ELEVATION		26.93	*28.13*
FDOT			
CRITICAL STORM EVENT: 100 YEAR, 8-HOUR PEAK STAGE		26.04	*27.24*
MINIMUM EARTHEN BERM ELEVATION		27.04	*28.24*
MINIMUM HARDEN BERM ELEVATION		26.54	*27.74*



CS-01 MODIFIED TYPE 'E' CONTROL STRUCTURE DETAIL

N.T.S.

GRADING AND DRAINAGE LEGEND:

	R/W PAVEMENT		TOP OF BANK
	STANDARD-DUTY PAVEMENT		TOE OF SLOPE
	CONCRETE		EDGE OF WATER
	RUBBLE RIP-RAP		PROPERTY LINE
	PROPOSED BUILDING		RIGHT OF WAY
	EASEMENT		EASEMENT
	PERIMETER BERM	#.##%	DRAINAGE FLOW ARROW WITH PERCENTAGE
	'V' BOTTOM DITCH/SWALE	16.50	PAVEMENT (NAVD)
+.2	EXISTING TOPO (NAVD)	16.5	GROUND (NAVD)
16.83 / 16.50	TOP OF CURB / PAVEMENT		

DRAINAGE SYSTEM

EXISTING	PROPOSED	
		MITERED END SECTION (MES)
		DRAINAGE PIPE W/ MES
		DRAINAGE PIPE
		DRAINAGE STRUCTURE W/ PIPE

B / SD-09 — CROSS SECTION LOCATION
B = NAME OF SECTION
SD-2 = SHEET WITH SECTION

GENERAL DRAINAGE NOTES:

1. THE LENGTH OF STORM DRAIN PIPES SHOWN ON PLANS ARE APPROXIMATE AND HAVE BEEN MEASURED FROM THE INSIDE FACE OF STRUCTURE.
2. LOCATIONS OF DRAINAGE STRUCTURES MAY BE FIELD ADJUSTED TO PRESERVE EXISTING VEGETATION AS APPROVED BY THE ENGINEER.
3. THE CONTRACTOR IS REQUIRED TO ADJUST ALL EXISTING AND PROPOSED VALVE BOXES, MANHOLE RIMS, GRATES, ETC. AS REQUIRED TO MATCH PROPOSED GRADES.
4. EXISTING OFF-SITE DRAINAGE PATTERNS SHALL BE MAINTAINED DURING CONSTRUCTION
5. SWALE PROFILES SHOWN ARE TO TOP OF SOD. CONTRACTOR TO ADJUST SWALE GRADING ACCORDINGLY.
6. UNLESS OTHERWISE NOTED, SOD A 12" STRIP BEHIND ALL CONCRETE CURB AND VALLEY GUTTER AND AT EDGE OF PAVEMENT PER F.D.O.T. REQUIREMENTS. REMAINING AREAS WITHIN THE ROAD R/W SHALL BE SEEDED, FERTILIZED AND MULCHED. SOD OR SEED & MULCH (AS DIRECTED BY ENGINEER) ALL SWALES, BERMS, RETENTION AREAS, AND SLOPES WITHIN LIMITS OF CONSTRUCTION.
7. THE CONTRACTOR SHALL REMOVE ALL UNSUITABLE MATERIAL ENCOUNTERED FROM FILL AREAS PRIOR TO PLACEMENT OF FILL. ALL UNSUITABLE MATERIAL EXCAVATED SHALL BE STOCKPILED OR REMOVED AS DIRECTED BY OWNER.
8. REAR LOT LINE ELEVATIONS ARE SUBJECT TO FIELD CHANGE FOR THE PURPOSE OF MATCHING THE EXISTING NATURAL CONDITIONS AND/OR RETAINING EXISTING VEGETATION.
9. DURING CONSTRUCTION, THROAT INLET OPENINGS, WHERE APPROPRIATE, SHALL BE COVERED WITH FILTER FABRIC (MIRAFI 140N OR APPROVED EQUAL) TO PREVENT DEBRIS AND FILL FROM FALLING INTO THE INLET.
10. CASING AND/OR CONDUIT SHALL EXTEND 5' BEYOND THE EDGE OF PAVEMENT, BACK OF CURB AND/OR SIDEWALK AT EACH END.
11. OPTIONAL BRICK PAVERS MAY BE INSTALLED IN LIEU OF ASPHALT PAVEMENT AT THE DISCRETION OF THE OWNER. SEE TYPICAL SECTIONS AND DETAILS FOR PAVEMENT SECTIONS.
12. THE TOP OF ALL ENDWALLS SHALL BE NO HIGHER THAN ONE FOOT (1') BELOW THE LAKE CONTROL ELEVATION. THE CONTRACTOR SHALL REVIEW PROPOSED STRUCTURE LOCATIONS WITH THE ENGINEER AFTER STAKEOUT AND BEFORE COMMENCING CONSTRUCTION. ALL DRAINAGE STRUCTURES TO BE LOCATED ON THE LAKE BANK SHALL BE FIELD ADJUSTED SO AS TO INSET THE STRUCTURE, TO THE GREATEST EXTENT POSSIBLE, IN THE LAKE BANK.
13. YARD DRAINS SHALL BE ADDED IN GREEN AREAS AS NECESSARY AT THE DIRECTION OF THE LANDSCAPE ARCHITECT OR ENGINEER.
14. CONTRACTOR TO COORDINATE IRRIGATION CONDUIT LOCATIONS WITH IRRIGATION CONTRACTOR.
15. AT THE REQUEST OF THE OWNER, STORM WATER MANAGEMENT FACILITIES HAVE BEEN DESIGNED TO CONFORM WITH THE MINIMUM REQUIREMENTS OF LEE COUNTY AND/OR ESTABLISHED SFWMD PERMIT DESIGN CRITERIA FOR THIS AREA. DOWNSTREAM DRAINAGE BASIN OUTFALL IS REGULATED BY OTHERS, THE ENGINEER CANNOT WARRANT THE WATER MANAGEMENT SYSTEM TO FUNCTION PROPERLY DURING EXTREME STORM EVENTS.
16. ALL PERIMETER BERMS AND ANY NECESSARY PERIMETER GRADING ARE TO BE CONSTRUCTED FIRST IN ORDER TO RETAIN ALL SURFACE WATER RUNOFF ON SITE WITH THE ONLY DISCHARGE ALLOWED TO BE THROUGH APPROVED WEIRS.
17. ALL TRENCHES TO BE BACKFILLED WITH CLEAN DRY SAND, FINE LIME ROCK OR PIT SHELL MECH. AND TO BE COMPACTED TO AT LEAST 95% OF MAX. DENSITY, OR 98% IF UNDER PAVED AREA OF ROADWAY, AS DETERMINED BY AASHTO SPECIFICATION T-180.
18. ALL MITERED END SECTIONS SHALL BE RIP-RAPPED ACCORDINGLY.

ROOF DRAINS:

CONTRACTOR AND/OR DEVELOPER SHALL COORDINATE BETWEEN THE ARCHITECT AND THE SITE ENGINEER TO ENSURE THAT ADEQUATE MEASURES ARE IN PLACE FOR ROOF DRAINS (I.E. DOWN SPOUTS ARE DIRECTLY CONNECTED TO THE STORM SEWER SYSTEM, DOWN SPOUTS DISCHARGE DIRECTLY INTO DETENTION AREAS WITH ADEQUATE RIP-RAP PROPOSED, AND/OR DOWN SPOUTS SHEET FLOW ACROSS PAVEMENT AREAS THAT HAVE ADEQUATE STABILIZATION AT THE TRAILING EDGE OF THE PAVEMENT). THE SITE ENGINEER IS NOT RESPONSIBLE FOR EROSION ASSOCIATED WITH ROOF DRAINS.

VERTICAL DATUM: NAVD

THIS DRAWING AND ALL ELEVATIONS SHOWN ARE BASED ON THE NATIONAL AMERICAN VERTICAL DATUM (NAVD 88)
*ADD 1.2' TO CONVERT TO NGVD 29'

NO.	REVISION	DATE

RAY AVENUE GAS STATION
PAVING, GRADING & DRAINAGE PLAN

604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF.
STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NUMBER 97911
THIS ITEM HAS BEEN DIGITALLY SIGNED & SEALED BY JOSHUA A. EISENOFF, P.E.
ON THE DATE INDICATED HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

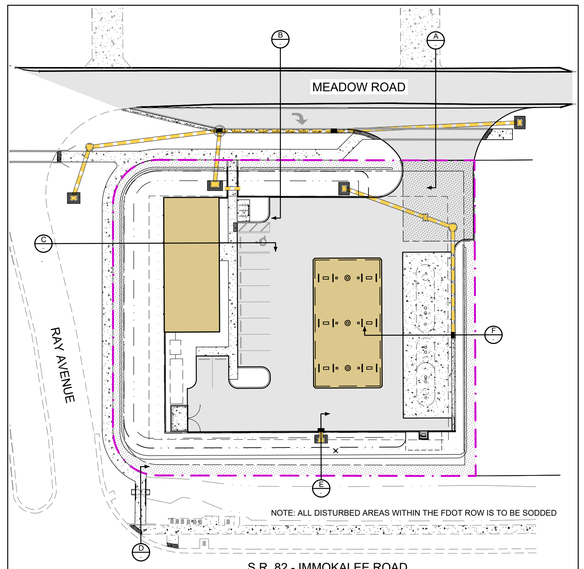
JOSHUA A. EISENOFF, P.E.
FL. PE #97911
VALID ONLY WITH STAMPED SEAL

SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-08
08 OF 16

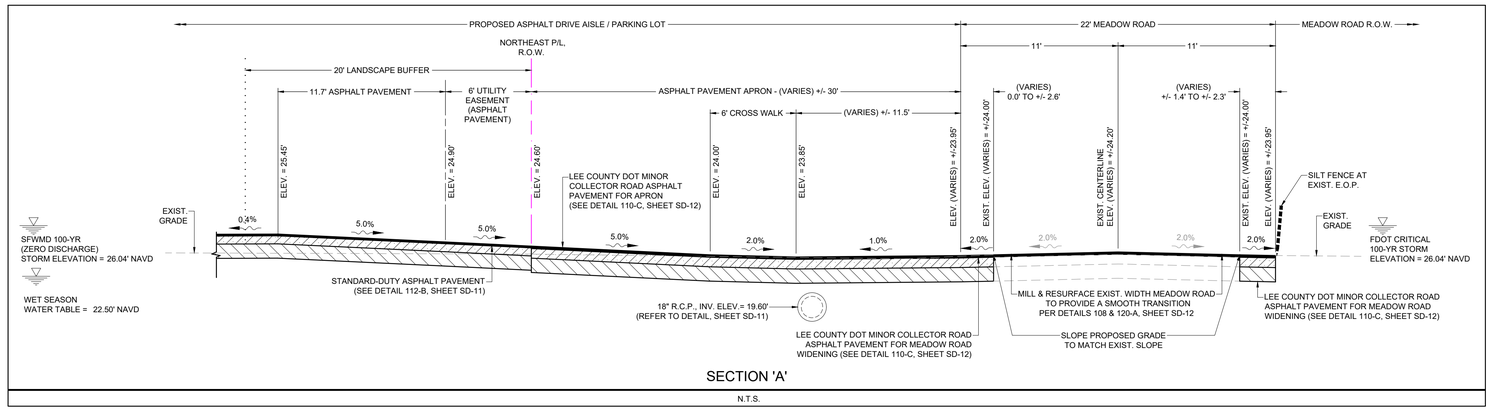
230704 - I

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CAD\ FILES\01-QA1 CADD - VERSION 102-PRODUCTIONS PLANS\230704-SD



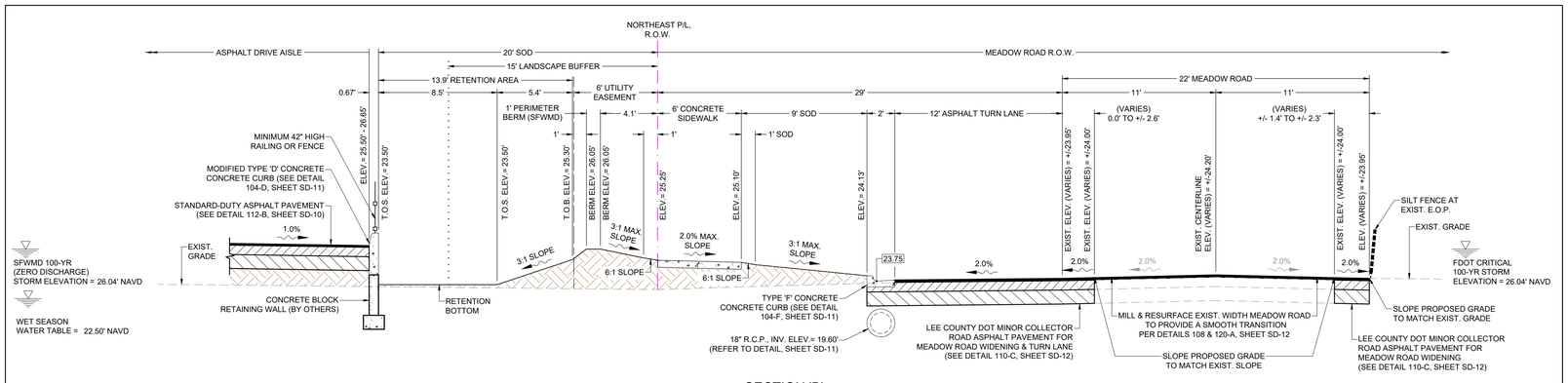
CROSS SECTION KEY MAP

N.T.S.



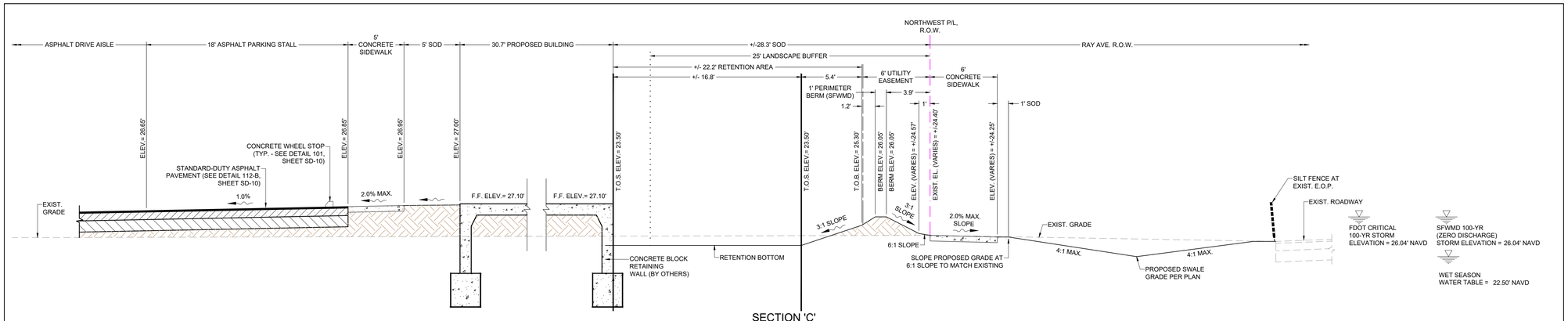
SECTION 'A'

N.T.S.



SECTION 'B'

N.T.S.



SECTION 'C'

N.T.S.

VERTICAL DATUM: NAVD
THIS DRAWING AND ALL ELEVATIONS SHOWN ARE BASED ON THE NATIONAL AMERICAN VERTICAL DATUM (NAVD 88) *ADD 1.2' TO CONVERT TO NGVD 29*

CROSS SECTION NOTE:
SWALE PROFILES SHOWN ARE TO TOP OF SOD. CONTRACTOR TO ADJUST SWALE GRADING ACCORDINGLY. ALL SLOPES TO BE SODDED. ANY NATURAL GROUND AREA DISTURBED BY THE CONTRACTOR SHALL BE SODDED.

REV	REVISION	DATE

RAY AVENUE GAS STATION
PROJECT CROSS SECTIONS - A

604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF
STATE OF FLORIDA PROFESSIONAL ENGINEER, LICENSE NO. 97911. THIS ITEM HAS BEEN DIGITALLY SIGNED & SEALED BY JOSHUA A. EISENOFF, P.E. ON THE DATE INDICATED HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED & SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JOSHUA A. EISENOFF, P.E.
FL. PE #97911

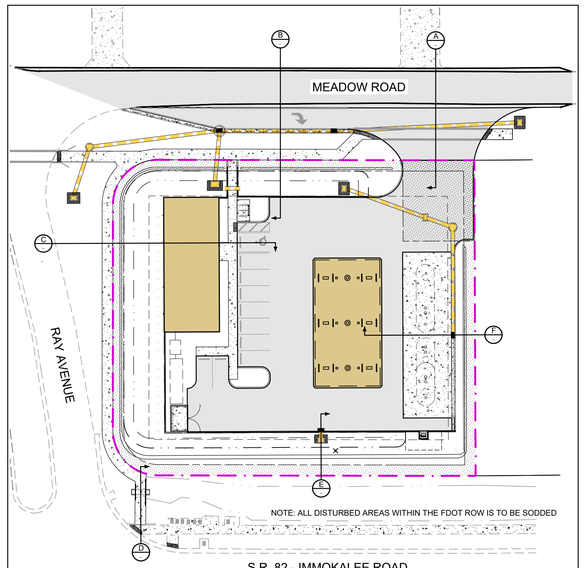
VALID ONLY WITH STAMPED SEAL

SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-09
09 OF 16

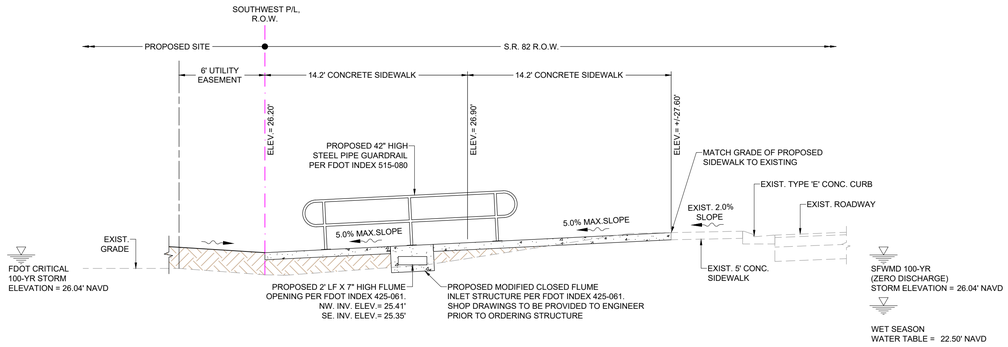
230704 - 1

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CADD FILES\01-QAI CADD - VERSION I102-PRODUCTIONS PLANS\230704-SD



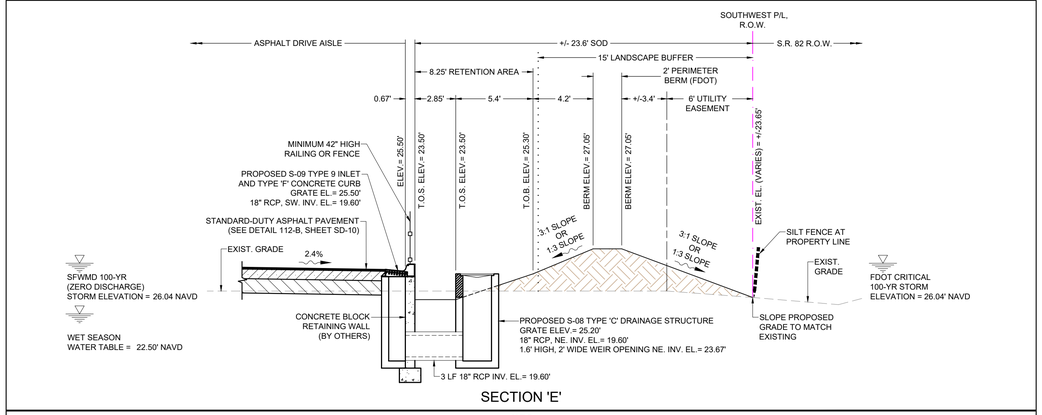
CROSS SECTION KEY MAP

N.T.S.



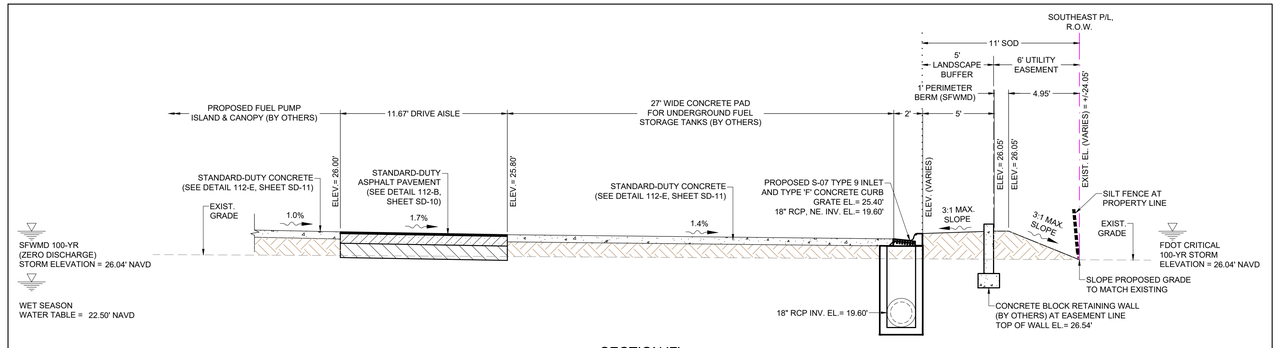
SECTION 'D'

N.T.S.



SECTION 'E'

N.T.S.



SECTION 'F'

N.T.S.

VERTICAL DATUM: NAVD
THIS DRAWING AND ALL ELEVATIONS SHOWN ARE BASED ON THE NATIONAL AMERICAN VERTICAL DATUM (NAVD 88) *ADD 1.2' TO CONVERT TO NGVD 29*

CROSS SECTION NOTE:
SWALE PROFILES SHOWN ARE TO TOP OF SOD. CONTRACTOR TO ADJUST SWALE GRADING ACCORDINGLY. ALL SLOPES TO BE SODDED. ANY NATURAL GROUND AREA DISTURBED BY THE CONTRACTOR SHALL BE SODDED.

REV	REVISION	DATE

RAY AVENUE GAS STATION
PROJECT CROSS SECTIONS - B

604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF, STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NUMBER 97911. THIS ITEM HAS BEEN DIGITALLY SIGNED & SEALED BY JOSHUA A. EISENOFF, P.E. ON THE DATE INDICATED HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JOSHUA A. EISENOFF, P.E.
FL. PE #97911

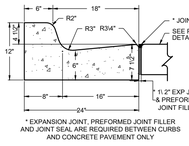
VALID ONLY WITH STAMPED SEAL

SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-10
10 OF 16

230704 - I

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CADD FILES\01-QAI CADD - VERSION I\02-PRODUCTIONS PLANS\230704-SD



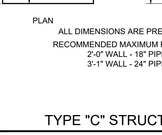
* EXPANSION JOINT, PREFORMED JOINT FILLER AND JOINT SEAL ARE REQUIRED BETWEEN CURBS AND CONCRETE PAVEMENT ONLY

REFER TO FDOT INDEX 520-001

DETAIL 104-F
REV: 01-01-2020

TYPE 'F' CURB DETAIL

N.T.S.



* EXPANSION JOINT, PREFORMED JOINT FILLER AND JOINT SEAL ARE REQUIRED BETWEEN CURBS AND CONCRETE PAVEMENT ONLY

REFER TO FDOT INDEX 520-001

DETAIL 104-D
REV: 01-01-2020

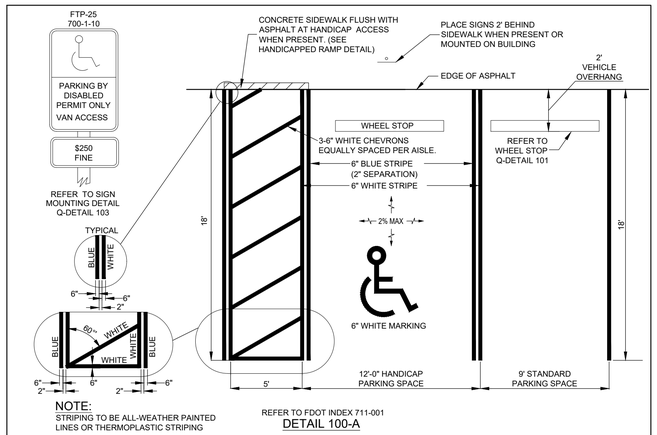
MODIFIED TYPE 'D' CURB DETAIL

N.T.S.

DETAIL 113-B

6" CURB END TRANSITION DETAIL

N.T.S.



NOTE:
STRIPING TO BE ALL-WEATHER PAINTED LINES OR THERMOPLASTIC STRIPING

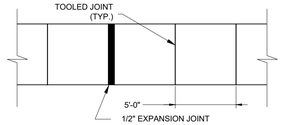
REFER TO FDOT INDEX.711-001

DETAIL 100-A

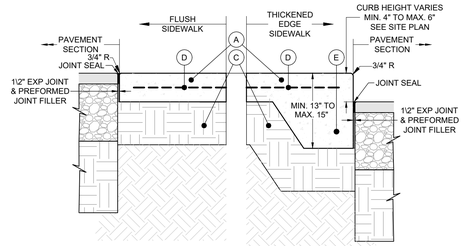
TYPICAL HANDICAP STALL & 18'x9' PARKING STALL W/ WHEEL STOPS DETAIL

N.T.S.

1. SIDEWALK TRUE MEASURE DEPTH THROUGH DRIVE/ENTRANCES SHALL BE 6" MIN.
2. CONCRETE STRENGTH SHALL BE 3000 PSI MINIMUM
3. MINIMUM SIDEWALK THICKNESS SHALL BE 4".
4. CONTROL JOINTS @ 5' O.C.
5. CONSTRUCT AN EXPANSION JOINT @ SPACING SHOWN BELOW AT END OF DAYS WORK, POINTS OF CURVATURE, & ADJOINING STRUCTURES.



PLAN VIEW



A 4" THICK TYPE 1 REINFORCED PORTLAND CEMENT CONCRETE WITH A 3,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS. 30' CONTROL JOINT SPACING WITH A MINIMUM 1/2" - 2" SAWCUT DEPTH FILLED WITH SEALANT PER FDOT. BROOM FINISH

C 6" MINIMUM TYPE B STABILIZED SUB-GRADE MIN. (LBR-40), COMPACTED TO 98% OF THE MAX, DENSITY AS DETERMINED BY AASHTO T-180

D 6" x 6" 10/10 WELDED WIRE FABRIC SHALL CONFORM TO ASTAM A-185 AND BE FREE OF RUST AND OIL. INSTALL IN LENGTHS AS LONG AS POSSIBLE, W/ MIN. 1' OVER LAP. PLACE AT 2" ABOVE BOTTOM OF CONCRETE.

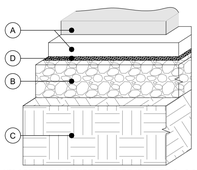
E #5 RE-BAR PLACED 2" FROM OUTSIDE EDGE OF CONRETE

SECTION VIEW

DETAIL 118
REV: 01-01-2020

FLUSH AND THICKENED EDGE SIDEWALK DETAIL

N.T.S.



A 1 - 1-1/2" ASPHALTIC CONCRETE TYPE (S-III) OR 2 - 0-3/4" LIFTS

B 8" MINIMUM LIMEROCK BASE (LBR-100), COMPACTED TO 98% OF THE MAX, DENSITY AS DETERMINED BY AASHTO T-134

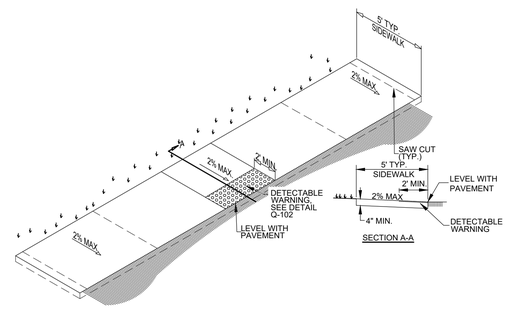
C 12" MINIMUM TYPE B STABILIZED SUB-GRADE MIN. (LBR-40), COMPACTED TO 98% OF THE MAX, DENSITY AS DETERMINED BY AASHTO T-180

D TACK COAT

DETAIL 112-B
REV: 02-19-2021

STANDARD-DUTY ASPHALT PAVEMENT DETAIL

N.T.S.

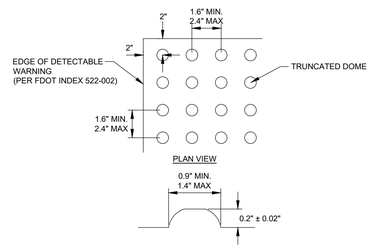


SECTION A-A

DETAIL 117-A
REV: 01-01-2020

FLUSH SIDEWALK AT HANDICAP RAMP

N.T.S.



TOP WIDTH OF DOME SHALL BE A MINIMUM OF 50% AND A MAX OF 65% OF THE BASE DIAMETER

TRUNCATED DOME

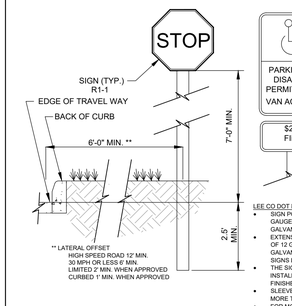
ALL DETECTABLE WARNING SURFACES SHALL BE CAST-IN-PLACE (WET-SET AND ANCHORED IN CONCRETE). ALL SIDEWALK CURB RAMPS SHALL HAVE DETECTABLE WARNING SURFACES THAT EXTEND THE FULL WIDTH OF THE RAMP AND A LENGTH OF 24" IN THE DIRECTION OF TRAVEL.

DETECTABLE WARNING DETAIL

DETAIL 102
REV: 01-01-2020

DETECTABLE WARNING DETAIL

N.T.S.



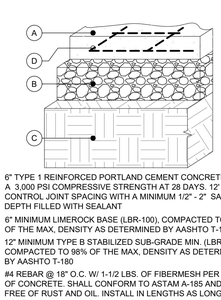
LEE CO DOT NOTES

- SIGN POST SHALL BE A MINIMUM OF 14 GAUGE STEEL, 2" SQUARE WITH A GALVANIZED COATING.
- EXTENSION SLEEVES, SHALL BE A MINIMUM OF 12 GAUGE STEEL, 2.25" SQUARE WITH A GALVANIZED COATING & REQUIRED FOR ALL SIGNS LARGER THAN 4 SQUARE FT.
- THE SIGN POST AND SLEEVE SHALL BE INSTALLED A MINIMUM OF 30" BELOW THE FINISHED GRADE.
- SLEEVES, IF USED, MAY NOT PROTRUDE MORE THAN 3" ABOVE THE FINISHED GRADE.
- FOR MORE INFO, REFER TO LEE COUNTY DOT TRAFFIC DESIGN STANDARD FOR SIGN INSTALLATION

DETAIL 103
REV: 01-01-2020

SIGN MOUNTING DETAIL

N.T.S.



A 6" TYPE 1 REINFORCED PORTLAND CEMENT CONCRETE WITH A 3,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS. 12' X 12' CONTROL JOINT SPACING WITH A MINIMUM 1/2" - 2" SAWCUT DEPTH FILLED WITH SEALANT

B 6" MINIMUM LIMEROCK BASE (LBR-100), COMPACTED TO 98% OF THE MAX, DENSITY AS DETERMINED BY AASHTO T-134

C 12" MINIMUM TYPE B STABILIZED SUB-GRADE MIN. (LBR-40), COMPACTED TO 98% OF THE MAX, DENSITY AS DETERMINED BY AASHTO T-180

D #4 REBAR @ 18" O.C. W/ 1-1/2 LBS. OF FIBERMESH PER CU. YD. OF CONCRETE. SHALL CONFORM TO ASTAM A-185 AND BE FREE OF RUST AND OIL. INSTALL IN LENGTHS AS LONG AS POSSIBLE, W/ MIN. 1' OVER LAP. PLACE AT 2" BELOW TOP OF CONCRETE.

DETAIL 112-E
REV: 01-01-2020

STANDARD-DUTY CONCRETE DETAIL

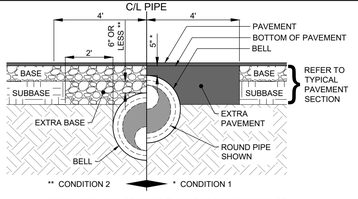
N.T.S.

ALL DIMENSIONS ARE PRE CAST

RECOMMENDED MAXIMUM PIPE SIZE:
2'-0" WALL - 18" PIPE
3'-1" WALL - 24" PIPE

TYPE "C" STRUCTURE DETAIL

N.T.S.



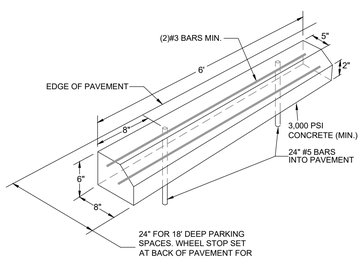
* 1 WHEN (BELL) TOP OF PIPE IS 5" OR LESS FROM BOTTOM OF ASPHALT COURSE, CONTRACTOR SHALL BE REQUIRED TO USE ASPHALT BASE STRUCTURAL COURSE OVER PIPE AND 4' BEYOND EACH SIDE OF CENTER OF PIPE. (ABC BASE SHALL MEET FDOT SPECIFICATIONS)

** 2 CONTRACTOR SHALL COMPACT EXTRA LIMEROCK BASE FROM THE BOTTOM OF THE SUBGRADE TO THE BOTTOM OF THE BASE, 2' EACH SIDE, WHENEVER PIPE ENCROACHES INTO THE SUBGRADE 6" OR GREATER. DENSITY TESTS WILL BE REQUIRED EACH SIDE OF PIPE IN EXTRA BASE.

DETAIL 107

EXTRA BASE DETAIL

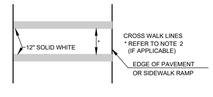
N.T.S.



DETAIL 101

TYPICAL WHEEL STOPS DETAIL

N.T.S.



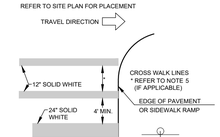
NOTE:

1. ALL MARKINGS WITHIN THE LEE COUNTY R.O.W TO BE THERMOPLASTIC.
2. CROSSWALK WIDTH, EXCEED WIDTH OF THE ADJACENT SIDEWALK, BUT DO NOT MAKE LESS THAN 6' FOR INTERSECTION CROSSWALKS AND 10' FOR MIDBLOCK CROSSWALKS. MEASURE WIDTH FROM THE INSIDE OF THE TRANSVERSE CROSSWALK MARKINGS.
3. WHEN THE SPECIAL CROSSWALK IS NOT PERPENDICULAR TO THE LANE LINES, MAKE THE LONGITUDINAL MARKINGS PARALLEL TO THE LANE LINES.
4. REFER TO FDOT INDEX 711-001 FOR GUIDANCE.
5. REFER TO FDOT INDEX 522-002 FOR WHEN CURB RAMPS ARE PRESENT.
6. NOT TO EXCEED 2% CROSS SLOPE AND 6% LONGITUDINAL SLOPE.

DETAIL 105-C

CROSSWALK DETAIL

N.T.S.



NOTE:

1. STOP BAR SHALL BE PLACED AT THE DESIRED STOPPING POINT.
2. STOP BAR SHOULD BE PLACED NO MORE THAN (30 FT) OR LESS THAN (4 FT) FROM THE NEAREST EDGE OF THE INTERSECTING TRAVEL WAY, IN ACCORDANCE WITH MUTCD SECTION 3B.16.
3. STOP LINES SHALL BE PLACED TO ALLOW SUFFICIENT SIGHT DISTANCE TO ALL OTHER APPROACHES TO AN INTERSECTION.
4. ALL MARKINGS WITHIN THE LEE COUNTY R.O.W TO BE THERMOPLASTIC.
5. CROSSWALK WIDTH SHALL MATCH EXISTING OR PROPOSED SIDEWALKS (IF APPLICABLE).
6. REFER TO FDOT INDEX 711-001 FOR GUIDANCE.

DETAIL 105-A

STOP BAR & SIGN W/ CROSSWALK DETAIL

N.T.S.

RAY AVENUE GAS STATION
PROJECT DETAILS - A
604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

REV	REVISION	DATE

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF, STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NUMBER 97911. THIS ITEM HAS BEEN DIGITALLY SIGNED & SEALED BY JOSHUA A. EISENOFF, P.E. ON THE DATE INDICATED HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JOSHUA A. EISENOFF, P.E.
FL. PE #97911

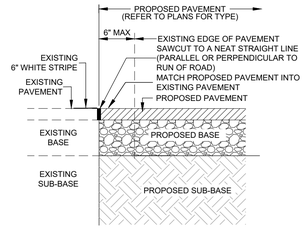
VALID ONLY WITH STAMPED SEAL

SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-11
11 OF 16

230704 - I

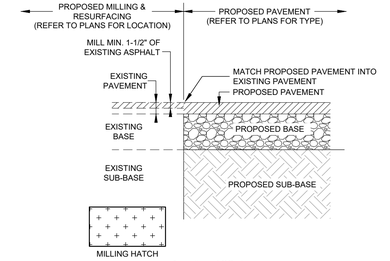
P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CAD FILES\01-QA1 CADD - VERSION 1\02-PRODUCTIONS PLANS\230704-SD



DETAIL 108

PAVEMENT SAWCUT DETAIL

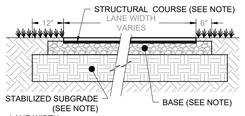
N.T.S.



DETAIL 120-A

PAVEMENT MILLING DETAIL

N.T.S.



LANE WIDTH
REFER TO LCDC:
SEC. 10-296(e) ROAD DESIGN

BASE COMPACTED TO 98% OF THE MAX. DENSITY AS DETERMINED BY AASHTO T-180

STABILIZED SUBGRADE COMPACTED TO 98% OF THE MAX. DENSITY AS DETERMINED BY AASHTO T-180

1. STABILIZED SUBGRADE TO EXTEND 1-FOOT BEYOND EDGE OF PAVEMENT AND OR BACK OF CURBING.
2. WHEN CURBING IS PROPOSED LIMEROCK BASE TO EXTEND 6-INCHES BEYOND EDGE OF PAVEMENT.

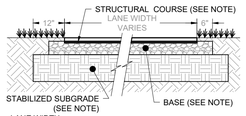
SIDEWALK
FRICTION COURSE:
NONE
STRUCTURAL COURSE:
6" PORTLAND CEMENT CONCRETE
BASE:
FDOT OPTIONAL BASE GROUP 1 (4" OF COMPACTED LIMEROCK)
SUBGRADE:
6" THICK STABILIZED SUBGRADE (LBR40)

DETAIL 110-H

REV: 02-19-2021

LC-DOT SIDEWALK DETAIL

N.T.S.



LANE WIDTH
REFER TO LCDC:
SEC. 10-296(e) ROAD DESIGN

BASE COMPACTED TO 98% OF THE MAX. DENSITY AS DETERMINED BY AASHTO T-180

STABILIZED SUBGRADE COMPACTED TO 98% OF THE MAX. DENSITY AS DETERMINED BY AASHTO T-180

1. STABILIZED SUBGRADE TO EXTEND 1-FOOT BEYOND EDGE OF PAVEMENT AND OR BACK OF CURBING.
2. WHEN CURBING IS PROPOSED LIMEROCK BASE TO EXTEND 6-INCHES BEYOND EDGE OF PAVEMENT.

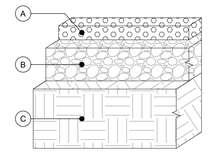
MINOR COLLECTOR
FRICTION COURSE:
NONE
STRUCTURAL COURSE:
1-1/2" ASPHALTIC CONCRETE FDOT TYPE S-I OR SUPERPAVE 12.5
BASE:
FDOT OPTIONAL BASE GROUP 6 (8" OF COMPACTED LIMEROCK)
SUBGRADE:
12" THICK STABILIZED SUBGRADE (LBR40)

DETAIL 110-C

REV: 01-01-2020

LC-DOT MINOR COLLECTOR PAVEMENT DETAIL

N.T.S.



A 3" #57 CRUSHED STONE

B 6" MINIMUM LIMEROCK BASE (LBR-100), COMPACTED TO 98% OF THE MAX, DENSITY AS DETERMINED BY AASHTO T-134

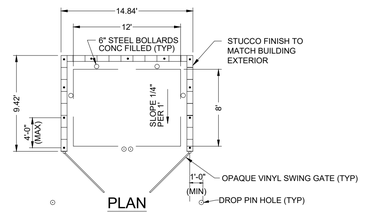
C 8" MINIMUM TYPE B STABILIZED SUB-GRADE MIN. (LBR-40), COMPACTED TO 98% OF THE MAX, DENSITY AS DETERMINED BY AASHTO T-180

DETAIL 111-A

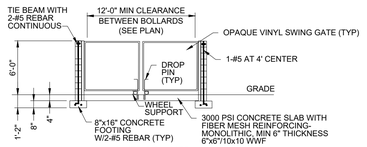
REV: 08-10-2020

GRAVEL CONSTRUCTION ENTRANCE DETAIL

N.T.S.



PLAN



ELEVATION

NOTE:

1. PROVIDE TWO GATES W/STOP(S).
2. DUMPSTERS AND CONTAINERS WITH AN INDIVIDUAL CAPACITY OF 1.5 CU. FT. OR MORE SHALL NOT BE STORED IN BUILDINGS OR PLACED WITHIN 10 FT. OF COMBUSTIBLE WALLS, OPENINGS, OR COMBUSTIBLE ROOF EAVE LINES. LATEST EDITION NFPA 1: CHAPTER 19.

DUMPSTER DETAIL

N.T.S.

REV	REVISION	DATE

RAY AVENUE GAS STATION
PROJECT DETAILS - B
604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF,
STATE OF FLORIDA, PROFESSIONAL
ENGINEER, LICENSE NO. 97911. THIS ITEM
HAS BEEN DIGITALLY SIGNED & SEALED BY
JOSHUA A. EISENOFF, P.E.
ON THE DATE ADJACENT TO THE SEAL. PRINTED
COPIES OF THIS DOCUMENT ARE NOT
CONSIDERED SIGNED & SEALED AND THE
SIGNATURE MUST BE VERIFIED ON ANY
ELECTRONIC COPIES.

JOSHUA A. EISENOFF, P.E.
FL. PE #97911

VALID ONLY WITH STAMPED SEAL

SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-12
12 OF 16

230704 - I

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CADD FILES\01-QAI CADD - VERSION I\02-PRODUCTIONS P LANS\230704-SD

GENERAL FGUA UTILITY NOTES:

1. ALL WORK SHALL CONFORM TO LATEST REVISION OF THE LEE COUNTY UTILITIES OPERATIONAL MANUAL WHICH IS ON FILE AT THE DIVISION OF LEE COUNTY UTILITIES OFFICE (1500 MONROE STREET, P.O. BOX 398, FORT MYERS, FLORIDA 33902).
2. ANY QUANTITIES SHOWN ON PLANS ARE NOT VERIFIED BY LCU.
3. AT THE SITE KEEP AND MAINTAIN ONE RECORD COPY OF ALL CONTRACT DOCUMENTS, REFERENCE DOCUMENTS AND ALL TECHNICAL DOCUMENTS SUBMITTED IN GOOD ORDER. AS THE WORK PROGRESSES THE ENGINEER OR HIS DESIGNATED REPRESENTATIVE SHALL RECORD ON ONE SET OF REPRODUCIBLE DRAWINGS ALL CHANGES AND DEVIATIONS FROM THE ORIGINAL PLANS. HE SHALL RECORD THE EXACT LOCATION OF ALL CHANGES IN VERTICAL AND HORIZONTAL ALIGNMENT BY OFFSETS AND TIES AT EACH: SEWER, WATER, ELECTRIC, GAS, COMMUNICATION AND OTHER SERVICES BY OFF-SET DISTANCE TO PERMANENT IMPROVEMENTS SUCH AS BUILDING AND CURBS.
4. A PRE-CONSTRUCTION MEETING IS REQUIRED BEFORE WORK MAY BEGIN. LEE COUNTY UTILITIES SHALL BE NOTIFIED 48 HOURS PRIOR TO PROJECT MOBILIZATION.
5. ALL WORK AND MATERIALS, WHICH DO NOT CONFORM TO LCU SPECIFICATIONS, ARE SUBJECT TO REMOVAL AND REPLACEMENT AT THE CONTRACTOR'S EXPENSE.
6. ANY WORK PERFORMED WITHOUT THE KNOWLEDGE OF LCU IS SUBJECT TO RE-EXCAVATION, REMOVAL AND REPLACEMENT OF SAME TO BE DONE AT THE CONTRACTOR'S EXPENSE.
7. THE CONTRACTOR SHALL PROVIDE SUFFICIENT PERSONNEL AND EQUIPMENT ON THE JOB AT ALL TIMES DURING CONSTRUCTION TO SATISFY THE SPECIFICATIONS AND TO COMPLETE WORK.
8. LCU INSPECTION STAFF MAY OBSERVE PROJECT CONSTRUCTION.
9. THE CONTRACTOR IS TO UNCOVER ALL EXISTING LINES BEING TIED INTO AND VERIFY GRADES BEFORE BEGINNING CONSTRUCTION.
10. IT IS THE CONTRACTOR'S RESPONSIBILITY TO LOCATE AND TAKE ALL POSSIBLE PRECAUTIONS TO AVOID ANY DAMAGE TO ALL UNDERGROUND PIPELINES, TELEPHONE, CABLE TV, ELECTRIC LINES/CONDUITS AND STRUCTURES IN ADVANCE OF ANY CONSTRUCTION. LCU WILL NOT GUARANTEE ANY LOCATIONS AS SHOWN ON THESE PLANS OR THOSE OMITTED FROM THESE PLANS. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MAY OCCUR BY HIS FAILURE TO EXACTLY LOCATE AND PROTECT EXISTING UTILITIES AND STRUCTURES.
11. CONTRACTOR SHALL VERIFY ALL QUANTITIES SHOWN ON THE PLANS. IF ANY DISCREPANCIES IN QUANTITIES ARE FOUND, THE CONTRACTOR SHALL NOTIFY THE PROJECT ENGINEER AND LCU.
12. ALL REGULATORY AND PERMITTING AGENCIES' REQUIREMENTS SHALL BE COMPLIED WITH.
13. APPROPRIATE TURBIDITY CONTROL DEVICES (E.G. SILT FENCES, HAY BAILS) WILL BE UTILIZED DURING ALL PHASES OF INSTALLATION AND GRADING. CONTRACTOR IS RESPONSIBLE FOR SUBMITTING THE NOTICE OF INTENT AND NOTICE OF TERMINATION TO THE EPA IN COMPLIANCE WITH LEE COUNTY'S NPDES PERMIT. CONTRACTOR IS RESPONSIBLE FOR DEVELOPING AND MAINTAINING AN EFFECTIVE STORM WATER POLLUTION PREVENTION PLAN.
14. IN THE EVENT THAT HARD LIMESTONE FORMATION IS ENCOUNTERED, MAKING IT IMPOSSIBLE TO EXCAVATE TO THE DEPTH REQUIRED UNDER THIS CONTRACT, THE CONTRACTOR MAY BE ALLOWED TO REDUCE THE PIPE COVER TO NOT LESS THAN TWO (2) FEET WHILE MAINTAINING THE LCU REQUIRED THICKNESS OF BEDDING UNDER THE PIPE. SUCH DEVIATION FROM THE PLANS MUST BE FIRST BE APPROVED BY LCU AND THE ENGINEER PRIOR TO THE PIPE LAYING. IF TWO FEET OF COVER OR MORE CAN NOT BE ATTAINED, THE CONTRACTOR SHALL PROVIDE OTHER METHOD OF CONSTRUCTION OR PIPE PROTECTION WHICH SHALL FIRST BE APPROVED BY LCU AND THE ENGINEER, AT NO ADDITIONAL COST TO THE COUNTY.
15. LOCATIONS, ELEVATIONS, AND DIMENSIONS OF EXISTING UTILITIES, STRUCTURES AND OTHER FEATURES ARE SHOWN ACCORDING TO THE BEST INFORMATION AVAILABLE AT THE TIME OF PREPARATION OF THESE PLANS, BUT DO NOT SUPPORT TO BE ABSOLUTELY CORRECT. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL VERIFY AND BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL EXISTING UTILITIES, STRUCTURES, AND OTHER FEATURES AFFECTING HIS WORK. ANYTHING NOT SHOWN ON THESE DRAWINGS SHOULD BE BROUGHT TO THE ATTENTION OF THE ENGINEER AND SHALL NOT CONSTITUTE AN EXTRA, UNLESS APPROVED BY THE ENGINEER.
16. THE CONTRACTOR SHALL CONTACT THE ENGINEER AND LCU IMMEDIATELY CONCERNING ANY CONFLICTS WITH LCU UTILITIES/STRUCTURES ARISING DURING CONSTRUCTION OF ANY FACILITIES SHOWN ON THESE DRAWINGS.
17. TRAFFIC MUST BE MAINTAINED AT ALL TIMES AS PER LEE COUNTY DEPARTMENT OF TRANSPORTATION (LCDOT) AND PER FLORIDA DEPARTMENT OF TRANSPORTATION (FDOT) STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, LATEST EDITION.
18. THE CONTRACTOR SHALL VERIFY ALL UTILITIES AND PROVIDE AT LEAST 48 HOURS NOTICE TO THE INDIVIDUAL UTILITY COMPANIES AND FDOT AND LCDOT PRIOR TO CONSTRUCTION.
19. THE CONTRACTOR SHALL REPLACE ALL PAVEMENT, CURBS, DRIVEWAYS, SIDEWALKS, FENCES, ETC., WITH THE SAME TYPE OF MATERIAL THAT WAS REMOVED DURING CONSTRUCTION OR AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHALL RESTORE ALL AREAS AFFECTED BY THE CONSTRUCTION TO ITS ORIGINAL CONDITION, OR BETTER.
20. WITHIN THE FDOT AND LCDOT RIGHT-OF-WAY, ALL DISTURBED AREAS SHALL RECEIVE GRASSING (SEEDING) OR SODDING MATERIALS IN ACCORDANCE WITH FDOT SPECIFICATIONS. THOSE AREAS THAT ARE CLASSIFIED AS DRAINAGE DITCHES SHALL RECEIVE FULL SOLID SOD.
21. THE INFORMATION PROVIDED IN THESE PLANS IS SOLELY TO ASSIST THE CONTRACTOR IN ASSESSING THE NATURE AND EXTENT OF CONDITIONS WHICH MAY BE ENCOUNTERED DURING THE COURSE OF WORK. ALL CONTRACTORS ARE DIRECTED, PRIOR TO BIDDING, TO CONDUCT WHATEVER INVESTIGATIONS THEY MAY DEEM NECESSARY TO ARRIVE AT THEIR OWN CONCLUSION REGARDING THE ACTUAL CONDITIONS THAT WILL BE ENCOUNTERED, AND UPON WHICH THEIR BIDS WILL BE BASED.
22. ALL FRAMES, COVERS, VALVE BOXES, METER BOXES, AND MANHOLES SHALL BE ADJUSTED TO FINISHED GRADE UPON COMPLETION OF PAVING OR RELATED CONSTRUCTION. ALL VALVE PADS SHALL BE POURED IN PLACE. NO PRE-FORMED VALVE PADS ALLOWED.
23. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ANY DEWATERING, CLEARING OR TREE REMOVAL PERMITS REQUIRED FOR THIS PROJECT.
24. ALL TRENCHES TO BE BACKFILLED WITH CLEAN DRY SAND, FINE LIME ROCK OR PIT SHELL MECH. AND TO BE COMPACTED TO AT LEAST 95% OF MAX. DENSITY, OR 98% IF UNDER PAVED AREA OF ROADWAY, AS DETERMINED BY AASHTO SPECIFICATION T-180.
25. PRIOR TO ANY UTILITY CONSTRUCTION, CONTRACTOR IS RESPONSIBLE TO ASSURE THAT PERMITS HAVE BEEN ISSUED BY THE APPROPRIATE UTILITY AND GOVERNMENTAL AUTHORITY.
26. LOCATIONS OF PROPOSED UTILITY LINES ARE SUBJECT TO MODIFICATION TO PRESERVE EXISTING VEGETATION AS DETERMINED BY ENGINEER AND CFM.
27. CONTRACTOR SHALL MAINTAIN A MIN. OF 10' HORIZONTAL AND 18" VERTICAL SEPARATION BETWEEN POTABLE WATERLINES AND STORM SEWER, FORCE MAINS AND SANITARY SEWER.
28. EXERCISE CARE TO PROTECT THE ROOTS OF TREES TO REMAIN. WITHIN THE BRANCH SPREAD OF SUCH TREES, PERFORM ALL TRENCHING BY HAND. OPEN THE TRENCH ONLY WHEN UTILITIES CAN BE INSTALLED IMMEDIATELY. PRUNE INJURED ROOTS CLEANLY AND BACKFILL AS SOON AS POSSIBLE.
29. CONTRACTOR TO COORDINATE WITH ELECTRIC, TELEPHONE AND CABLE COMPANIES TO ASSURE THAT NO SANITARY SEWER, WATERLINE OR IRRIGATION SERVICES ARE INSTALLED UNDER PROPOSED TRANSFORMERS.
30. CONTRACTOR TO PROVIDE UTILITY RECORD DRAWINGS TO OWNER PRIOR TO FINAL PAYMENT REQUEST.

GENERAL FGUA UTILITY NOTES, CONTINUE:

31. ALL CONSTRUCTION SHALL MEET LEE COUNTY STANDARDS AND SPECIFICATIONS
32. AN FGUA REPRESENTATIVE IS TO BE PRESENT DURING CONNECTIONS TO EXISTING FACILITIES WITH 72 HOUR NOTIFICATION
33. ALL UTILITIES WITHIN THE PROPERTY LINES WILL BE OWNED AND MAINTAINED BY THE PROPERTY OWNER
34. ANY UTILITIES (PIPES, VALVES, AND ASSOCIATED HARDWARE) TO BE CONVEYED TO FGUA WILL BE LOCATED WITHIN AN EXISTING PUBLIC UTILITY EASEMENT, RIGHT-OF-WAY (ROW) AND/OR DEDICATED FGUA UTILITY EASEMENT
35. ANY REAL PROPERTY (LIFT STATIONS, PUMP HOUSES, WELLS, ETC) WILL NOT BE LOCATED WITHIN THE ROW AND WILL BE CONVEYED TO FGUA VIA A SPECIFIC WARRANTY DEED
36. LOW PRESSURE AIR TEST, AND VIDEO INSPECTION ARE REQUIRED FOR THE GRAVITY COLLECTION SYSTEM - WATER SYSTEMS ARE TESTED TO A MINIMUM 150 PSI FOR TWO-HOUR DURATION AS PER AWWA C 600 STANDARDS. FIRE SYSTEMS ARE NOT THE FGUA RESPONSIBILITY
37. INSTALL 10 GAUGE TRACER WIRE ON ALL MAIN PIPELINES. WIRE TO EXTEND INTO EACH BOX WITH SUFFICIENT EXCESS TO BRING WIRE THREE FEET ABOVE FINISH GRADE
38. ALL WATER AND/OR WASTEWATER LINES PASSING UNDER PAVED AREAS WILL BE CONSTRUCTED WITH A CASING AND SHALL APPLY WITH LOCAL OR STATE DOT STANDARDS
39. A REDUCED PRESSURE ZONE (RPZ), BACKFLOW PREVENTION ASSEMBLY (BFP) IS REQUIRED. ACCEPTABLE MODELS FEBCO 825Y, WATTS 909, WILKINS 975XL, OR CONBRACO 400.
40. FULL CIRCLE STAINLESS STEEL TAPPING SADDLE SLEEVE WITH RESILIENT SEATED TAPPING VALVE FOR PROPOSED WATER AND FORCE MAIN CONNECTIONS TO BE USED.
41. WATER TAPPING SADDLES TO BE DOUBLE STRAP BRASS OR DUCTILE IRON OR EPOXY COATED STEEL WITH STAINLESS STEEL HARDWARE. TIE STRAPS AND BOLTS SHALL BE CORROSION RESISTANT ALLOY STEEL.
42. ALL FIRE FLOW LINES WILL RUN SEPARATE FORM THE DOMESTIC WATER MAINS AND GAVE A FIRE DETECTION METER AND A FIRE LINE BACKFLOW PREVENTION ASSEMBLY (BFP) ASSEMBLY SHALL INCLUDE A LOW FLOW BY-PASS METER W/ RPZ
43. MANHOLE FRAMES & COVERS, AND CO CAPS ARE TO BE WATER TIGHT, IF NEEDED EXISTING MANHOLE TOPS ARE TO BE ADJUSTED TO PROPOSED GRADE.
44. INFLOW PROTECTION DISHES AREA REQUIRED FOR ALL MANHOLES; STAINLESS STEEL IN PAVED AREAS, ABS IN UNPAVED AREAS.
45. A CLEANOUT SHALL BE PROVIDED AT THE ROW/PL FOR ALL SERVICE CONNECTIONS.
46. THE FGUA SHALL OWN AND MAINTAIN ALL WATER SERVICE LINES UP TO THE POINT OF CONNECTION. ALL WATER MAINS SHALL BE MINIMUM OF FOUR INCHES (4") IN DIAMETER AND ALL SUCH WATER MAINS SHALL BE CONSTRUCTED IN DEDICATED RIGH-OF-WAY OR APPROPRIATE UTILITY EASEMENTS DEDICATED TO THE FGUA.
47. ALL SEWER FORCE MAIN LINES AND GRAVITY SERVICE LINES ACCEPTED MUST BE AT LEAST FOUR INCHES (4") IN DIAMETER AND EIGHT INCHES (8"), RESPECTIVELY EXCEPT LATERAL SEWER SERVICE LINES WHICH SHALL BO NO LESS THAN SIX INCHES (6") IN DIAMETER AND ACCEPTED BY THE FGUA TO A CLEANOUT LOCATED ON THE EDGE OF THE RIGHT-OF-WAY OR AN EASEMENT.
48. A BALL CHECK VALVE FOLLOWED BY A PLUG VALVE WILL BE INSTALLED ON FORCE MAINS LOCATED IN PUE OR R/W AT THE PROPERTY LINE DISCHARGING FROM PRIVATELY OWNED LIFT STATIONS CONNECTING INTO THE UTILITY MAINS.
49. CONNECTIONS TO EXISTING MANHOLES AND/OR LIFT STATIONS SHALL BE BY CORE BORE WITH KOR-N-SEAL CONNECTORS.
50. ALL CONNECTIONS WILL BE MADE AS HOT TAPS. ANY EXCEPTIONS TO THIS WILL REQUIRE FGUA APPROVAL. ALL HOT TAP CONNECTIONS SHALL BE PERFORMED BY A LICENSED, UNDERGROUND CONTRACTOR, WITH ADVANCE NOTIFICATION (72 HOURS) TO THE FGUA INSPECTOR

ADA ACCESSIBILTY NOTES:

1. ALL HANDICAPPED PARKING SPACES AND ACCESS AISLES ADJACENT TO THE HANDICAP PARKING SPACES SHALL HAVE A MAXIMUM OF 2% SLOPE IN ALL DIRECTIONS (INCLUDES RUNNING SLOPE AND CROSS SLOPE).
2. AN ACCESSIBLE ROUTE FROM THE PUBLIC STREET OR SIDEWALK TO ALL BUILDING ENTRANCES MUST BE PROVIDED. THIS ACCESSIBLE ROUTE SHALL BE A MINIMUM OF 60" WIDE. THE RUNNING SLOPE OF AN ACCESSIBLE ROUTE SHALL NOT EXCEED 5% AND THE CROSS SLOPE SHALL NOT EXCEED 2%.
3. SLOPES EXCEEDING 5% BUT LESS THAN 8% WILL REQUIRE A RAMP AND MUST CONFORM TO THE REQUIREMENTS FOR RAMP DESIGN (HANDRAILS, CURBS, LANDINGS). NO RAMP SHALL EXCEED AN 8% RUNNING SLOPE OR 2% CROSS SLOPE.
4. IF A RAMP RUN HAS A RISE GREATER THEN 6 INCHES OR A HORIZONTAL PROJECTION GREATER THEN 72 INCHES, THEN IT SHALL HAVE HANDRAILS ON BOTH SIDES. HANDRAILS ARE NOT REQUIRED ON CURB RAMPS OR ADJACENT TO SEATING IN ASSEMBLY AREAS.
5. IN THE CASE THAT A NEW SIDEWALK WILL BE CONSTRUCTED IN THE RIGHT OF WAY OF A SITE THE RUNNING SLOPE OF THE SIDEWALK SHALL NOT EXCEED 5% AND THE CROSS SLOPE SHALL NOT EXCEED 2%. THIS STANDARD APPLIES TO CROSS WALKS IN THE DRIVEWAY AS WELL AND WILL REQUIRE SPECIAL ATTENTION DURING STAKING TO MAKE SURE THE 2% CROSS SLOPE IS MET IN THE CROSS WALK.
6. IT WILL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO ENSURE THAT THE HANDICAP PARKING SPACES, ACCESSIBLE ROUTES, AND SIDEWALKS/CROSSWALKS ARE CONSTRUCTED TO MEET ADA REQUIREMENTS.
7. ANY REQUIREMENTS LISTED ABOVE THAT CAN NOT BE MET SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION IMMEDIATELY. ANYTHING NOT BUILT TO THE ABOVE STANDARDS WILL REQUIRE REMOVAL AND REPLACEMENT OF THE NON COMPLIANT AREAS AT THE GENERAL CONTRACTORS COST.

GENERAL LEE COUNTY NOTES:

1. ALL CONSTRUCTION TO BE IN ACCORDANCE WITH F.D.O.T. STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, LATEST EDITION, AND THE F.D.O.T. ROADWAY AND TRAFFIC DESIGN STANDARDS, LATEST EDITION.
2. THE CONTRACTOR SHALL RETAIN ON THE WORK SITE AT ALL TIMES COPIES OF ALL PERMITS NECESSARY FOR CONSTRUCTION.
3. ANY DEVIATIONS FROM THESE PLANS BY THE OWNER OR CONTRACTOR REQUIRES PRIOR APPROVAL OF THE ENGINEER. CONTRACTOR AND/OR ANY OF HIS SUBCONTRACTORS SHALL NOTIFY THE ENGINEER IMMEDIATELY OF ANY DISCREPANCY OR ERRORS IN THE PLANS OR IF EXISTING CONDITIONS ARE NOT AS SHOWN ON THE PLANS.
4. THE CONTRACTOR SHALL NOTIFY THE OWNER AND CONTACT ALL UTILITY COMPANIES FOR LOCATIONS OF EXISTING UTILITIES IN THE AREA 72 HOURS (MINIMUM) PRIOR TO COMMENCING CONSTRUCTION.
5. THE CONTRACTOR SHALL NOTIFY THE LEE COUNTY DIVISION OF DEVELOPMENT SERVICES A MINIMUM OF 72 HOURS PRIOR TO ALL INSPECTIONS REQUIRED BY THE LEE COUNTY LAND DEVELOPMENT CODE.
6. THE LOCATION OF EXISTING UTILITIES, PAVEMENT, VEGETATION, AND MISCELLANEOUS IMPROVEMENTS ARE APPROXIMATE ONLY. THE EXACT LOCATIONS SHALL BE VERIFIED BY THE CONTRACTOR IN THE FIELD.
7. EXISTING IMPROVEMENTS SHALL BE RESTORED TO A CONDITION EQUIVALENT TO THAT WHICH EXISTED PRIOR TO COMMENCING CONSTRUCTION, AT NO ADDITIONAL COST TO OWNER.
8. A VEGETATION PERMIT IS REQUIRED PRIOR TO ANY SITE WORK. PROTECTIVE BARRICADES MUST BE CONSTRUCTED AND INSPECTED PRIOR TO PERMIT ISSUANCE.
9. OWNER SHALL INSURE THAT THE GENERAL CONTRACTOR COMPLIES WITH ALL REQUIREMENTS OF STORM WATER POLLUTION PREVENTION (SWPS) ON THIS PLAN. IF THE PROJECT AREA IS MORE THAN 1 ACRE, THE OWNER SHALL FILE A NOTICE OF INTENT (NOI) WITH BOTH THE DEP AND LEE COUNTY DEVELOPMENT SERVICES AT LEAST 48 HRS. PRIOR TO THE START OF CONSTRUCTION.
10. ARTIFICIAL LIGHTING USED TO ILLUMINATE PREMISES SHALL BE DIRECTED AWAY FROM ADJACENT PROPERTIES AND STREETS, SHINING ONLY ON THE SUBJECT SITE. ALL LIGHT FIXTURES TO BE FURNISHED WITH FLAT LENSES AND HOUSE SHIELDS FULL CUTOFF TYPE TO LIMIT LIGHT POLLUTION FOR THE ADJOINING PROPERTIES, REFER TO LDC SECTION 34-625 (OUTDOOR LIGHTING STANDARDS). LIGHTING DESIGN BY OTHERS.
11. PER LEE TRAN, NO PUBLIC TRANSIT AMENITIES ARE REQUIRED IN CONJUNCTION WITH THIS PROJECT.
12. PROPOSED BUILDING FOOTPRINTS, PATIOS AND ACCESSORY STRUCTURES MAY BE MODIFIED WITHIN THE ALLOWABLE SETBACKS.
13. COASTAL SETBACK LINES DO NOT APPLY TO THIS PARCEL.
14. THERE ARE NO APPARENT ENVIRONMENTALLY SENSITIVE LANDS OR HABITAT FOR ENDANGERED SPECIES ON THIS PARCEL.
15. THERE ARE NO APPARENT FLOWING WELLS ON THIS PARCEL; IF ANY ARE FOUND DURING CONSTRUCTION, THEY WILL BE PLUGGED IN ACCORDANCE WITH SFWMD AND DEP PLUGGING REQUIREMENTS.
16. PRIOR TO THE START OF ANY CONSTRUCTION, PROVIDE A CONNECTION PERMIT FROM LEE COUNTY DOT OR FDOT WHICH EVER MAY BE APPLICABLE.
17. PRIOR TO THE ISSUANCE OF ANY CERTIFICATE OF COMPLIANCE, PROVIDE TO DEVELOPMENT SERVICES A NOTICE OF APPROVAL FROM LEE COUNTY DOT FOR WORK WITHIN THE COUNTY R.O.W.
18. FOR SEPTIC TANKS WHEN REQUIRED, ELEVATIONS, SIZES OF TANKS AND DRAIN FIELDS AND REQUIRED DRAIN FIELD ELEVATION SHALL BE OBTAINED FROM THE APPROVED SEPTIC TANK PERMIT.
19. YARD DRAINS SHALL BE ADDED IN GREEN AREAS AS NECESSARY AT THE DIRECTION OF THE LANDSCAPE ARCHITECT OR ENGINEER.
20. FOR THE UNDERGROUND SPRINKLER LINE SEE THE ENGINEER OF RECORDS 61G15 FIRE PROTECTION DOCUMENTS FROM THE POINT OF SERVICE (POS). THIS IS NOT THE CIVIL ENGINEER'S RESPONSIBILITY.
21. LCLDC 10-384 (c) WATER MAINS INSTALLATION: MINIMUM 12" FOR INDUSTRIAL AND 10" FOR COMMERCIAL. LCLDC 10-384 (d5) FIRE HYDRANT DESIGN AND SPACING: INDUSTRIAL AREAS SHALL BE 300 FEET APART AND COMMERCIAL AREAS SHALL BE 400 FEET APART. LCLDC 10-384 (5c) FIRE HYDRANTS REQUIRED WITHIN 400 FEET OF ANY PORTION OF THE BUILDING.
22. ALL RAMPS, SLOPES, WALKWAYS, SIDEWALKS INTENDED FOR DISABLE PERSON MUST MEET THE ADA STANDARDS FOR ACCESSIBLE DESIGN, LATEST EDITION.
23. THE CONTRACTOR SHALL BE RESPONSIBLE TO OBTAINED A TALL STRUCTURE PERMIT WITH LEE COUNTY PORT AUTHORITY IF CONSTRUCTION EQUIPMENT EXCEEDS THE DESIGNATED AIRSPACE NOTIFICATION HEIGHT. THE CONTACT FOR THIS PERMIT IS THE LCPA AIRPORT PLANNER AT 239-590-4609 AND THE PROCESS CAN BE FOUND AT HTTP://WWW.FLYLCPA.COM/TALLSTRUCTUREAPPLICATION.

GEOTECHNICAL NOTES:

1. UPON COMPLIANCE OF ALL FEDERAL, STATE AND LOCAL REGULATIONS BY THE OWNER, THIS SITE MAY BE SAFELY USED FOR BUILDING PURPOSES WITHOUT UNDUE DANGER FROM FLOOD. THE OWNER SHOULD RETAIN THE SERVICES OF A QUALIFIED GEOTECHNICAL ENGINEER TO DETERMINE IF SUBSURFACE CONDITIONS EXIST WHICH MAY ADVERSELY EFFECT THE DEVELOPMENT OF THIS SITE. THE OWNER SHOULD RETAIN THE SERVICES OF A QUALIFIED STRUCTURAL ENGINEER TO DETERMINE CORRECT FOUNDATION DESIGNS FOR ANY PROPOSED STRUCTURES ON THE SITE.
2. THERE WILL BE NO ADVERSE IMPACTS TO GROUND OR SURFACE WATERS, WETLANDS, FLOOD PLAINS OR RIVERINE AREAS; OR ANY KNOWN ARCHEOLOGICAL SITES EXPECTED BY THE DEVELOPMENT OF THIS SITE. A "CERTIFICATE TO DIG" IS NOT REQUIRED FOR THIS PROJECT PER LEE COUNTY ARCHEOLOGICAL SENSITIVITY MAP.
3. A VEGETATION PERMIT IS REQUIRED PRIOR TO ANY SITE WORK. PROTECTIVE BARRICADES MUST BE CONSTRUCTED AND INSPECTED PRIOR TO PERMIT ISSUANCE.
4. ANY EXISTING TOPSOIL AND UNSUITABLE ORGANIC SOILS (IF ENCOUNTERED) SHOULD BE STRIPPED AND UNDERCUT FROM WITHIN THE LIMITS OF THE PROPOSED PAVEMENTS, PROPOSED BUILDING FOOTPRINT AND A MINIMUM OF 5 FEET BEYOND THE BUILDING FOOTPRINT.
5. THE CONTRACTOR SHALL ENSURE THAT THE EXISTING SOILS WITHIN THE LIMITS OF THE DRY DETENTION AREAS AND TO A DEPTH OF AT THE LEAST THE WATER TABLE ARE SUITABLE FOR ADEQUATE PERCOLATION. MATERIAL IMPORTED FOR USE IN DRY DETENTION AREAS MUST ALSO BE SUITABLE FOR ADEQUATE PERCOLATION.

ENVIRONMENTAL NOTE:

FWC STAFF RECOMMENDS THE FOLLOWING MEASURES TO REDUCE NESTING POTENTIAL DURING CONSTRUCTION:

- CLEAR THE SITE ONLY WHEN READY TO BUILD, AND
- AVOID LEAVING CLEARED AREAS WITH LITTLE TO NO ACTIVITY FOR AN EXTENDED AMOUNT OF TIME.
- IF NESTING OCCURS CONTACT FWC STAFF TO DISCUSS NECESSARY NEST BUFFERS AND POTENTIAL PERMITTING ALTERNATIVES

TO REDUCE CONFLICTS WITH BEARS DURING DEVELOPMENT ACTIVITIES:

- MAINTAIN A CLEAN CONSTRUCTION SITE AND UTILIZE WILDLIFE-RESISTANT CONTAINERS FOR WORKERS TO USE FOR FOOD-RELATED AND OTHER WILDLIFE ATTRACTANT REFUSE
- FREQUENT TRASH REMOVAL AND THE USE OF PROPER FOOD STORAGE AND REMOVAL ON WORK SITES

NO.	REVISION	DATE

RAY AVENUE GAS STATION
PROJECT NOTES

604 MEADOW ROAD
LEHIGH ACRES, FLORIDA 33973

Quattrone & Associates, Inc.
Engineers, Planners & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

JOSHUA A. EISENOFF, STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NO. 97911. THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY JOSHUA A. EISENOFF, P.E. ON THE DATE INDICATED HERE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED & SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JOSHUA A. EISENOFF, P.E.
FL. PE #97911

VALID ONLY WITH STAMPED SEAL

SAVED BY: Jeff
SAVED ON: 2/17/26

SHEET
SD-13
13 OF 16

230704 - 1

P:\2023 Projects\230704 RAY AVENUE GAS STATION - ACTIVE\01-CADD FILES\01-QAI CADD - VERSION II\02-PRODUCTIONS PLANS\230704-SD

TABLE OF CONTENTS

Introduction **3**
Existing Conditions **3**
Proposed Project **3**
Floodplain Encroachment **3**
Tailwater Calculations **3**
Proposed Conditions **4-5**
Summary of Results **5**

Exhibit A – Aerial
Exhibit B – Soil Map
Exhibit C – FEMA Map
Exhibit D – Proposed ICPR Node Map
Exhibit E – Basin Area Exhibit
Appendix A – Site Calculations:

- Existing Land Use Table
- Proposed Land Use Table
- Existing Curve Number Calculations
- Proposed Curve Number Calculations
- Water Quality Calculations
- Proposed Stage Storage
- Wet Season High Water Table Map
- NRCS Custom Soil Survey

Appendix B – Proposed Conditions ICPR Reports

- Input Report
- Node Max Report
- 25-Year, 72-Hour Node Time Series
- 25-Year, 72-Hour Node Time Series (2nd Event)
- 72-Hour Drawdown Time Series

Appendix C – Existing Adjacent Permit

- SFWMD Application #110222-9

Appendix D – Pollutant Loading BMP Trains Report

I. INTRODUCTION

This Stormwater Management Report is being provided to the South Florida Water Management District for an Environmental Resource Permit for the proposed project, Ray Avenue Gas Station.

The subject project is located along the north side of SR-82 at the intersection of SR-82 and Ray Avenue. The Strap Number is 05-45-26-03-00035.0010; Section 05, Township 45, and Range 26E in Lehigh Acres, FL.

II. EXISTING CONDITIONS

Existing Conditions Overview:
The subject site is a vacant undeveloped commercially zoned parcel. In its existing conditions, stormwater is untreated and overland flows to the roadside swales along SR-82 and Meadow Road. The project site has an average elevation of 24.25' NAVD. The wet season water table was determined to be 22.50' NAVD from the permitted wet season water table per adjacent SFWMD Application #110222-9, adjacent Lehigh Acres Permits #19-07 and #24-04, the Lee County High Water Table Map shown in Appendix A, and discussions with Lehigh Acres Staff members.

FDEP WBID:
The existing site is located within a BMAP group 3 with a BMAPID of CALO.

III. PROPOSED PROJECT

The 0.80-acre site is proposed to be developed with a gas station and a convenience store and a covered pump area totaling 4,954 square feet. The proposed activities include the construction of the gas station, stormwater management system, paving, grading, associated utilities, and a driveway connection to Meadow Road. The stormwater management system will consist of a dry retention system, associated swales, storm pipes and storm structures to serve the project. Project limits and location maps are included as exhibits following this narrative.

IV. FLOODPLAIN ENCROACHMENT

The proposed project is located within an Unincorporated Flood Zone with no required Base Flood Elevation per Firm Community 125124, Panel No. 0475, Map # 12071C0475F dated 08/28/2008.

V. TAILWATER CALCULATIONS

The Proposed project discharges to an existing roadside swale along Meadow Road on the northeast side of the site. Tailwater elevations were determined by the elevation of the top of bank of the existing roadside swale within the ROW. With the above information the following tailwater definitions were calculated:

For the 10 Year, 1-day storm event, the tailwater elevation starts at elevation 22.50' at hour 0 and increases to elevation 23.90' at hour 18. The receiving water body recovers to 22.50' at hour 30.

The 25 year, 3-day storm event tailwater definition is calculated as starting at the wet season water table of 22.50' at hour 0. At hour 60, the tailwater elevation increases to 23.90'. At hour 360 the tailwater has fully recovered to elevation 22.50. Please refer to the ICPR input report that shows the above tailwater stages for the required storm events.

VI. PROPOSED CONDITIONS

Pollutant Loading:
Curve numbers for existing site conditions were determined using Table 2-2c of the TR-55 manual for wooded land under Hydrologic Soil Group D. Based on this table, an existing condition curve number of 77 was calculated.

Proposed condition curve numbers were developed using the proposed land use table in conjunction with Table 2-2a of the TR-55 manual, resulting in a weighted curve number of 84.43 for the project site. As a conservative assumption for the nutrient loading analysis, all proposed impervious areas were treated as directly connected impervious area (DCIA), representing 59.99 percent of the total drainage basin area. A non-DCIA curve number of 80 was applied for the pervious portion of the basin. The land use assumptions and detailed curve number calculations are provided in Appendix A of this report.

Using the curve numbers described above, a nutrient loading analysis was performed utilizing the BMP Trains program. Based on the results, the project is required to provide 0.083 acre-feet of retention prior to offsite discharge. The proposed design satisfies this requirement by providing on-site retention between elevations 23.50 feet and 24.43 feet NAVD, with a control structure establishing the first point of discharge at elevation 24.43 feet NAVD. The complete nutrient loading analysis is included in Appendix D of this report.

ICPR4 Modeling Events:
The proposed project was evaluated using the software Interconnected Pond Routing version 4.07.08 (ICPR). The following storms were modeled and evaluated as part of the ICPR4 software: 10 year, 24-hour; 25 year, 72-hour; 25 year, 72-hour R2 (back to back storm); & 100-year, 72 hour zero discharge.

ICPR4 Modeling Results:
Per the above, the 10-year, 25-year and 100-year storm events were modeled. The 10-year modeling results set the minimum elevation for the proposed roadways and parking while the 100-year sets the minimum elevation of the finish floor of the proposed buildings. The 25-year storm event modeling results are analyzed at hour 360 to establish recovery and the need for a 2nd 25-year storm event. Per the above, the first 25-year, 72-hour storm event stage at hour 360 was evaluated and compared to the starting elevation of 22.50'. Per the ICPR4 modeling results, the stage recovered to 24.43' which demonstrates the site does not fully recover. Per requirements of SFWMD, a second 25 year, 72-hour storm event was modeled with its starting stage at the end stage of the first storm. The peak elevation of this 2nd 25 year, 72-hour storm event will be the required minimum berm elevation.

Because retention is utilized for nutrient removal, a supplemental analysis was performed to demonstrate that the required nutrient loading volume is fully recovered within 72 hours. A drawdown analysis was conducted in ICPR beginning at the nutrient loading elevation of 24.43 feet NAVD. Per the results of this analysis, the proposed project recovers to elevation 22.76' NAVD which is below the toe of the retention areas indicating that all retention volume is available after 72-hours.

Soil percolation rates were obtained from the NRCS Custom Soil Resource Report for the soils present within the project area. According to the report, Wabasso Sand has a percolation rate of

68.54 micrometers per second, which converts to 9.72 feet per day after applying a factor of safety of 2.0. Soil percolation was considered solely for the purpose of the 72-hour drawdown analysis and was not relied upon in establishing design control elevations.

The maximum allowable flow rate for this site is 0.04 cfs. The proposed max flow rate during the 25-year, 72-hour storm event is 0.29 cfs. Although the proposed discharge rate exceeds the max allowable, a minimum 3" diameter orifice is used per the applicant handbook vol 2 section 5.2.

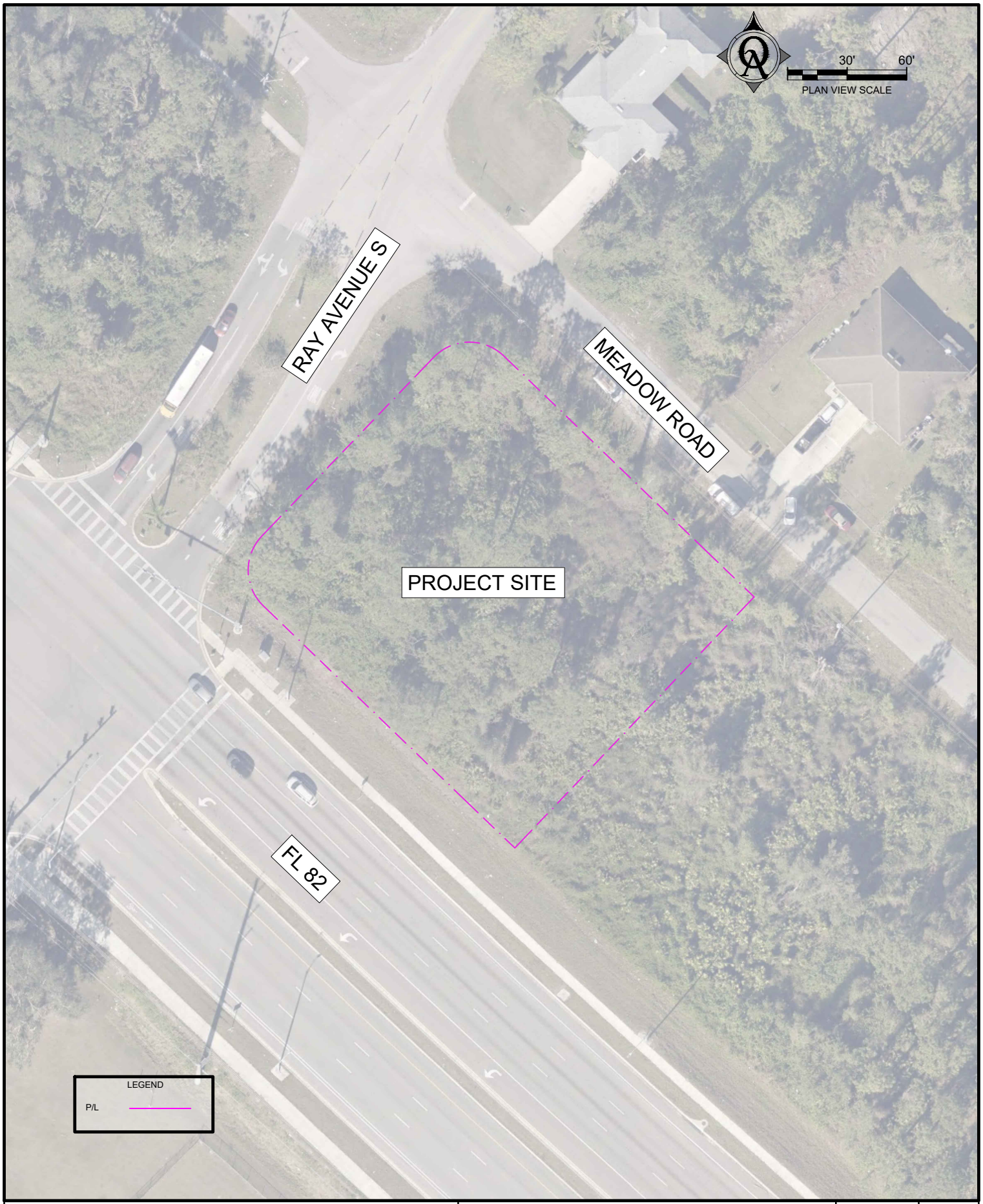
Please see the following table with the Basic Site & Modeling Information, Design Modeling Summary Data and Design Parameter Summary:

Basic Site & Modeling Information			
Basin Allowable Discharge Rate	30.1 csm		
Total Basin Area	0.80 ac		
72 Hour Retention Drawdown Elevation	22.76' NAVD (All Storage is Available)		
Control Elevation	22.50' NAVD		
Allowable Discharge Rate	0.04 cfs		
Design Modeling Summary Data			
	Rainfall (inch)	Peak Stage (ft-NAVD)	Peak Outfall Rate (cfs)
10 Year, 24-Hour Storm Event	6.50	25.37	
25 Year, 72-Hour Storm Event	11.00	26.02	0.29
25 Year, 72-Hour Storm Event (2nd Event)	11.00	26.04	
100 Year, 72-Hour Storm Event Zero Discharge	14.00	26.93	
Design Parameter Summary			
	Min. Required	Provided	Notes
Retention Volume Required (acre-feet)	0.08	0.08	Provided from 23.50' to 24.43'
Min. Parking Elevation (ft-NAVD)	25.37	25.37	
Min. Road Elevation (ft-NAVD)	25.37	25.37	
Min. Perimeter Berm (ft-NAVD)	26.04	26.04	25 Year, 72-Hour (2nd Event)
Min. Finished Floor (ft-NAVD)	26.93	26.93	

VII. SUMMARY OF RESULTS

Per the above calculated information, the proposed stormwater system is designed within the requirements of FDOT, SFWMD and Lee County. The proposed system maintains and improves offsite drainage patterns and does not propose any adverse impacts to surrounding parcels.

EXHIBIT A-AERIAL MAP



Quattrone & Associates, Inc.
Engineers, Planners, & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

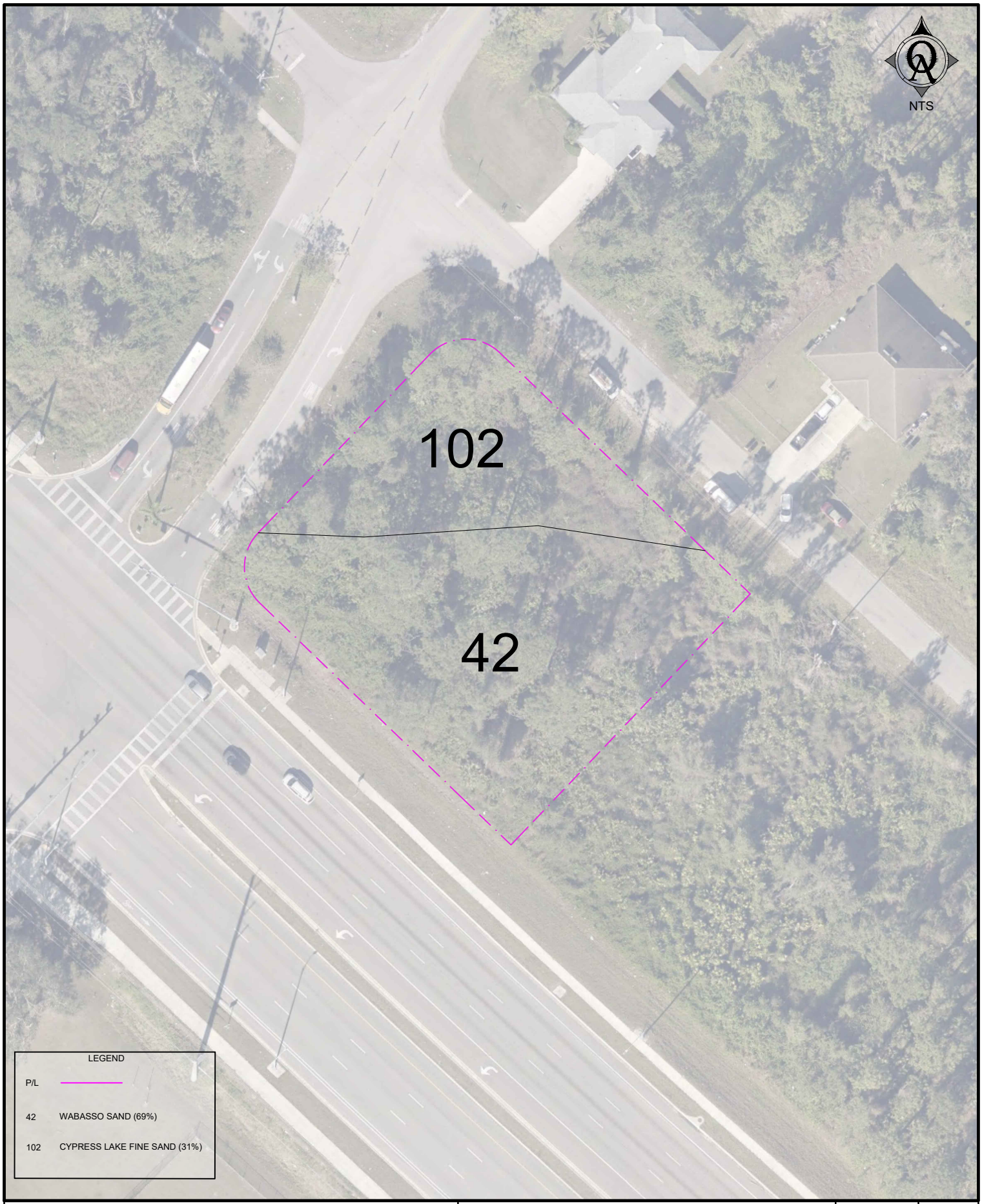
RAY AVENUE GAS STATION

AERIAL MAP

LEE COUNTY, FLORIDA 33973

SHEET
EX-A
01 OF 01

EXHIBIT B-SOIL MAP



Quattrone & Associates, Inc.
Engineers, Planners, & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

RAY AVENUE GAS STATION

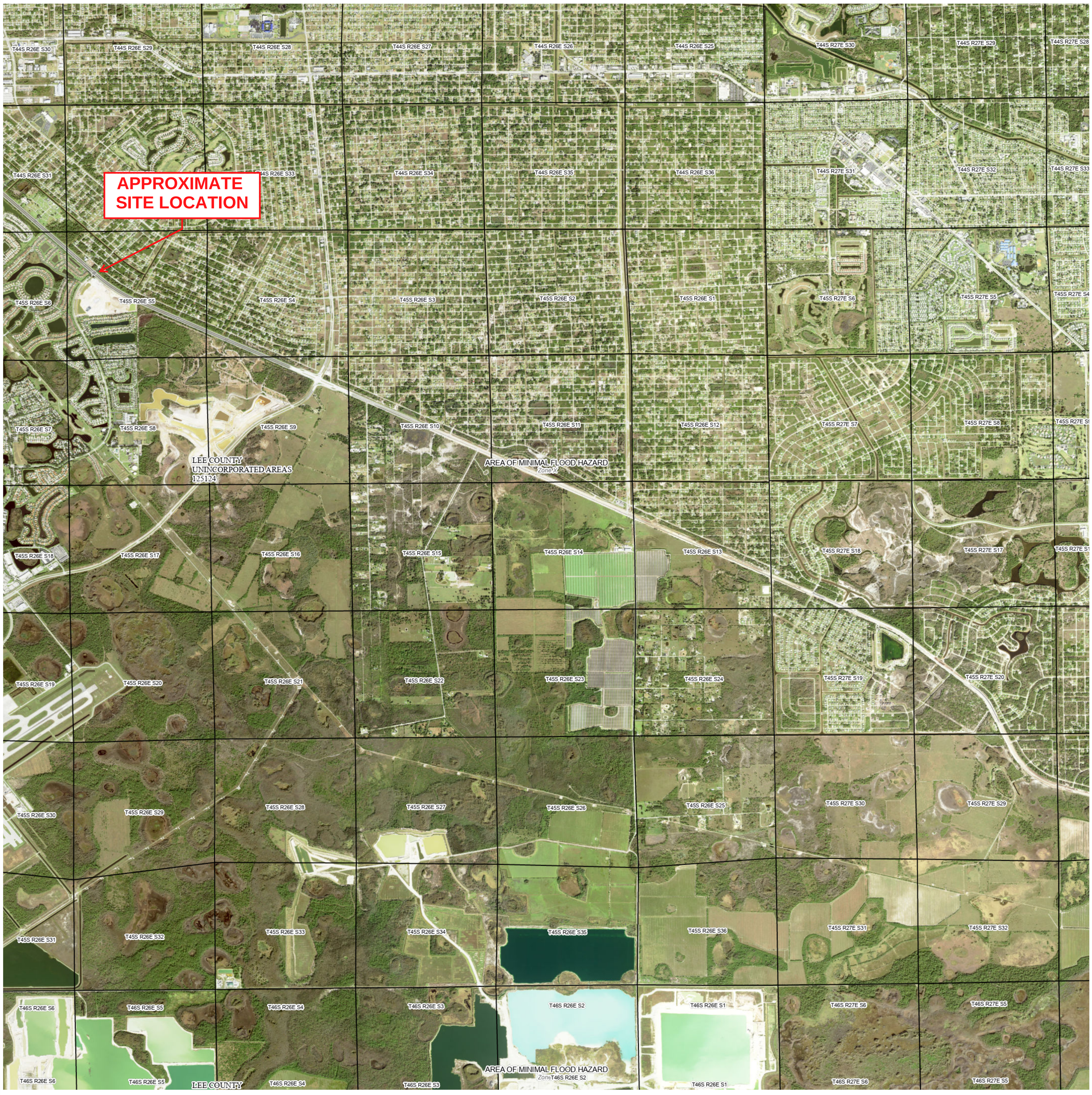
SOIL MAP

LEE COUNTY, FLORIDA 33973

SHEET
EX-B
01 OF 01

EXHIBIT C-FEMA MAP

81°45'1.72"W 26°37'56.37"N



81°37'28.3"W 26°29'33.66"N

FLOOD HAZARD INFORMATION

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR DRAFT FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
- Area with Reduced Flood Risk due to Levee See Notes *Zone X*
- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard *Zone X*
- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- 20.2 Cross Sections with 1% Annual Chance
- 17.5 Water Surface Elevation
- 8 Coastal Transect
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature
- 513 Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary

NOTES TO USERS

For information and questions about this Flood Insurance Rate Map (FIRM), available products associated with this FIRM, including historic versions, the current map date for each FIRM panel, how to order products, or the National Flood Insurance Program (NFIP) in general, please call the FEMA Map Information eXchange at 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA Flood Map Service Center website at https://msc.fema.gov. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the website.

Communities annexing land on adjacent FIRM panels must obtain a current copy of the adjacent panel as well as the current FIRM Index. These may be ordered directly from the Flood Map Service Center at the number listed above.

For community and countywide map dates, refer to the Flood Insurance Study Report for this jurisdiction.

To determine if flood insurance is available in this community, contact your Insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

Basemap information shown on this FIRM was provided in digital format by USDA, Farm Service Agency (FSA). This information was derived from NAIP, dated April 11, 2018.

This map was exported from FEMA's National Flood Hazard Layer (NFHL) on **5/29/2024 2:52 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. For additional information, please see the Flood Hazard Mapping Updates Overview Fact Sheet at https://www.fema.gov/media-library/assets/documents/118418

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date.

SCALE

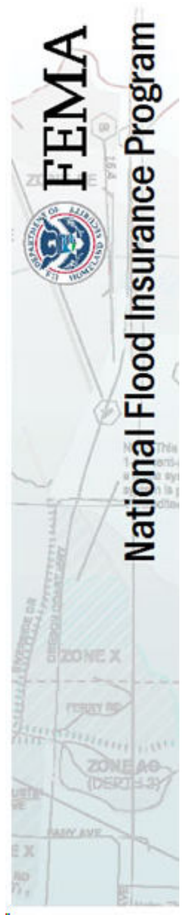
Map Projection:
GCS, Geodetic Reference System 1980;
Vertical Datum: No elevation features on this FIRM
For information about the specific vertical datum for elevation features, datum conversions, or vertical monuments used to create this map, please see the Flood Insurance Study (FIS) Report for your community at https://msc.fema.gov

N

1 inch = 2,000 feet 1:24,000

0 1,000 2,000 4,000 6,000 8,000 Feet

0 210 420 840 1,260 1,680 Meters



NATIONAL FLOOD INSURANCE PROGRAM

FLOOD INSURANCE RATE MAP

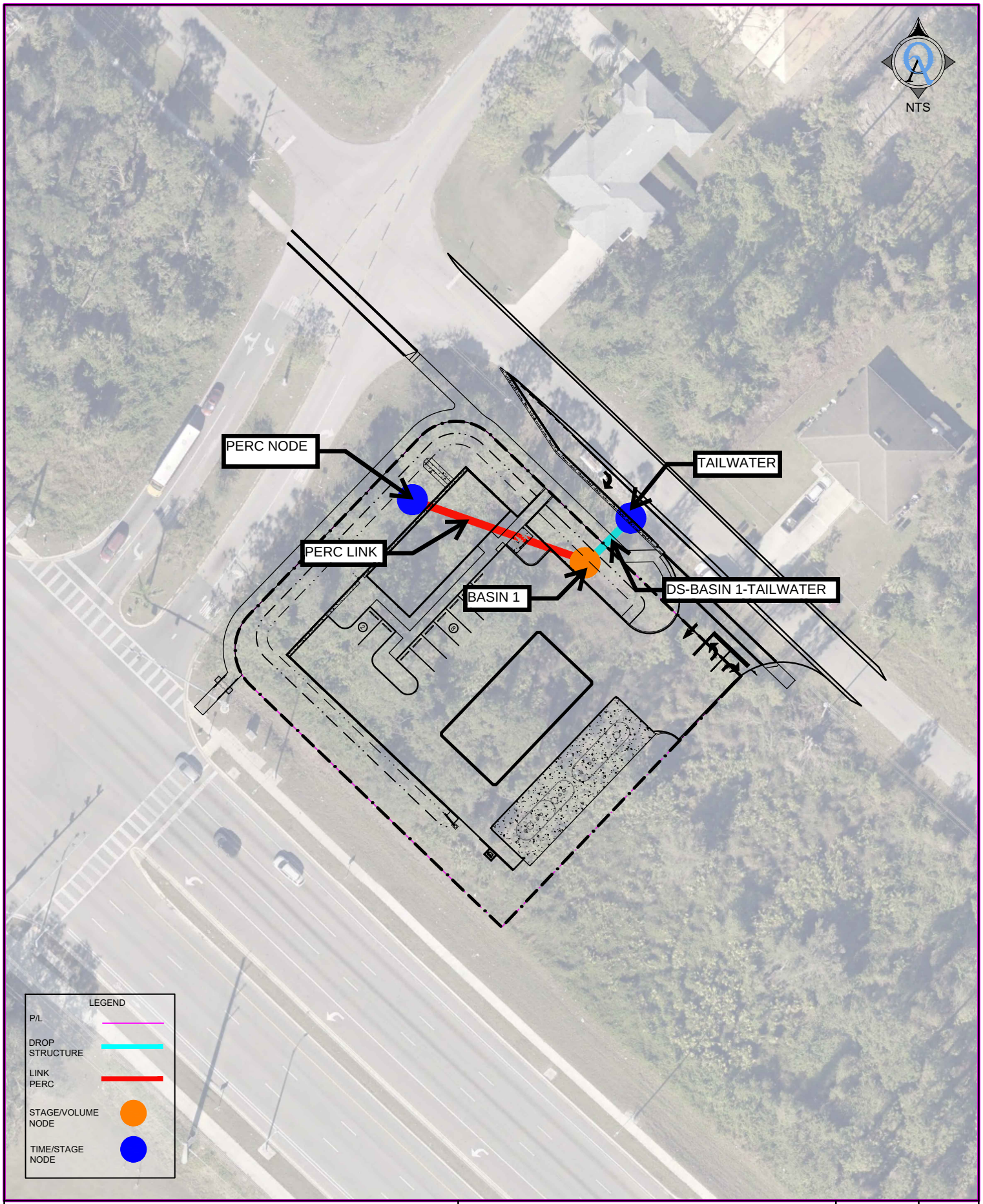
PANEL **475** OF **678**

Panel Contains:

COMMUNITY	NUMBER	PANEL
LEE COUNTY	125124	0475

MAP NUMBER
12071C0475F
EFFECTIVE DATE
August 28, 2008

EXHIBIT D-ICPR NODE MAP



Quattrone & Associates, Inc.
Engineers, Planners, & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

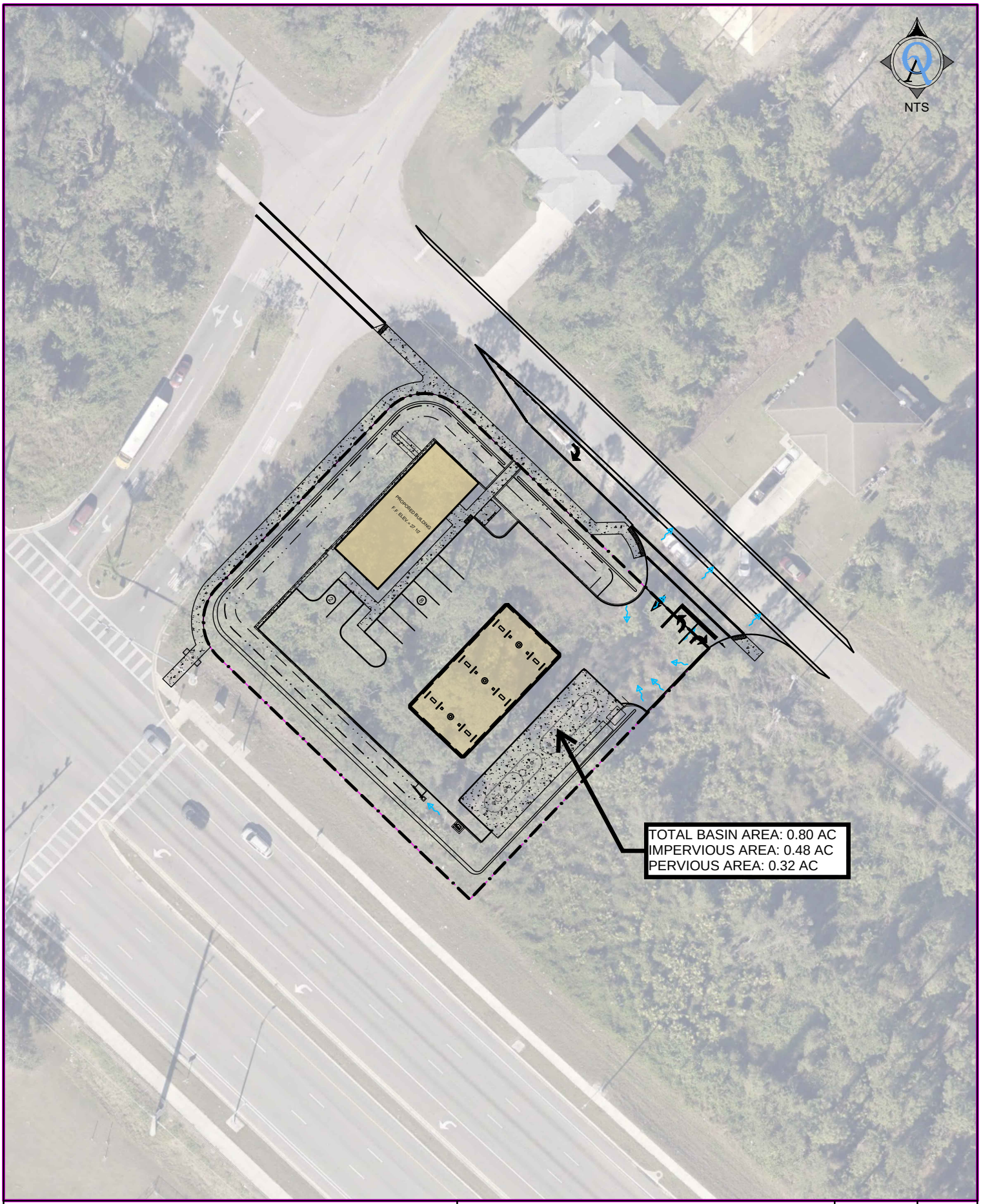
RAY AVENUE GAS STATION

PROPOSED NODE MAP

LEE COUNTY, FLORIDA 33973

SHEET
EX-D
01 OF 01

EXHIBIT E-BASIN AREA EXHIBIT



Quattrone & Associates, Inc.
Engineers, Planners, & Development Consultants
4301 Veronica Shoemaker Blvd - Fort Myers, Florida 33916 - 239-936-5222
Certificate of Authorization Number: 9465

RAY AVENUE GAS STATION

BASIN AREA EXHIBIT

LEE COUNTY, FLORIDA 33973

SHEET
EX-E
01 OF 01

Appendix A- Site Calculations

Existing Land Use Table

EXISTING PROJECT LANDUSE		
DESCRIPTION	AC	%
TOTAL AREA	0.80	100%
BUILDING GROUND FLOOR AREA	0.00	0%
PAVEMENT AREA	0.00	0%
OTHER IMPERVIOUS	0.00	0%
PERVIOUS ACREGE	0.80	100%
TOTAL IMPERVIOUS AREA	0.00	0%
TOTAL PERVIOUS AREA	0.80	100%

Proposed Land Use Table

PROPOSED PROJECT LANDUSE		
DESCRIPTION	**AC**	**%**
TOTAL BASIN AREA	0.80	100%
BUILDING GROUND FLOOR AREA	0.11	14%
PAVEMENT AREA	0.29	36%
OTHER IMPERVIOUS	0.08	10%
PERVIOUS ACREGE	0.32	40%
TOTAL IMPERVIOUS AREA	**0.48**	60%
TOTAL PERVIOUS AREA	**0.32**	40%
TOTAL PROJECT AREA	**0.80**	N/A

Existing Curve Number Calculations

Pre - Developed Site :								
	Area (ac)	Imp Area	Imp CN	CN * Area	Perv Area	Perv CN	CN * Area	CN (Weighted)
Undeveloped Land	0.80	0.00	98.00	0.00	0.80	77.00	61.62	77.00
Total Area	**0.80**						**Total Weighted CN:**	**77.00**
Perv CN based on TR-55 Woods (D) Good								

Proposed Curve Number Calculations

Post - Developed Site Basin 1:								
	Area (ac)	Imp Area	Imp CN	CN * Area	Perv Area	Perv CN	CN * Area	CN (Weighted)
Commercial Site	0.72	0.48	98.00	47.05	0.24	61.00	14.82	85.57
Dry Detention Bottom	0.08	0.00	98.00	0.00	0.08	77.00	5.95	77.00
Total	**0.80**						**Total Weighted CN:**	**84.74**

Water Quality Calculations

NUTRIENT LOADING

REQUIRED NUTRIENT LOADING VOLUME	0.084	AC-FT	Required Stage =	24.43 ' NAVD
PROVIDED NUTRIENT LOADING VOLUME	0.084	AC-FT	Actual Stage =	24.43 ' NAVD

Proposed Stage Storage Table

STAGE VS STORAGE CURVE BASIN 1

	LAKES	ROADS	MISC IMP	PERV	DETENTION	INDIGENOUS	TOTAL	EL DELTA
AREA (AC)	0.00	0.29	0.08	0.19	0.13	0.00	0.69	0.40
MIN EL (FT)	0.00	25.40	25.40	25.30	23.50	0.00		
MAX EL (FT)	0.00	27.10	27.10	27.10	25.30	0.00	TOTAL w/o INDIGENOUS	TOTAL w/ INDIGENOUS
ELEV.	STORAGE (AC-FT)							
22.50	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
22.90	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
23.30	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
23.70	0.000	0.000	0.000	0.000	0.016	0.000	0.016	0.016
24.10	0.000	0.000	0.000	0.000	0.051	0.000	0.051	0.051
24.50	0.000	0.000	0.000	0.000	0.091	0.000	0.091	0.091
24.90	0.000	0.000	0.000	0.000	0.136	0.000	0.136	0.136
25.30	0.000	0.000	0.000	0.000	0.185	0.000	0.185	0.185
25.70	0.000	0.008	0.002	0.009	0.236	0.000	0.254	0.254
26.10	0.000	0.042	0.011	0.034	0.287	0.000	0.374	0.374
26.50	0.000	0.103	0.028	0.077	0.339	0.000	0.546	0.546
26.90	0.000	0.191	0.051	0.137	0.390	0.000	0.769	0.769
27.30	0.000	0.303	0.081	0.211	0.441	0.000	1.037	1.037
27.70	0.000	0.419	0.112	0.288	0.493	0.000	1.312	1.312
28.10	0.000	0.535	0.143	0.365	0.544	0.000	1.586	1.586
28.50	0.000	0.650	0.174	0.442	0.595	0.000	1.861	1.861

Wet Season High Water Table Map

Legend

Elevation
- 5
- 10
- 15
- 20
- 25

WATERSHED
- 22 Bayshore Creek
- 37 Bedman Creek
- 41 Billy Creek
- 21 Chapel Branch
- Cocohatchee River
- 19 Cohn Branch
- 36 County Line
- 44 Cow Creek
- 33 Cypress Creek
- 20 Daughtrey Creek
- 20A Daughtrey Creek-East Branch
- 43 Deep Lagoon
- 2 Durden Creek
- 47A Estero River
- 31 Fichter Branch
- 30Franklin Run
- 5 Gator Slough
- 3 Greenwell Branch
- 47B Halfway Creek
- 16 Hancock Creek
- 45 Hendry Creek
- 38 Hickey Creek
- 49 Imperial River
- 26 Kickapoo Creek
- 49L Leitner Creek
- 4 Longview Run
- 18 Marsh Point
- 35 Miller's Gully
- 46B Mullock Creek
- 490 Oak Creek
- 39 Olga Creek
- 40 Orange River
- 28 Otter Creek
- 270 Owl Creek
- 25 Palm Creek
- 32 Park Branch
- 23 Popash Creek
- 17 Powell Creek
- 46A Six Mile Cypress
- 34 Spanish Creek
- 48 Spring Creek
- 24 Stroud Creek
- 29 Telegraph Creek
- 46C Ten Mile Canal
- 27 Trout Creek
- 42 Whiskey Creek
- 16Y Yellow Fever Creek
- 16E Yellow Fever Creek-East Branch
- 1 Yucca Pen Creek
- Well Locations

APPROXIMATE SITE LOCATION

Some contour lines may not be optimized due to unavailable data in certain boundaries

Disclaimer: Lee County Natural Resources Information accessed through this web page provide a visual display of data for your convenience. Every reasonable effort has been made to assure the accuracy of the maps and associated data. Lee County makes no warranty, representation or guaranty as to the content, sequence, accuracy, timeliness or completeness of any of the data provided herein. The user of these applications should not rely on the data provided herein for any reason. Lee County explicitly disclaims any representations and warranties, including, without limitation, the implied warranties of merchantability and fitness for a particular purpose. Lee County shall\ assume no liability for any errors, omissions, or inaccuracies in the information provided regardless of how caused. Lee County shall assume no liability for any decisions made or actions taken or not taken by the user of the applications in reliance upon any information or data furnished hereunder.

LEE COUNTY
NATURAL RESOURCES
1500 Monroe St.
P.O. Box 398
Fort Myers, FL 33903
http://www.lee-county.com

LEE COUNTY
Wet Season High Water Table Map
2023 (NAVD 88 Datum) Telemetry and Non-Telemetry Well Data

2 1 0 2 Miles

Created in ArcGIS using ArcMap
NAVD 88 Datum

ESRI

NRCS Custom Soil Survey

United States Department of Agriculture

NRCS

Natural Resources Conservation Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Lee County, Florida



December 16, 2025

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

Contents

Preface.....2
How Soil Surveys Are Made.....5
Soil Map.....8
Soil Map.....9
Legend.....10
Map Unit Legend.....11
Map Unit Descriptions.....11
Lee County, Florida.....13
42—Wabasso sand, limestone substratum, 0 to 2 percent slopes.....13
102—Cypress Lake fine sand-Urban land complex, 0 to 2 percent slopes.....15
Soil Information for All Uses.....18
Soil Properties and Qualities.....18
Soil Physical Properties.....18
Saturated Hydraulic Conductivity (Ksat).....18
References.....22

How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

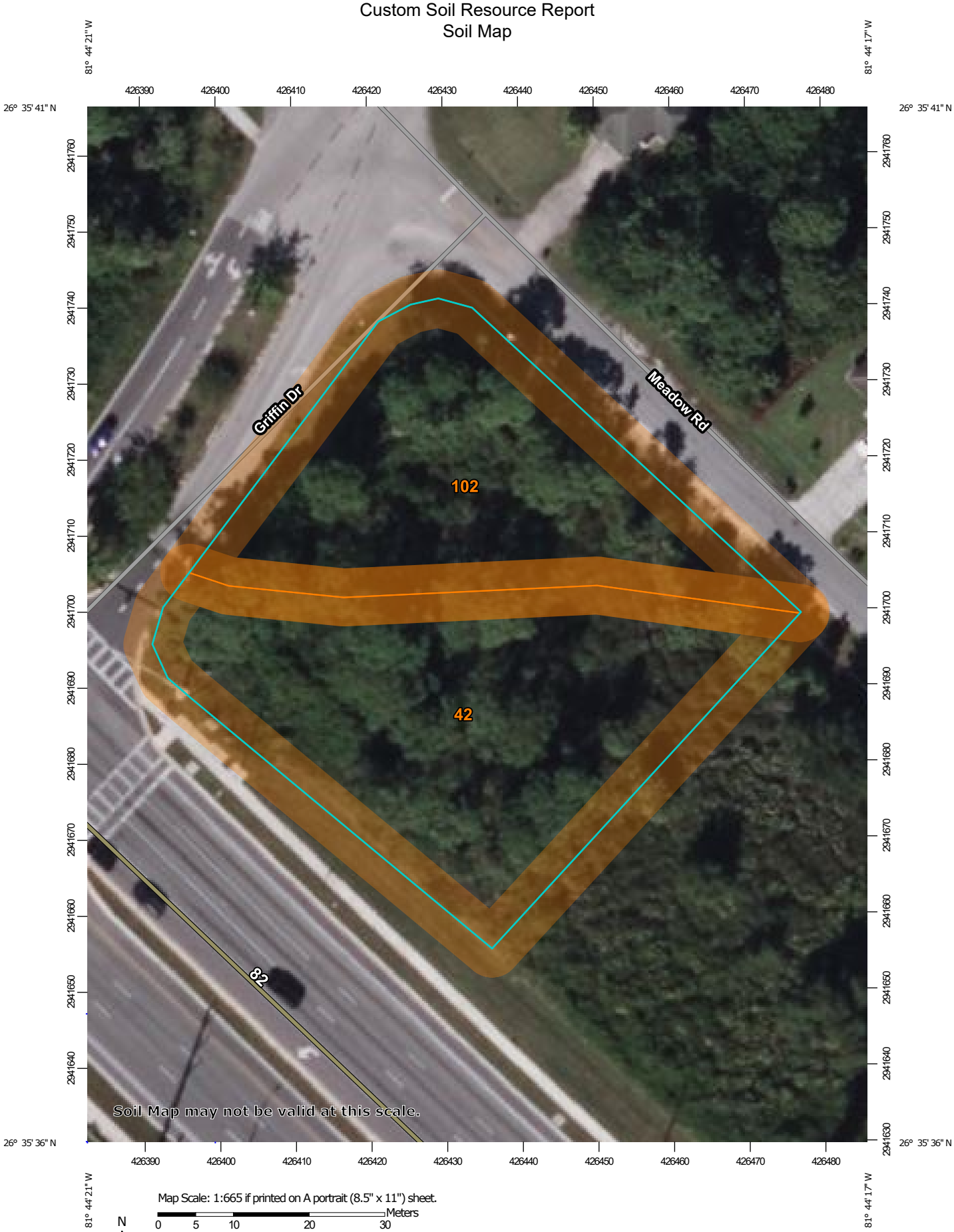
After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report
Soil Map



Map Scale: 1:665 if printed on A portrait (8.5" x 11") sheet.

N

Meters: 0 5 10 20 30

Feet: 0 30 60 120 180

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 17N WGS84

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Lee County, Florida
Survey Area Data: Version 23, Aug 29, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Nov 14, 2021—Nov 23, 2021

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
42	Wabasso sand, limestone substratum, 0 to 2 percent slopes	0.6	56.9%
102	Cypress Lake fine sand-Urban land complex, 0 to 2 percent slopes	0.4	43.1%
Totals for Area of Interest		**1.0**	**100.0%**

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The

delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Lee County, Florida

42—Wabasso sand, limestone substratum, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 2tzws
Elevation: 0 to 50 feet
Mean annual precipitation: 46 to 54 inches
Mean annual air temperature: 70 to 77 degrees F
Frost-free period: 355 to 365 days
Farmland classification: Farmland of unique importance

Map Unit Composition

Wabasso, limestone substratum, and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Wabasso, Limestone Substratum

Setting

Landform: Flatwoods on marine terraces
Landform position (three-dimensional): Tread, talf
Down-slope shape: Convex, linear
Across-slope shape: Linear
Parent material: Sandy and loamy marine deposits over limestone

Typical profile

A - 0 to 6 inches: sand
E - 6 to 25 inches: sand
Bh - 25 to 35 inches: sand
Btg - 35 to 45 inches: sandy clay loam
2R - 45 to 55 inches: bedrock

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: 13 to 54 inches to lithic bedrock
Drainage class: Poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 6 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum: 4.0
Available water supply, 0 to 60 inches: Low (about 3.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3w
Hydrologic Soil Group: C/D
Ecological site: F155XY120FL - Sandy Flatwoods and Hammocks
Forage suitability group: Sandy soils on flats of mesic or hydric lowlands (G155XB141FL)

Other vegetative classification: South Florida Flatwoods (R155XY003FL), Sandy soils on flats of mesic or hydric lowlands (G155XB141FL)
Hydric soil rating: No

Minor Components

Cypress lake
Percent of map unit: 6 percent
Landform: Flats on marine terraces, drainageways on marine terraces
Landform position (three-dimensional): Tread, talf, dip
Down-slope shape: Convex, linear
Across-slope shape: Linear, concave
Ecological site: F155XY130FL - Sandy over Loamy Flatwoods and Hammocks
Other vegetative classification: South Florida Flatwoods (R155XY003FL), Sandy over loamy soils on flats of hydric or mesic lowlands (G155XB241FL)
Hydric soil rating: Yes

Gentry
Percent of map unit: 3 percent
Landform: Depressions on marine terraces
Landform position (three-dimensional): Tread, dip
Down-slope shape: Concave
Across-slope shape: Concave
Ecological site: R155XY080FL - Sandy over Loamy Freshwater Isolated Marshes and Swamps
Other vegetative classification: Freshwater Marshes and Ponds (R155XY010FL), Sandy over loamy soils on stream terraces, flood plains, or in depressions (G155XB245FL)
Hydric soil rating: Yes

Gator
Percent of map unit: 3 percent
Landform: Depressions on marine terraces
Landform position (three-dimensional): Tread, dip
Down-slope shape: Concave
Across-slope shape: Concave
Ecological site: R155XY100FL - Organic Freshwater Isolated Marshes and Swamps
Other vegetative classification: Freshwater Marshes and Ponds (R155XY010FL), Organic soils in depressions and on flood plains (G155XB645FL)
Hydric soil rating: Yes

Brynwood
Percent of map unit: 3 percent
Landform: Flatwoods on marine terraces
Landform position (three-dimensional): Tread, talf
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F155XY120FL - Sandy Flatwoods and Hammocks
Other vegetative classification: South Florida Flatwoods (R155XY003FL), Sandy soils on flats of mesic or hydric lowlands (G155XB141FL)
Hydric soil rating: Yes

102—Cypress Lake fine sand-Urban land complex, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 2zldz
Elevation: 0 to 70 feet
Mean annual precipitation: 42 to 56 inches
Mean annual air temperature: 68 to 77 degrees F
Frost-free period: 350 to 365 days
Farmland classification: Not prime farmland

Map Unit Composition

Cypress lake and similar soils: 42 percent
Urban land: 36 percent
Minor components: 22 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Cypress Lake

Setting

Landform: Flatwoods on marine terraces, drainageways on marine terraces
Landform position (three-dimensional): Tread, talf, dip
Down-slope shape: Linear
Across-slope shape: Linear, concave
Parent material: Sandy and loamy marine deposits over limestone

Typical profile

A - 0 to 3 inches: fine sand
E - 3 to 14 inches: fine sand
E/B - 14 to 25 inches: fine sand
Btg - 25 to 30 inches: fine sandy loam
2R - 30 to 40 inches: bedrock

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: 8 to 40 inches to lithic bedrock
Drainage class: Poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: About 3 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 4 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum: 4.0
Available water supply, 0 to 60 inches: Very low (about 2.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3w

Hydrologic Soil Group: A/D
Ecological site: F155XY130FL - Sandy over Loamy Flatwoods and Hammocks
Forage suitability group: Sandy over loamy soils on flats of hydric or mesic lowlands (G155XB241FL)
Other vegetative classification: South Florida Flatwoods (R155XY003FL), Sandy over loamy soils on flats of hydric or mesic lowlands (G155XB241FL)
Hydric soil rating: Yes

Description of Urban Land

Setting

Landform: Flatwoods on marine terraces
Landform position (three-dimensional): Riser, talf
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: No parent material

Interpretive groups

Land capability classification (irrigated): None specified
Ecological site: F155XY130FL - Sandy over Loamy Flatwoods and Hammocks
Forage suitability group: Forage suitability group not assigned (G155XB999FL)
Other vegetative classification: Forage suitability group not assigned (G155XB999FL)
Hydric soil rating: Unranked

Minor Components

Brynwood

Percent of map unit: 8 percent
Landform: Flatwoods on marine terraces
Landform position (three-dimensional): Tread, talf
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F155XY120FL - Sandy Flatwoods and Hammocks
Other vegetative classification: South Florida Flatwoods (R155XY003FL), Sandy soils on flats of mesic or hydric lowlands (G155XB141FL)
Hydric soil rating: Yes

Wabasso

Percent of map unit: 6 percent
Landform: Flatwoods on marine terraces
Landform position (three-dimensional): Tread, talf
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F155XY120FL - Sandy Flatwoods and Hammocks
Other vegetative classification: South Florida Flatwoods (R155XY003FL), Sandy soils on flats of mesic or hydric lowlands (G155XB141FL)
Hydric soil rating: No

Pineda

Percent of map unit: 4 percent
Landform: Flats on marine terraces, drainageways on marine terraces
Landform position (three-dimensional): Tread, talf, dip
Down-slope shape: Linear
Across-slope shape: Linear, concave
Ecological site: F155XY130FL - Sandy over Loamy Flatwoods and Hammocks

Other vegetative classification: Slough (R155XY011FL), Sandy over loamy soils on flats of hydric or mesic lowlands (G155XB241FL)
Hydric soil rating: Yes

Ft. drum

Percent of map unit: 2 percent
Landform: Flatwoods on marine terraces
Landform position (three-dimensional): Tread, talf
Down-slope shape: Convex
Across-slope shape: Linear
Ecological site: F155XY120FL - Sandy Flatwoods and Hammocks
Other vegetative classification: Sandy soils on flats of mesic or hydric lowlands (G155XB141FL)
Hydric soil rating: No

Cypress lake

Percent of map unit: 2 percent
Landform: Drainageways on marine terraces, flatwoods on marine terraces
Landform position (three-dimensional): Tread, talf, dip
Down-slope shape: Linear
Across-slope shape: Linear, concave
Ecological site: F155XY130FL - Sandy over Loamy Flatwoods and Hammocks
Other vegetative classification: South Florida Flatwoods (R155XY003FL), Sandy over loamy soils on flats of hydric or mesic lowlands (G155XB241FL)
Hydric soil rating: No

Soil Information for All Uses

Soil Properties and Qualities

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

Soil Physical Properties

Soil Physical Properties are measured or inferred from direct observations in the field or laboratory. Examples of soil physical properties include percent clay, organic matter, saturated hydraulic conductivity, available water capacity, and bulk density.

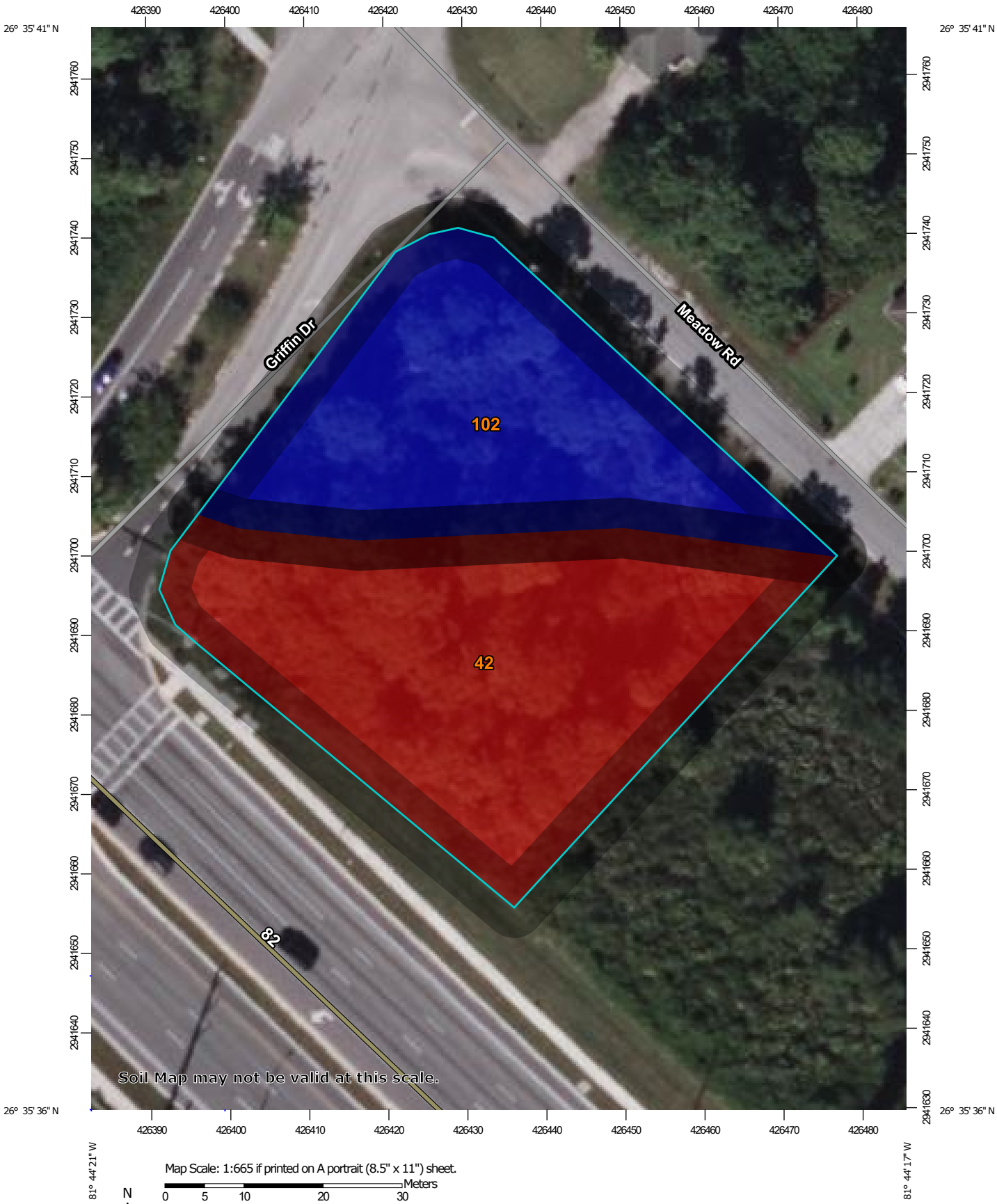
Saturated Hydraulic Conductivity (Ksat)

Saturated hydraulic conductivity (Ksat) refers to the ease with which pores in a saturated soil transmit water. The estimates are expressed in terms of micrometers per second. They are based on soil characteristics observed in the field, particularly structure, porosity, and texture. Saturated hydraulic conductivity is considered in the design of soil drainage systems and septic tank absorption fields.

For each soil layer, this attribute is actually recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.

The numeric Ksat values have been grouped according to standard Ksat class limits.

Map—Saturated Hydraulic Conductivity (Ksat)



Map Scale: 1:665 if printed on A portrait (8.5" x 11") sheet.

N

Meters: 0 5 10 20 30

Feet: 0 30 60 120 180

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 17N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- <= 68.5379
- > 68.5379 and <= 78.5287
- Not rated or not available

Soil Rating Lines

- <= 68.5379
- > 68.5379 and <= 78.5287
- Not rated or not available

Soil Rating Points

- <= 68.5379
- > 68.5379 and <= 78.5287
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Lee County, Florida
Survey Area Data: Version 23, Aug 29, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Nov 14, 2021—Nov 23, 2021

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Table—Saturated Hydraulic Conductivity (Ksat)

= 19.43 ft/day= 9.72 ft/day (F.S.= 2.0)

Map unit symbol	Map unit name	Rating (micrometers per second)	Acres in AOI	Percent of AOI
42	Wabasso sand, limestone substratum, 0 to 2 percent slopes	68.5379	0.6	56.9%
102	Cypress Lake fine sand-Urban land complex, 0 to 2 percent slopes	78.5287	0.4	43.1%
Totals for Area of Interest			**1.0**	**100.0%**

Rating Options—Saturated Hydraulic Conductivity (Ksat)

Units of Measure: micrometers per second

Aggregation Method: Dominant Component

Component Percent Cutoff: None Specified

Tie-break Rule: Fastest

Interpret Nulls as Zero: No

Layer Options (Horizon Aggregation Method): All Layers (Weighted Average)

References

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.

Federal Register. July 13, 1994. Changes in hydric soils of the United States.

Federal Register. September 18, 2002. Hydric soils of the United States.

Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.

National Research Council. 1995. Wetlands: Characteristics and boundaries.

Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_054262

Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053577

Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053580

Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.

United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.

United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/home/?cid=nrcs142p2_053374

United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/landuse/rangepasture/?cid=stelprdb1043084

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

Appendix B: Proposed Conditions ICPR Reports

ICPR Input Report

Simple Basin: BASIN 1

Scenario:	Scenario1
Node:	BASIN 1
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	10.0000 min
Max Allowable Q:	999.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH256
Peaking Factor:	256.0
Area:	0.8000 ac
Curve Number:	84.7
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Node: BASIN 1

Scenario:	Scenario1
Type:	Stage/Volume
Base Flow:	0.00 cfs
Initial Stage:	22.50 ft
Warning Stage:	999.00 ft

Stage [ft]	Volume [ac-ft]	Volume [ft3]
22.50	0.00	0
22.90	0.00	0
23.30	0.00	0
23.70	0.02	697
24.10	0.05	2222
24.50	0.09	3964
24.90	0.14	5924
25.30	0.19	8059
25.70	0.25	11064
26.10	0.37	16291
26.50	0.55	23784
26.90	0.77	33498
27.30	1.04	45172
27.70	1.31	57151
28.10	1.59	69086
28.50	1.86	81065

Comment:

Node: PERC NODE

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 22.50 ft
Warning Stage: 999.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	22.50
0	0	0	999.0000	22.50

Comment:

Node: TAILWATER

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 22.50 ft
Warning Stage: 999.00 ft
Boundary Stage: TAILWATER

Comment:

Drop Structure Link: DS-BASIN 1-TAILWATER

Scenario: Scenario1
From Node: BASIN 1
To Node: TAILWATER
Link Count: 1
Flow Direction: Both
Solution: Combine
Increments: 0
Pipe Count: 1
Damping: 0.0100 ft
Length: 50.00 ft
FHWA Code: 1
Entr Loss Coef: 0.20
Exit Loss Coef: 0.50
Bend Loss Coef: 0.00
Bend Location: 0.00 dec
Energy Switch: Energy

Upstream Pipe		Downstream Pipe	
Invert:	10.00 ft	Invert:	10.00 ft
Manning's N:	0.0120	Manning's N:	0.0120
Geometry: Circular		Geometry: Circular	
Max Depth:	1.50 ft	Max Depth:	1.50 ft
Bottom Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000
Top Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000

Pipe Comment:

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0100 ft	Ref Node:	
Weir Type:	Sharp Crested Vertical	Top Clip	
Geometry Type:	Circular	Default:	0.00 ft
Invert:	24.43 ft	Op Table:	
Control Elevation:	24.43 ft	Ref Node:	
Max Depth:	0.25 ft	Discharge Coefficients	
		Weir Default:	3.200
		Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component			
Weir:	2	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Horizontal	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	26.02 ft	Op Table:	
Control Elevation:	26.02 ft	Ref Node:	
Max Depth:	4.50 ft	Discharge Coefficients	
Max Width:	3.00 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Drop Structure Comment:

Percolation Link: PERC LINK			
Scenario:	Scenario1	Surface Area Option:	Vary Based on Stage/Area Table
From Node:	BASIN 1		
To Node:	PERC NODE	Vertical Flow Termination:	Horizontal Flow Algorithm
Link Count:	1	Perimeter 1:	842.00 ft
Flow Direction:	None	Perimeter 2:	1234.00 ft
Aquifer Base Elevation:	12.50 ft	Perimeter 3:	4357.00 ft
Water Table Elevation:	22.50 ft	Distance P1 to P2:	50.00 ft
Annual Recharge Rate:	0 ipy	Distance P2 to P3:	450.00 ft
Horizontal Conductivity:	9.720 fpd	# of Cells P1 to P2:	5
Vertical Conductivity:	4.860 fpd	# of Cells P2 to P3:	45
Fillable Porosity:	0.200		
Layer Thickness:	1.00 ft		

Comment:

Simulation: 72 H Drawdown

Scenario: Scenario1
Run Date/Time: 12/22/2025 1:40:55 PM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set: No Rainfall
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec		
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	No Rainfall
Max dZ:	1.0000 ft		
Link Optimizer Tol:	0.0001 ft		
Edge Length Option:	Automatic		
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 100Y-72H

Scenario: Scenario1
Run Date/Time: 12/22/2025 11:47:22 AM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	30.0000
Max Calculation Time:		60.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000
0	0	0	60.0000	15.0000
0	0	0	80.0000	60.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set: 25-year
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft
Edge Length Option: Automatic

IA Recovery Time: 24.0000 hr
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~SFWMD-72
Rainfall Amount: 14.00 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: SFWMD 10Y-24H

Scenario: Scenario1
Run Date/Time: 12/22/2025 1:43:15 PM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000

End Time:	0	0	0	30.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set: 10-year
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft
Edge Length Option: Automatic

IA Recovery Time: 24.0000 hr
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FLMOD
Rainfall Amount: 6.50 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: SFWMD 25Y-72H

Scenario: Scenario1
Run Date/Time: 12/22/2025 11:41:02 AM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000
0	0	0	60.0000	15.0000
0	0	0	80.0000	60.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set: 25-year
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec		
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	Global
Max dZ:	1.0000 ft		
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~SFWMD-72
		Rainfall Amount:	11.00 in
Edge Length Option:	Automatic	Storm Duration:	72.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 25Y-72H R2

Scenario: Scenario1
Run Date/Time: 12/22/2025 11:42:23 AM
Program Version: ICPR4 4.07.08

General

Run Mode: Hot Start
Hot Start Simulation: SFWMD 25Y-72H

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000
Hot Start Time:	0	0	0	359.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000
0	0	0	60.0000	15.0000
0	0	0	80.0000	60.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set: 25-year
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft
Edge Length Option: Automatic

IA Recovery Time: 24.0000 hr
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~SFWMD-72
Rainfall Amount: 11.50 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Boundary Stage: TAILWATER

Boundary Stage Set: 10-year

Year	Month	Day	Hour [hr]	Stage [ft]
0	0	0	0.0000	22.50
0	0	0	18.0000	23.90
0	0	0	30.0000	22.50

Comment:

Boundary Stage: TAILWATER

Boundary Stage Set: 25-year

Year	Month	Day	Hour [hr]	Stage [ft]
0	0	0	0.0000	22.50
0	0	0	60.0000	23.90
0	0	0	360.0000	22.50

Comment:

Boundary Stage: TAILWATER

Boundary Stage Set: No Rainfall

Year	Month	Day	Hour [hr]	Stage [ft]
0	0	0	0.0000	22.50
0	0	0	999.0000	22.50

Comment:

ICPR Node Max Report

Scenario	Sim	Node Name	Maximum Stage [ft]	Maximum Total Outflow Rate [cfs]
Scenario1	SFWMD 100Y-72H	BASIN 1	26.93	0.00
Scenario1	SFWMD 10Y-24H	BASIN 1	25.37	0.21
Scenario1	SFWMD 25Y-72H	BASIN 1	26.02	0.29
Scenario1	SFWMD 25Y-72H R2	BASIN 1	26.04	0.47

ICPR 25-Year, 72 Hour Node Time Series

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	0.0000	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	0.2511	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	0.5050	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	0.7527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	1.0027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	1.2527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	1.5027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	1.7527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	2.0027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	2.2527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	2.5027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	2.7527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	3.0027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	3.2527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	3.5027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	3.7527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	4.0027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	4.2527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	4.5027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	4.7527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	5.0027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	5.2527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	5.5027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	5.7527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	6.0027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	6.2527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	6.5027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	6.7527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	7.0027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	7.2527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	7.5027	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	7.7527	22.50
Scenario1	SFWMD 25Y-72H	BASIN 1	8.0027	22.51
Scenario1	SFWMD 25Y-72H	BASIN 1	8.2527	22.52
Scenario1	SFWMD 25Y-72H	BASIN 1	8.5027	22.53
Scenario1	SFWMD 25Y-72H	BASIN 1	8.7527	22.55
Scenario1	SFWMD 25Y-72H	BASIN 1	9.0027	22.58
Scenario1	SFWMD 25Y-72H	BASIN 1	9.2544	22.61
Scenario1	SFWMD 25Y-72H	BASIN 1	9.5028	22.64
Scenario1	SFWMD 25Y-72H	BASIN 1	9.7516	22.67
Scenario1	SFWMD 25Y-72H	BASIN 1	10.0009	22.72
Scenario1	SFWMD 25Y-72H	BASIN 1	10.2521	22.76

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	10.5031	22.81
Scenario1	SFWMD 25Y-72H	BASIN 1	10.7533	22.86
Scenario1	SFWMD 25Y-72H	BASIN 1	11.0002	22.92
Scenario1	SFWMD 25Y-72H	BASIN 1	11.2550	22.97
Scenario1	SFWMD 25Y-72H	BASIN 1	11.5018	23.01
Scenario1	SFWMD 25Y-72H	BASIN 1	11.7518	23.03
Scenario1	SFWMD 25Y-72H	BASIN 1	12.0018	23.05
Scenario1	SFWMD 25Y-72H	BASIN 1	12.2518	23.08
Scenario1	SFWMD 25Y-72H	BASIN 1	12.5018	23.09
Scenario1	SFWMD 25Y-72H	BASIN 1	12.7518	23.11
Scenario1	SFWMD 25Y-72H	BASIN 1	13.0018	23.13
Scenario1	SFWMD 25Y-72H	BASIN 1	13.2518	23.15
Scenario1	SFWMD 25Y-72H	BASIN 1	13.5018	23.16
Scenario1	SFWMD 25Y-72H	BASIN 1	13.7518	23.18
Scenario1	SFWMD 25Y-72H	BASIN 1	14.0018	23.20
Scenario1	SFWMD 25Y-72H	BASIN 1	14.2518	23.21
Scenario1	SFWMD 25Y-72H	BASIN 1	14.5018	23.23
Scenario1	SFWMD 25Y-72H	BASIN 1	14.7518	23.24
Scenario1	SFWMD 25Y-72H	BASIN 1	15.0018	23.25
Scenario1	SFWMD 25Y-72H	BASIN 1	15.2518	23.27
Scenario1	SFWMD 25Y-72H	BASIN 1	15.5018	23.28
Scenario1	SFWMD 25Y-72H	BASIN 1	15.7518	23.30
Scenario1	SFWMD 25Y-72H	BASIN 1	16.0018	23.31
Scenario1	SFWMD 25Y-72H	BASIN 1	16.2518	23.32
Scenario1	SFWMD 25Y-72H	BASIN 1	16.5018	23.34
Scenario1	SFWMD 25Y-72H	BASIN 1	16.7518	23.35
Scenario1	SFWMD 25Y-72H	BASIN 1	17.0018	23.36
Scenario1	SFWMD 25Y-72H	BASIN 1	17.2518	23.37
Scenario1	SFWMD 25Y-72H	BASIN 1	17.5018	23.38
Scenario1	SFWMD 25Y-72H	BASIN 1	17.7518	23.39
Scenario1	SFWMD 25Y-72H	BASIN 1	18.0018	23.40
Scenario1	SFWMD 25Y-72H	BASIN 1	18.2518	23.41
Scenario1	SFWMD 25Y-72H	BASIN 1	18.5018	23.42
Scenario1	SFWMD 25Y-72H	BASIN 1	18.7518	23.43
Scenario1	SFWMD 25Y-72H	BASIN 1	19.0018	23.44
Scenario1	SFWMD 25Y-72H	BASIN 1	19.2518	23.45
Scenario1	SFWMD 25Y-72H	BASIN 1	19.5018	23.46
Scenario1	SFWMD 25Y-72H	BASIN 1	19.7518	23.47
Scenario1	SFWMD 25Y-72H	BASIN 1	20.0018	23.48
Scenario1	SFWMD 25Y-72H	BASIN 1	20.2518	23.49
Scenario1	SFWMD 25Y-72H	BASIN 1	20.5018	23.50
Scenario1	SFWMD 25Y-72H	BASIN 1	20.7518	23.51

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	21.0018	23.52
Scenario1	SFWMD 25Y-72H	BASIN 1	21.2518	23.52
Scenario1	SFWMD 25Y-72H	BASIN 1	21.5018	23.53
Scenario1	SFWMD 25Y-72H	BASIN 1	21.7518	23.54
Scenario1	SFWMD 25Y-72H	BASIN 1	22.0018	23.55
Scenario1	SFWMD 25Y-72H	BASIN 1	22.2518	23.56
Scenario1	SFWMD 25Y-72H	BASIN 1	22.5018	23.57
Scenario1	SFWMD 25Y-72H	BASIN 1	22.7518	23.57
Scenario1	SFWMD 25Y-72H	BASIN 1	23.0018	23.58
Scenario1	SFWMD 25Y-72H	BASIN 1	23.2518	23.59
Scenario1	SFWMD 25Y-72H	BASIN 1	23.5018	23.60
Scenario1	SFWMD 25Y-72H	BASIN 1	23.7518	23.61
Scenario1	SFWMD 25Y-72H	BASIN 1	24.0018	23.61
Scenario1	SFWMD 25Y-72H	BASIN 1	24.2518	23.62
Scenario1	SFWMD 25Y-72H	BASIN 1	24.5018	23.63
Scenario1	SFWMD 25Y-72H	BASIN 1	24.7518	23.65
Scenario1	SFWMD 25Y-72H	BASIN 1	25.0018	23.66
Scenario1	SFWMD 25Y-72H	BASIN 1	25.2518	23.67
Scenario1	SFWMD 25Y-72H	BASIN 1	25.5018	23.68
Scenario1	SFWMD 25Y-72H	BASIN 1	25.7518	23.69
Scenario1	SFWMD 25Y-72H	BASIN 1	26.0018	23.70
Scenario1	SFWMD 25Y-72H	BASIN 1	26.2518	23.71
Scenario1	SFWMD 25Y-72H	BASIN 1	26.5018	23.72
Scenario1	SFWMD 25Y-72H	BASIN 1	26.7518	23.73
Scenario1	SFWMD 25Y-72H	BASIN 1	27.0018	23.74
Scenario1	SFWMD 25Y-72H	BASIN 1	27.2518	23.75
Scenario1	SFWMD 25Y-72H	BASIN 1	27.5018	23.76
Scenario1	SFWMD 25Y-72H	BASIN 1	27.7518	23.77
Scenario1	SFWMD 25Y-72H	BASIN 1	28.0018	23.78
Scenario1	SFWMD 25Y-72H	BASIN 1	28.2518	23.79
Scenario1	SFWMD 25Y-72H	BASIN 1	28.5018	23.80
Scenario1	SFWMD 25Y-72H	BASIN 1	28.7518	23.81
Scenario1	SFWMD 25Y-72H	BASIN 1	29.0018	23.82
Scenario1	SFWMD 25Y-72H	BASIN 1	29.2518	23.84
Scenario1	SFWMD 25Y-72H	BASIN 1	29.5018	23.85
Scenario1	SFWMD 25Y-72H	BASIN 1	29.7518	23.86
Scenario1	SFWMD 25Y-72H	BASIN 1	30.0018	23.87
Scenario1	SFWMD 25Y-72H	BASIN 1	30.2518	23.88
Scenario1	SFWMD 25Y-72H	BASIN 1	30.5018	23.89
Scenario1	SFWMD 25Y-72H	BASIN 1	30.7518	23.90
Scenario1	SFWMD 25Y-72H	BASIN 1	31.0018	23.91
Scenario1	SFWMD 25Y-72H	BASIN 1	31.2518	23.92

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	31.5018	23.93
Scenario1	SFWMD 25Y-72H	BASIN 1	31.7518	23.94
Scenario1	SFWMD 25Y-72H	BASIN 1	32.0018	23.95
Scenario1	SFWMD 25Y-72H	BASIN 1	32.2518	23.96
Scenario1	SFWMD 25Y-72H	BASIN 1	32.5018	23.97
Scenario1	SFWMD 25Y-72H	BASIN 1	32.7518	23.97
Scenario1	SFWMD 25Y-72H	BASIN 1	33.0018	23.98
Scenario1	SFWMD 25Y-72H	BASIN 1	33.2518	23.99
Scenario1	SFWMD 25Y-72H	BASIN 1	33.5018	24.00
Scenario1	SFWMD 25Y-72H	BASIN 1	33.7518	24.01
Scenario1	SFWMD 25Y-72H	BASIN 1	34.0018	24.02
Scenario1	SFWMD 25Y-72H	BASIN 1	34.2518	24.03
Scenario1	SFWMD 25Y-72H	BASIN 1	34.5018	24.04
Scenario1	SFWMD 25Y-72H	BASIN 1	34.7518	24.05
Scenario1	SFWMD 25Y-72H	BASIN 1	35.0018	24.06
Scenario1	SFWMD 25Y-72H	BASIN 1	35.2518	24.07
Scenario1	SFWMD 25Y-72H	BASIN 1	35.5018	24.08
Scenario1	SFWMD 25Y-72H	BASIN 1	35.7518	24.09
Scenario1	SFWMD 25Y-72H	BASIN 1	36.0018	24.10
Scenario1	SFWMD 25Y-72H	BASIN 1	36.2518	24.11
Scenario1	SFWMD 25Y-72H	BASIN 1	36.5018	24.12
Scenario1	SFWMD 25Y-72H	BASIN 1	36.7518	24.13
Scenario1	SFWMD 25Y-72H	BASIN 1	37.0018	24.14
Scenario1	SFWMD 25Y-72H	BASIN 1	37.2518	24.14
Scenario1	SFWMD 25Y-72H	BASIN 1	37.5018	24.15
Scenario1	SFWMD 25Y-72H	BASIN 1	37.7518	24.16
Scenario1	SFWMD 25Y-72H	BASIN 1	38.0018	24.17
Scenario1	SFWMD 25Y-72H	BASIN 1	38.2518	24.18
Scenario1	SFWMD 25Y-72H	BASIN 1	38.5018	24.19
Scenario1	SFWMD 25Y-72H	BASIN 1	38.7518	24.20
Scenario1	SFWMD 25Y-72H	BASIN 1	39.0018	24.21
Scenario1	SFWMD 25Y-72H	BASIN 1	39.2518	24.22
Scenario1	SFWMD 25Y-72H	BASIN 1	39.5018	24.23
Scenario1	SFWMD 25Y-72H	BASIN 1	39.7518	24.24
Scenario1	SFWMD 25Y-72H	BASIN 1	40.0018	24.25
Scenario1	SFWMD 25Y-72H	BASIN 1	40.2518	24.26
Scenario1	SFWMD 25Y-72H	BASIN 1	40.5018	24.27
Scenario1	SFWMD 25Y-72H	BASIN 1	40.7518	24.28
Scenario1	SFWMD 25Y-72H	BASIN 1	41.0018	24.29
Scenario1	SFWMD 25Y-72H	BASIN 1	41.2518	24.30
Scenario1	SFWMD 25Y-72H	BASIN 1	41.5018	24.30
Scenario1	SFWMD 25Y-72H	BASIN 1	41.7518	24.31

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	42.0018	24.32
Scenario1	SFWMD 25Y-72H	BASIN 1	42.2518	24.33
Scenario1	SFWMD 25Y-72H	BASIN 1	42.5018	24.34
Scenario1	SFWMD 25Y-72H	BASIN 1	42.7518	24.35
Scenario1	SFWMD 25Y-72H	BASIN 1	43.0018	24.36
Scenario1	SFWMD 25Y-72H	BASIN 1	43.2518	24.37
Scenario1	SFWMD 25Y-72H	BASIN 1	43.5018	24.38
Scenario1	SFWMD 25Y-72H	BASIN 1	43.7518	24.39
Scenario1	SFWMD 25Y-72H	BASIN 1	44.0018	24.40
Scenario1	SFWMD 25Y-72H	BASIN 1	44.2518	24.41
Scenario1	SFWMD 25Y-72H	BASIN 1	44.5018	24.42
Scenario1	SFWMD 25Y-72H	BASIN 1	44.7518	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	45.0018	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	45.2518	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	45.5018	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	45.7518	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	46.0018	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	46.2518	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	46.5018	24.49
Scenario1	SFWMD 25Y-72H	BASIN 1	46.7518	24.50
Scenario1	SFWMD 25Y-72H	BASIN 1	47.0018	24.50
Scenario1	SFWMD 25Y-72H	BASIN 1	47.2518	24.51
Scenario1	SFWMD 25Y-72H	BASIN 1	47.5018	24.52
Scenario1	SFWMD 25Y-72H	BASIN 1	47.7518	24.53
Scenario1	SFWMD 25Y-72H	BASIN 1	48.0018	24.53
Scenario1	SFWMD 25Y-72H	BASIN 1	48.2518	24.54
Scenario1	SFWMD 25Y-72H	BASIN 1	48.5018	24.54
Scenario1	SFWMD 25Y-72H	BASIN 1	48.7518	24.55
Scenario1	SFWMD 25Y-72H	BASIN 1	49.0018	24.56
Scenario1	SFWMD 25Y-72H	BASIN 1	49.2518	24.56
Scenario1	SFWMD 25Y-72H	BASIN 1	49.5018	24.57
Scenario1	SFWMD 25Y-72H	BASIN 1	49.7518	24.57
Scenario1	SFWMD 25Y-72H	BASIN 1	50.0018	24.57
Scenario1	SFWMD 25Y-72H	BASIN 1	50.2518	24.58
Scenario1	SFWMD 25Y-72H	BASIN 1	50.5018	24.58
Scenario1	SFWMD 25Y-72H	BASIN 1	50.7518	24.59
Scenario1	SFWMD 25Y-72H	BASIN 1	51.0018	24.59
Scenario1	SFWMD 25Y-72H	BASIN 1	51.2518	24.60
Scenario1	SFWMD 25Y-72H	BASIN 1	51.5018	24.60
Scenario1	SFWMD 25Y-72H	BASIN 1	51.7518	24.61
Scenario1	SFWMD 25Y-72H	BASIN 1	52.0018	24.61
Scenario1	SFWMD 25Y-72H	BASIN 1	52.2518	24.62

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	52.5018	24.63
Scenario1	SFWMD 25Y-72H	BASIN 1	52.7518	24.63
Scenario1	SFWMD 25Y-72H	BASIN 1	53.0018	24.64
Scenario1	SFWMD 25Y-72H	BASIN 1	53.2518	24.65
Scenario1	SFWMD 25Y-72H	BASIN 1	53.5018	24.66
Scenario1	SFWMD 25Y-72H	BASIN 1	53.7518	24.66
Scenario1	SFWMD 25Y-72H	BASIN 1	54.0018	24.67
Scenario1	SFWMD 25Y-72H	BASIN 1	54.2518	24.68
Scenario1	SFWMD 25Y-72H	BASIN 1	54.5018	24.69
Scenario1	SFWMD 25Y-72H	BASIN 1	54.7518	24.70
Scenario1	SFWMD 25Y-72H	BASIN 1	55.0018	24.71
Scenario1	SFWMD 25Y-72H	BASIN 1	55.2518	24.72
Scenario1	SFWMD 25Y-72H	BASIN 1	55.5018	24.74
Scenario1	SFWMD 25Y-72H	BASIN 1	55.7518	24.75
Scenario1	SFWMD 25Y-72H	BASIN 1	56.0018	24.76
Scenario1	SFWMD 25Y-72H	BASIN 1	56.2518	24.77
Scenario1	SFWMD 25Y-72H	BASIN 1	56.5018	24.79
Scenario1	SFWMD 25Y-72H	BASIN 1	56.7518	24.80
Scenario1	SFWMD 25Y-72H	BASIN 1	57.0018	24.81
Scenario1	SFWMD 25Y-72H	BASIN 1	57.2518	24.83
Scenario1	SFWMD 25Y-72H	BASIN 1	57.5018	24.85
Scenario1	SFWMD 25Y-72H	BASIN 1	57.7518	24.87
Scenario1	SFWMD 25Y-72H	BASIN 1	58.0018	24.89
Scenario1	SFWMD 25Y-72H	BASIN 1	58.2518	24.91
Scenario1	SFWMD 25Y-72H	BASIN 1	58.5018	24.93
Scenario1	SFWMD 25Y-72H	BASIN 1	58.7501	24.96
Scenario1	SFWMD 25Y-72H	BASIN 1	59.0032	25.00
Scenario1	SFWMD 25Y-72H	BASIN 1	59.2523	25.04
Scenario1	SFWMD 25Y-72H	BASIN 1	59.5032	25.10
Scenario1	SFWMD 25Y-72H	BASIN 1	59.7502	25.29
Scenario1	SFWMD 25Y-72H	BASIN 1	60.0005	25.62
Scenario1	SFWMD 25Y-72H	BASIN 1	60.2505	25.84
Scenario1	SFWMD 25Y-72H	BASIN 1	60.5008	25.93
Scenario1	SFWMD 25Y-72H	BASIN 1	60.7540	25.98
Scenario1	SFWMD 25Y-72H	BASIN 1	61.0020	26.00
Scenario1	SFWMD 25Y-72H	BASIN 1	61.2520	26.01
Scenario1	SFWMD 25Y-72H	BASIN 1	61.5020	26.02
Scenario1	SFWMD 25Y-72H	BASIN 1	61.7520	26.02
Scenario1	SFWMD 25Y-72H	BASIN 1	62.0020	26.02
Scenario1	SFWMD 25Y-72H	BASIN 1	62.2520	26.02
Scenario1	SFWMD 25Y-72H	BASIN 1	62.5020	26.02
Scenario1	SFWMD 25Y-72H	BASIN 1	62.7520	26.02

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	63.0020	26.01
Scenario1	SFWMD 25Y-72H	BASIN 1	63.2520	26.01
Scenario1	SFWMD 25Y-72H	BASIN 1	63.5020	26.00
Scenario1	SFWMD 25Y-72H	BASIN 1	63.7520	25.99
Scenario1	SFWMD 25Y-72H	BASIN 1	64.0020	25.99
Scenario1	SFWMD 25Y-72H	BASIN 1	64.2520	25.98
Scenario1	SFWMD 25Y-72H	BASIN 1	64.5020	25.97
Scenario1	SFWMD 25Y-72H	BASIN 1	64.7520	25.96
Scenario1	SFWMD 25Y-72H	BASIN 1	65.0020	25.95
Scenario1	SFWMD 25Y-72H	BASIN 1	65.2520	25.94
Scenario1	SFWMD 25Y-72H	BASIN 1	65.5020	25.93
Scenario1	SFWMD 25Y-72H	BASIN 1	65.7520	25.92
Scenario1	SFWMD 25Y-72H	BASIN 1	66.0020	25.91
Scenario1	SFWMD 25Y-72H	BASIN 1	66.2520	25.90
Scenario1	SFWMD 25Y-72H	BASIN 1	66.5020	25.89
Scenario1	SFWMD 25Y-72H	BASIN 1	66.7520	25.88
Scenario1	SFWMD 25Y-72H	BASIN 1	67.0020	25.87
Scenario1	SFWMD 25Y-72H	BASIN 1	67.2520	25.86
Scenario1	SFWMD 25Y-72H	BASIN 1	67.5020	25.85
Scenario1	SFWMD 25Y-72H	BASIN 1	67.7520	25.84
Scenario1	SFWMD 25Y-72H	BASIN 1	68.0020	25.83
Scenario1	SFWMD 25Y-72H	BASIN 1	68.2520	25.82
Scenario1	SFWMD 25Y-72H	BASIN 1	68.5020	25.80
Scenario1	SFWMD 25Y-72H	BASIN 1	68.7520	25.79
Scenario1	SFWMD 25Y-72H	BASIN 1	69.0020	25.78
Scenario1	SFWMD 25Y-72H	BASIN 1	69.2520	25.76
Scenario1	SFWMD 25Y-72H	BASIN 1	69.5020	25.75
Scenario1	SFWMD 25Y-72H	BASIN 1	69.7520	25.73
Scenario1	SFWMD 25Y-72H	BASIN 1	70.0020	25.72
Scenario1	SFWMD 25Y-72H	BASIN 1	70.2520	25.70
Scenario1	SFWMD 25Y-72H	BASIN 1	70.5020	25.69
Scenario1	SFWMD 25Y-72H	BASIN 1	70.7520	25.68
Scenario1	SFWMD 25Y-72H	BASIN 1	71.0020	25.66
Scenario1	SFWMD 25Y-72H	BASIN 1	71.2520	25.65
Scenario1	SFWMD 25Y-72H	BASIN 1	71.5020	25.63
Scenario1	SFWMD 25Y-72H	BASIN 1	71.7520	25.62
Scenario1	SFWMD 25Y-72H	BASIN 1	72.0020	25.60
Scenario1	SFWMD 25Y-72H	BASIN 1	72.2520	25.59
Scenario1	SFWMD 25Y-72H	BASIN 1	72.5020	25.57
Scenario1	SFWMD 25Y-72H	BASIN 1	72.7520	25.54
Scenario1	SFWMD 25Y-72H	BASIN 1	73.0020	25.52
Scenario1	SFWMD 25Y-72H	BASIN 1	73.2520	25.50

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	73.5020	25.47
Scenario1	SFWMD 25Y-72H	BASIN 1	73.7520	25.45
Scenario1	SFWMD 25Y-72H	BASIN 1	74.0020	25.43
Scenario1	SFWMD 25Y-72H	BASIN 1	74.2520	25.40
Scenario1	SFWMD 25Y-72H	BASIN 1	74.5020	25.38
Scenario1	SFWMD 25Y-72H	BASIN 1	74.7520	25.35
Scenario1	SFWMD 25Y-72H	BASIN 1	75.0020	25.32
Scenario1	SFWMD 25Y-72H	BASIN 1	75.2520	25.30
Scenario1	SFWMD 25Y-72H	BASIN 1	75.5020	25.27
Scenario1	SFWMD 25Y-72H	BASIN 1	75.7520	25.24
Scenario1	SFWMD 25Y-72H	BASIN 1	76.0020	25.22
Scenario1	SFWMD 25Y-72H	BASIN 1	76.2520	25.19
Scenario1	SFWMD 25Y-72H	BASIN 1	76.5020	25.16
Scenario1	SFWMD 25Y-72H	BASIN 1	76.7520	25.14
Scenario1	SFWMD 25Y-72H	BASIN 1	77.0020	25.11
Scenario1	SFWMD 25Y-72H	BASIN 1	77.2520	25.08
Scenario1	SFWMD 25Y-72H	BASIN 1	77.5020	25.06
Scenario1	SFWMD 25Y-72H	BASIN 1	77.7520	25.03
Scenario1	SFWMD 25Y-72H	BASIN 1	78.0020	25.01
Scenario1	SFWMD 25Y-72H	BASIN 1	78.2520	24.98
Scenario1	SFWMD 25Y-72H	BASIN 1	78.5020	24.96
Scenario1	SFWMD 25Y-72H	BASIN 1	78.7520	24.93
Scenario1	SFWMD 25Y-72H	BASIN 1	79.0020	24.91
Scenario1	SFWMD 25Y-72H	BASIN 1	79.2520	24.88
Scenario1	SFWMD 25Y-72H	BASIN 1	79.5020	24.86
Scenario1	SFWMD 25Y-72H	BASIN 1	79.7520	24.84
Scenario1	SFWMD 25Y-72H	BASIN 1	80.0020	24.81
Scenario1	SFWMD 25Y-72H	BASIN 1	80.2520	24.79
Scenario1	SFWMD 25Y-72H	BASIN 1	80.5020	24.77
Scenario1	SFWMD 25Y-72H	BASIN 1	80.7520	24.75
Scenario1	SFWMD 25Y-72H	BASIN 1	81.0020	24.73
Scenario1	SFWMD 25Y-72H	BASIN 1	81.2520	24.72
Scenario1	SFWMD 25Y-72H	BASIN 1	81.5020	24.70
Scenario1	SFWMD 25Y-72H	BASIN 1	81.7520	24.68
Scenario1	SFWMD 25Y-72H	BASIN 1	82.0020	24.67
Scenario1	SFWMD 25Y-72H	BASIN 1	82.2520	24.66
Scenario1	SFWMD 25Y-72H	BASIN 1	82.5020	24.65
Scenario1	SFWMD 25Y-72H	BASIN 1	82.7520	24.63
Scenario1	SFWMD 25Y-72H	BASIN 1	83.0020	24.62
Scenario1	SFWMD 25Y-72H	BASIN 1	83.2520	24.61
Scenario1	SFWMD 25Y-72H	BASIN 1	83.5020	24.60
Scenario1	SFWMD 25Y-72H	BASIN 1	83.7520	24.60

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	84.0020	24.59
Scenario1	SFWMD 25Y-72H	BASIN 1	84.2520	24.58
Scenario1	SFWMD 25Y-72H	BASIN 1	84.5020	24.58
Scenario1	SFWMD 25Y-72H	BASIN 1	84.7520	24.57
Scenario1	SFWMD 25Y-72H	BASIN 1	85.0020	24.56
Scenario1	SFWMD 25Y-72H	BASIN 1	85.2520	24.56
Scenario1	SFWMD 25Y-72H	BASIN 1	85.5020	24.55
Scenario1	SFWMD 25Y-72H	BASIN 1	85.7520	24.55
Scenario1	SFWMD 25Y-72H	BASIN 1	86.0020	24.54
Scenario1	SFWMD 25Y-72H	BASIN 1	86.2520	24.54
Scenario1	SFWMD 25Y-72H	BASIN 1	86.5020	24.54
Scenario1	SFWMD 25Y-72H	BASIN 1	86.7520	24.53
Scenario1	SFWMD 25Y-72H	BASIN 1	87.0020	24.53
Scenario1	SFWMD 25Y-72H	BASIN 1	87.2520	24.53
Scenario1	SFWMD 25Y-72H	BASIN 1	87.5020	24.52
Scenario1	SFWMD 25Y-72H	BASIN 1	87.7520	24.52
Scenario1	SFWMD 25Y-72H	BASIN 1	88.0020	24.52
Scenario1	SFWMD 25Y-72H	BASIN 1	88.2520	24.52
Scenario1	SFWMD 25Y-72H	BASIN 1	88.5020	24.51
Scenario1	SFWMD 25Y-72H	BASIN 1	88.7520	24.51
Scenario1	SFWMD 25Y-72H	BASIN 1	89.0020	24.51
Scenario1	SFWMD 25Y-72H	BASIN 1	89.2520	24.51
Scenario1	SFWMD 25Y-72H	BASIN 1	89.5020	24.51
Scenario1	SFWMD 25Y-72H	BASIN 1	89.7520	24.50
Scenario1	SFWMD 25Y-72H	BASIN 1	90.0020	24.50
Scenario1	SFWMD 25Y-72H	BASIN 1	90.2520	24.50
Scenario1	SFWMD 25Y-72H	BASIN 1	90.5020	24.50
Scenario1	SFWMD 25Y-72H	BASIN 1	90.7520	24.50
Scenario1	SFWMD 25Y-72H	BASIN 1	91.0020	24.50
Scenario1	SFWMD 25Y-72H	BASIN 1	91.2520	24.49
Scenario1	SFWMD 25Y-72H	BASIN 1	91.5020	24.49
Scenario1	SFWMD 25Y-72H	BASIN 1	91.7520	24.49
Scenario1	SFWMD 25Y-72H	BASIN 1	92.0020	24.49
Scenario1	SFWMD 25Y-72H	BASIN 1	92.2520	24.49
Scenario1	SFWMD 25Y-72H	BASIN 1	92.5020	24.49
Scenario1	SFWMD 25Y-72H	BASIN 1	92.7520	24.49
Scenario1	SFWMD 25Y-72H	BASIN 1	93.0020	24.49
Scenario1	SFWMD 25Y-72H	BASIN 1	93.2520	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	93.5020	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	93.7520	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	94.0020	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	94.2520	24.48

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	94.5020	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	94.7520	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	95.0020	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	95.2520	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	95.5020	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	95.7520	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	96.0020	24.48
Scenario1	SFWMD 25Y-72H	BASIN 1	96.2520	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	96.5020	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	96.7520	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	97.0020	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	97.2520	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	97.5020	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	97.7520	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	98.0020	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	98.2520	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	98.5020	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	98.7520	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	99.0020	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	99.2520	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	99.5020	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	99.7520	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	100.0020	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	100.2520	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	100.5020	24.47
Scenario1	SFWMD 25Y-72H	BASIN 1	100.7520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	101.0020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	101.2520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	101.5020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	101.7520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	102.0020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	102.2520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	102.5020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	102.7520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	103.0020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	103.2520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	103.5020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	103.7520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	104.0020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	104.2520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	104.5020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	104.7520	24.46

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	105.0020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	105.2520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	105.5020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	105.7520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	106.0020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	106.2520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	106.5020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	106.7520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	107.0020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	107.2520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	107.5020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	107.7520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	108.0020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	108.2520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	108.5020	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	108.7520	24.46
Scenario1	SFWMD 25Y-72H	BASIN 1	109.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	109.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	109.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	109.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	110.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	110.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	110.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	110.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	111.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	111.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	111.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	111.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	112.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	112.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	112.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	112.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	113.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	113.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	113.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	113.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	114.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	114.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	114.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	114.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	115.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	115.2520	24.45

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	115.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	115.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	116.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	116.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	116.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	116.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	117.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	117.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	117.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	117.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	118.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	118.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	118.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	118.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	119.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	119.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	119.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	119.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	120.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	120.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	120.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	120.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	121.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	121.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	121.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	121.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	122.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	122.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	122.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	122.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	123.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	123.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	123.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	123.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	124.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	124.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	124.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	124.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	125.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	125.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	125.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	125.7520	24.45

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	126.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	126.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	126.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	126.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	127.0020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	127.2520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	127.5020	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	127.7520	24.45
Scenario1	SFWMD 25Y-72H	BASIN 1	128.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	128.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	128.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	128.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	129.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	129.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	129.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	129.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	130.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	130.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	130.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	130.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	131.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	131.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	131.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	131.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	132.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	132.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	132.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	132.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	133.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	133.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	133.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	133.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	134.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	134.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	134.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	134.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	135.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	135.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	135.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	135.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	136.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	136.2520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	136.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	136.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	137.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	137.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	137.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	137.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	138.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	138.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	138.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	138.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	139.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	139.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	139.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	139.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	140.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	140.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	140.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	140.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	141.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	141.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	141.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	141.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	142.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	142.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	142.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	142.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	143.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	143.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	143.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	143.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	144.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	144.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	144.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	144.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	145.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	145.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	145.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	145.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	146.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	146.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	146.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	146.7520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	147.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	147.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	147.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	147.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	148.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	148.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	148.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	148.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	149.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	149.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	149.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	149.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	150.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	150.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	150.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	150.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	151.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	151.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	151.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	151.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	152.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	152.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	152.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	152.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	153.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	153.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	153.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	153.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	154.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	154.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	154.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	154.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	155.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	155.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	155.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	155.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	156.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	156.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	156.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	156.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	157.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	157.2520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	157.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	157.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	158.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	158.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	158.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	158.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	159.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	159.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	159.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	159.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	160.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	160.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	160.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	160.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	161.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	161.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	161.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	161.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	162.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	162.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	162.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	162.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	163.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	163.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	163.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	163.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	164.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	164.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	164.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	164.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	165.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	165.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	165.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	165.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	166.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	166.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	166.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	166.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	167.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	167.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	167.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	167.7520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	168.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	168.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	168.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	168.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	169.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	169.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	169.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	169.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	170.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	170.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	170.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	170.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	171.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	171.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	171.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	171.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	172.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	172.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	172.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	172.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	173.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	173.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	173.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	173.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	174.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	174.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	174.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	174.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	175.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	175.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	175.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	175.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	176.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	176.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	176.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	176.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	177.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	177.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	177.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	177.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	178.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	178.2520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	178.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	178.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	179.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	179.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	179.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	179.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	180.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	180.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	180.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	180.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	181.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	181.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	181.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	181.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	182.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	182.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	182.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	182.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	183.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	183.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	183.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	183.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	184.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	184.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	184.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	184.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	185.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	185.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	185.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	185.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	186.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	186.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	186.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	186.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	187.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	187.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	187.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	187.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	188.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	188.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	188.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	188.7520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	189.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	189.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	189.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	189.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	190.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	190.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	190.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	190.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	191.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	191.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	191.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	191.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	192.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	192.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	192.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	192.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	193.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	193.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	193.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	193.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	194.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	194.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	194.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	194.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	195.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	195.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	195.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	195.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	196.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	196.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	196.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	196.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	197.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	197.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	197.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	197.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	198.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	198.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	198.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	198.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	199.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	199.2520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	199.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	199.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	200.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	200.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	200.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	200.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	201.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	201.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	201.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	201.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	202.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	202.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	202.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	202.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	203.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	203.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	203.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	203.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	204.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	204.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	204.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	204.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	205.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	205.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	205.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	205.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	206.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	206.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	206.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	206.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	207.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	207.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	207.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	207.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	208.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	208.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	208.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	208.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	209.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	209.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	209.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	209.7520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	210.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	210.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	210.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	210.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	211.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	211.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	211.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	211.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	212.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	212.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	212.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	212.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	213.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	213.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	213.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	213.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	214.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	214.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	214.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	214.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	215.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	215.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	215.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	215.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	216.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	216.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	216.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	216.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	217.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	217.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	217.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	217.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	218.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	218.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	218.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	218.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	219.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	219.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	219.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	219.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	220.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	220.2520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	220.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	220.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	221.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	221.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	221.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	221.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	222.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	222.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	222.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	222.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	223.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	223.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	223.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	223.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	224.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	224.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	224.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	224.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	225.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	225.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	225.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	225.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	226.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	226.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	226.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	226.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	227.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	227.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	227.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	227.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	228.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	228.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	228.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	228.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	229.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	229.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	229.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	229.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	230.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	230.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	230.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	230.7520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	231.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	231.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	231.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	231.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	232.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	232.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	232.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	232.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	233.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	233.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	233.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	233.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	234.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	234.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	234.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	234.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	235.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	235.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	235.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	235.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	236.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	236.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	236.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	236.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	237.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	237.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	237.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	237.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	238.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	238.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	238.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	238.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	239.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	239.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	239.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	239.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	240.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	240.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	240.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	240.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	241.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	241.2520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	241.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	241.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	242.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	242.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	242.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	242.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	243.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	243.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	243.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	243.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	244.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	244.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	244.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	244.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	245.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	245.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	245.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	245.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	246.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	246.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	246.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	246.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	247.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	247.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	247.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	247.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	248.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	248.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	248.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	248.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	249.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	249.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	249.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	249.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	250.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	250.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	250.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	250.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	251.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	251.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	251.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	251.7520	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	252.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	252.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	252.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	252.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	253.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	253.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	253.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	253.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	254.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	254.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	254.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	254.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	255.0020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	255.2520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	255.5020	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	255.7520	24.44
Scenario1	SFWMD 25Y-72H	BASIN 1	256.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	256.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	256.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	256.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	257.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	257.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	257.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	257.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	258.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	258.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	258.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	258.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	259.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	259.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	259.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	259.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	260.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	260.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	260.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	260.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	261.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	261.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	261.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	261.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	262.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	262.2520	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	262.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	262.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	263.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	263.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	263.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	263.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	264.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	264.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	264.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	264.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	265.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	265.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	265.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	265.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	266.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	266.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	266.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	266.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	267.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	267.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	267.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	267.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	268.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	268.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	268.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	268.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	269.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	269.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	269.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	269.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	270.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	270.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	270.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	270.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	271.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	271.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	271.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	271.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	272.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	272.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	272.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	272.7520	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	273.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	273.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	273.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	273.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	274.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	274.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	274.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	274.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	275.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	275.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	275.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	275.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	276.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	276.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	276.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	276.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	277.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	277.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	277.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	277.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	278.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	278.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	278.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	278.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	279.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	279.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	279.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	279.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	280.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	280.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	280.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	280.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	281.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	281.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	281.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	281.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	282.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	282.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	282.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	282.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	283.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	283.2520	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	283.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	283.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	284.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	284.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	284.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	284.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	285.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	285.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	285.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	285.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	286.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	286.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	286.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	286.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	287.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	287.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	287.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	287.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	288.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	288.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	288.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	288.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	289.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	289.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	289.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	289.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	290.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	290.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	290.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	290.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	291.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	291.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	291.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	291.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	292.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	292.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	292.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	292.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	293.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	293.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	293.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	293.7520	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	294.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	294.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	294.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	294.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	295.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	295.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	295.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	295.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	296.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	296.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	296.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	296.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	297.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	297.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	297.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	297.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	298.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	298.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	298.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	298.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	299.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	299.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	299.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	299.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	300.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	300.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	300.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	300.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	301.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	301.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	301.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	301.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	302.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	302.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	302.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	302.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	303.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	303.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	303.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	303.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	304.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	304.2520	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	304.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	304.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	305.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	305.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	305.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	305.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	306.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	306.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	306.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	306.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	307.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	307.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	307.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	307.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	308.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	308.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	308.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	308.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	309.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	309.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	309.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	309.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	310.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	310.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	310.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	310.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	311.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	311.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	311.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	311.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	312.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	312.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	312.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	312.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	313.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	313.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	313.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	313.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	314.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	314.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	314.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	314.7520	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	315.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	315.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	315.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	315.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	316.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	316.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	316.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	316.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	317.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	317.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	317.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	317.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	318.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	318.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	318.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	318.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	319.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	319.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	319.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	319.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	320.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	320.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	320.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	320.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	321.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	321.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	321.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	321.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	322.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	322.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	322.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	322.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	323.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	323.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	323.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	323.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	324.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	324.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	324.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	324.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	325.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	325.2520	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	325.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	325.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	326.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	326.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	326.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	326.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	327.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	327.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	327.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	327.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	328.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	328.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	328.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	328.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	329.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	329.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	329.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	329.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	330.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	330.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	330.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	330.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	331.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	331.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	331.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	331.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	332.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	332.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	332.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	332.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	333.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	333.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	333.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	333.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	334.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	334.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	334.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	334.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	335.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	335.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	335.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	335.7520	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	336.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	336.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	336.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	336.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	337.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	337.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	337.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	337.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	338.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	338.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	338.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	338.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	339.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	339.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	339.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	339.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	340.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	340.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	340.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	340.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	341.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	341.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	341.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	341.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	342.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	342.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	342.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	342.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	343.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	343.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	343.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	343.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	344.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	344.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	344.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	344.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	345.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	345.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	345.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	345.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	346.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	346.2520	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	346.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	346.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	347.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	347.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	347.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	347.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	348.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	348.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	348.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	348.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	349.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	349.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	349.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	349.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	350.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	350.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	350.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	350.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	351.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	351.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	351.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	351.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	352.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	352.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	352.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	352.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	353.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	353.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	353.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	353.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	354.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	354.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	354.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	354.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	355.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	355.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	355.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	355.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	356.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	356.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	356.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	356.7520	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H	BASIN 1	357.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	357.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	357.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	357.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	358.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	358.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	358.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	358.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	359.0020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	359.2520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	359.5020	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	359.7520	24.43
Scenario1	SFWMD 25Y-72H	BASIN 1	360.0020	24.43

ICPR 25-Year, 72 Hour (2nd Event) Node Time Series

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	0.0000	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	0.2511	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	0.5050	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	0.7527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	1.0027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	1.2527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	1.5027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	1.7527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	2.0027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	2.2527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	2.5027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	2.7527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	3.0027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	3.2527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	3.5027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	3.7527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	4.0027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	4.2527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	4.5027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	4.7527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	5.0027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	5.2527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	5.5027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	5.7527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	6.0027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	6.2527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	6.5027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	6.7527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	7.0027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	7.2527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	7.5027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	7.7527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	8.0027	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	8.2527	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	8.5027	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	8.7527	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	9.0027	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	9.2527	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	9.5027	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	9.7527	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	10.0027	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	10.2527	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	10.5027	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	10.7527	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	11.0027	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	11.2527	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	11.5027	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	11.7527	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	12.0027	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	12.2527	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	12.5027	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	12.7527	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	13.0027	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	13.2527	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	13.5027	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	13.7527	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	14.0027	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	14.2527	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	14.5027	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	14.7527	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	15.0027	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	15.2527	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	15.5027	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	15.7527	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	16.0027	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	16.2527	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	16.5027	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	16.7527	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	17.0027	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	17.2527	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	17.5027	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	17.7527	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	18.0027	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	18.2527	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	18.5027	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	18.7527	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	19.0027	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	19.2527	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	19.5027	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	19.7527	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	20.0027	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	20.2527	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	20.5027	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	20.7527	24.52

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	21.0027	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	21.2527	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	21.5027	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	21.7527	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	22.0027	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	22.2527	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	22.5027	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	22.7527	24.53
Scenario1	SFWMD 25Y-72H R2	BASIN 1	23.0027	24.53
Scenario1	SFWMD 25Y-72H R2	BASIN 1	23.2527	24.53
Scenario1	SFWMD 25Y-72H R2	BASIN 1	23.5027	24.53
Scenario1	SFWMD 25Y-72H R2	BASIN 1	23.7527	24.53
Scenario1	SFWMD 25Y-72H R2	BASIN 1	24.0027	24.53
Scenario1	SFWMD 25Y-72H R2	BASIN 1	24.2527	24.53
Scenario1	SFWMD 25Y-72H R2	BASIN 1	24.5027	24.54
Scenario1	SFWMD 25Y-72H R2	BASIN 1	24.7527	24.54
Scenario1	SFWMD 25Y-72H R2	BASIN 1	25.0027	24.54
Scenario1	SFWMD 25Y-72H R2	BASIN 1	25.2527	24.54
Scenario1	SFWMD 25Y-72H R2	BASIN 1	25.5027	24.55
Scenario1	SFWMD 25Y-72H R2	BASIN 1	25.7527	24.55
Scenario1	SFWMD 25Y-72H R2	BASIN 1	26.0027	24.55
Scenario1	SFWMD 25Y-72H R2	BASIN 1	26.2527	24.55
Scenario1	SFWMD 25Y-72H R2	BASIN 1	26.5027	24.56
Scenario1	SFWMD 25Y-72H R2	BASIN 1	26.7527	24.56
Scenario1	SFWMD 25Y-72H R2	BASIN 1	27.0027	24.56
Scenario1	SFWMD 25Y-72H R2	BASIN 1	27.2527	24.56
Scenario1	SFWMD 25Y-72H R2	BASIN 1	27.5027	24.56
Scenario1	SFWMD 25Y-72H R2	BASIN 1	27.7527	24.56
Scenario1	SFWMD 25Y-72H R2	BASIN 1	28.0027	24.57
Scenario1	SFWMD 25Y-72H R2	BASIN 1	28.2527	24.57
Scenario1	SFWMD 25Y-72H R2	BASIN 1	28.5027	24.57
Scenario1	SFWMD 25Y-72H R2	BASIN 1	28.7527	24.57
Scenario1	SFWMD 25Y-72H R2	BASIN 1	29.0027	24.57
Scenario1	SFWMD 25Y-72H R2	BASIN 1	29.2527	24.57
Scenario1	SFWMD 25Y-72H R2	BASIN 1	29.5027	24.57
Scenario1	SFWMD 25Y-72H R2	BASIN 1	29.7527	24.58
Scenario1	SFWMD 25Y-72H R2	BASIN 1	30.0027	24.58
Scenario1	SFWMD 25Y-72H R2	BASIN 1	30.2527	24.58
Scenario1	SFWMD 25Y-72H R2	BASIN 1	30.5027	24.58
Scenario1	SFWMD 25Y-72H R2	BASIN 1	30.7527	24.58
Scenario1	SFWMD 25Y-72H R2	BASIN 1	31.0027	24.58
Scenario1	SFWMD 25Y-72H R2	BASIN 1	31.2527	24.58

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	31.5027	24.58
Scenario1	SFWMD 25Y-72H R2	BASIN 1	31.7527	24.58
Scenario1	SFWMD 25Y-72H R2	BASIN 1	32.0027	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	32.2527	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	32.5027	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	32.7527	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	33.0027	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	33.2527	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	33.5027	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	33.7527	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	34.0027	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	34.2527	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	34.5027	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	34.7527	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	35.0027	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	35.2527	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	35.5027	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	35.7527	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	36.0027	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	36.2527	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	36.5027	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	36.7527	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	37.0027	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	37.2527	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	37.5027	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	37.7527	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	38.0027	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	38.2527	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	38.5027	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	38.7527	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	39.0027	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	39.2527	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	39.5027	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	39.7527	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	40.0027	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	40.2527	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	40.5027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	40.7527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	41.0027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	41.2527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	41.5027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	41.7527	24.61

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	42.0027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	42.2527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	42.5027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	42.7527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	43.0027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	43.2527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	43.5027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	43.7527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	44.0027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	44.2527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	44.5027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	44.7527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	45.0027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	45.2527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	45.5027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	45.7527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	46.0027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	46.2527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	46.5027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	46.7527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	47.0027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	47.2527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	47.5027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	47.7527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	48.0027	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	48.2527	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	48.5027	24.62
Scenario1	SFWMD 25Y-72H R2	BASIN 1	48.7527	24.62
Scenario1	SFWMD 25Y-72H R2	BASIN 1	49.0027	24.62
Scenario1	SFWMD 25Y-72H R2	BASIN 1	49.2527	24.62
Scenario1	SFWMD 25Y-72H R2	BASIN 1	49.5027	24.62
Scenario1	SFWMD 25Y-72H R2	BASIN 1	49.7527	24.62
Scenario1	SFWMD 25Y-72H R2	BASIN 1	50.0027	24.62
Scenario1	SFWMD 25Y-72H R2	BASIN 1	50.2527	24.62
Scenario1	SFWMD 25Y-72H R2	BASIN 1	50.5027	24.63
Scenario1	SFWMD 25Y-72H R2	BASIN 1	50.7527	24.63
Scenario1	SFWMD 25Y-72H R2	BASIN 1	51.0027	24.63
Scenario1	SFWMD 25Y-72H R2	BASIN 1	51.2527	24.63
Scenario1	SFWMD 25Y-72H R2	BASIN 1	51.5027	24.64
Scenario1	SFWMD 25Y-72H R2	BASIN 1	51.7527	24.64
Scenario1	SFWMD 25Y-72H R2	BASIN 1	52.0027	24.64
Scenario1	SFWMD 25Y-72H R2	BASIN 1	52.2527	24.65

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	52.5027	24.65
Scenario1	SFWMD 25Y-72H R2	BASIN 1	52.7527	24.66
Scenario1	SFWMD 25Y-72H R2	BASIN 1	53.0027	24.66
Scenario1	SFWMD 25Y-72H R2	BASIN 1	53.2527	24.67
Scenario1	SFWMD 25Y-72H R2	BASIN 1	53.5027	24.68
Scenario1	SFWMD 25Y-72H R2	BASIN 1	53.7527	24.69
Scenario1	SFWMD 25Y-72H R2	BASIN 1	54.0027	24.70
Scenario1	SFWMD 25Y-72H R2	BASIN 1	54.2527	24.70
Scenario1	SFWMD 25Y-72H R2	BASIN 1	54.5027	24.71
Scenario1	SFWMD 25Y-72H R2	BASIN 1	54.7527	24.72
Scenario1	SFWMD 25Y-72H R2	BASIN 1	55.0027	24.73
Scenario1	SFWMD 25Y-72H R2	BASIN 1	55.2527	24.74
Scenario1	SFWMD 25Y-72H R2	BASIN 1	55.5027	24.76
Scenario1	SFWMD 25Y-72H R2	BASIN 1	55.7527	24.77
Scenario1	SFWMD 25Y-72H R2	BASIN 1	56.0027	24.78
Scenario1	SFWMD 25Y-72H R2	BASIN 1	56.2527	24.79
Scenario1	SFWMD 25Y-72H R2	BASIN 1	56.5027	24.81
Scenario1	SFWMD 25Y-72H R2	BASIN 1	56.7527	24.82
Scenario1	SFWMD 25Y-72H R2	BASIN 1	57.0027	24.84
Scenario1	SFWMD 25Y-72H R2	BASIN 1	57.2527	24.86
Scenario1	SFWMD 25Y-72H R2	BASIN 1	57.5027	24.87
Scenario1	SFWMD 25Y-72H R2	BASIN 1	57.7527	24.90
Scenario1	SFWMD 25Y-72H R2	BASIN 1	58.0027	24.92
Scenario1	SFWMD 25Y-72H R2	BASIN 1	58.2527	24.94
Scenario1	SFWMD 25Y-72H R2	BASIN 1	58.5027	24.97
Scenario1	SFWMD 25Y-72H R2	BASIN 1	58.7531	24.99
Scenario1	SFWMD 25Y-72H R2	BASIN 1	59.0038	25.03
Scenario1	SFWMD 25Y-72H R2	BASIN 1	59.2532	25.08
Scenario1	SFWMD 25Y-72H R2	BASIN 1	59.5024	25.14
Scenario1	SFWMD 25Y-72H R2	BASIN 1	59.7504	25.33
Scenario1	SFWMD 25Y-72H R2	BASIN 1	60.0001	25.66
Scenario1	SFWMD 25Y-72H R2	BASIN 1	60.2501	25.88
Scenario1	SFWMD 25Y-72H R2	BASIN 1	60.5023	25.98
Scenario1	SFWMD 25Y-72H R2	BASIN 1	60.7551	26.03
Scenario1	SFWMD 25Y-72H R2	BASIN 1	61.0078	26.04
Scenario1	SFWMD 25Y-72H R2	BASIN 1	61.2578	26.04
Scenario1	SFWMD 25Y-72H R2	BASIN 1	61.5078	26.04
Scenario1	SFWMD 25Y-72H R2	BASIN 1	61.7578	26.04
Scenario1	SFWMD 25Y-72H R2	BASIN 1	62.0078	26.03
Scenario1	SFWMD 25Y-72H R2	BASIN 1	62.2578	26.03
Scenario1	SFWMD 25Y-72H R2	BASIN 1	62.5078	26.03
Scenario1	SFWMD 25Y-72H R2	BASIN 1	62.7578	26.02

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	63.0078	26.02
Scenario1	SFWMD 25Y-72H R2	BASIN 1	63.2578	26.01
Scenario1	SFWMD 25Y-72H R2	BASIN 1	63.5078	26.01
Scenario1	SFWMD 25Y-72H R2	BASIN 1	63.7578	26.00
Scenario1	SFWMD 25Y-72H R2	BASIN 1	64.0078	26.00
Scenario1	SFWMD 25Y-72H R2	BASIN 1	64.2578	25.99
Scenario1	SFWMD 25Y-72H R2	BASIN 1	64.5078	25.98
Scenario1	SFWMD 25Y-72H R2	BASIN 1	64.7578	25.97
Scenario1	SFWMD 25Y-72H R2	BASIN 1	65.0078	25.96
Scenario1	SFWMD 25Y-72H R2	BASIN 1	65.2578	25.95
Scenario1	SFWMD 25Y-72H R2	BASIN 1	65.5078	25.94
Scenario1	SFWMD 25Y-72H R2	BASIN 1	65.7578	25.93
Scenario1	SFWMD 25Y-72H R2	BASIN 1	66.0078	25.92
Scenario1	SFWMD 25Y-72H R2	BASIN 1	66.2578	25.91
Scenario1	SFWMD 25Y-72H R2	BASIN 1	66.5078	25.90
Scenario1	SFWMD 25Y-72H R2	BASIN 1	66.7578	25.89
Scenario1	SFWMD 25Y-72H R2	BASIN 1	67.0078	25.88
Scenario1	SFWMD 25Y-72H R2	BASIN 1	67.2578	25.87
Scenario1	SFWMD 25Y-72H R2	BASIN 1	67.5078	25.86
Scenario1	SFWMD 25Y-72H R2	BASIN 1	67.7578	25.85
Scenario1	SFWMD 25Y-72H R2	BASIN 1	68.0078	25.84
Scenario1	SFWMD 25Y-72H R2	BASIN 1	68.2578	25.83
Scenario1	SFWMD 25Y-72H R2	BASIN 1	68.5078	25.82
Scenario1	SFWMD 25Y-72H R2	BASIN 1	68.7578	25.80
Scenario1	SFWMD 25Y-72H R2	BASIN 1	69.0078	25.79
Scenario1	SFWMD 25Y-72H R2	BASIN 1	69.2578	25.78
Scenario1	SFWMD 25Y-72H R2	BASIN 1	69.5078	25.76
Scenario1	SFWMD 25Y-72H R2	BASIN 1	69.7578	25.75
Scenario1	SFWMD 25Y-72H R2	BASIN 1	70.0078	25.74
Scenario1	SFWMD 25Y-72H R2	BASIN 1	70.2578	25.72
Scenario1	SFWMD 25Y-72H R2	BASIN 1	70.5078	25.71
Scenario1	SFWMD 25Y-72H R2	BASIN 1	70.7578	25.69
Scenario1	SFWMD 25Y-72H R2	BASIN 1	71.0078	25.68
Scenario1	SFWMD 25Y-72H R2	BASIN 1	71.2578	25.67
Scenario1	SFWMD 25Y-72H R2	BASIN 1	71.5078	25.65
Scenario1	SFWMD 25Y-72H R2	BASIN 1	71.7578	25.64
Scenario1	SFWMD 25Y-72H R2	BASIN 1	72.0078	25.62
Scenario1	SFWMD 25Y-72H R2	BASIN 1	72.2578	25.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	72.5078	25.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	72.7578	25.57
Scenario1	SFWMD 25Y-72H R2	BASIN 1	73.0078	25.54
Scenario1	SFWMD 25Y-72H R2	BASIN 1	73.2578	25.52

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	73.5078	25.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	73.7578	25.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	74.0078	25.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	74.2578	25.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	74.5078	25.40
Scenario1	SFWMD 25Y-72H R2	BASIN 1	74.7578	25.38
Scenario1	SFWMD 25Y-72H R2	BASIN 1	75.0078	25.35
Scenario1	SFWMD 25Y-72H R2	BASIN 1	75.2578	25.32
Scenario1	SFWMD 25Y-72H R2	BASIN 1	75.5078	25.30
Scenario1	SFWMD 25Y-72H R2	BASIN 1	75.7578	25.27
Scenario1	SFWMD 25Y-72H R2	BASIN 1	76.0078	25.24
Scenario1	SFWMD 25Y-72H R2	BASIN 1	76.2578	25.22
Scenario1	SFWMD 25Y-72H R2	BASIN 1	76.5078	25.19
Scenario1	SFWMD 25Y-72H R2	BASIN 1	76.7578	25.16
Scenario1	SFWMD 25Y-72H R2	BASIN 1	77.0078	25.14
Scenario1	SFWMD 25Y-72H R2	BASIN 1	77.2578	25.11
Scenario1	SFWMD 25Y-72H R2	BASIN 1	77.5078	25.08
Scenario1	SFWMD 25Y-72H R2	BASIN 1	77.7578	25.06
Scenario1	SFWMD 25Y-72H R2	BASIN 1	78.0078	25.03
Scenario1	SFWMD 25Y-72H R2	BASIN 1	78.2578	25.01
Scenario1	SFWMD 25Y-72H R2	BASIN 1	78.5078	24.98
Scenario1	SFWMD 25Y-72H R2	BASIN 1	78.7578	24.96
Scenario1	SFWMD 25Y-72H R2	BASIN 1	79.0078	24.93
Scenario1	SFWMD 25Y-72H R2	BASIN 1	79.2578	24.91
Scenario1	SFWMD 25Y-72H R2	BASIN 1	79.5078	24.88
Scenario1	SFWMD 25Y-72H R2	BASIN 1	79.7578	24.86
Scenario1	SFWMD 25Y-72H R2	BASIN 1	80.0078	24.84
Scenario1	SFWMD 25Y-72H R2	BASIN 1	80.2578	24.81
Scenario1	SFWMD 25Y-72H R2	BASIN 1	80.5078	24.79
Scenario1	SFWMD 25Y-72H R2	BASIN 1	80.7578	24.77
Scenario1	SFWMD 25Y-72H R2	BASIN 1	81.0078	24.75
Scenario1	SFWMD 25Y-72H R2	BASIN 1	81.2578	24.73
Scenario1	SFWMD 25Y-72H R2	BASIN 1	81.5078	24.72
Scenario1	SFWMD 25Y-72H R2	BASIN 1	81.7578	24.70
Scenario1	SFWMD 25Y-72H R2	BASIN 1	82.0078	24.68
Scenario1	SFWMD 25Y-72H R2	BASIN 1	82.2578	24.67
Scenario1	SFWMD 25Y-72H R2	BASIN 1	82.5078	24.66
Scenario1	SFWMD 25Y-72H R2	BASIN 1	82.7578	24.65
Scenario1	SFWMD 25Y-72H R2	BASIN 1	83.0078	24.63
Scenario1	SFWMD 25Y-72H R2	BASIN 1	83.2578	24.62
Scenario1	SFWMD 25Y-72H R2	BASIN 1	83.5078	24.61
Scenario1	SFWMD 25Y-72H R2	BASIN 1	83.7578	24.60

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	84.0078	24.60
Scenario1	SFWMD 25Y-72H R2	BASIN 1	84.2578	24.59
Scenario1	SFWMD 25Y-72H R2	BASIN 1	84.5078	24.58
Scenario1	SFWMD 25Y-72H R2	BASIN 1	84.7578	24.58
Scenario1	SFWMD 25Y-72H R2	BASIN 1	85.0078	24.57
Scenario1	SFWMD 25Y-72H R2	BASIN 1	85.2578	24.56
Scenario1	SFWMD 25Y-72H R2	BASIN 1	85.5078	24.56
Scenario1	SFWMD 25Y-72H R2	BASIN 1	85.7578	24.55
Scenario1	SFWMD 25Y-72H R2	BASIN 1	86.0078	24.55
Scenario1	SFWMD 25Y-72H R2	BASIN 1	86.2578	24.54
Scenario1	SFWMD 25Y-72H R2	BASIN 1	86.5078	24.54
Scenario1	SFWMD 25Y-72H R2	BASIN 1	86.7578	24.54
Scenario1	SFWMD 25Y-72H R2	BASIN 1	87.0078	24.53
Scenario1	SFWMD 25Y-72H R2	BASIN 1	87.2578	24.53
Scenario1	SFWMD 25Y-72H R2	BASIN 1	87.5078	24.53
Scenario1	SFWMD 25Y-72H R2	BASIN 1	87.7578	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	88.0078	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	88.2578	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	88.5078	24.52
Scenario1	SFWMD 25Y-72H R2	BASIN 1	88.7578	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	89.0078	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	89.2578	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	89.5078	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	89.7578	24.51
Scenario1	SFWMD 25Y-72H R2	BASIN 1	90.0078	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	90.2578	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	90.5078	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	90.7578	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	91.0078	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	91.2578	24.50
Scenario1	SFWMD 25Y-72H R2	BASIN 1	91.5078	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	91.7578	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	92.0078	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	92.2578	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	92.5078	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	92.7578	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	93.0078	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	93.2578	24.49
Scenario1	SFWMD 25Y-72H R2	BASIN 1	93.5078	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	93.7578	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	94.0078	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	94.2578	24.48

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	94.5078	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	94.7578	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	95.0078	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	95.2578	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	95.5078	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	95.7578	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	96.0078	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	96.2578	24.48
Scenario1	SFWMD 25Y-72H R2	BASIN 1	96.5078	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	96.7578	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	97.0078	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	97.2578	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	97.5078	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	97.7578	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	98.0078	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	98.2578	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	98.5078	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	98.7578	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	99.0078	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	99.2578	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	99.5078	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	99.7578	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	100.0078	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	100.2578	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	100.5078	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	100.7578	24.47
Scenario1	SFWMD 25Y-72H R2	BASIN 1	101.0078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	101.2578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	101.5078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	101.7578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	102.0078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	102.2578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	102.5078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	102.7578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	103.0078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	103.2578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	103.5078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	103.7578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	104.0078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	104.2578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	104.5078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	104.7578	24.46

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	105.0078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	105.2578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	105.5078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	105.7578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	106.0078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	106.2578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	106.5078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	106.7578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	107.0078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	107.2578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	107.5078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	107.7578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	108.0078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	108.2578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	108.5078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	108.7578	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	109.0078	24.46
Scenario1	SFWMD 25Y-72H R2	BASIN 1	109.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	109.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	109.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	110.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	110.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	110.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	110.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	111.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	111.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	111.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	111.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	112.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	112.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	112.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	112.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	113.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	113.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	113.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	113.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	114.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	114.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	114.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	114.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	115.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	115.2578	24.45

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	115.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	115.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	116.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	116.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	116.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	116.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	117.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	117.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	117.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	117.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	118.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	118.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	118.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	118.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	119.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	119.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	119.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	119.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	120.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	120.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	120.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	120.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	121.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	121.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	121.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	121.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	122.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	122.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	122.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	122.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	123.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	123.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	123.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	123.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	124.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	124.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	124.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	124.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	125.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	125.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	125.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	125.7578	24.45

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	126.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	126.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	126.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	126.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	127.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	127.2578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	127.5078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	127.7578	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	128.0078	24.45
Scenario1	SFWMD 25Y-72H R2	BASIN 1	128.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	128.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	128.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	129.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	129.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	129.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	129.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	130.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	130.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	130.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	130.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	131.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	131.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	131.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	131.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	132.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	132.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	132.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	132.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	133.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	133.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	133.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	133.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	134.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	134.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	134.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	134.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	135.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	135.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	135.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	135.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	136.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	136.2578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	136.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	136.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	137.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	137.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	137.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	137.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	138.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	138.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	138.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	138.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	139.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	139.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	139.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	139.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	140.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	140.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	140.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	140.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	141.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	141.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	141.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	141.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	142.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	142.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	142.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	142.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	143.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	143.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	143.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	143.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	144.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	144.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	144.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	144.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	145.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	145.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	145.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	145.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	146.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	146.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	146.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	146.7578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	147.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	147.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	147.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	147.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	148.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	148.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	148.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	148.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	149.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	149.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	149.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	149.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	150.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	150.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	150.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	150.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	151.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	151.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	151.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	151.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	152.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	152.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	152.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	152.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	153.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	153.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	153.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	153.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	154.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	154.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	154.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	154.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	155.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	155.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	155.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	155.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	156.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	156.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	156.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	156.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	157.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	157.2578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	157.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	157.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	158.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	158.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	158.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	158.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	159.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	159.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	159.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	159.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	160.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	160.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	160.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	160.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	161.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	161.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	161.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	161.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	162.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	162.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	162.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	162.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	163.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	163.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	163.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	163.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	164.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	164.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	164.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	164.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	165.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	165.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	165.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	165.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	166.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	166.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	166.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	166.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	167.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	167.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	167.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	167.7578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	168.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	168.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	168.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	168.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	169.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	169.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	169.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	169.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	170.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	170.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	170.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	170.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	171.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	171.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	171.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	171.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	172.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	172.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	172.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	172.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	173.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	173.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	173.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	173.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	174.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	174.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	174.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	174.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	175.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	175.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	175.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	175.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	176.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	176.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	176.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	176.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	177.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	177.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	177.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	177.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	178.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	178.2578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	178.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	178.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	179.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	179.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	179.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	179.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	180.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	180.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	180.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	180.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	181.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	181.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	181.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	181.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	182.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	182.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	182.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	182.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	183.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	183.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	183.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	183.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	184.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	184.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	184.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	184.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	185.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	185.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	185.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	185.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	186.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	186.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	186.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	186.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	187.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	187.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	187.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	187.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	188.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	188.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	188.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	188.7578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	189.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	189.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	189.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	189.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	190.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	190.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	190.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	190.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	191.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	191.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	191.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	191.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	192.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	192.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	192.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	192.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	193.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	193.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	193.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	193.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	194.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	194.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	194.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	194.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	195.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	195.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	195.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	195.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	196.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	196.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	196.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	196.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	197.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	197.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	197.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	197.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	198.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	198.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	198.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	198.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	199.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	199.2578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	199.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	199.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	200.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	200.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	200.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	200.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	201.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	201.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	201.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	201.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	202.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	202.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	202.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	202.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	203.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	203.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	203.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	203.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	204.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	204.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	204.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	204.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	205.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	205.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	205.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	205.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	206.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	206.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	206.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	206.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	207.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	207.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	207.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	207.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	208.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	208.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	208.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	208.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	209.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	209.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	209.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	209.7578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	210.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	210.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	210.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	210.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	211.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	211.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	211.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	211.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	212.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	212.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	212.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	212.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	213.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	213.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	213.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	213.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	214.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	214.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	214.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	214.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	215.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	215.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	215.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	215.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	216.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	216.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	216.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	216.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	217.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	217.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	217.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	217.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	218.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	218.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	218.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	218.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	219.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	219.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	219.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	219.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	220.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	220.2578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	220.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	220.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	221.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	221.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	221.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	221.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	222.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	222.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	222.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	222.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	223.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	223.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	223.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	223.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	224.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	224.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	224.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	224.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	225.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	225.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	225.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	225.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	226.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	226.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	226.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	226.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	227.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	227.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	227.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	227.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	228.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	228.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	228.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	228.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	229.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	229.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	229.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	229.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	230.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	230.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	230.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	230.7578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	231.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	231.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	231.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	231.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	232.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	232.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	232.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	232.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	233.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	233.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	233.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	233.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	234.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	234.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	234.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	234.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	235.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	235.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	235.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	235.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	236.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	236.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	236.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	236.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	237.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	237.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	237.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	237.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	238.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	238.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	238.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	238.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	239.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	239.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	239.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	239.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	240.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	240.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	240.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	240.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	241.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	241.2578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	241.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	241.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	242.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	242.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	242.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	242.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	243.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	243.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	243.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	243.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	244.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	244.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	244.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	244.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	245.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	245.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	245.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	245.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	246.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	246.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	246.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	246.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	247.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	247.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	247.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	247.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	248.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	248.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	248.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	248.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	249.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	249.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	249.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	249.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	250.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	250.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	250.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	250.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	251.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	251.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	251.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	251.7578	24.44

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	252.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	252.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	252.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	252.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	253.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	253.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	253.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	253.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	254.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	254.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	254.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	254.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	255.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	255.2578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	255.5078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	255.7578	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	256.0078	24.44
Scenario1	SFWMD 25Y-72H R2	BASIN 1	256.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	256.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	256.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	257.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	257.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	257.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	257.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	258.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	258.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	258.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	258.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	259.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	259.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	259.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	259.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	260.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	260.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	260.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	260.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	261.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	261.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	261.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	261.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	262.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	262.2578	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	262.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	262.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	263.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	263.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	263.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	263.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	264.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	264.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	264.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	264.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	265.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	265.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	265.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	265.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	266.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	266.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	266.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	266.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	267.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	267.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	267.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	267.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	268.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	268.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	268.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	268.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	269.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	269.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	269.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	269.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	270.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	270.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	270.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	270.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	271.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	271.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	271.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	271.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	272.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	272.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	272.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	272.7578	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	273.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	273.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	273.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	273.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	274.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	274.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	274.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	274.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	275.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	275.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	275.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	275.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	276.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	276.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	276.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	276.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	277.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	277.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	277.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	277.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	278.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	278.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	278.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	278.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	279.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	279.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	279.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	279.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	280.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	280.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	280.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	280.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	281.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	281.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	281.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	281.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	282.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	282.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	282.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	282.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	283.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	283.2578	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	283.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	283.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	284.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	284.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	284.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	284.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	285.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	285.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	285.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	285.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	286.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	286.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	286.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	286.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	287.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	287.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	287.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	287.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	288.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	288.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	288.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	288.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	289.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	289.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	289.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	289.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	290.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	290.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	290.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	290.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	291.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	291.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	291.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	291.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	292.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	292.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	292.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	292.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	293.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	293.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	293.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	293.7578	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	294.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	294.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	294.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	294.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	295.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	295.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	295.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	295.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	296.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	296.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	296.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	296.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	297.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	297.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	297.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	297.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	298.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	298.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	298.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	298.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	299.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	299.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	299.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	299.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	300.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	300.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	300.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	300.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	301.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	301.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	301.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	301.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	302.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	302.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	302.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	302.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	303.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	303.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	303.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	303.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	304.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	304.2578	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	304.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	304.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	305.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	305.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	305.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	305.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	306.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	306.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	306.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	306.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	307.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	307.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	307.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	307.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	308.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	308.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	308.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	308.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	309.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	309.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	309.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	309.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	310.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	310.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	310.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	310.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	311.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	311.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	311.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	311.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	312.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	312.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	312.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	312.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	313.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	313.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	313.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	313.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	314.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	314.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	314.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	314.7578	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	315.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	315.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	315.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	315.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	316.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	316.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	316.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	316.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	317.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	317.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	317.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	317.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	318.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	318.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	318.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	318.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	319.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	319.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	319.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	319.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	320.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	320.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	320.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	320.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	321.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	321.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	321.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	321.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	322.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	322.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	322.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	322.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	323.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	323.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	323.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	323.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	324.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	324.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	324.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	324.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	325.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	325.2578	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	325.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	325.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	326.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	326.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	326.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	326.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	327.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	327.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	327.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	327.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	328.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	328.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	328.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	328.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	329.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	329.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	329.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	329.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	330.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	330.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	330.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	330.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	331.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	331.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	331.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	331.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	332.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	332.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	332.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	332.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	333.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	333.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	333.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	333.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	334.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	334.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	334.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	334.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	335.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	335.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	335.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	335.7578	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	336.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	336.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	336.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	336.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	337.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	337.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	337.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	337.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	338.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	338.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	338.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	338.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	339.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	339.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	339.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	339.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	340.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	340.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	340.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	340.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	341.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	341.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	341.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	341.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	342.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	342.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	342.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	342.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	343.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	343.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	343.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	343.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	344.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	344.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	344.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	344.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	345.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	345.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	345.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	345.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	346.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	346.2578	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	346.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	346.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	347.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	347.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	347.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	347.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	348.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	348.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	348.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	348.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	349.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	349.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	349.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	349.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	350.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	350.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	350.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	350.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	351.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	351.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	351.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	351.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	352.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	352.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	352.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	352.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	353.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	353.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	353.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	353.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	354.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	354.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	354.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	354.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	355.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	355.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	355.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	355.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	356.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	356.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	356.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	356.7578	24.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	SFWMD 25Y-72H R2	BASIN 1	357.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	357.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	357.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	357.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	358.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	358.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	358.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	358.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	359.0078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	359.2578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	359.5078	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	359.7578	24.43
Scenario1	SFWMD 25Y-72H R2	BASIN 1	360.0078	24.43

72-Hour Drawdown Analysis

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	72 H Drawdown	BASIN 1	0.0000	24.43
Scenario1	72 H Drawdown	BASIN 1	0.2511	24.38
Scenario1	72 H Drawdown	BASIN 1	0.5018	24.33
Scenario1	72 H Drawdown	BASIN 1	0.7530	24.28
Scenario1	72 H Drawdown	BASIN 1	1.0001	24.23
Scenario1	72 H Drawdown	BASIN 1	1.2538	24.18
Scenario1	72 H Drawdown	BASIN 1	1.5013	24.13
Scenario1	72 H Drawdown	BASIN 1	1.7512	24.08
Scenario1	72 H Drawdown	BASIN 1	2.0004	24.04
Scenario1	72 H Drawdown	BASIN 1	2.2540	24.00
Scenario1	72 H Drawdown	BASIN 1	2.5030	23.97
Scenario1	72 H Drawdown	BASIN 1	2.7527	23.94
Scenario1	72 H Drawdown	BASIN 1	3.0014	23.91
Scenario1	72 H Drawdown	BASIN 1	3.2514	23.88
Scenario1	72 H Drawdown	BASIN 1	3.5014	23.86
Scenario1	72 H Drawdown	BASIN 1	3.7514	23.83
Scenario1	72 H Drawdown	BASIN 1	4.0014	23.81
Scenario1	72 H Drawdown	BASIN 1	4.2514	23.79
Scenario1	72 H Drawdown	BASIN 1	4.5014	23.77
Scenario1	72 H Drawdown	BASIN 1	4.7514	23.75
Scenario1	72 H Drawdown	BASIN 1	5.0014	23.73
Scenario1	72 H Drawdown	BASIN 1	5.2514	23.71
Scenario1	72 H Drawdown	BASIN 1	5.5014	23.69
Scenario1	72 H Drawdown	BASIN 1	5.7514	23.68
Scenario1	72 H Drawdown	BASIN 1	6.0014	23.66
Scenario1	72 H Drawdown	BASIN 1	6.2514	23.64
Scenario1	72 H Drawdown	BASIN 1	6.5014	23.63
Scenario1	72 H Drawdown	BASIN 1	6.7514	23.61
Scenario1	72 H Drawdown	BASIN 1	7.0014	23.60
Scenario1	72 H Drawdown	BASIN 1	7.2514	23.58
Scenario1	72 H Drawdown	BASIN 1	7.5014	23.57
Scenario1	72 H Drawdown	BASIN 1	7.7514	23.55
Scenario1	72 H Drawdown	BASIN 1	8.0014	23.54
Scenario1	72 H Drawdown	BASIN 1	8.2514	23.53
Scenario1	72 H Drawdown	BASIN 1	8.5014	23.51
Scenario1	72 H Drawdown	BASIN 1	8.7514	23.50
Scenario1	72 H Drawdown	BASIN 1	9.0014	23.49
Scenario1	72 H Drawdown	BASIN 1	9.2514	23.48
Scenario1	72 H Drawdown	BASIN 1	9.5014	23.46
Scenario1	72 H Drawdown	BASIN 1	9.7514	23.45
Scenario1	72 H Drawdown	BASIN 1	10.0014	23.44
Scenario1	72 H Drawdown	BASIN 1	10.2514	23.43

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	72 H Drawdown	BASIN 1	10.5014	23.41
Scenario1	72 H Drawdown	BASIN 1	10.7514	23.40
Scenario1	72 H Drawdown	BASIN 1	11.0014	23.39
Scenario1	72 H Drawdown	BASIN 1	11.2514	23.38
Scenario1	72 H Drawdown	BASIN 1	11.5014	23.37
Scenario1	72 H Drawdown	BASIN 1	11.7514	23.36
Scenario1	72 H Drawdown	BASIN 1	12.0014	23.35
Scenario1	72 H Drawdown	BASIN 1	12.2514	23.34
Scenario1	72 H Drawdown	BASIN 1	12.5014	23.33
Scenario1	72 H Drawdown	BASIN 1	12.7514	23.31
Scenario1	72 H Drawdown	BASIN 1	13.0014	23.30
Scenario1	72 H Drawdown	BASIN 1	13.2514	23.29
Scenario1	72 H Drawdown	BASIN 1	13.5014	23.28
Scenario1	72 H Drawdown	BASIN 1	13.7514	23.27
Scenario1	72 H Drawdown	BASIN 1	14.0014	23.27
Scenario1	72 H Drawdown	BASIN 1	14.2514	23.26
Scenario1	72 H Drawdown	BASIN 1	14.5014	23.25
Scenario1	72 H Drawdown	BASIN 1	14.7514	23.24
Scenario1	72 H Drawdown	BASIN 1	15.0014	23.23
Scenario1	72 H Drawdown	BASIN 1	15.2514	23.22
Scenario1	72 H Drawdown	BASIN 1	15.5014	23.22
Scenario1	72 H Drawdown	BASIN 1	15.7514	23.21
Scenario1	72 H Drawdown	BASIN 1	16.0014	23.20
Scenario1	72 H Drawdown	BASIN 1	16.2514	23.19
Scenario1	72 H Drawdown	BASIN 1	16.5014	23.19
Scenario1	72 H Drawdown	BASIN 1	16.7514	23.18
Scenario1	72 H Drawdown	BASIN 1	17.0014	23.17
Scenario1	72 H Drawdown	BASIN 1	17.2514	23.17
Scenario1	72 H Drawdown	BASIN 1	17.5014	23.16
Scenario1	72 H Drawdown	BASIN 1	17.7514	23.15
Scenario1	72 H Drawdown	BASIN 1	18.0014	23.15
Scenario1	72 H Drawdown	BASIN 1	18.2514	23.14
Scenario1	72 H Drawdown	BASIN 1	18.5014	23.13
Scenario1	72 H Drawdown	BASIN 1	18.7514	23.13
Scenario1	72 H Drawdown	BASIN 1	19.0014	23.12
Scenario1	72 H Drawdown	BASIN 1	19.2514	23.12
Scenario1	72 H Drawdown	BASIN 1	19.5014	23.11
Scenario1	72 H Drawdown	BASIN 1	19.7514	23.11
Scenario1	72 H Drawdown	BASIN 1	20.0014	23.10
Scenario1	72 H Drawdown	BASIN 1	20.2514	23.10
Scenario1	72 H Drawdown	BASIN 1	20.5014	23.09
Scenario1	72 H Drawdown	BASIN 1	20.7514	23.09

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	72 H Drawdown	BASIN 1	21.0014	23.08
Scenario1	72 H Drawdown	BASIN 1	21.2514	23.08
Scenario1	72 H Drawdown	BASIN 1	21.5014	23.07
Scenario1	72 H Drawdown	BASIN 1	21.7514	23.07
Scenario1	72 H Drawdown	BASIN 1	22.0014	23.06
Scenario1	72 H Drawdown	BASIN 1	22.2514	23.06
Scenario1	72 H Drawdown	BASIN 1	22.5014	23.05
Scenario1	72 H Drawdown	BASIN 1	22.7514	23.05
Scenario1	72 H Drawdown	BASIN 1	23.0014	23.05
Scenario1	72 H Drawdown	BASIN 1	23.2514	23.04
Scenario1	72 H Drawdown	BASIN 1	23.5014	23.04
Scenario1	72 H Drawdown	BASIN 1	23.7514	23.03
Scenario1	72 H Drawdown	BASIN 1	24.0014	23.03
Scenario1	72 H Drawdown	BASIN 1	24.2514	23.03
Scenario1	72 H Drawdown	BASIN 1	24.5014	23.02
Scenario1	72 H Drawdown	BASIN 1	24.7514	23.02
Scenario1	72 H Drawdown	BASIN 1	25.0014	23.02
Scenario1	72 H Drawdown	BASIN 1	25.2514	23.01
Scenario1	72 H Drawdown	BASIN 1	25.5014	23.01
Scenario1	72 H Drawdown	BASIN 1	25.7514	23.01
Scenario1	72 H Drawdown	BASIN 1	26.0014	23.00
Scenario1	72 H Drawdown	BASIN 1	26.2514	23.00
Scenario1	72 H Drawdown	BASIN 1	26.5014	23.00
Scenario1	72 H Drawdown	BASIN 1	26.7514	22.99
Scenario1	72 H Drawdown	BASIN 1	27.0014	22.99
Scenario1	72 H Drawdown	BASIN 1	27.2514	22.99
Scenario1	72 H Drawdown	BASIN 1	27.5014	22.98
Scenario1	72 H Drawdown	BASIN 1	27.7514	22.98
Scenario1	72 H Drawdown	BASIN 1	28.0014	22.98
Scenario1	72 H Drawdown	BASIN 1	28.2514	22.97
Scenario1	72 H Drawdown	BASIN 1	28.5014	22.97
Scenario1	72 H Drawdown	BASIN 1	28.7514	22.97
Scenario1	72 H Drawdown	BASIN 1	29.0014	22.96
Scenario1	72 H Drawdown	BASIN 1	29.2514	22.96
Scenario1	72 H Drawdown	BASIN 1	29.5014	22.96
Scenario1	72 H Drawdown	BASIN 1	29.7514	22.96
Scenario1	72 H Drawdown	BASIN 1	30.0014	22.95
Scenario1	72 H Drawdown	BASIN 1	30.2514	22.95
Scenario1	72 H Drawdown	BASIN 1	30.5014	22.95
Scenario1	72 H Drawdown	BASIN 1	30.7514	22.95
Scenario1	72 H Drawdown	BASIN 1	31.0014	22.94
Scenario1	72 H Drawdown	BASIN 1	31.2514	22.94

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	72 H Drawdown	BASIN 1	31.5014	22.94
Scenario1	72 H Drawdown	BASIN 1	31.7514	22.94
Scenario1	72 H Drawdown	BASIN 1	32.0014	22.93
Scenario1	72 H Drawdown	BASIN 1	32.2514	22.93
Scenario1	72 H Drawdown	BASIN 1	32.5014	22.93
Scenario1	72 H Drawdown	BASIN 1	32.7514	22.93
Scenario1	72 H Drawdown	BASIN 1	33.0014	22.93
Scenario1	72 H Drawdown	BASIN 1	33.2514	22.92
Scenario1	72 H Drawdown	BASIN 1	33.5014	22.92
Scenario1	72 H Drawdown	BASIN 1	33.7514	22.92
Scenario1	72 H Drawdown	BASIN 1	34.0014	22.92
Scenario1	72 H Drawdown	BASIN 1	34.2514	22.92
Scenario1	72 H Drawdown	BASIN 1	34.5014	22.91
Scenario1	72 H Drawdown	BASIN 1	34.7514	22.91
Scenario1	72 H Drawdown	BASIN 1	35.0014	22.91
Scenario1	72 H Drawdown	BASIN 1	35.2514	22.91
Scenario1	72 H Drawdown	BASIN 1	35.5014	22.91
Scenario1	72 H Drawdown	BASIN 1	35.7514	22.90
Scenario1	72 H Drawdown	BASIN 1	36.0014	22.90
Scenario1	72 H Drawdown	BASIN 1	36.2514	22.90
Scenario1	72 H Drawdown	BASIN 1	36.5014	22.90
Scenario1	72 H Drawdown	BASIN 1	36.7514	22.90
Scenario1	72 H Drawdown	BASIN 1	37.0014	22.89
Scenario1	72 H Drawdown	BASIN 1	37.2514	22.89
Scenario1	72 H Drawdown	BASIN 1	37.5014	22.89
Scenario1	72 H Drawdown	BASIN 1	37.7514	22.89
Scenario1	72 H Drawdown	BASIN 1	38.0014	22.89
Scenario1	72 H Drawdown	BASIN 1	38.2514	22.89
Scenario1	72 H Drawdown	BASIN 1	38.5014	22.88
Scenario1	72 H Drawdown	BASIN 1	38.7514	22.88
Scenario1	72 H Drawdown	BASIN 1	39.0014	22.88
Scenario1	72 H Drawdown	BASIN 1	39.2514	22.88
Scenario1	72 H Drawdown	BASIN 1	39.5014	22.88
Scenario1	72 H Drawdown	BASIN 1	39.7514	22.88
Scenario1	72 H Drawdown	BASIN 1	40.0014	22.88
Scenario1	72 H Drawdown	BASIN 1	40.2514	22.87
Scenario1	72 H Drawdown	BASIN 1	40.5014	22.87
Scenario1	72 H Drawdown	BASIN 1	40.7514	22.87
Scenario1	72 H Drawdown	BASIN 1	41.0014	22.87
Scenario1	72 H Drawdown	BASIN 1	41.2514	22.87
Scenario1	72 H Drawdown	BASIN 1	41.5014	22.87
Scenario1	72 H Drawdown	BASIN 1	41.7514	22.87

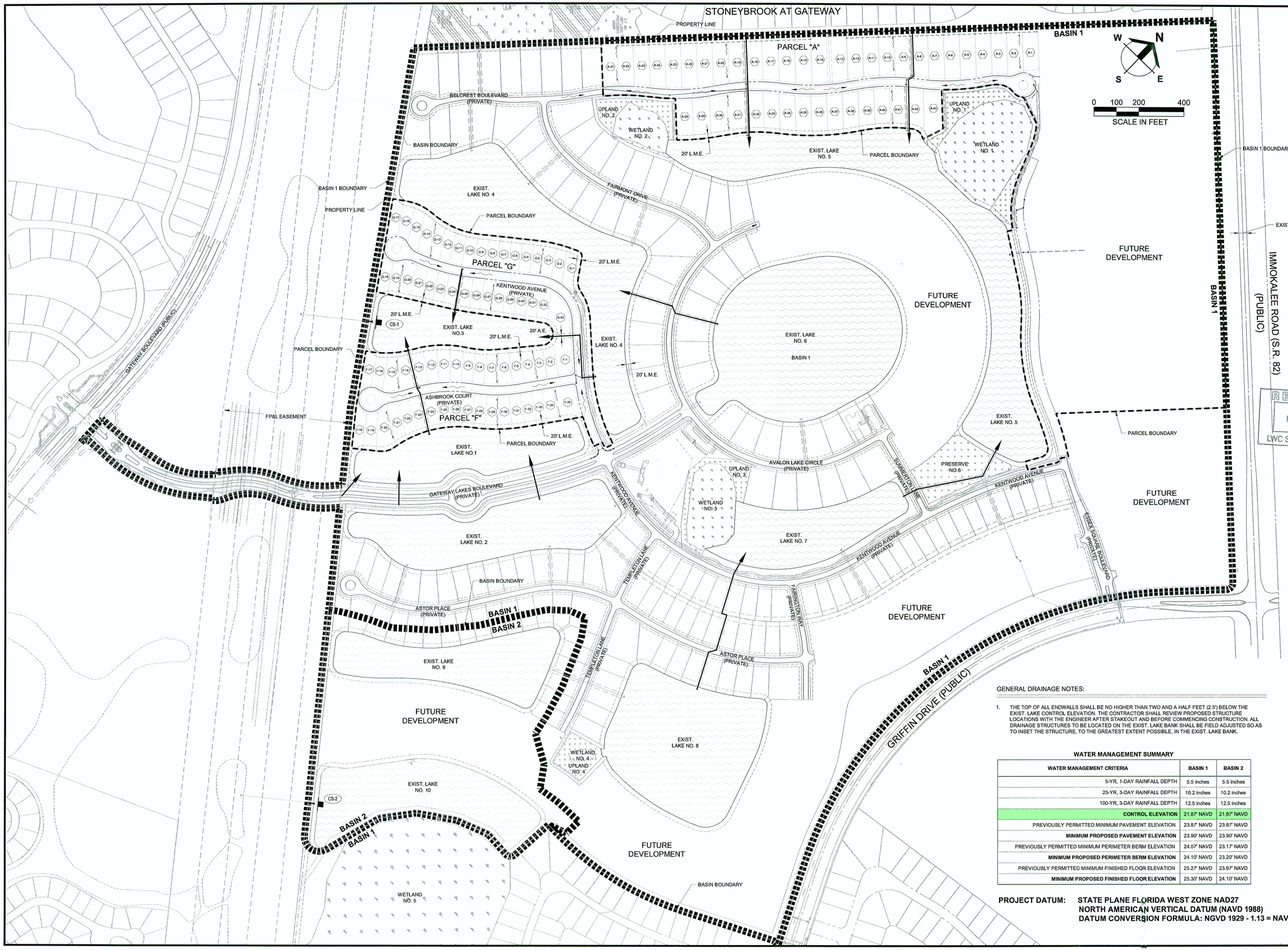
Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	72 H Drawdown	BASIN 1	42.0014	22.86
Scenario1	72 H Drawdown	BASIN 1	42.2514	22.86
Scenario1	72 H Drawdown	BASIN 1	42.5014	22.86
Scenario1	72 H Drawdown	BASIN 1	42.7514	22.86
Scenario1	72 H Drawdown	BASIN 1	43.0014	22.86
Scenario1	72 H Drawdown	BASIN 1	43.2514	22.86
Scenario1	72 H Drawdown	BASIN 1	43.5014	22.86
Scenario1	72 H Drawdown	BASIN 1	43.7514	22.85
Scenario1	72 H Drawdown	BASIN 1	44.0014	22.85
Scenario1	72 H Drawdown	BASIN 1	44.2514	22.85
Scenario1	72 H Drawdown	BASIN 1	44.5014	22.85
Scenario1	72 H Drawdown	BASIN 1	44.7514	22.85
Scenario1	72 H Drawdown	BASIN 1	45.0014	22.85
Scenario1	72 H Drawdown	BASIN 1	45.2514	22.85
Scenario1	72 H Drawdown	BASIN 1	45.5014	22.85
Scenario1	72 H Drawdown	BASIN 1	45.7514	22.84
Scenario1	72 H Drawdown	BASIN 1	46.0014	22.84
Scenario1	72 H Drawdown	BASIN 1	46.2514	22.84
Scenario1	72 H Drawdown	BASIN 1	46.5014	22.84
Scenario1	72 H Drawdown	BASIN 1	46.7514	22.84
Scenario1	72 H Drawdown	BASIN 1	47.0014	22.84
Scenario1	72 H Drawdown	BASIN 1	47.2514	22.84
Scenario1	72 H Drawdown	BASIN 1	47.5014	22.84
Scenario1	72 H Drawdown	BASIN 1	47.7514	22.83
Scenario1	72 H Drawdown	BASIN 1	48.0014	22.83
Scenario1	72 H Drawdown	BASIN 1	48.2514	22.83
Scenario1	72 H Drawdown	BASIN 1	48.5014	22.83
Scenario1	72 H Drawdown	BASIN 1	48.7514	22.83
Scenario1	72 H Drawdown	BASIN 1	49.0014	22.83
Scenario1	72 H Drawdown	BASIN 1	49.2514	22.83
Scenario1	72 H Drawdown	BASIN 1	49.5014	22.83
Scenario1	72 H Drawdown	BASIN 1	49.7514	22.83
Scenario1	72 H Drawdown	BASIN 1	50.0014	22.83
Scenario1	72 H Drawdown	BASIN 1	50.2514	22.82
Scenario1	72 H Drawdown	BASIN 1	50.5014	22.82
Scenario1	72 H Drawdown	BASIN 1	50.7514	22.82
Scenario1	72 H Drawdown	BASIN 1	51.0014	22.82
Scenario1	72 H Drawdown	BASIN 1	51.2514	22.82
Scenario1	72 H Drawdown	BASIN 1	51.5014	22.82
Scenario1	72 H Drawdown	BASIN 1	51.7514	22.82
Scenario1	72 H Drawdown	BASIN 1	52.0014	22.82
Scenario1	72 H Drawdown	BASIN 1	52.2514	22.82

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	72 H Drawdown	BASIN 1	52.5014	22.82
Scenario1	72 H Drawdown	BASIN 1	52.7514	22.81
Scenario1	72 H Drawdown	BASIN 1	53.0014	22.81
Scenario1	72 H Drawdown	BASIN 1	53.2514	22.81
Scenario1	72 H Drawdown	BASIN 1	53.5014	22.81
Scenario1	72 H Drawdown	BASIN 1	53.7514	22.81
Scenario1	72 H Drawdown	BASIN 1	54.0014	22.81
Scenario1	72 H Drawdown	BASIN 1	54.2514	22.81
Scenario1	72 H Drawdown	BASIN 1	54.5014	22.81
Scenario1	72 H Drawdown	BASIN 1	54.7514	22.81
Scenario1	72 H Drawdown	BASIN 1	55.0014	22.81
Scenario1	72 H Drawdown	BASIN 1	55.2514	22.81
Scenario1	72 H Drawdown	BASIN 1	55.5014	22.80
Scenario1	72 H Drawdown	BASIN 1	55.7514	22.80
Scenario1	72 H Drawdown	BASIN 1	56.0014	22.80
Scenario1	72 H Drawdown	BASIN 1	56.2514	22.80
Scenario1	72 H Drawdown	BASIN 1	56.5014	22.80
Scenario1	72 H Drawdown	BASIN 1	56.7514	22.80
Scenario1	72 H Drawdown	BASIN 1	57.0014	22.80
Scenario1	72 H Drawdown	BASIN 1	57.2514	22.80
Scenario1	72 H Drawdown	BASIN 1	57.5014	22.80
Scenario1	72 H Drawdown	BASIN 1	57.7514	22.80
Scenario1	72 H Drawdown	BASIN 1	58.0014	22.80
Scenario1	72 H Drawdown	BASIN 1	58.2514	22.79
Scenario1	72 H Drawdown	BASIN 1	58.5014	22.79
Scenario1	72 H Drawdown	BASIN 1	58.7514	22.79
Scenario1	72 H Drawdown	BASIN 1	59.0014	22.79
Scenario1	72 H Drawdown	BASIN 1	59.2514	22.79
Scenario1	72 H Drawdown	BASIN 1	59.5014	22.79
Scenario1	72 H Drawdown	BASIN 1	59.7514	22.79
Scenario1	72 H Drawdown	BASIN 1	60.0014	22.79
Scenario1	72 H Drawdown	BASIN 1	60.2514	22.79
Scenario1	72 H Drawdown	BASIN 1	60.5014	22.79
Scenario1	72 H Drawdown	BASIN 1	60.7514	22.79
Scenario1	72 H Drawdown	BASIN 1	61.0014	22.79
Scenario1	72 H Drawdown	BASIN 1	61.2514	22.79
Scenario1	72 H Drawdown	BASIN 1	61.5014	22.78
Scenario1	72 H Drawdown	BASIN 1	61.7514	22.78
Scenario1	72 H Drawdown	BASIN 1	62.0014	22.78
Scenario1	72 H Drawdown	BASIN 1	62.2514	22.78
Scenario1	72 H Drawdown	BASIN 1	62.5014	22.78
Scenario1	72 H Drawdown	BASIN 1	62.7514	22.78

Scenario	Sim	Node Name	Relative Time [hrs]	Stage [ft]
Scenario1	72 H Drawdown	BASIN 1	63.0014	22.78
Scenario1	72 H Drawdown	BASIN 1	63.2514	22.78
Scenario1	72 H Drawdown	BASIN 1	63.5014	22.78
Scenario1	72 H Drawdown	BASIN 1	63.7514	22.78
Scenario1	72 H Drawdown	BASIN 1	64.0014	22.78
Scenario1	72 H Drawdown	BASIN 1	64.2514	22.78
Scenario1	72 H Drawdown	BASIN 1	64.5014	22.78
Scenario1	72 H Drawdown	BASIN 1	64.7514	22.78
Scenario1	72 H Drawdown	BASIN 1	65.0014	22.77
Scenario1	72 H Drawdown	BASIN 1	65.2514	22.77
Scenario1	72 H Drawdown	BASIN 1	65.5014	22.77
Scenario1	72 H Drawdown	BASIN 1	65.7514	22.77
Scenario1	72 H Drawdown	BASIN 1	66.0014	22.77
Scenario1	72 H Drawdown	BASIN 1	66.2514	22.77
Scenario1	72 H Drawdown	BASIN 1	66.5014	22.77
Scenario1	72 H Drawdown	BASIN 1	66.7514	22.77
Scenario1	72 H Drawdown	BASIN 1	67.0014	22.77
Scenario1	72 H Drawdown	BASIN 1	67.2514	22.77
Scenario1	72 H Drawdown	BASIN 1	67.5014	22.77
Scenario1	72 H Drawdown	BASIN 1	67.7514	22.77
Scenario1	72 H Drawdown	BASIN 1	68.0014	22.77
Scenario1	72 H Drawdown	BASIN 1	68.2514	22.77
Scenario1	72 H Drawdown	BASIN 1	68.5014	22.77
Scenario1	72 H Drawdown	BASIN 1	68.7514	22.77
Scenario1	72 H Drawdown	BASIN 1	69.0014	22.76
Scenario1	72 H Drawdown	BASIN 1	69.2514	22.76
Scenario1	72 H Drawdown	BASIN 1	69.5014	22.76
Scenario1	72 H Drawdown	BASIN 1	69.7514	22.76
Scenario1	72 H Drawdown	BASIN 1	70.0014	22.76
Scenario1	72 H Drawdown	BASIN 1	70.2514	22.76
Scenario1	72 H Drawdown	BASIN 1	70.5014	22.76
Scenario1	72 H Drawdown	BASIN 1	70.7514	22.76
Scenario1	72 H Drawdown	BASIN 1	71.0014	22.76
Scenario1	72 H Drawdown	BASIN 1	71.2514	22.76
Scenario1	72 H Drawdown	BASIN 1	71.5014	22.76
Scenario1	72 H Drawdown	BASIN 1	71.7514	22.76
Scenario1	72 H Drawdown	BASIN 1	72.0014	22.76

Appendix C – Existing Adjacent Permit

SFWMD APPLICATION #110222-9



GENERAL DRAINAGE NOTES:

1. THE TOP OF ALL ENDWALLS SHALL BE NO HIGHER THAN TWO AND A HALF FEET (2.5') BELOW THE EXIST. LAKE CONTROL ELEVATION. THE CONTRACTOR SHALL REVIEW PROPOSED STRUCTURE LOCATIONS WITH THE ENGINEER AFTER STAKEOUT AND BEFORE COMMENCING CONSTRUCTION. ALL DRAINAGE STRUCTURES TO BE LOCATED ON THE EXIST. LAKE BANK SHALL BE FIELD ADJUSTED SO AS TO INSET THE STRUCTURE, TO THE GREATEST EXTENT POSSIBLE, IN THE EXIST. LAKE BANK.

WATER MANAGEMENT SUMMARY

WATER MANAGEMENT CRITERIA	BASIN 1	BASIN 2
5-YR, 1-DAY RAINFALL DEPTH	5.5 inches	5.5 inches
25-YR, 3-DAY RAINFALL DEPTH	10.2 inches	10.2 inches
100-YR, 3-DAY RAINFALL DEPTH	12.5 inches	12.5 inches
CONTROL ELEVATION	21.87' NAVD	21.87' NAVD
PREVIOUSLY PERMITTED MINIMUM PAVEMENT ELEVATION	23.87' NAVD	23.87' NAVD
MINIMUM PROPOSED PAVEMENT ELEVATION	23.90' NAVD	23.90' NAVD
PREVIOUSLY PERMITTED MINIMUM PERIMETER BERM ELEVATION	24.07' NAVD	23.17' NAVD
MINIMUM PROPOSED PERIMETER BERM ELEVATION	24.10' NAVD	23.20' NAVD
PREVIOUSLY PERMITTED MINIMUM FINISHED FLOOR ELEVATION	25.27' NAVD	23.97' NAVD
MINIMUM PROPOSED FINISHED FLOOR ELEVATION	25.30' NAVD	24.10' NAVD

PROJECT DATUM: STATE PLANE FLORIDA WEST ZONE NAD27
NORTH AMERICAN VERTICAL DATUM (NAVD 1988)
DATUM CONVERSION FORMULA: NGVD 1929 - 1.13 = NAVD 1988

Barraco and Associates, Inc.
CIVIL ENGINEERING - LAND SURVEYING
LAND PLANNING - LANDSCAPE DESIGN
www.barraco.net
2271 McGREGOR BLVD., SUITE 100
POST OFFICE DRAWER 2800
FORT MYERS, FLORIDA 33902-2800
PHONE (239) 461-3170
FAX (239) 461-3169
FLORIDA CERTIFICATES OF AUTHORIZATION
ENGINEERING 7995 - SURVEYING LB-6840

PREPARED FOR

WCI Communities LLC
24301 WALDEN CENTER DRIVE
BONITA SPRINGS, FLORIDA 34134
PHONE (239) 947-2600
FAX (239) 498-8273
WWW.WCICOMMUNITIES.COM

PROJECT DESCRIPTION

HAMPTON PARK AT GATEWAY

PARCELS A,F&G

PART OF SECTION 31
TOWNSHIP 44 SOUTH
RANGE 26 EAST AND
PART OF SECTIONS 5 AND 6
TOWNSHIP 45 SOUTH
RANGE 26 EAST
LEE COUNTY, FLORIDA

RECEIVED
FEB 22 2011
LWC SERVICE CENTER

PROJECT ENGINEER

ENGINEER OF RECORD:
CARL A. BARRACO, P.E.
FOR THE FIRM
2271 McGREGOR BOULEVARD
FORT MYERS, FLORIDA 33901
FLORIDA P.E. NO. 38536
CARLB@BARRACO.NET

NOT VALID WITHOUT EMBOSSED SEAL, SIGNATURE AND DATE
© COPYRIGHT 2011, BARRACO AND ASSOCIATES, INC.
REPRODUCTION, CHANGES OR ASSIGNMENTS ARE PROHIBITED

FILE NAME	22983A11.DWG
LAYOUT	LAYOUT1
LOCATION	J:\22983\DWG\DO\
PLOT DATE	TUE. 2-22-2011 - 4:07 PM
PLOT BY	JAN BILDZUKEWICZ
DESIGN BY	GUY SAPEN

CROSS REFERENCED DRAWINGS

BASEPLAN = 22983A00.DWG
MASTER = 21925A00.DWG

PLAN REVISIONS

PLAN STATUS

ERP SUBMITTAL
2011-02-22
NOT FOR CONSTRUCTION

MASTER DRAINAGE PLAN

PROJECT / FILE NO.	SHEET NUMBER
22983	5

Appendix D: Pollutant Loading BMP Trains Report

Simple Basin: BASIN 1

Scenario:	Scenario1
Node:	BASIN 1
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	10.0000 min
Max Allowable Q:	999.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH256
Peaking Factor:	256.0
Area:	0.8000 ac
Curve Number:	84.7
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Node: BASIN 1

Scenario:	Scenario1
Type:	Stage/Volume
Base Flow:	0.00 cfs
Initial Stage:	22.50 ft
Warning Stage:	999.00 ft

Stage [ft]	Volume [ac-ft]	Volume [ft3]
22.50	0.00	0
22.90	0.00	0
23.30	0.00	0
23.70	0.02	697
24.10	0.05	2222
24.50	0.09	3964
24.90	0.14	5924
25.30	0.19	8059
25.70	0.25	11064
26.10	0.37	16291
26.50	0.55	23784
26.90	0.77	33498
27.30	1.04	45172
27.70	1.31	57151
28.10	1.59	69086
28.50	1.86	81065

Comment:

Node: PERC NODE

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 22.50 ft
Warning Stage: 999.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	22.50
0	0	0	999.0000	22.50

Comment:

Node: TAILWATER

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 22.50 ft
Warning Stage: 999.00 ft
Boundary Stage: TAILWATER

Comment:

Drop Structure Link: DS-BASIN 1-TAILWATER

Scenario: Scenario1
From Node: BASIN 1
To Node: TAILWATER
Link Count: 1
Flow Direction: Both
Solution: Combine
Increments: 0
Pipe Count: 1
Damping: 0.0100 ft
Length: 50.00 ft
FHWA Code: 1
Entr Loss Coef: 0.20
Exit Loss Coef: 0.50
Bend Loss Coef: 0.00
Bend Location: 0.00 dec
Energy Switch: Energy

Upstream Pipe		Downstream Pipe	
Invert:	10.00 ft	Invert:	10.00 ft
Manning's N:	0.0120	Manning's N:	0.0120
Geometry: Circular		Geometry: Circular	
Max Depth:	1.50 ft	Max Depth:	1.50 ft
Bottom Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000
Top Clip			
Default:	0.00 ft	Default:	0.00 ft
Op Table:		Op Table:	
Ref Node:		Ref Node:	
Manning's N:	0.0000	Manning's N:	0.0000

Pipe Comment:

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0100 ft	Ref Node:	
Weir Type:	Sharp Crested Vertical	Top Clip	
Geometry Type:	Circular	Default:	0.00 ft
Invert:	24.43 ft	Op Table:	
Control Elevation:	24.43 ft	Ref Node:	
Max Depth:	0.25 ft	Discharge Coefficients	
		Weir Default:	3.200
		Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component			
Weir:	2	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Horizontal	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	26.02 ft	Op Table:	
Control Elevation:	26.02 ft	Ref Node:	
Max Depth:	4.50 ft	Discharge Coefficients	
Max Width:	3.00 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Drop Structure Comment:

Percolation Link: PERC LINK			
Scenario:	Scenario1	Surface Area Option:	Vary Based on Stage/Area Table
From Node:	BASIN 1		
To Node:	PERC NODE	Vertical Flow Termination:	Horizontal Flow Algorithm
Link Count:	1	Perimeter 1:	842.00 ft
Flow Direction:	None	Perimeter 2:	1234.00 ft
Aquifer Base Elevation:	12.50 ft	Perimeter 3:	4357.00 ft
Water Table Elevation:	22.50 ft	Distance P1 to P2:	50.00 ft
Annual Recharge Rate:	0 ipy	Distance P2 to P3:	450.00 ft
Horizontal Conductivity:	9.720 fpd	# of Cells P1 to P2:	5
Vertical Conductivity:	4.860 fpd	# of Cells P2 to P3:	45
Fillable Porosity:	0.200		
Layer Thickness:	1.00 ft		

Comment:

Simulation: 72 H Drawdown

Scenario: Scenario1
Run Date/Time: 12/22/2025 1:40:55 PM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set: No Rainfall
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec		
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	No Rainfall
Max dZ:	1.0000 ft		
Link Optimizer Tol:	0.0001 ft		
Edge Length Option:	Automatic		
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 100Y-72H

Scenario: Scenario1
Run Date/Time: 12/22/2025 11:47:22 AM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	30.0000
Max Calculation Time:		60.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000
0	0	0	60.0000	15.0000
0	0	0	80.0000	60.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set: 25-year
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft
Edge Length Option: Automatic

IA Recovery Time: 24.0000 hr
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~SFWMD-72
Rainfall Amount: 14.00 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: SFWMD 10Y-24H

Scenario: Scenario1
Run Date/Time: 12/22/2025 1:43:15 PM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000

End Time:	0	0	0	30.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set: 10-year
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft
Edge Length Option: Automatic

IA Recovery Time: 24.0000 hr
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~FLMOD
Rainfall Amount: 6.50 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: SFWMD 25Y-72H

Scenario: Scenario1
Run Date/Time: 12/22/2025 11:41:02 AM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	60.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000
0	0	0	60.0000	15.0000
0	0	0	80.0000	60.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set: 25-year
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight Fact:	0.5 dec		
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	Global
Max dZ:	1.0000 ft		
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~SFWMD-72
		Rainfall Amount:	11.00 in
Edge Length Option:	Automatic	Storm Duration:	72.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area (1D):	100 ft2
		Energy Switch (1D):	Energy

Comment:

Simulation: SFWMD 25Y-72H R2

Scenario: Scenario1
Run Date/Time: 12/22/2025 11:42:23 AM
Program Version: ICPR4 4.07.08

General

Run Mode: Hot Start
Hot Start Simulation: SFWMD 25Y-72H

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	360.0000
Hot Start Time:	0	0	0	359.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	30.0000	0.1000
Max Calculation Time:		30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000
0	0	0	60.0000	15.0000
0	0	0	80.0000	60.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set: 25-year
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft
Edge Length Option: Automatic

IA Recovery Time: 24.0000 hr
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~SFWMD-72
Rainfall Amount: 11.50 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Boundary Stage: TAILWATER

Boundary Stage Set: 10-year

Year	Month	Day	Hour [hr]	Stage [ft]
0	0	0	0.0000	22.50
0	0	0	18.0000	23.90
0	0	0	30.0000	22.50

Comment:

Boundary Stage: TAILWATER

Boundary Stage Set: 25-year

Year	Month	Day	Hour [hr]	Stage [ft]
0	0	0	0.0000	22.50
0	0	0	60.0000	23.90
0	0	0	360.0000	22.50

Comment:

Boundary Stage: TAILWATER

Boundary Stage Set: No Rainfall

Year	Month	Day	Hour [hr]	Stage [ft]
0	0	0	0.0000	22.50
0	0	0	999.0000	22.50

Comment:

FLORIDA DEPARTMENT OF
Environmental Protection

Bob Martinez Center
2600 Blair Stone Road
Tallahassee, Florida 32399-2400

Ron DeSantis
Governor

Jeanette Nuñez
Lt. Governor

Shawn Hamilton
Secretary

SELF-CERTIFICATION FOR A STORMWATER MANAGEMENT SYSTEM IN UPLANDS SERVING LESS THAN 10 ACRES OF TOTAL PROJECT AREA AND LESS THAN 2 ACRES OF IMPERVIOUS SURFACES

Owner(s)/Permittee(s):	CELESTIAL LIBERTY DREAMS LLC
File No:	0452260001EG
File Name:	RAY AVENUE GAS STATION
Site Address:	602 MEADOW RD Lehigh Acres FL - 33973
County:	Lee
Latitude:	26° 35' 38.6757"
Longitude:	-81° 44' 19.8007"
Total Project Area:	0.80
Total Impervious Surface Area:	0.47
Approximate Date of Commencement of Construction:	02/14/2024
Registered Florida Professional:	Joshua Eisenoff
License No.:	97911
Company:	Quattrone & Associates, Inc.

Date: August 20, 2024

Joshua Eisenoff certified through the Department's Enterprise Self-Service Application portal that the project described above was designed by the above-named Florida registered professional to meet the following requirements:

(a)The total project area involves less than 10 acres and less than 2 acres of impervious surface;
(b)Activities will not impact wetlands or other surface waters;
(c)Activities are not conducted in, on, or over wetlands or other surface waters;
(d)Drainage facilities will not include pipes having diameters greater than 24 inches, or the hydraulic equivalent, and will not use pumps in any manner;
(e)The project is not part of a larger common plan, development, or sale; and
(f)The project does not:
1.Cause adverse water quantity or flooding impacts to receiving water and adjacent lands;

2.Cause adverse impacts to existing surface water storage and conveyance capabilities;
3.Cause a violation of state water quality standards; or
4.Cause an adverse impact to the maintenance of surface or ground water levels or surface water flows established pursuant to s. 373.042 or a work of the district established pursuant to s. 373.086, F.S.

This certification was submitted before initiation of construction of the above project. The system is designed, and will be operated and maintained in accordance with applicable rules adopted pursuant to part IV of chapter 373, F.S. There is a rebuttable presumption that the discharge from such system will comply with state water quality standards.Therefore, construction, alteration, and maintenance of the stormwater management system serving this project is authorized in accordance with s.403.814(12), F.S.

In accordance with s. 373.416(2), F.S., if ownership of the property or the stormwater management system is sold or transferred to another party, continued operation of the system is authorized only if notice is provided to the Department within 30 days of the sale or transfer.This notice can be submitted to:

FDEP South District
2295 Victoria Ave
Ft. Myers, FL33901

This certification was submitted along with the following electronic documents:

If you have submitted this certification as a Florida Registered Professional, you may wish to sign and seal this certification, and return a copy to the Department, in accordance with your professional practice act requirements under Florida Statutes.

I, Joshua Eisenoff, License No. 97911, do hereby certify that the above information is true and accurate, based upon my knowledge, information and belief. In the space below, affix signature, date, seal, company name, address and certificate of authorization (if applicable).

This sealed certification may be submitted to the Department, either electronically (as an attachment in Adobe PDF or other secure, digital format) at FTMERP_Compliance@dep.state.fl.us, or as a hardcopy, at the postal address below:

FDEP South District
2295 Victoria Ave
Ft. Myers, FL33901

PART IV #26-41

Recommended Action

Engineering Review:

By: _____ 3/9/2026 **Approved** **X** **Denied** ___

District Field Review:

By: _____ **Approved** ___ **Denied** ___

District Manager: _____ **Date**: _____

Comments:

The referenced permit application for Ray Avenue Gas Station at 602 Meadow Rd., in Lehigh Acres, submitted by Quattrone & Associates, Inc., has been reviewed and is recommended for approval with the following stipulations/comments:

1. Appendix D of the Stormwater Management Report references the Pollutant Loading BMP Trains Report; however, it appears that Appendix D is a copy of the ICPR Input Report. Prior to construction, please revise the Stormwater Management Report to include the Pollutant Loading BMP Trains Report.

2. The CS-01 Structure Detail on Sheet 8 of the Civil Plans shows the wet season water table at elevation 22.3' NAVD; however, the Stormwater Management Report states it is at elevation 22.5' NAVD. Prior to construction, update Sheet 8 of the Civil Plans to match the elevation specified in the Stormwater Management Report.

3. The permittee shall submit signed and sealed record drawings within thirty (30) days of project completion. The drawings must include sufficient detail to demonstrate that the permitted facilities have been constructed in substantial compliance with LA-MSID–approved plans and design.

4. Prior to commencing construction, the permittee shall provide LA-MSID with copies of all applicable permits from other governing agencies.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: 2024/2025 Independent Financial Audit - Purvis, Gray & Company

Meeting Date: March 16, 2026

Agenda Item # 3B

☐ **CONSENT** ☒ **ACTION** ☐ **OLD BUSINESS** ☐ **NEW BUSINESS**

Request Initiated By: ☐ **COMMISSIONER** _____ ☐ **ATTORNEY**

☐ **BOARD** ☒ **MANAGER** ☐ **ENGINEER** ☐ **AUDITOR**

1 Recommended motion: Move to accept the FY 2024-2025 Independent Audit from Purvis & Gray.

2 What the action accomplishes: Provides audited data from an independent auditor, that showcases the health and compliance of LA-MSID.

3 Fiscal Impact: Estimated:
Audit & Financial Statement Fee - $38,500
Single Audit Fee - $3,000
Total Audit Fees - $41,500

4 Staff comments: The independent audit report expresses an unmodified opinion, which means the District's financial statements present a true and fair view. There were no material weaknesses that were found or reported by the auditor.

5 Staff review and recommended approval:

☒ **MANAGER** ☒ **ASSISTANT DISTRICT MGR**

☐ **ATTORNEY** ☐ **FIELD DIRECTOR**

☐ **ENGINEER** ☒ **FINANCE DIRECTOR**

6 Board Action:

☐ **APPROVED** ☐ **DENIED** ☐ **DEFERRED**

PURVIS GRAY

COMMUNICATION WITH THOSE CHARGED WITH GOVERNANCE

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

We have audited the financial statements of the governmental activities, each major fund, and the aggregate remaining fund information of the Lehigh Acres Municipal Services Improvement District (the District) for the year ended September 30, 2025. Professional standards require that we provide you with information about our responsibilities under generally accepted auditing standards, *Government Auditing Standards,* and Chapter 10.550, *Rules of the Auditor General*, as well as certain information related to the planned scope and timing of our audit. We have communicated such information in our engagement letter to you dated October 20, 2023, and our required communication letter dated August 21, 2025. Professional standards also require that we communicate to you the following information related to our audit.

Significant Audit Matters

Qualitative Aspects of Accounting Practices

Accounting Policies
Management is responsible for the selection and use of appropriate accounting policies. The significant accounting policies used by the District are described in Note 1 to the financial statements. Governmental Accounting Standards Board (GASB) Statement No. 101, *Compensated Absences*, and GASB Statement No. 102, *Certain Risk Disclosures*, were adopted during 2025 and were determined to have no impact on the District's financial statements. The application of existing policies was not changed during the year. We noted no transactions entered into by the District during the year for which there is a lack of authoritative guidance or consensus. All significant transactions have been recognized in the financial statements in the proper period.

Accounting Estimates
Accounting estimates are an integral part of the financial statements prepared by management and are based on management's knowledge and experience about past and current events and assumptions about future events. Certain accounting estimates are particularly sensitive because of their significance to the financial statements and because of the possibility that future events affecting them may differ significantly from those expected. The most significant estimates affecting the District's financial statements were:

- The original cost of certain land and improvements constructed or acquired prior to 1998 were estimated in that year based on the best available information at the time.
- Depreciation expense is based on industry useful lives using the straight-line method.
- Accrued compensated absences are based on unused vacation, sick, and personal hours earned as of year-end multiplied by the employee's current pay rate, plus payroll taxes, where applicable.

- As a participating employer in the Florida Retirement System (FRS), the District has recorded its share of the FRS' net pension liability and related deferred outflows/inflows on its statement of net position. These numbers are estimates based on assumptions underlying actuarial reports, and there are additional assumptions embedded in the employer allocations. If these assumptions were changed, the reported amounts could be significantly affected. In addition, because the FRS operates on a fiscal year ending June 30, there is a difference between the measurement date of these numbers (June 30, 2025) and the reporting date (September 30, 2025).

- Management's estimate of the total Other Postemployment Benefits (OPEB) liability related deferred inflows and outflows of resources, and expenses for OPEB at September 30, 2025, is an estimate based on actuarial studies performed by a qualified actuary retained by the District. The actuarial valuation is based upon actuarial methods and assumptions which are selected based on risk and market factors affecting governmental entities of similar sizes, OPEB plan policies, and employee census information. The total OPEB liability and deferred outflows and inflows of resources allocated to the District are determined by the actuary and also considered a significant estimate.

- Management's estimate related to the initial measurement of subscription-based information technology arrangements assets and liabilities, interest expense, term, present value of payments, and fair value of underlying assets is all based on terms in the agreements and the discount rate, based on the implementation of GASB Statement No. 96, *Subscription-Based Information Technology Arrangements*.

- Management's estimate related to the initial measurement of lease assets and liabilities, interest expense, lease term, present value of lease payments, and fair value of underlying assets is all based on terms in the agreements and the discount rate, based on the implementation of GASB Statement No. 87, *Leases*.

We evaluated the methods, assumptions, and data used to develop the above estimates in determining that they are reasonable in relation to the financial statements taken as a whole.

The financial statement disclosures are neutral, consistent, and clear.

Difficulties Encountered in Performing the Audit
We encountered no significant difficulties in dealing with management in performing and completing our audit.

Corrected and Uncorrected Misstatements
Professional standards require us to accumulate all known and likely misstatements identified during the audit, other than those that are clearly trivial, and communicate them to the appropriate level of management. Management has corrected all such misstatements. The following material misstatement detected as a result of audit procedures was corrected by management:

- During the current fiscal year, the District recognized Federal Emergency Management Agency obligations related to Hurricane Milton in the amount of $52,835 as due from other governments and unavailable revenue. However, federal reimbursement was received by the District in October 2025, which is within the District's availability period. This resulted in unavailable revenue being overstated while intergovernmental revenues were understated by the aforementioned amount.

Disagreements with Management
For purposes of this letter, a disagreement with management is a financial accounting, reporting, or auditing matter, whether or not resolved to our satisfaction, that could be significant to the financial statements or the auditor's report. We are pleased to report that no such disagreements arose during the course of our audit.

Management Representations
We have requested certain representations from management that are included in the management representation letter dated March 9, 2026.

Management Consultations with Other Independent Accountants
In some cases, management may decide to consult with other accountants about auditing and accounting matters, similar to obtaining a "second opinion" on certain situations. If a consultation involves application of an accounting principle to the District's financial statements or a determination of the type of auditor's opinion that may be expressed on those statements, our professional standards require the consulting accountant to check with us to determine that the consultant has all the relevant facts. To our knowledge, there were no such consultations with other accountants.

Other Audit Matters
We generally discuss a variety of matters, including the application of accounting principles and auditing standards, with management each year prior to retention as the District's auditors. However, these discussions occurred in the normal course of our professional relationship and our responses were not a condition to our retention.

The following are other audit matters we would like to bring to the attention of the District:

Upcoming Accounting Pronouncements
- GASB Statement No. 103, *Financial Model Improvements*—will become effective for the fiscal year ending September 30, 2026. This statement updates the guidelines for Management's Discussion & Analysis, unusual and infrequent items, proprietary fund statements, major component unit presentation, and budgetary comparison information to enhance effectiveness and comparability in information provided to financial statement users.

- GASB Statement No. 104, *Disclosure of Certain Capital Assets*—will become effective for the fiscal year ending September 30, 2026. This statement requires certain types of capital assets to be disclosed separately in the capital assets note disclosures. Separate disclosure of lease assets, intangible right-to-use assets, and subscription-based information technology arrangements will now be required, as well as additional disclosures for capital assets held for sale.

Significant Risks Identified
We have identified the risk of uncertain estimates being used to value OPEB and FRS liabilities, as well as their related activity, and have obtained an understanding of the District's related controls, including control activities relevant to such risks.

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

Other Matters

We applied certain limited procedures to the management's discussion and analysis (MD&A) and the required supplementary information (RSI), other than the MD&A, as listed in the table of contents (collectively, the required supplementary information), which is RSI that supplements the basic financial statements. Our procedures consisted of inquiries of management regarding the methods of preparing the information and comparing the information for consistency with management's responses to our inquiries, the basic financial statements, and other knowledge we obtained during our audit of the basic financial statements. We did not audit the RSI and do not express an opinion or provide any assurance on the RSI.

We were engaged to report on other supplementary information as listed in the table of contents, which accompanies the financial statements but is not RSI. With respect to this supplementary information, we made certain inquiries of management and evaluated the form, content, and methods of preparing the information to determine that the information complies with accounting principles generally accepted in the United States of America, the method of preparing it has not changed from the prior period, and the information is appropriate and complete in relation to our audit of the financial statements. We compared and reconciled the other supplementary information to the underlying accounting records used to prepare the financial statements or to the financial statements themselves.

Restriction on Use

This information is intended solely for the information and use of the Board of Commissioners and management of the District and is not intended to be, and should not be, used by anyone other than these specified parties.

Purvis Gray

March 9, 2026
Sarasota, Florida

2025

Lehigh Acres Municipal Services Improvement District

Financial Statements and Independent Auditor's Report

September 30, 2025

PURVIS GRAY

CERTIFIED PUBLIC ACCOUNTANTS

FINANCIAL STATEMENTS
AND
INDEPENDENT AUDITOR'S REPORT

LEHIGH ACRES MUNICIPAL SERVICES
IMPROVEMENT DISTRICT

SEPTEMBER 30, 2025

TABLE OF CONTENTS

Independent Auditor's Report 1-3

Management's Discussion and Analysis 4-12

Basic Financial Statements
Government-Wide Financial Statements:
Statement of Net Position 13
Statement of Activities 14
Fund Financial Statements:
Governmental Funds:
Balance Sheet - Governmental Funds 15
Reconciliation of the Balance Sheet - Governmental Funds to the Statement of Net Position 16
Statement of Revenues, Expenditures, and Changes in Fund Balance - Governmental Funds 17
Reconciliation of the Statement of Revenues, Expenditures, and Changes in Fund Balance - Governmental Funds to the Statement of Activities 18
Notes to the Financial Statements 19-44

Required Supplementary Information
Budget to Actual Comparison - General Fund:
Schedule of Revenues, Expenditures, and Changes in Fund Balance - Budget and Actual - General Fund 45
Schedules of Pension and Other Postemployment Benefits:
Schedule of Changes in the District's Total Other Postemployment Benefits Liability and Related Ratios 46
Schedule of the District's Proportionate Share of the Net Pension Liability 47
Schedule of District's Contributions 48
Notes to the Required Supplementary Information 49

FINANCIAL STATEMENTS
AND
INDEPENDENT AUDITOR'S REPORT

LEHIGH ACRES MUNICIPAL SERVICES
IMPROVEMENT DISTRICT

SEPTEMBER 30, 2025

TABLE OF CONTENTS

Supplementary Information
Balance Sheet - Non-Major Governmental Funds 50
Schedule of Revenues, Expenditures, and Changes
In Fund Balance - Non-Major Governmental Funds 51

Compliance Section
Schedule of Expenditures of State Financial Assistance 52
Notes to the Schedule of Expenditures of State Financial Assistance 53
Independent Auditor's Report on Internal Control Over Financial
Reporting and on Compliance and Other Matters Based on an
Audit of Financial Statements Performed in Accordance with
Government Auditing Standards 54-55
Independent Auditor's Report on Compliance for Each Major State
Project and on Internal Control Over Compliance Required by
Chapter 10.550, *Rules of the Auditor General* 56-58
Schedule of Findings and Questioned Costs 59-60
Independent Accountant's Report on Compliance with
Florida Statutes, Section 218.415 - Investments of Public Funds 61
Management Letter 62-64

PURVIS GRAY

INDEPENDENT AUDITOR'S REPORT

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

Report on the Audit of the Financial Statements

Opinions
We have audited the accompanying financial statements of the governmental activities, each major fund, and the aggregate remaining fund information of Lehigh Acres Municipal Services Improvement District (the District) as of and for the year ended September 30, 2025, and the related notes to the financial statements, which collectively comprise the District's basic financial statements as listed in the table of contents.

In our opinion, the financial statements referred to above present fairly, in all material respects, the respective financial position of the governmental activities, each major fund, and the aggregate remaining fund information of the District, as of September 30, 2025, and the respective changes in financial position for the year then ended in accordance with accounting principles generally accepted in the United States of America.

Basis for Opinions
We conducted our audit in accordance with auditing standards generally accepted in the United States of America and the standards applicable to financial audits contained in *Government Auditing Standards*, issued by the Comptroller General of the United States. Our responsibilities under those standards are further described in the Auditor's Responsibilities for the Audit of the Financial Statements section of our report. We are required to be independent of the District and to meet our other ethical responsibilities, in accordance with the relevant ethical requirements relating to our audit. We believe that the audit evidence we have obtained is sufficient and appropriate to provide a basis for our audit opinions.

Responsibilities of Management for the Financial Statements
Management is responsible for the preparation and fair presentation of the financial statements in accordance with accounting principles generally accepted in the United States of America, and for the design, implementation, and maintenance of internal control relevant to the preparation and fair presentation of financial statements that are free from material misstatement, whether due to fraud or error.

In preparing the financial statements, management is required to evaluate whether there are conditions or events, considered in the aggregate, that raise substantial doubt about the District's ability to continue as a going concern for twelve months beyond the financial statement date, including any currently known information that may raise substantial doubt shortly thereafter.

INDEPENDENT AUDITOR'S REPORT

Auditor's Responsibilities for the Audit of the Financial Statements

Our objectives are to obtain reasonable assurance about whether the financial statements as a whole are free from material misstatement, whether due to fraud or error, and to issue an auditor's report that includes our opinions. Reasonable assurance is a high level of assurance but is not absolute assurance and, therefore, is not a guarantee that an audit conducted in accordance with generally accepted auditing standards and *Government Auditing Standards* will always detect a material misstatement when it exists. The risk of not detecting a material misstatement resulting from fraud is higher than for one resulting from error, as fraud may involve collusion, forgery, intentional omissions, misrepresentations, or the override of internal control. Misstatements are considered material if there is a substantial likelihood that, individually or in the aggregate, they would influence the judgment made by a reasonable user based on the financial statements.

In performing an audit in accordance with generally accepted auditing standards and *Government Auditing Standards*, we:

- Exercise professional judgment and maintain professional skepticism throughout the audit.

- Identify and assess the risks of material misstatement of the financial statements, whether due to fraud or error, and design and perform audit procedures responsive to those risks. Such procedures include examining, on a test basis, evidence regarding amounts and disclosures in the financial statements.

- Obtain an understanding of internal control relevant to the audit in order to design audit procedures that are appropriate in the circumstances, but not for the purpose of expressing an opinion on the effectiveness of the District's internal control. Accordingly, no such opinion is expressed.

- Evaluate the appropriateness of accounting policies used and the reasonableness of significant accounting estimates made by management, as well as evaluate the overall presentation of the financial statements.

- Conclude whether, in our judgment, there are conditions or events, considered in the aggregate, that raise substantial doubt about the District's ability to continue as a going concern for a reasonable period of time.

We are required to communicate with those charged with governance regarding, among other matters, the planned scope and timing of the audit, significant audit findings, and certain internal control-related matters that we identified during the audit.

Required Supplementary Information

Accounting principles generally accepted in the United States of America require that the management's discussion and analysis (MD&A) and the required supplementary information, other than MD&A, as listed in the table of contents (collectively, the required supplementary information), be presented to supplement the basic financial statements. Such information is the responsibility of management and,

although not a part of the basic financial statements, is required by the Governmental Accounting Standards Board, who considers it to be an essential part of financial reporting for placing the basic financial statements in an appropriate operational, economic, or historical context. We have applied certain limited procedures to the required supplementary information in accordance with auditing standards generally accepted in the United States of America, which consisted of inquiries of management about the methods of preparing the information and comparing the information for consistency with management's responses to our inquiries, the basic financial statements, and other knowledge we obtained during our audit of the basic financial statements. We do not express an opinion or provide any assurance on the information because the limited procedures do not provide us with sufficient evidence to express an opinion or provide any assurance.

Supplementary Information

Our audit was conducted for the purpose of forming opinions on the financial statements that collectively comprise the District's basic financial statements. The accompanying combining and individual non-major fund financial schedules and schedule of expenditures of state financial assistance, as required by Chapter 10.550, *Rules of the Auditor General* of the State of Florida, are presented for purposes of additional analysis and are not a required part of the basic financial statements. Such information is the responsibility of management and was derived from and relates directly to the underlying accounting and other records used to prepare the basic financial statements. The information has been subjected to the auditing procedures applied in the audit of the basic financial statements and certain additional procedures, including comparing and reconciling such information directly to the underlying accounting and other records used to prepare the basic financial statements or to the basic financial statements themselves, and other additional procedures in accordance with auditing standards generally accepted in the United States of America. In our opinion, the combining and individual non-major fund schedules and schedule of expenditures of state financial assistance are fairly stated in all material respects in relation to the basic financial statements as a whole.

Other Reporting Required by *Government Auditing Standards*

In accordance with *Government Auditing Standards*, we have also issued our report dated March 9, 2026 on our consideration of the District's internal control over financial reporting and on our tests of its compliance with certain provisions of laws, regulations, contracts and grant agreements, and other matters. The purpose of that report is solely to describe the scope of our testing of internal control over financial reporting and compliance and the results of that testing, and not to provide an opinion on the effectiveness of the District's internal control over financial reporting or on compliance. That report is an integral part of an audit performed in accordance with *Government Auditing Standards* in considering the District's internal control over financial reporting and compliance.

Purvis Gray

March 9, 2026
Sarasota, Florida

MANAGEMENT'S DISCUSSION AND ANALYSIS

MANAGEMENT'S DISCUSSION AND ANALYSIS

Lehigh Acres Municipal Services Improvement District's (the District) discussion and analysis is designed to offer readers of the District's financial statements a narrative overview and analysis of the financial activities of the District for the fiscal year ended September 30, 2025. Readers are encouraged to read the Management's Discussion and Analysis in conjunction with the District's financial statements, which begin on page 13.

The District was created on June 10, 2015, by an act of the Florida Legislature. The act dissolved the East County Water Control District (the predecessor entity) and essentially transferred all assets, obligations, and rights, etc., of that entity to the new District.

Financial Highlights

* The assets (plus deferred outflows) of the District exceeded its liabilities (plus deferred inflows) at the close of the most recent fiscal year by $38,932,771 (*net position*). Of this amount, $6,936,334 (*unrestricted net position*) may be used to meet the District's ongoing obligations to citizens and creditors.

* The District's net position increased by $1,622,752 for the fiscal year ended September 30, 2025.

* As of the close of the current fiscal year, the District's governmental funds reported combined ending fund balances of $8,251,291, an increase of $569,428 from the previous fiscal year. Other than the non-spendable amount of $16,033, the total amount of fund balance is *available for spending* at the District's discretion (*assigned and unassigned fund balance*).

* At the end of the current fiscal year, unassigned fund balance for the general fund was $5,531,068.

Overview of the Financial Statements

This discussion and analysis is intended to serve as an introduction to the District's basic financial statements. The District's basic financial statements comprise three components: 1) government-wide financial statements, 2) fund financial statements, and 3) notes to the financial statements. This report also contains other supplementary information in addition to the basic financial statements themselves.

Government-Wide Financial Statements. The *government-wide financial statements* are designed to provide readers with a broad overview of the District's finances, in a manner similar to a private-sector business.

The *statement of net position* presents information on all of the District's assets, deferred outflows, liabilities, and deferred inflows, with the difference between the two reported as *net position*. Over time, increases or decreases in net position may serve as a useful indicator of whether the financial position of the District is improving or deteriorating.

The *statement of activities* presents information showing how the District's net position changed during the most recent fiscal year. All changes in net position are reported as soon as the underlying event given rise to the change occurs, *regardless of the timing of related cash flows*. Thus, revenues and expenses are reported in this statement for some items that will only result in cash flows in future fiscal periods (e.g., uncollected assessments and earned but unused vacation leave).

Both of the government-wide financial statements report only functions of the District that are principally supported by non-ad valorem assessments and intergovernmental revenues (*governmental activities*). The governmental activity of the District is water control. The District has no business-type activities.

The government-wide financial statements can be found on pages 13 and 14 of this report.

Fund Financial Statements. A *fund* is a grouping of related accounts that is used to maintain control over resources that have been segregated for specific activities or objectives. The District, like other governmental entities, uses fund accounting to ensure and demonstrate compliance with finance-related legal requirements.

Governmental funds are used to account for essentially the same functions reported as *governmental activities* in the government-wide financial statements. However, unlike the government-wide financial statements, governmental fund financial statements focus on *near-term inflows and outflows of spendable resources*, as well as on *balances of spendable resources* available at the end of the fiscal year. Such information may be useful in evaluating the District's near-term financing requirements.

Because the focus of governmental funds is narrower than that of the government-wide financial statements, it is useful to compare the information presented for *governmental funds* with similar information presented for *governmental activities* in the government-wide financial statements. By doing so, readers may better understand the long-term impact of the District's near-term financing decisions. Both the governmental fund balance sheet and the governmental fund statement of revenues, expenditures, and changes in fund balance provide a reconciliation to facilitate this comparison between governmental funds and governmental activities.

The District maintains five individual governmental funds. Information is presented separately in the governmental fund balance sheet and in the governmental fund statement of revenues, expenditures, and changes in fund balances for the major funds.

The District adopts an annual appropriated budget for all funds. To demonstrate compliance with the adopted budget, a budgetary comparison statement has been provided for the general fund as *required supplementary information,* which can be found on page 45 of this report.

The basic governmental fund financial statements can be found on pages 15-18 of this report.

Notes to the Financial Statements. The notes provide additional information that is essential to a full understanding of the data provided in the government-wide and fund financial statements. The notes to the financial statements can be found on pages 19 through 44 of this report.

Government-Wide Financial Analysis

As noted earlier, net position may serve over time as a useful indicator of a government's financial position. In the case of the District, assets (plus deferred outflows) exceeded liabilities (plus deferred inflows) by $38,932,771 at the close of the most recent fiscal year.

Remainder of page intentionally left blank

The following schedule is a summary of the statement of net position found on page 13 of this report, with comparisons to the prior year.

Lehigh Acres Municipal Services Improvement District's Net Position

	Governmental Activities 2025	Governmental Activities 2024
Assets		
Current and other assets	$ 10,452,966	$ 9,089,776
Capital assets	34,703,338	35,375,187
	45,156,304	44,464,963
Deferred Outflows of Resources	536,282	619,961
Liabilities		
Current and other liabilities	726,496	673,237
Non-current liabilities	4,921,324	6,117,279
	5,647,820	6,790,516
Deferred Inflows of Resources	1,111,995	984,389
Net Position		
Net investment in capital assets	31,996,437	32,144,781
Unrestricted	6,936,334	5,165,238
Total net position	$ 38,932,771	$ 37,310,019

By far, the largest portion of the District's net position (82%) reflects its investment in capital assets (e.g., land, buildings, improvements, equipment, intangible assets including SBITA assets, and construction in progress) less any related debt used to acquire those assets that is still outstanding. The District uses these capital assets to carry out its statutory responsibilities; consequently, these assets are *not* available for future spending. Although the District's investment in its capital assets is reported net of related debt, it should be noted that the resources needed to repay this debt must be provided from other sources, since the capital assets themselves cannot be used to liquidate these liabilities.

Due to the fact that none of the District's net position is subject to external restrictions on how it may be used, $6,936,334 is unrestricted and may be used to meet the District's ongoing obligations to citizens and creditors.

When compared to last fiscal year, there was an increase of $1,771,096 in the unrestricted net position in connection with the District's governmental activities.

Lehigh Acres Municipal Services Improvement District's Changes in Net Position

	Governmental Activities 2025	Governmental Activities 2024
Revenues		
Program revenues:		
Charges for services – permit fees	$ 311,312	$ 79,814
Intergovernmental revenue	807,429	2,453,647
General Revenues		
Non-ad valorem assessments	7,946,543	6,548,343
Other	450,860	478,615
Total revenues	9,516,144	9,560,419
Expenses		
Water control	7,893,392	7,761,388
Total expenses	7,893,392	7,761,388
Change in net position	1,622,752	1,799,031
Net position – beginning	37,310,019	35,510,988
Net position – ending	$ 38,932,771	$ 37,310,019

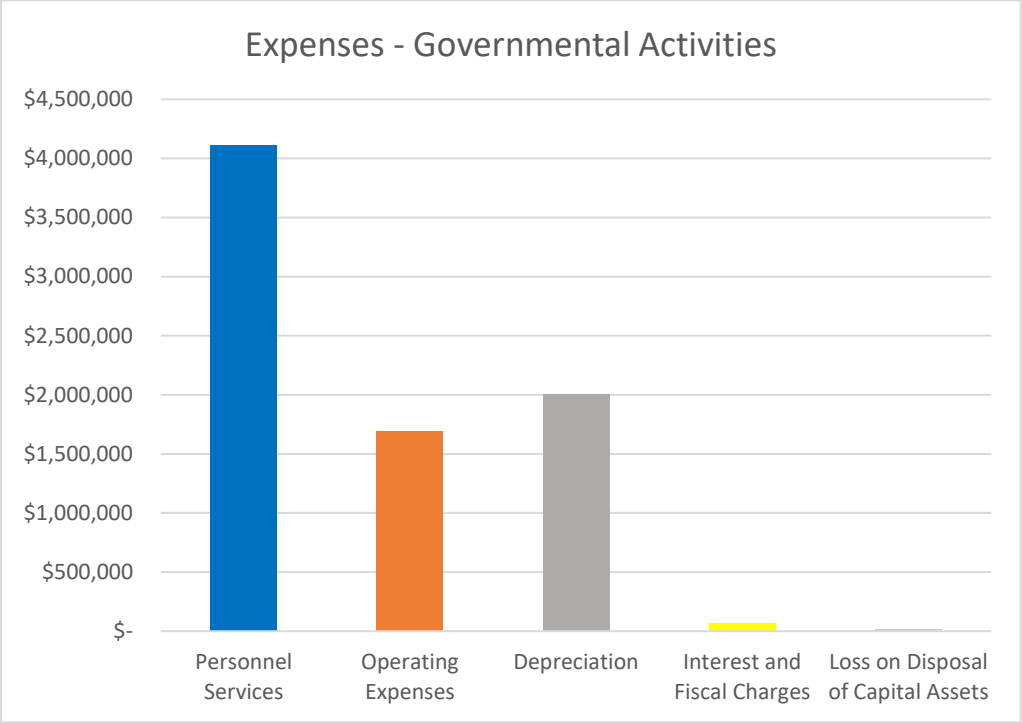
During fiscal year 2025, the District's total net position increased by $1,622,752 compared to an increase of $1,799,031 in the prior year. The reasons for the changes are discussed below:

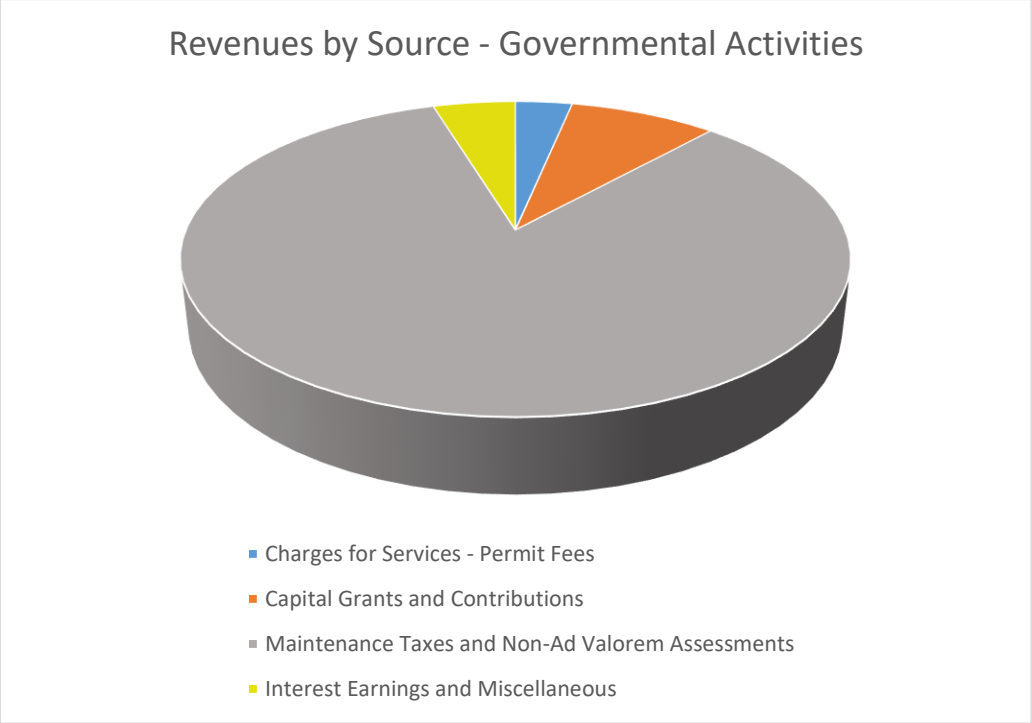
* In fiscal year 2025, intergovernmental revenue decreased by $1,646,218, which was due to less grant activity in fiscal year 2025 vs. fiscal year 2024.

* Non-ad valorem assessments increased in fiscal year 2025 by $1,398,200 compared to fiscal year 2024, which is primarily due to an increase in the non-ad valorem assessment rate per acre in fiscal year 2025.

* Total expenses increased by $132,004 in fiscal year 2025 vs. fiscal year 2024, with a $43,238 increase in personnel services due to increases in regular salaries and related expenses. Most of the remaining increase is attributable to higher depreciation expense resulting from newly acquired capital assets.

Shown below are graphs of 2025 expenses and revenues:





Financial Analysis of the Government's Funds

As noted earlier, the District uses fund accounting to ensure and demonstrate compliance with finance-related legal requirements.

Governmental Funds. The focus of the District's *governmental funds* is to provide information on near-term inflows, outflows, and balances of *spendable* resources. Such information is useful in assessing the District's financing requirements. In particular, *unassigned fund balance* may serve as a useful measure of a government's net resources available for spending at the end of the fiscal year.

At September 30, 2025, the District's governmental funds reported combined ending fund balances of $8,251,291, an increase of $569,428 in fiscal year 2025 compared to fiscal year 2024. Of the year-end balance, $16,033 is non-spendable and $2,785,327 is assigned for various purposes. $5,449,931 (66%) of the year-end balance constitutes *unassigned fund balance* and is available for spending at the District's discretion.

The general fund is the chief operating fund of the District. At the end of the current fiscal year, unassigned fund balance of the general fund was $5,531,068, an increase of $16,691. This increased less than 1% from 2024.

The Capital Equipment Fund reported a significant deficit fund balance of $(1,072,165) at the end of fiscal year 2024, all of which was unassigned. During fiscal year 2025, the deficit decreased substantially to $(81,137), all of which was unassigned. The improvement was primarily the result of transfers from the General Fund.

The capital infrastructure projects fund has a total fund balance of $274,357 at year-end, of which is all assigned for capital expenditures.

The bridge renewal and replacement fund has a total fund balance of $1,352,801 at year-end, of which all is assigned for bridge replacement.

The District's three capital project funds (discussed above) have a combined total fund balance of $1,546,021 in fiscal year 2025, which is a $235,726 (17.99%) increase in fund balance compared to fiscal year 2024. The change is mainly due to an increase in transfers into the capital equipment fund to cover prior year deficit and a large decrease in the capital infrastructure project fund due to projects in 2025.

Capital Asset and Debt Administration

Capital Assets. The District's capital assets, as of September 30, 2025, amount to $34,703,338 (net of accumulated depreciation), which includes land, land improvements, buildings, equipment, intangible assets (easements, computer software, and SBITAs), and construction in progress. There was a net decrease of $671,849 in the District's capital assets in fiscal year 2025 vs. fiscal year 2024.

Major capital asset events during the current fiscal year included the following:

* Construction in progress had a net increase of $851,639.
* Land improvements additions for the year totaled $150,247.
* Equipment additions for the year totaled $414,035.
* Depreciation expense totaled $2,008,306.

The schedule below shows the District's capital asset balances (book value) at the end of the current and prior year:

Lehigh Acres Municipal Services Improvement District's Capital Assets
(Net of Depreciation)

	Governmental Activities 2025	Governmental Activities 2024
Land	$ 3,290,932	$ 3,290,932
Intangibles – easements	1,297	1,297
Buildings, net	1,401,882	1,442,962
Land improvements, net	20,823,391	22,079,929
Equipment, net	3,237,157	3,449,816
Intangibles – software	186	725
Intangibles – SBITAs	42,240	54,912
Construction in progress	5,906,253	5,054,614
Total assets	$ 34,703,338	$ 35,375,187

Additional information on the District's capital assets can be found in Note 5 on page 30 of this report.

Long-Term Debt. At the end of the current fiscal year, the District had total notes payable outstanding of $2,356,669 and a total SBITA payable of $44,158.

Lehigh Acres Municipal Services Improvement District
Long-Term Debt

	Governmental Activities 2025	Governmental Activities 2024
Notes payable	$ 2,356,669	$ 3,139,321
SBITA payable	44,158	56,509
Total	$ 2,400,827	$ 3,195,830

The District's total debt saw a net decrease of ($795,003) (24.88%) during the current fiscal year as a result of scheduled principal repayment.

Additional information on the District's long-term debt can be found in Note 6 on pages 31-32 of this report.

Economic Factors and Next Year's Budgets and Rates

The area in which the District operates shows signs of rapid population growth. During fiscal year 2025, with an estimated population of 126,557, the District was ranked as one of the fastest growing communities in the country, by population.

The District's primary source of funding is non-ad valorem assessments. The assessment rate for fiscal year 2026 (tax year 2025) was $172.48 per acre. In fiscal year 2025, there was a $30 per acre non-ad valorem assessment rate increase, which was mainly for the purpose of covering the increasing costs of operating while providing residents with the same high standard and quality services. The majority of the landowners own one-quarter or one-half acre lots. The uncollectible rate remains budgeted at 3% of the total non-ad valorem assessed.

Unassigned fund balance continues to be budgeted at a minimum of 25% of budgeted operating expenditures.

Requests for Information

This financial report is designed to provide a general overview of the District's finances for all those with an interest in the government's finances. Questions concerning any of the information provided in this report or requests for additional financial information should be addressed to the District Manager, 601 East County Lane, Lehigh Acres, Florida 33936.

Remainder of page intentionally left blank

BASIC FINANCIAL STATEMENTS

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
STATEMENT OF NET POSITION
SEPTEMBER 30, 2025

	Governmental Activities
Assets	
Cash and Cash Equivalents	$ 8,844,743
Lease Receivable	654,869
Due from Other Governments	927,842
Other Receivables	9,479
Prepaid Items	14,358
Utility Deposits	1,675
Capital Assets:	
Non-Depreciable Land, Construction in Progress, and Easements	9,198,482
Depreciable Buildings, Land Improvements, Furniture and Fixtures, and Equipment (Net of ($45,237,009) Accumulated Depreciation)	25,504,856
Total Assets	45,156,304
Deferred Outflows of Resources	
Deferred Outflows - Pension	536,282
Total Deferred Outflows of Resources	536,282
Total Assets and Deferred Outflows of Resources	45,692,586
Liabilities	
Accounts and Contracts Payable	479,250
Retainage Payable	38,040
Deposits	5,441
Accrued Liabilities	197,878
Accrued Interest on Long-Term Obligations	5,887
Subscription-Based Information Technology Arrangements (SBITAs) Payable:	
Due Within One Year	12,757
Due in More than One Year	31,401
Non-Current Liabilities:	
Due Within One Year	981,509
Due in More than One Year	3,895,657
Total Liabilities	5,647,820
Deferred Inflows of Resources	
Deferred Inflows - Pensions	467,908
Lease Related	644,087
Total Deferred Inflows of Resources	1,111,995
Net Position	
Net Investment in Capital Assets	31,996,437
Unrestricted	6,936,334
Total Net Position	38,932,771
Total Liabilities, Deferred Inflows of Resources, and Net Position	$ 45,692,586

See accompanying notes to financial statements.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
STATEMENT OF ACTIVITIES
YEAR ENDED SEPTEMBER 30, 2025

	Governmental Activities
Expenses	
Water Control:	
Personnel Services	$ 4,106,655
Operating Expenses	1,689,746
Depreciation	2,008,306
Interest and Fiscal Charges	69,058
Loss on Disposal of Capital Assets	19,627
Total Expenses	7,893,392
Program Revenue	
Charges for Services - Permit Fees	311,312
Capital Grants and Contributions	807,429
Total Program Revenue	1,118,741
Net Program Expenses	(6,774,651)
General Revenues	
Maintenance Taxes and Non-Ad Valorem Assessments	7,946,543
Interest Earnings and Miscellaneous	450,860
Total General Revenues	8,397,403
Change in Net Position	1,622,752
Net Position, Beginning of Year	37,310,019
Net Position, End of Year	$ 38,932,771

See accompanying notes to financial statements.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
BALANCE SHEET - GOVERNMENTAL FUNDS
SEPTEMBER 30, 2025

ASSETS

	Major Funds					
	General	Capital Equipment	Capital Infrastructure Projects	Bridge Renewal & Replacement	Non-Major Governmental Fund	Total Governmental Funds
Assets						
Cash and Cash Equivalents	$ 5,730,026	$ 145,326	$ 749,378	$ 1,363,329	$ 856,684	$ 8,844,743
Lease Receivable	654,869	-	-	-	-	654,869
Due from Other Governments	38,028	-	889,814	-	-	927,842
Due from Other Funds	458,774	-	-	-	-	458,774
Other Receivables	9,479	-	-	-	-	9,479
Prepaid Items	14,358	-	-	-	-	14,358
Utility Deposits	1,675	-	-	-	-	1,675
Total Assets	6,907,209	145,326	1,639,192	1,363,329	856,684	10,911,740

LIABILITIES, DEFERRED INFLOWS, AND FUND BALANCES

Liabilities						
Accounts and Contracts Payable	211,217	-	268,033	-	-	479,250
Retainage Payable	-	-	38,040	-	-	38,040
Due to Other Funds	-	226,463	221,783	10,528	-	458,774
Deposits	5,441	-	-	-	-	5,441
Accrued Liabilities	197,878	-	-	-	-	197,878
Total Liabilities	414,536	226,463	527,856	10,528	-	1,179,383
Deferred Inflows						
Unavailable Grant Revenues	-	-	836,979	-	-	836,979
Lease Related	644,087	-	-	-	-	644,087
Total Deferred Inflows	644,087	-	836,979	-	-	1,481,066
Fund Balances						
Non-Spendable:						
Prepaid Items and Utility Deposits	16,033	-	-	-	-	16,033
Assigned for:						
Debt Service	-	-	-	-	856,684	856,684
Capital Expenditures	-	-	274,357	-	-	274,357
Bridge Replacement	-	-	-	1,352,801	-	1,352,801
Subsequent Year's Expenditures	301,485	-	-	-	-	301,485
Unassigned	5,531,068	(81,137)	-	-	-	5,449,931
Total Fund Balances	5,848,586	(81,137)	274,357	1,352,801	856,684	8,251,291
Total Liabilities, Deferred Inflows, and Fund Balances	$ 6,907,209	$ 145,326	$ 1,639,192	$ 1,363,329	$ 856,684	$10,911,740

See accompanying notes to financial statements.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
RECONCILIATION OF THE BALANCE SHEET - GOVERNMENTAL FUNDS
TO THE STATEMENT OF NET POSITION
SEPTEMBER 30, 2025

Total Fund Balances for Governmental Funds		$ 8,251,291
Amounts reported for governmental activities in the statement of net position are different because:		
Revenues are deferred in governmental funds when both the measurable and available criteria are not met under the modified accrual basis of accounting. Under the full accrual basis of accounting these revenues would be recognized when earned regardless of when they are received		836,979
Capital assets used in governmental activities are not financial resources and, therefore, are not reported in the governmental funds:		
Land	$ 3,290,932	
Intangibles - Easements	1,297	
Construction in Progress	5,906,253	
Building, Infrastructure, Equipment, and Intangibles (including SBITAs)	70,741,865	
(Less Accumulated Depreciation)	(45,237,009)	34,703,338
Long-term liabilities are not due and payable in the current period and, therefore, are not reported in the funds:		
Notes Payable	(2,356,669)	
SBITAs Payable	(44,158)	
Accrued Interest	(5,887)	
Compensated Absences	(394,540)	(2,801,254)
The net pension liability and related deferred outflows and inflows of resources are not recorded in the fund financial statements because they do not utilize current resources:		
Net Pension Liability	(2,000,629)	
Deferred Outflows of Resources	536,282	
Deferred Inflows of Resources	(467,908)	(1,932,255)
The other postemployment benefits liability is not recorded in the fund financial statements because it does not utilize current resources:		
Other Postemployment Benefit Liability		(125,328)
Total Net Position of Governmental Activities		$ 38,932,771

See accompanying notes to financial statements.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
STATEMENT OF REVENUES, EXPENDITURES, AND
CHANGES IN FUND BALANCES - GOVERNMENTAL FUNDS
FOR THE YEAR ENDED SEPTEMBER 30, 2025

	Major Funds					
	General	Capital Equipment	Capital Infrastructure Projects	Bridge Renewal & Replacement	Non-Major Governmental Fund	Total Governmental Funds
Revenues						
Non-Ad Valorem Assessments	$ 7,626,577	$ 34,966	$ -	$ 285,000	$ -	$ 7,946,543
Permits	-	311,312	-	-	-	311,312
Intergovernmental Revenues	-	-	52,835	-	-	52,835
Miscellaneous Revenue	41,654	1,500	60	-	-	43,214
Interest Income	241,344	6,380	32,898	52,882	20,547	354,051
Total Revenues	7,909,575	354,158	85,793	337,882	20,547	8,707,955
Expenditures						
Current:						
Physical Environment:						
Personnel Services	4,114,469	-	168,594	-	-	4,283,063
Operating Expenditures	1,680,128	-	149,589	341	-	1,830,058
Capital Outlay	-	414,035	861,574	-	-	1,275,609
Debt Service:						
Principal	12,351	-	-	-	782,653	795,004
Interest and Fiscal Charges	1,653	-	-	-	66,572	68,225
Total Expenditures	5,808,601	414,035	1,179,757	341	849,225	8,251,959
Excess (Deficiency) of Revenues Over (Under) Expenditures	2,100,974	(59,877)	(1,093,964)	337,541	(828,678)	455,996
Other Financing Sources (Uses)						
Transfers In	-	938,594	-	-	849,225	1,787,819
Transfers (Out)	(1,787,819)	-	-	-	-	(1,787,819)
Insurance Proceeds	-	53,595	-	-	-	53,595
Proceeds from the Sale of Capital Assets	-	58,716	1,121	-	-	59,837
Total Other Financing Sources (Uses)	(1,787,819)	1,050,905	1,121	-	849,225	113,432
Net Changes in Fund Balances	313,155	991,028	(1,092,843)	337,541	20,547	569,428
Fund Balances, Beginning of Year, As Previously Presented	5,535,431	(1,072,165)	1,367,200	-	1,851,397	7,681,863
Change Within Financial Reporting Entity (Non-Major to Major Fund)	-	-	-	1,015,260	(1,015,260)	-
Fund Balances, Beginning of Year, As Adjusted or Restated	5,535,431	(1,072,165)	1,367,200	1,015,260	836,137	7,681,863
Fund Balances, End of Year	$ 5,848,586	$ (81,137)	$ 274,357	$ 1,352,801	$ 856,684	$ 8,251,291

See accompanying notes to financial statements.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
RECONCILIATION OF THE STATEMENT OF REVENUES, EXPENDITURES,
AND CHANGES IN FUND BALANCES - GOVERNMENTAL FUNDS
TO THE STATEMENT OF ACTIVITIES
YEAR ENDED SEPTEMBER 30, 2025

Net Changes in Fund Balances - Total Governmental Funds		$ 569,428
The change in net position reported for governmental activities in the statement of activities is different because:		
Under the modified accrual basis of accounting used in governmental funds, revenues are recognized when they are earned, measurable, and available. In the statement of activities, however, which is presented on the accrual basis, revenues are recognized when they are earned and measurable.		754,594
Governmental funds report capital outlays as expenditures. However, in the statement of activities, the cost of capital assets acquired is depreciated over their estimated useful lives and reported as depreciation expense:		
Expenditures for Capital Assets	$ 1,415,921	
Net Book Value of Capital Assets Disposed	(79,464)	
(Less Current Year Depreciation)	(2,008,306)	(671,849)
Long-term liabilities are not due and payable in the current period and, therefore, are not reported in the governmental funds. In addition, principal payments are recorded as expenditures in the fund statements, and proceeds from debt issuances are recorded as other financing sources:		
Principal Payments		795,004
Under the modified accrual basis of accounting used in governmental funds, expenditures are not recognized for transactions that are not normally paid with expendable available financial resources. In the statement of activities, however, which is presented on the accrual basis, expenses and liabilities are reported regardless of when financial resources are available.		
Adjustments are as follows:		
Changes in Accrued Interest	(834)	
Changes in Compensated Absences	(1,557)	(2,391)
Change in the other postemployment liability does not require the use of current financial resources and, therefore, is not reported as an expenditure in governmental funds.		43,872
Changes in the net pension liability and pension related deferred outflows and inflows of resources do not require the use of current financial resources and, therefore, are not reported as expenditures in governmental funds.		134,094
Change in Net Position of Governmental Activities		$ 1,622,752

See accompanying notes to financial statements.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
NOTES TO THE FINANCIAL STATEMENTS

Note 1 - Summary of Significant Accounting Policies

Organization

The Lehigh Acres Municipal Services Improvement District (the District) is an independent special district in Lee and Hendry Counties, created June 10, 2015, under Chapter 2015-202, Laws of Florida, and Chapter 2017-216, Laws of Florida. The District is organized and exists for all purposes and shall hold all powers set forth in its charter and Chapters 189 and 197, Florida Statutes. The District charter may be amended only by special act of the Legislature. The District has the power to provide signage, drainage and water control, public improvements, and community facilities; to designate, create, implement, and maintain conservation, mitigation, and wildlife habitats; to fund, plan, establish, acquire, construct or reconstruct, enlarge or extend, equip, operate, and maintain systems, facilities, and basic infrastructure for conservation areas, mitigation areas, and wildlife habitat, including the maintenance of any plant or animal species, and any related interest in real or personal property.

This District is governed by an at-large elected five (5) member Board of Commissioners (the Board) serving staggered four (4) year terms.

Predecessor Entity

Prior to June 10, 2015, the District was known as the East County Water Control District (ECWCD), an independent special taxing district originally created pursuant to a decree of the Twelfth (12th) Circuit Court, Lee County, entered in Chancery Number 12,429 on May 5, 1958. Laws of Florida, Chapter 63-1549 reaffirmed the ECWCD's creation and provided authorization to levy and enforce the ECWCD's taxation power in July of 1963, as amended under the provisions of Florida Statutes, Chapter 298. The enabling legislation was repealed, reenacted, and codified by Laws of Florida, Chapter 2000-423, effective July 3, 2000. The ECWCD subsequently amended its codification via Laws of Florida, Chapter 2003-315, 2005-308, 2006-319, 2009-260, and 2012-254.

The ECWCD had been formed for the purpose of preserving and protecting water resources by drainage, reclamation, conservation, mitigation, irrigation, and water management in the eastern portion of Lee County and the western portion of Hendry County. The ECWCD also had authority under Laws of Florida, Chapter 67-901 to construct, maintain, and regulate navigational and boating facilities within the District. As a result of the codification (Laws of Florida, Chapter 2000-423), the ECWCD also had the authority to operate and maintain certain recreational parks within the District.

On June 10, 2015, the Legislature of the State of Florida passed an act, Chapter 2015-202, which dissolved ECWCD and created the District, primarily for the purpose of expanding the potential powers of the predecessor entity. According to the act, "As of the effective date of this act, all property, whether real, personal, or mixed, that is owned, possessed, or controlled by the ECWCD and all other assets, contracts, obligations, and liabilities of the ECWCD are hereby transferred and vested in the Lehigh Acres Municipal Services Improvement District. All contracts and obligations of the ECWCD existing on the effective date of this act shall remain in full force and effect, and this act shall in no way affect the validity of such contracts or obligations". Also, according to the act, "the members of the former Board of Commissioners of the ECWCD shall constitute the five-member Board of District Commissioners of the Lehigh Acres Municipal Services Improvement District, with existing 4-year terms of office to remain intact". On June 6, 2017, the Florida Legislature amended Chapter 2015-202, Laws of Florida, by enacting Chapter 2017-216, Laws of Florida, to expand the District boundaries.

Reporting Entity

The District's financial statements were prepared in accordance with Governmental Accounting Standards Board (GASB) Statements related to *The Financial Reporting Entity,* which establishes standards for defining and reporting on the financial reporting entity. The financial reporting entity is defined as the District, organizations for which the District is financially accountable, and other organizations for which the nature and significance of the relationship with the District are such that exclusion would cause the reporting entity's financial statements to be misleading or incomplete. The District is financially accountable for a component unit if it appoints a voting majority of the organization's governing board and is able to impose its will on that organization or there is a potential for the organization to provide specific financial benefits to, or impose specific financial burdens on, the District. Based on criteria established by GASB Statements, there are no potential component units required to be included in the District's financial statements.

Government-Wide and Fund Financial Statements

The basic financial statements consist of the government-wide financial statements, the fund financial statements, and the notes to the financial statements. The government-wide financial statements (the statement of net position and the statement of activities) report information on all of the non-fiduciary activities of the primary government. These activities are supported by general governmental and intergovernmental revenues.

The statement of activities demonstrates the degree to which the direct expenses of a given function are offset by program revenues. Direct expenses are those that are clearly identifiable with a specific function. Program revenues include: 1) charges to customers or applicants who purchase, use, or directly benefit from goods, services, or privileges provided by a given function; and 2) grants and contributions that are restricted to meeting the operational or capital requirements of a particular function. Taxes and other items not properly included among program revenues are reported instead as general revenues.

Separate financial statements are provided for governmental funds with the major individual funds reported in separate columns.

Measurement Focus and Basis of Accounting

Since the governmental fund financial statements are presented on a different measurement focus and basis of accounting than the government-wide statements' governmental activities column, a reconciliation is presented on the page following each statement, which briefly explains the adjustments necessary to reconcile the fund based financial statements to the governmental activities column of the government-wide presentation.

Government-Wide Financial Statements

Government-wide financial statements are reported using the economic resources measurement focus and the accrual basis of accounting. Under the accrual basis of accounting, revenues, expenses, gains, losses, assets, and liabilities resulting from exchange and exchange-like transactions are recognized when the exchange takes place. Revenues, expenses, gains, losses, assets, and liabilities resulting from non-exchange transactions are recognized in accordance with the requirements of GASB Statement No. 33, *Accounting and Financial Reporting for Non-Exchange Transactions*. Amounts paid to acquire capital assets are capitalized as assets in the government-wide financial statements, rather than reported as expenditures. Proceeds of long-term debt are recorded as liabilities in the government-wide financial

statements, rather than as other financing sources. Amounts paid to reduce long-term indebtedness of the reporting government are reported as a reduction of the related liability in the government-wide financial statements, rather than as expenditures. Revenues are recorded when earned and expenses are recorded when a liability is incurred, regardless of the timing of related cash flows. Non-ad valorem assessments are recognized as revenues in the year for which they are levied. Grants and similar items are recognized as revenue as soon as all eligibility requirements have been met.

Governmental Fund Financial Statements
Governmental fund financial statements are reported using the current financial resources measurement focus and the modified accrual basis of accounting.

Revenues are recognized as soon as they are both measurable and available. Revenues are considered to be available when they are collectible within the current period, or soon enough thereafter, to pay liabilities of the current period. For this purpose, the District considers revenues to be available if they are collected within sixty days of the end of the current fiscal period. Expenditures are generally recognized under the modified accrual basis of accounting when the related fund liability is incurred.

Exceptions to this general rule include principal and interest on long-term debt, as well as expenditures related to compensated absences, which are recognized when due.

Revenues susceptible to accrual are assessments, expenditure reimbursements, and intergovernmental revenues. These have been recognized as revenues in the period they meet the availability criteria. In addition, revenue from federal and state reimbursement type grants are recognized as revenue when the availability and eligibility requirements have been met. All other revenue items are considered to be measurable and available only when the District receives cash.

Major Funds and Basis of Presentation
The accounts of the District are organized on the basis of funds, each of which is considered a separate accounting entity. The operations of each fund are accounted for with a separate set of self-balancing accounts that comprise its assets, liabilities, fund balance, revenues, and expenditures. Government resources are allocated to and accounted for in individual funds based upon the purpose for which they are to be spent and the means by which spending activities are controlled.

The District's major funds are presented in separate columns on the governmental fund financial statements. The definition of a major fund is one that meets certain criteria set forth in GASB Statement No. 34, *Basic Financial Statements-and Management's Discussion and Analysis-for State and Local Governments*. The funds that do not meet the criteria of a major fund are considered non-major funds and are combined into a single column on the governmental fund financial statements.

The District reports the following major governmental funds:

- ***General Fund***—The General Fund is the District's primary operating fund. It accounts for all financial resources of the District, except those required to be accounted for in another fund.

- ***Capital Equipment Fund***—The Capital Equipment Fund is used to account for the accumulation of resources for capital equipment, maintenance, and replacement.

- ***Capital Infrastructure Projects Fund***—The Capital Infrastructure Projects Fund is used to account for financial resources to be used for the acquisition, construction, or improvement of major capital facilities or infrastructure.

- ***Bridge Renewal and Replacement Fund***—The Bridge Renewal and Replacement Fund is used to account for the accumulation of resources for bridge renewal and replacement.

Additionally, the District reports the following non-major funds:

- ***Debt Service Fund***—The Debt Service Fund is used to account for the accumulation of resources for the payment of long-term debt principal, interest, and related costs.

The effect of all interfund activity (transfers in/out and due from/due to) between governmental funds has been removed from the government-wide financial statements.

Amounts reported as program revenues include: 1) revenues generated by fees charged for permits that allow drainage into the District systems or for the use of District rights-of-way, 2) operating grants and contributions, and 3) capital grants and contributions. Internally dedicated resources are reported as general revenues rather than as program revenues. Likewise, general revenues include all assessments.

When both restricted and unrestricted resources are available for use, it is the District's policy to use restricted resources first, then unrestricted resources as they are needed, unless there are legal documents/contracts that prohibit doing this, such as in grant agreements requiring dollar for dollar spending. Additionally, the District would first use committed fund balance, followed by assigned fund balance, and then unassigned fund balance when expenditures are incurred for purposes for which amounts in any of the unrestricted fund balance classifications could be used.

Assets, Deferred Outflows, Liabilities, Deferred Inflows, and Net Position

Cash and Cash Equivalents
The District's cash and cash equivalents are considered to be cash on hand, demand deposits, and investments. The District's investments consist of the Florida State Board of Administration's (SBA) Florida PRIME (formerly known as the Local Government Surplus Funds Trust Fund Pooled Investment Account) and the Florida Cooperative Liquid Assets Securities System (FLCLASS). All investments are recorded at fair value unless the investment qualifies as an external investment pool under the guidance of GASB Statement No. 79, which allows under certain criteria, these investments to be recorded at amortized cost. The Florida PRIME Fund is an external investment pool reported at amortized cost, which approximates fair value. FLCLASS is an external investment pool reported at fair value.

Interfund Receivables and Payables
During the course of its operations, the District has numerous transactions between funds to construct or acquire assets and service debt. Transactions between funds that were not paid for or received as of September 30, 2025, have been recorded as due from and due to other funds in the fund financial statements.

Receivables
Accounts receivable are shown net of an allowance for uncollectible, when appropriate. Receivables, including due from other governments, are reported at the net realizable value. Leases receivable are reported at the present value of lease payments expected to be received during the lease term, reduced by any provision for estimated uncollectible amounts. There are no estimated uncollectible amounts at September 30, 2025. Receivables are further described in Note 4.

Prepaid Items
Certain payments to vendors reflect costs applicable to future accounting periods and are recorded as prepaid items in both government-wide and fund financial statements.

Restricted Assets
Certain assets of governmental funds are restricted as to use. Such assets, consisting primarily of cash and receivables, may include debt proceeds, state and federal grants, and amounts held for debt service.

Capital Assets
Capital assets, which include land, easements, buildings, land improvements, equipment, and intangible assets including subscription-based information technology arrangements (SBITAs) are reported in the government-wide financial statements in the statement of net position.

The District has adopted a minimum capitalization threshold for capital expenditures of $5,000. Assets purchased with a cost of less than $5,000 are included as operating expenditures in the appropriate fund.

All purchased capital assets are valued at cost where historical records are available and at an estimated historical cost where no historical records exist. The costs of substantially all capital assets acquired or constructed prior to 1998 have been estimated. Donated capital assets are valued at their estimated fair market value on the date donated. Public domain (infrastructure) capital assets consisting of certain improvements other than buildings, including canals, curbs, culverts, and excavations, are capitalized.

The District has segregated the cost of land under and surrounding its canals into the “land” category of capital assets and, as such, it is not depreciated. The cost of excavation of those canals is categorized as “infrastructure”. The District continually maintains its canals and drainage system and has elected to depreciate such system improvements over periods ranging 10-50 years.

Maintenance, repairs, and minor renovations are not capitalized. The acquisition of land and construction projects utilizing resources received from federal and state agencies are capitalized when the related expenditure is incurred.

Expenditures that materially increase values, change capacities, or extend useful lives are capitalized. Upon sale or retirement, the cost is eliminated from the respective accounts.

Expenditures for capital assets are recorded in the fund statements as current expenditures. However, such expenditures are not reflected as expenditures in the government-wide statements but rather capitalized and depreciated.

Depreciable capital assets are depreciated using the straight-line method over the following estimated useful lives:

Assets	Years
Buildings	10-50
Land Improvements	10-50
Equipment	4-25
Software	5
SBITAs	Contract Term

Compensated Absences

The District's employees accumulate annual leave based on the number of years of continuous service. Upon termination of employment, employees can receive payment of accumulated annual leave, sick leave benefits, personal time, and compensatory time if certain criteria are met. The costs of vacation, sick leave benefits, personal time, and compensatory time (compensated absences) are expended in the respective operating funds when payments are made but are accrued as incurred in the government-wide financial statements – statement of net position as non-current liabilities.

Long-Term Obligations

In the government-wide financial statements, long-term debt, and other long-term obligations, including subscription liabilities, are reported as liabilities in the governmental activities statement of net position, increased, or reduced, by related unamortized premiums or discounts. In the fund financial statements, the face amount of debt issued, plus any premium or net of any discount, is reported as other financing sources in the year of issuance.

Issuance costs are reported as debt service expenditures in the year of issuance.

Fund Balance

The District implemented the provisions of GASB Statement No. 54, *Fund Balance Reporting and Governmental Fund Type Definitions,* during the fiscal year ended September 30, 2011, as required. The purpose of GASB Statement No. 54 was to improve the consistency and usefulness of fund balance information to the financial statement user. The statement establishes fund balance classifications that comprise a hierarchy based primarily on the extent to which the organization is bound to honor constraints on the specific purpose for which amounts in the funds can be spent. Based on the extent to which these external or internal constraints have on the spending of fund balances, they are classified as the following:

- ***Non-Spendable Fund Balance***—Amounts that are: 1) not in spendable form or 2) legally or contractually required to be maintained intact.

- ***Restricted Fund Balance***—Amounts that have externally enforceable limitations on use.

- ***Committed Fund Balance***—Amounts that have self-imposed limitations (via Board Resolution) set in place prior to the end of the reporting period.

- ***Assigned Fund Balance***—Pursuant to Resolution 2011-08 adopted by the Board on July 18, 2011, assigned fund balances include "spendable fund balance amounts established by management of the District that are intended to be used for specific purposes and are neither considered restricted nor committed". In addition, residual balances in the capital projects and debt service funds are considered assigned for the general purpose of the respective fund.

- ***Unassigned Fund Balance***—The residual net resources for the general fund in excess of non-spendable, restricted, committed, and assigned fund balance (i.e., surplus). Unassigned fund balance may also include negative balances if expenditures exceed amounts restricted, committed, or assigned (i.e., deficit).

Minimum Level of Unassigned Fund Balance

Board Resolution 2011-08 adopted July 18, 2011, also specified certain actions to be taken when unassigned fund balance in the general fund fell below certain minimum levels. If, after the annual audit, prior committed or assigned fund balance causes the unassigned fund balance to fall below 25% of general fund operating expenditures, the District manager will so advise the Board in order for the necessary action to be taken to restore the unassigned fund balance to 25% of general fund operating expenditures. The District manager will prepare and submit a plan for committed and/or assigned fund balance reductions, expenditure reductions, and/or revenue increases to the Board. The District shall take action necessary to restore the unassigned fund balance to acceptable levels within two years.

Net Position

Equity in the government-wide statement of net position is displayed in three categories: 1) net investment in capital assets, 2) restricted, or 3) unrestricted. The net investment in capital assets component of net position consists of capital assets, net of accumulated depreciation reduced by the outstanding debt incurred to acquire, construct, or improve those assets. Deferred outflows of resources and deferred inflows of resources that are attributable to the acquisition, construction, or improvement of capital assets or related debt are also included in this component of net position. The restricted component of net position consists of restricted assets reduced by liabilities and deferred inflows of resources related to those assets. The unrestricted component of net position is the net amount of assets, deferred outflows of resources, liabilities, and deferred inflows of resources that are not included in the determination of net investment in capital assets or the restricted component of net position.

Encumbrances

Encumbrance accounting, under which purchase orders, contracts, and other commitments for the expenditure of monies are recorded as reservations of budget, is not employed by the District because presently, it is not necessary to assure effective budgetary or cash planning control.

Other Policies

Management Estimates

The preparation of basic financial statements in conformity with accounting principles generally accepted in the United States of America requires the District to make estimates and assumptions that affect the reported amounts of assets, liabilities, fund equity, and disclosure of contingent assets and liabilities at the date of the basic financial statements and the reported amounts of revenues and expenditures during the reporting period. Actual results could differ from those estimates.

Non-Ad Valorem Maintenance and Benefit Assessments

Maintenance assessments, Florida Statute 298.54, and benefit assessments, Florida Statute 298.36, are levied each November 1 after formal adoption of the District's budget and become due and payable upon receipt of the Notice of Levy. Non-ad valorem assessments are based on the size of real property parcels that are provided by the Lee County and Hendry County Property Appraisers. The current year's levy is based on 46,782 acres. Discounts are allowed for payment of the assessments prior to March 1 of the following year. Assessments become delinquent on April 1 and are subject to the issuance of tax sale certificates if unpaid by June 1.

The billing and collection of all maintenance and benefit assessments is performed for the District by the Tax Collectors for Lee and Hendry Counties. These non-ad valorem assessments are recognized as revenue when received from the Tax Collectors.

Under the provisions of Chapter 2015-202, Laws of Florida, and applicable provisions of Chapter 298 of the Florida Statutes, the Board levied a uniform maintenance tax of approximately $142.15 per acre of land within Lee County and Hendry County. The maintenance assessment proceeds are used by the District to pay the operating costs for the year ended September 30, 2025. These costs include stormwater operations, surveys, engineering, legal and accounting fees, and other expenses as judged necessary by the Board.

There were no non-ad valorem assessments levied under the provisions of Chapter 298.36, Florida Statutes, for the fiscal year ended September 30, 2025; instead, maintenance assessments are used to pay debt service for replacement of existing infrastructure.

Change Within the Reporting Entity
The District made a change to report the bridge renewal and replacement fund as a major fund when it was previously reported as non-major. This represents a change within the reporting entity, the result of which had no effect on the beginning fund balance of governmental funds, nor the net position for governmental activities appearing in the government-wide statements.

Note 2 - Stewardship, Compliance, and Accountability

Budgetary Information
Annual budgets are legally adopted for the general fund and the debt service fund. The adoption of the annual budget also includes capital project funds, for the respective year, along with approval of the five-year capital improvement plan.

Reports that compare the budget to actual results for all funds of the District are prepared and presented to the Board on a monthly basis to ensure control through fund management.

Budgets for all funds are prepared on a modified accrual basis of accounting. This means that revenues are recognized when they become measurable and available, and expenditures are recognized when they are incurred. Presentation of budget versus actual results is contained in the budgetary comparison schedules.

The District follows these procedures in establishing budgetary data for the general fund, debt service fund, and capital project funds:

- During the summer of each year, District management submits to the Board a proposed operating budget for the fiscal year commencing on October 1. The operating budget includes proposed expenditures and the means of financing them.

- Public hearings are conducted to obtain public comments.

- The budget is adopted by resolution of the Board.

- Budgets for all funds are prepared on a basis consistent with accounting principles generally accepted in the United States of America.

- Budget transfers can be made throughout the year between expenditure accounts by approval of the Board. The level of control for appropriations is exercised at the fund level.

- Budget amounts, as shown in these basic financial statements, are as originally adopted or as amended by the Board.

- Appropriations lapse at year-end.

- Supplemental appropriations made during the fiscal year are included in the "Final Budget" columns on the budgetary comparison schedules for all governmental funds. Supplemental budgetary appropriations were made during the year for the following:

    - A transfer of $543,521 was made in FY25 to reimburse the General Fund for items that were budgeted and expended in FY24. The FY25 budget was increased to authorize and cover this transfer.

Note 3 - Deposits, Cash Equivalents, and Investments

Cash Deposits

The District's bank deposits are held in qualified public depositories (QPDs) pursuant to Chapter 280, Florida Statutes. Under this Chapter, each QPD is required to pledge collateral to the State Treasurer against the public deposits. In the event of default by a QPD, all claims for public deposits would be satisfied by the State Treasurer from the proceeds of federal deposit insurance, pledged collateral of the QPD in default and, if necessary, assessments against other QPDs of the same type as the depositor in default. At September 30, 2025, cash totaled $2,185,354 (including cash on hand of $3,807). Cash equivalents at September 30, 2025, include investments in local government investment pools of $6,659,389. At September 30, 2025, the carrying amount of the District's cash and cash equivalents was $8,844,743, and the bank balances were $8,756,086. The difference is due to cash on hand and checks that were written but not yet paid by the bank.

Investments

The District had not adopted an investment policy as of September 30, 2025, but adopted one after the fiscal year-end in February 2026. Therefore, the District followed Chapter 218.415, Florida Statutes, to establish guidelines for efficient management of its cash reserves during fiscal year 2025. The District holds assets which are labeled on the balance sheet as cash equivalents but are defined as investments. The District's investments are recorded at fair value unless the investment qualifies as an external investment pool under the guidance of GASB Statement No. 79.

The District's investments at year-end are listed below, with ratings from Standards and Poor's. The District held no other types of investments during the year ended September 30, 2025.

	Maturity (WAM)	Amortized Cost	Credit Rating
Investments			
Investment Pools:			
Florida PRIME Local Government Surplus Trust	47 days	$ 1,701,596	AAAm
FLCLASS	30 days	4,957,793	AAAm
Total Investments		$ 6,659,389	

Florida PRIME:
The District holds investments in a qualified external investment pool that measures all of its investments at amortized cost. The above investments are recorded at amortized cost at September 30, 2025. Florida PRIME is a local government investment pool administered by the SBA and managed by professional money managers. Florida PRIME invests exclusively in short-term, high-quality fixed-income securities rated in the highest short-term rating category by one or more nationally recognized statistical rating agencies, or securities of comparable quality.

With regard to redemption gates, Chapter 218.409(8)(a), Florida Statutes, states, "The principal, and any part thereof, of each account constituting the trust fund is subject to payment at any time from the moneys in the trust fund. However, the Executive Director may, in good faith, on the occurrence of an event that has a material impact on liquidity or operations of the trust fund, for 48 hours limit contributions to or withdrawals from the trust fund to ensure that the Board can invest moneys entrusted to it in exercising its fiduciary responsibility. Such action must be immediately disclosed to all participants, the Trustees, the Joint Legislative Auditing Committee, and the Investment Advisory Council. The Trustees shall convene an emergency meeting as soon as practicable from the time the Executive Director has instituted such measures and review the necessity of those measures. If the Trustees are unable to convene an emergency meeting before the expiration of the 48-hour moratorium on contributions and withdrawals, the moratorium may be extended by the Executive Director until the Trustees are able to meet to review the necessity for the moratorium. If the Trustees agree with such measures, the Trustees shall vote to continue the measures for up to an additional 15 days. The Trustees must convene and vote to continue any such measures before the expiration of the time limit set, but in no case may the time limit set by the Trustees exceed 15 days".

With regard to liquidity fees, Florida Statute Section 218.409(4) provides authority for the SBA to impose penalties for early withdrawal, subject to disclosure in the enrollment materials of the amount and purpose of such fees. At present, no such disclosure has been made.

As of September 30, 2025, there were no redemption fees, maximum transaction amounts, or any other requirements that serve to limit a participant's daily access to 100 percent of their account value.

FLCLASS:
The FLCLASS is an independent local government investment pool designed to provide a safe and competitive investment alternative for Florida governmental entities. FLCLASS does not meet the requirements of GASB 79 and is managed as a stable value net asset value pool, which approximates fair value. FLCASS measures its investments at fair value and, therefore, investments in the pools are not required to be categorized within the fair value hierarchy for purposes of Paragraph 81a(2) of GASB Statement No. 72.

Foreign Currency Risk—The pool was not exposed to foreign currency risk during the year.

Custodial Credit Risk—For an investment, custodial credit risk is the risk that, in the event of the failure of the counterparty, the District will not be able to recover the value of the investments or collateral securities that are in the possession of an outside party. The District investments are not evidenced by securities that exist in physical or book entry form. Therefore, the custodial credit risk associated with these investments is minimal.

Credit Risk—For investments, credit risk is generally the risk that an issuer of an investment will not fulfill its obligation to the holder of the investment. This is measured by the assignment of a rating by a nationally recognized statistical rating organization. Investment ratings by investment type are included in the preceding summary of investments.

Concentration Risk—The District places no limit on the amount the District may invest in any one issuer but emphasizes diversifying the investment portfolio so that the impact of potential losses from any one type of security or from any one individual issuer will be minimized.

Interest Rate Risk—Interest rate risk is the risk that changes in interest rates will adversely affect the fair value of an investment. The District minimizes this risk by structuring the portfolio so that securities mature to meet cash flow requirements for ongoing operations and thereby avoiding the need to sell securities on the open market prior to maturity.

Note 4 - Receivables and Accrued Liabilities

Receivables as of September 30, 2025, for the District's governmental activities are as follows:

Receivables	
Lease	$ 654,869
Intergovernmental	927,842
Other Receivables	9,479
Total Receivables	$ 1,592,190

Lease receivables totaling $654,869 at September 30, 2025, includes the present value of lease payments expected to be received during the lease term, in which the District leases a parcel of land, including the air space above it, for the placement of a communication facility (cell tower). The term of this agreement is 5 years with 9 renewal options for an additional 5 years each, which the lessee is reasonably expected to utilize. Each year, the monthly rent, starting at $1,800 a month, increases by 2%.

Lease revenue and interest income recognized in relation to this lease for the year ended September 30, 2025, is $13,257 and $17,974, respectively.

Accrued liabilities at September 30, 2025, were as follows:

	General Fund
Accrued Liabilities	
Payroll	$ 197,878
Total Accrued Liabilities	$ 197,878

Note 5 - Capital Assets

The following is a summary of changes in capital asset activity for the year ended September 30, 2025:

	Balance at October 1, 2024	Additions	Deletions	Balance at September 30, 2025
Capital Assets Not Being Depreciated				
Land (Including Canals)	$ 3,290,932	$ -	$ -	$ 3,290,932
Intangibles - Easements	1,297	-	-	1,297
Construction in Progress	5,054,614	1,001,886	(150,247)	5,906,253
Total Capital Assets Not Being Depreciated	8,346,843	1,001,886	(150,247)	9,198,482
Capital Assets Being Depreciated				
Buildings	2,062,036	-	-	2,062,036
Land Improvements	60,497,151	150,247	-	60,647,398
Equipment	7,929,491	414,035	(446,984)	7,896,542
Intangibles - Software	55,633	-	-	55,633
Right-to-Use Subscription Asset	80,256	-	-	80,256
Total Capital Assets Being Depreciated	70,624,567	564,282	(446,984)	70,741,865
Less Accumulated Depreciation				
Buildings	(619,074)	(41,080)	-	(660,154)
Land Improvements	(38,417,222)	(1,406,785)	-	(39,824,007)
Equipment	(4,479,675)	(547,230)	367,520	(4,659,385)
Intangibles - Software	(54,908)	(539)	-	(55,447)
Right-to-Use Subscription Asset	(25,344)	(12,672)	-	(38,016)
Total Accumulated Depreciation	(43,596,223)	(2,008,306)	367,520	(45,237,009)
Total Capital Assets Being Depreciated, Net	27,028,344	(1,444,024)	(79,464)	25,504,856
Capital Assets, Net	$ 35,375,187	$ (442,138)	$ (229,711)	$ 34,703,338

Depreciation expense in the amount of $2,008,306, was reported as a separate line item in the statement of activities.

SBITAs

The District has one subscription arrangement that requires recognition under GASB Statement No. 96 and is labeled above as a Right-to-Use Subscription Asset. Under the standard, this asset was required to be restated as of October 1, 2022, and has an original cost of $80,256. The subscription arrangement is a ten-year agreement and lasts through fiscal year 2029. The asset is required to be amortized over the term of the contract, as that is considered to be the useful life of the asset. The accumulated amortization as of September 30, 2025, was $38,016 and approximately 3 years remain on the contract. There is a related subscription liability described further in Note 6.

Construction Commitments

The District has several construction projects in progress as of September 30, 2025. The District's commitments at year-end are as follows:

	Expended - To-Date	Remaining Commitment
Construction Projects		
Robust Phase II	$ 211,894	$ 750,107
GS-10 Caloosahatchee Crosslink Project	34,585	1,147,284
Total Construction Projects	$ 246,479	$ 1,897,391

Note 6 - Long-Term Debt

Notes Payable

All of the District's notes payable as of September 30, 2025, arose through direct borrowings.

Notes payable outstanding at September 30, 2025, consist of the following for governmental activities:

	Collateral[(1)]	Purpose of Issue	Amount Issued	Amount Outstanding	Interest Rate	Maximum Annual Debt Service
FDEP 2005 SW109030 Direct Borrowing	Lien on Maintenance Taxes	Construction	$ 2,146,951	$ 534,748	2.94%	$ 142,680
FDEP SW109041 Direct Borrowing	Lien on Maintenance Taxes	Construction	1,913,708	519,360	2.68%	123,285
Series 2017 Direct Borrowing	Lien on Maintenance Taxes	Construction	5,457,854	1,302,561	2.00%	583,260
Total			$ 9,518,513	$ 2,356,669		

(1) All notes are secured by pledged revenues of maintenance taxes less related maintenance expenses, which amount to $2,151,605.

The following is a summary of changes in long-term debt for the fiscal year ended September 30, 2025:

	Balance October 1, 2024	Additions	Reductions	Balance September 30, 2025	Amounts Due Within One Year
Notes Payable	$ 3,139,321	$ -	$ (782,652)	$ 2,356,669	$ 800,350
SBITAs Payable	56,509	-	(12,351)	44,158	12,757
Compensated Absences	392,983	315,950	(314,393)	394,540	174,968
Net Pension Liability	2,359,266	-	(358,637)	2,000,629	-
Other Postemployment Benefits	169,200	16,104	(59,976)	125,328	6,191
	$ 6,117,279	$ 332,054	$ (1,528,009)	$ 4,921,324	$ 994,266

The following are the annual requirements to service the notes payable outstanding at September 30, 2025:

Year Ending September 30,	Principal	Interest	Total
2026	$ 800,350	$ 48,874	$ 849,224
2027	818,459	30,766	849,225
2028	418,189	14,891	433,080
2029	258,847	7,117	265,964
2030	60,824	815	61,639
Total	$ 2,356,669	$ 102,463	$ 2,459,132

SBITAs

The District has one right-to-use subscription through December 2028 based on a ten-year non-cancellable agreement, which is required to be recognized under GASB Statement No. 96. As of September 30, 2025, the value of the subscription liability was $44,158. The District is required to make annual principal and interest payments of $14,004. The SBITA is discounted using a rate of 3.25%, which is based on the S&P Municipal Bond 20-Year High Grade Rate Index published by S&P Dow Jones Indices. This rate was used as an estimate of the interest rate the District would have to pay to borrow funds for a similar term and amount.

The following are the annual requirements to service the SBITAs outstanding at September 30, 2025:

Year Ending September 30,	Principal	Interest	Total
2026	$ 12,757	$ 1,247	$ 14,004
2027	13,182	822	14,004
2028	13,614	390	14,004
2029	4,605	27	4,632
Total	$ 44,158	$ 2,486	$ 46,644

Note 7 - Interfund Activity

Interfund Transfers

Interfund transfers at September 30, 2025, are as follows:

Transfer-Out Fund	Transfer-In Fund: Capital Equipment	Transfer-In Fund: Debt Service	Total
General	$ 938,594	$ 849,225	$ 1,787,819
Total Transfers	$ 938,594	$ 849,225	$ 1,787,819

Transfers are used to move revenues from the fund with collection authorization to the debt service fund for future debt service principal and interest payments.

Interfund Receivables and Payables

Interfund Due To and Due From at September 30, 2025, are as follows:

Due From Funds	Due To Funds: General	Total
Capital Infrastructure Projects	$ 221,783	$ 221,783
Capital Equipment	226,463	226,463
Bridge Renewal & Replacement	10,528	10,528
Total	$ 458,774	$ 458,774

The interfund amounts are primarily the result of: 1) exchange activities for labor and equipment across funds. These amounts are expected to be repaid within one year.

Note 8 - Risk Management

The District is exposed to various risks of loss related to torts; theft of, damage to, and destruction of assets; errors and omissions; and natural disasters for which the District carries commercial insurance.

The District has also purchased commercial excess coverage for workers' compensation which covers claims as presented below:

	Amount
Each Accident	$ 1,000,000
Each Employee	1,000,000
Policy Limit	1,000,000

Note 9 - Commitments and Contingencies

Litigation, Claims, and Assessments
The District is involved from time to time in certain routine litigation, the substance of which, either as liabilities or recoveries, would not materially affect the financial position of the District. Although the final outcome of the lawsuits and claims or the exact amount of costs and/or potential recovery is not presently determinable, in the opinion of the District's legal counsel, the resolution of these matters will not have a materially adverse effect on the financial condition of the District. The District plans to contest these matters unless first settled. Potential losses, if any, may be recovered through third-party insurance.

Grants
Amounts received from grant agencies are subject to audit and adjustment by grantor agencies, principally the federal government. Any disallowed claims, including amounts already collected, may constitute a liability of the applicable funds. The amount, if any, of expenditures that may be disallowed by the grantor cannot be determined at this time, although the District expects such amounts, if any, to be immaterial.

Note 10 - Employee Retirement Plan

General Information About the Florida Retirement System (FRS)
The FRS was created in Chapter 121, Florida Statutes, to provide a defined benefit pension plan for participating public employees. The FRS was amended in 1998 to add the Deferred Retirement Option Program (DROP) under the defined benefit plan and amended in 2000 to provide a defined contribution plan alternative to the defined benefit plan for FRS members effective July 1, 2002. This integrated defined contribution pension plan is the FRS Investment Plan. Chapter 112, Florida Statutes, established the Retiree Health Insurance Subsidy (HIS) Program, a cost-sharing multiple-employer defined benefit pension plan, to assist retired members of any state-administered retirement system in paying the costs of health insurance.

Essentially all regular employees of the District are eligible to enroll as members of the state-administered FRS. Provisions relating to the FRS are established by Chapters 121 and 122, Florida Statutes; Chapter 112 Part IV, Florida Statutes; Chapter 238, Florida Statutes; and FRS Rules, Chapter 60S, *Florida Administrative Code*; wherein eligibility, contributions, and benefits are defined and described in detail. Such provisions may be amended at any time by further action from the Florida Legislature. The FRS is a single retirement system administered by the Florida Department of Management Services, Division of Retirement, and consists of two cost-sharing, multiple-employer defined benefit plans and other non-integrated programs. An annual comprehensive financial report of the FRS, which includes its financial statements, required supplementary information, actuarial report, and other relevant information, is available from the Florida Department of Management Services' web site (www.dms.myflorida.com).

The District's contributions totaled $425,997 for the fiscal year ended September 30, 2025 (all plans).

Payables to the Pension Plan
The District reported a payable of $20,468 for the outstanding amount of contributions to the plan required for the fiscal year ended September 30, 2025.

FRS Pension Plan

Plan Description
The FRS Pension Plan (the Plan) is a cost-sharing, multiple-employer qualified defined benefit pension plan with a DROP available for eligible employees. The general classes of membership applicable to the District are as follows:

- *Regular Class* – Members of the FRS who do not qualify for membership in the other classes.
- *Senior Management Service Class* – Members in senior management level positions.
- *Elected Officers Class* – Elected officers.

Employees enrolled in the Plan prior to July 1, 2011, vest at six years of creditable service and employees enrolled in the Plan on or after July 1, 2011, vest at eight years of creditable service. All vested members enrolled prior to July 1, 2011, are eligible for normal retirement benefits at age 62 or at any age after 30 years of service, except for members classified as special risk who are eligible for normal retirement benefits at age 55 or at any age after 25 years of service. All members enrolled in the Plan on or after July 1, 2011, once vested, are eligible for normal retirement benefits at age 65 or any time after 33 years of creditable service, except for members classified as special risk who are eligible for normal retirement benefits at age 60 or at any age after 30 years of service. Members of the Plan may include up to 4 years of credit for military service toward creditable service.

The Plan also includes an early retirement provision; however, there is a benefit reduction for each year a member retires before his or her normal retirement date. The Plan provides retirement, disability, death benefits, and annual cost-of-living adjustments to eligible participants.

DROP, subject to provisions of Section 121.091, Florida Statutes, permits employees eligible for normal retirement under the Plan to defer receipt of monthly benefit payments while continuing employment with an FRS employer. An employee may participate in DROP for a period not to exceed 96 months after electing to participate, except that certain instructional personnel may extend beyond 96 months. During the period of DROP participation, deferred monthly benefits are held in the FRS Trust Fund and accrue interest. The net pension liability does not include amounts for DROP participants, as these members are considered retired and are not accruing additional pension benefits.

Benefits Provided
Benefits under the Plan are computed on the basis of age and/or years of service, average final compensation, and service credit. Credit for each year of service is expressed as a percentage of the average final compensation. For members initially enrolled before July 1, 2011, the average final compensation is the average of the five highest fiscal years' earnings; for members initially enrolled on or after July 1, 2011, the average final compensation is the average of the eight highest fiscal years' earnings. The total percentage value of the benefit received is determined by calculating the total value of all

service, which is based on the retirement class to which the member belonged when the service credit was earned. Members are eligible for in-line-of-duty or regular disability and survivors' benefits. The following chart shows the percentage value for each year of service credit earned:

Class, Initial Enrollment, and Retirement Age/Years of Service	Percent Value
Regular Class Members Initially Enrolled Before July 1, 2011:	
Retirement up to age 62 or up to 30 years of service	1.60
Retirement at age 63 or with 31 years of service	1.63
Retirement at age 64 or with 32 years of service	1.65
Retirement at age 65 or with 33 or more years of service	1.68
Regular Class Members Initially Enrolled on or After July 1, 2011:	
Retirement up to age 65 or up to 33 years of service	1.60
Retirement at age 66 or with 34 years of service	1.63
Retirement at age 67 or with 35 years of service	1.65
Retirement at age 68 or with 36 or more years of service	1.68
Senior Management Service Class	2.00
Elected Officers Class – Others	3.00

As provided in Section 121.101, Florida Statutes, if the member is initially enrolled in the FRS before July 1, 2011, and all service credit was accrued before July 1, 2011, the annual cost-of-living adjustment is 3% per year. If the member is initially enrolled before July 1, 2011, and has service credit on or after July 1, 2011, there is an individually calculated cost-of-living adjustment. The annual cost-of-living adjustment is a proportion of 3% determined by dividing the sum of the pre-July 2011 service credit by the total service credit at retirement multiplied by 3%. Plan members initially enrolled on or after July 1, 2011, will not have a cost-of-living adjustment after retirement.

Contributions

The Florida Legislature establishes contribution rates for participating employers and employees. Contribution rates during the District's 2024-2025 fiscal year were as follows:

	Year Ended June 30, 2025 Percent of Gross Salary		Year Ended June 30, 2026 Percent of Gross Salary	
Class	Employee	Employer	Employee	Employer
FRS, Regular	3.00	11.57	3.00	11.97
FRS, Senior Management Services	3.00	32.46	3.00	31.18
Elected Officers	3.00	56.62	3.00	52.51
DROP – Applicable to Members from All of the Above Classes	0.00	19.13	0.00	20.02
FRS, Reemployment Retiree	(1)	(1)	(1)	(1)

Note: (1) Contribution rates are dependent upon retirement class in which reemployed.

The District's contributions to the Plan totaled $239,996 for the fiscal year ended September 30, 2025. This excludes the HIS defined benefit pension plan contributions.

Pension Liabilities, Pension Expense, Deferred Outflows of Resources, and Deferred Inflows of Resources Related to Pensions

At September 30, 2025, the District reported a liability of $1,241,534 for its proportionate share of the net pension liability. The net pension liability was measured as of June 30, 2025, and the total pension liability used to calculate the net pension liability was determined by an actuarial valuation as of July 1, 2025. The District's proportionate share of the net pension liability was based on the District's 2024-2025 fiscal year contributions relative to the fiscal year 2024-2025 fiscal year contributions of all participating members. At June 30, 2025, the District's proportion was 0.00400041485%, which was an increase of 0.00009723120% from its proportion measured as of June 30, 2024.

For the year ended September 30, 2025, the District recognized pension expense of $126,825 related to the Plan. At September 30, 2025, the District reported deferred outflows of resources and deferred inflows of resources related to pensions from the following sources:

Description	Deferred Outflows of Resources	Deferred Inflows of Resources
Employer Contributions after Measurement Date	$ 55,381	$ -
Difference Between Expected and Actual Experience	132,609	-
Changes of Assumptions	144,174	-
Changes in Proportion and Difference Between District Contributions and Proportionate Share of Contributions	79,911	48,371
Net Difference Between Projected and Actual Earnings on Pension Plan Investments	-	207,287
Total	$ 412,075	$ 255,658

The deferred outflows of resources related to pensions resulting from District contributions subsequent to the measurement date, totaling $55,381 will be recognized as a reduction of the net pension liability in the subsequent fiscal year. Other amounts reported as deferred outflows of resources and deferred inflows of resources related to pensions will be recognized in pension expense as follows:

Fiscal Year Ending	Amount
2026	$ 212,003
2027	(34,436)
2028	(42,053)
2029	(34,478)
Total	$ 101,036

Actuarial Assumptions

The total pension liability in the July 1, 2025 actuarial valuation was determined using the individual entry age cost method, and the following actuarial assumptions:

Inflation	2.40%
Salary Increases	3.50% Average, Including Inflation
Discount Rate	6.70%
Long-Term Expected Rate of Return, Net of Investment Expense	6.70%

Mortality rates were based on the PUB-2010 based table, which varies by member category and sex, projected generationally with Scale MP-2021.

The actuarial assumptions used in the July 1, 2025 valuation were based on the results of an actuarial experience study for the period July 1, 2018 through June 30, 2023.

The long-term expected rate of return on pension plan investments was not based on historical returns but instead is based on a forward-looking capital market economic model. The allocation policy's description of each asset class was used to map the target allocation to the asset classes shown below. Each asset class assumption is based on a consistent set of underlying assumptions and includes an adjustment for the inflation assumption.

The target allocation and best estimates of arithmetic and geometric real rates of return for each major asset class are summarized in the following table:

Asset Class	Target Allocation (1)	Annual Arithmetic Return	Compound Annual (Geometric) Return	Standard Deviation
Cash	1.0%	3.2%	3.2%	1.1%
Fixed Income	29.0%	5.5%	5.4%	4.0%
Global Equity	45.0%	8.5%	6.9%	18.3%
Real Estate	12.0%	8.4%	7.1%	16.8%
Private Equity	11.0%	12.4%	8.8%	28.4%
Strategic Investments	2.0%	6.5%	6.1%	8.7%
Total	100%			
Assumed Inflation – Mean			2.4%	1.5%

Note: (1) As Outlined in the Plan's Investment Policy.

Discount Rate

The discount rate used to measure the total pension liability was 6.70%. The Plan's fiduciary net position was projected to be available to make all projected future benefit payments of current active and inactive employees. Therefore, the discount rate for calculating the total pension liability is equal to the long-term expected rate of return. The discount rate was 6.70% in the July 1, 2025 valuation.

Sensitivity of the District's Proportionate Share of the Net Position Liability to Changes in the Discount Rate

The following presents the District's proportionate share of the net pension liability calculated using the discount rate of (6.70%), as well as what the District's proportionate share of the net pension liability would be if it were calculated using a discount rate that is 1-percentage-point lower (5.70%) or 1-percentage-point higher (7.70%) than the current rate:

FRS – District:

	1% Decrease (5.70%)	Current Discount Rate (6.70%)	1% Increase (7.70%)
District's Proportionate Share of the Net Pension Liability	$ 2,436,491	$ 1,241,534	$ 239,699

Pension Plan Fiduciary Net Position
Detailed information about the Plan's fiduciary net position is available in the separately issued FRS Pension Plan and Other State Administered Systems Annual Comprehensive Financial Report.

HIS Pension Plan

Plan Description
The HIS Pension Plan (HIS Plan) is a cost-sharing, multiple-employer defined benefit pension plan established and administered in accordance with Section 112.363, Florida Statutes. The Florida Legislature established and amends the contribution requirements and benefit terms of the HIS Plan. The benefit is a monthly payment to assist retirees of state-administered retirement systems in paying their health insurance costs.

Benefits Provided
For the fiscal year ended June 30, 2025, eligible retirees and beneficiaries received a monthly HIS payment equal to the number of years of service credited at retirement multiplied by $7.50. The minimum payment is $45 and the maximum payment is $225 per month, pursuant to Section 112.363, Florida Statutes. To be eligible to receive a HIS Plan benefit, a retiree under one of the state-administered retirement systems must provide proof of health insurance coverage, which may include Medicare.

Contributions
The HIS Plan is funded by required contributions from FRS participating employers as set by the Florida Legislature. Employer contributions are a percentage of gross compensation for all active FRS members. For the HIS Plan fiscal year ended June 30, 2025, the contribution rate was 2.00% of payroll, pursuant to Section 112.363, Florida Statutes. The District contributed 100% of its statutorily required contributions for the current and preceding 4 years. HIS Plan contributions are deposited in a separate trust fund from which payments are authorized. HIS Plan benefits are not guaranteed and are subject to annual legislative appropriation.

In the event the legislative appropriation or available funds fail to provide full subsidy benefits to all participants, benefits may be reduced or canceled.

The District's contributions to the HIS Plan totaled $53,206 for the fiscal year ended September 30, 2025.

Pension Liabilities, Pension Expense, Deferred Outflows of Resources, and Deferred Inflows of Resources Related to Pensions
At September 30, 2025, the District reported a net pension liability of $759,095 for its proportionate share of the HIS Plan's net pension liability. The net pension liability was measured as of June 30, 2025, and the total pension liability used to calculate the net pension liability was determined by an actuarial valuation as of July 1, 2025. The District's proportionate share of the net pension liability was based on the District's 2024-2025 fiscal year contributions relative to the total 2024-2025 fiscal year contributions of all participating members. At June 30, 2025, the District's proportionate share was 0.00592235856%, which was an increase of 0.00026052061% from its proportionate share measured as of June 30, 2024.

For the fiscal year ended September 30, 2025, the District recognized pension expense of $91,951 related to the HIS Plan. In addition, the District reported deferred outflows of resources and deferred inflows of resources related to pensions from the following sources:

Description	Deferred Outflows of Resources	Deferred Inflows of Resources
Employer Contributions After Measurement Date	$ 11,936	$ -
Difference Between Expected and Actual Experience	4,531	1,204
Changes of Assumptions	6,719	183,606
Changes in Proportion and Difference Between District Contributions and Proportionate Share of Contributions	101,021	26,808
Net Difference Between Projected and Actual Earnings on Pension Plan Investments	-	632
Total	$ 124,207	$ 212,250

The deferred outflows of resources related to pensions, resulting from District contributions to the HIS Plan subsequent to the measurement date totaling $11,936, will be recognized as a reduction of the net pension liability in the subsequent fiscal year. Other amounts reported as deferred outflows of resources and deferred inflows of resources related to pensions will be recognized in pension expense as follows:

Fiscal Year Ending	Amount
2026	$ (24,879)
2027	(29,230)
2028	(23,174)
2029	(12,496)
2030	(10,199)
Total	$ (99,978)

Actuarial Assumptions

The total pension liability in the July 1, 2025 actuarial valuation was determined using the individual entry age cost method, and the following actuarial assumptions:

Inflation	2.40%
Salary Increases	3.50% Average, Including Inflation
Discount Rate	5.20%
Municipal Bond Index	5.20%

Mortality rates were based on the Generational PUB-2010 with Projection Scale MP-2021.

The actuarial assumptions used in the July 1, 2025 valuation were based on the results of an actuarial experience study for the period July 1, 2018 through June 30, 2023.

Discount Rate

The discount rate used to measure the total pension liability was 5.20%. In general, the discount rate for calculating the total pension liability is equal to the single rate equivalent to discounting at the long-term expected rate of return for benefit payments prior to the projected depletion date. Because the HIS

benefit is essentially funded on a pay-as-you-go basis, the depletion date is considered to be immediate, and the single equivalent discount rate is equal to the municipal bond rate selected by the HIS Plan sponsor. The Bond Buyer General Obligation 20-Bond Municipal Bond Index was adopted as the applicable municipal bond index.

Sensitivity of the District's Proportionate Share of the Net Pension Liability to Changes in the Discount Rate
The following presents the District's proportionate share of the net pension liability calculated using the discount rate of (5.20%), as well as what the District's proportionate share of the net pension liability would be if it were calculated using a discount rate that is 1-percentage-point lower (4.20%) or 1-percentage-point higher (6.20%) than the current rate:

	1% Decrease (4.20%)	Current Discount Rate (5.20%)	1% Increase (6.20%)
District's Proportionate Share of the Net Pension Liability	$ 856,002	$ 759,095	$ 677,821

Pension Plan Fiduciary Net Position
Detailed information about the HIS Plan's fiduciary net position is available in the separately issued FRS Pension Plan and Other State Administered Systems Annual Comprehensive Financial Report.

FRS – Defined Contribution Pension Plan
The District contributes to the FRS Investment Plan (the Investment Plan), a defined contribution pension plan, for its eligible employees electing to participate in the Investment Plan. The Investment Plan is administered by the SBA and is reported in the SBA's annual financial statements and in the State of Florida Annual Comprehensive Financial Report. Service retirement benefits are based upon the value of the member's account upon retirement.

As provided in Section 121.4501, Florida Statutes, eligible FRS members may elect to participate in the Investment Plan in lieu of the FRS defined benefit plan. District employees participating in DROP are not eligible to participate in the Investment Plan. Employer and employee contributions, including amounts contributed to individual member's accounts, are defined by law, but the ultimate benefit depends in part on the performance of investment funds. Benefit terms, including contribution requirements, for the Investment Plan are established and may be amended by the Florida Legislature. The Investment Plan is funded with the same employer and employee contribution rates that are based on salary and membership class (Regular Class, Elected County Officers, etc.), as the FRS defined benefit plan contributions are directed to individual member accounts, and the individual members allocate contributions and account balances among various approved investment choices. Allocations to the investment member's accounts during the 2024-2025 fiscal year were as follows:

	Year Ended June 30, 2024 Percent of Gross Compensation		Year Ended June 30, 2025 Percent of Gross Compensation	
Class	Employee	Employer	Employee	Employer
FRS, Regular Class	3.00	8.30	3.00	8.30
FRS, Senior Management Service Class	3.00	9.70	3.00	9.70
FRS, Elected County Officers	3.00	13.34	3.00	13.34

For all membership classes, employees are immediately vested in their own contributions and are vested after one year of service for employer contributions and investment earnings. If an accumulated benefit obligation for service credit originally earned under the FRS Pension Plan is transferred to the Investment Plan, the member must have the years of service required for FRS Pension Plan vesting (including the service credit represented by the transferred funds) to be vested for these funds and the earnings on the funds. Non-vested employer contributions are placed in a suspense account for up to five years. If the employee returns to FRS-covered employment within the five-year period, the employee will regain control over their account. If the employee does not return within the five-year period, the employee will forfeit the accumulated account balance. Costs of administering the Investment Plan, including the FRS Financial Guidance Program, are funded through employer contributions of 0.06% of payroll and by forfeit benefits of Investment Plan members. For the fiscal year ended September 30, 2025, the information for the amount of forfeitures was unavailable from the SBA; however, management believes that these amounts, if any, would be immaterial to the District.

After termination and applying to receive benefits, the member may rollover vested funds to another qualified plan, structure a periodic payment under the Investment Plan, receive a lump-sum distribution, leave the funds invested for future distribution, or any combination of these options. Disability coverage is provided; the member may either transfer the account balance to the FRS Pension Plan when approved for disability retirement to receive guaranteed lifetime monthly benefits under the FRS Pension Plan or remain in the Investment Plan and rely upon that account balance for retirement income.

The District's Investment Plan contributions totaled $133,126 for the fiscal year ended September 30, 2025.

Note 11 - Deferred Compensation Plan

The District offers its employees a deferred compensation plan created in accordance with Internal Revenue Service (IRS) Code Section 457. The plan, available to all District employees, permits them to defer a portion of their salary until future years. The deferred compensation is not available to employees until termination, retirement, death, or hardship distribution criteria as defined in IRS Code Section 457. Because the assets of the plan are held in trust and are the sole property of the participants, no balances or financial information relative to the plan is reported in the basic financial statements.

Note 12 - Other Postemployment Benefits (OPEB)

Plan Description
The District's Retiree Health Care Plan (OPEB Plan) is a single-employer defined benefit postemployment health care plan that covers eligible retired employees of the District. The OPEB Plan, which was established by practice, is administered by the District and allows employees who retire and meet retirement eligibility requirements under one of the District's retirement plans to continue medical insurance coverage as a participant in the District's plan. The OPEB Plan does not issue a stand-alone report.

Participant data as of the most recent actuarial valuation date is shown below:

	September 30, 2024
Inactive Plan Members or Beneficiaries Currently Receiving Benefits	3
Active Employees	38
Total	41

Benefits Provided

The benefits provided are the same as those provided for active employees. Spouses and dependents of eligible retirees are also eligible for medical coverage. All employees of the District are eligible to receive postemployment health care benefits. The retiree is responsible for paying the entire monthly premium for healthcare coverage and that of any covered spouse or eligible dependents.

Funding Policy

For the OPEB Plan, contribution requirements of the District are established and may be amended through action from the Board of District Commissioners. Currently, the District's OPEB benefits are unfunded. The required contributions are based on pay-as-you-go financing requirements. There is no separate trust fund or equivalent arrangement into which the District would make contributions to advance-fund the obligation, as it does for its pension plan, the FRS. Therefore, ultimate subsidies which are provided over time are financed directly by general assets of the District, which are invested in very short-term income instruments.

Actuarial Methods and Assumptions

In any long-term actuarial valuation, certain demographic, economic, and behavioral assumptions are made concerning the population, the investment discount rates, and the benefits provided. These actuarial assumptions form the basis for the actuarial model which is used to project the future population, the future benefits provided, and the contributions collected. Then the investment discount rate assumption is used to discount those projected net OPEB benefits to a present value. This and other related present values are used to calculate the annual OPEB cost.

The Total OPEB Liability was determined by an actuarial valuation as of September 30, 2024, using the following actuarial assumptions:

Inflation	2.50%
Salary Increases	2.50%
Discount Rate	4.06%
Initial Trend Rate	6.75%
Ultimate Trend Rate	4.00%
Years to Ultimate	50

For all lives, mortality rates were PubG-2010 Mortality Tables projected to the valuation date using Projection Scale MP-2020.

Given the District's decision not to fund the program, all future benefit payments were discounted using a high-quality municipal bond rate of 4.06%. The high-quality municipal bond rate was based on the measurement date of the S&P Municipal Bond 20 Year High Grade Rate Index as published by S&P Dow Jones Indices. The S&P Municipal 20 Year High Grade Rate Index consists of bonds in the S&P Municipal Bond Index with a maturity of 20 years. The average rating of the 20 bonds is roughly equivalent to Moody's Investors Service's Aa2 rating and Standard & Poor's Corp.'s AA rating.

Total OPEB Liability

The District's annual OPEB expense pursuant to GASB Statement No. 75 is based on the change in the annual total OPEB liability. The District has elected to calculate the total OPEB liability and related information using the alternative measurement method permitted by GASB Statement No. 75 for employers in plans with fewer than one hundred plan participants. The total OPEB liability was measured as of September 30, 2024. The following table shows the change in the District's OPEB liability:

	Total OPEB Liability
Balance at September 30, 2024	$ 169,200
Changes for the Year	
Service Cost	7,798
Interest	8,306
Differences Between Expected and Actual Experience	(27,674)
Changes of Assumptions	(19,253)
Benefit Payments	(13,049)
Net Changes	(43,872)
Balance at September 30, 2025	$ 125,328

Changes in assumptions reflect a change in the discount rate from 4.87% for the fiscal year ended September 30, 2024, to 4.06% for the fiscal year ended September 30, 2025. Also reflected as assumption changes are updated health care costs and premiums.

Sensitivity of the Total OPEB Liability to Changes in the Discount Rate

The following presents the total OPEB liability of the District, as well as what the District's total OPEB liability would be if it were calculated using a discount rate that is 1-percentage-point lower (3.06%) or 1-percentage-point higher (5.06%) than the current discount rate:

	1% Decrease (3.06%)	Discount Rate (4.06%)	1% Increase (5.06%)
Total OPEB Liability	$ 137,472	$ 125,328	$ 114,384

Sensitivity of the Total OPEB Liability to Changes in Healthcare Cost Trend Rates

The following presents the total OPEB liability for the District, as well as what the District's total OPEB liability would be if it were calculated using healthcare cost trend rates that are 1-percentage-point lower (5.75%, decreasing to 3.00%) or 1-percentage-point higher (7.75%, decreasing to 5.00%) than the current healthcare cost trend rates:

	1% Decrease (5.75% Decreasing to 3.00%)	Healthcare Cost Trend Rates (6.75% Decreasing to 4.00%)	1% Increase (7.75% Decreasing to 5.00%)
Total OPEB Liability	$ 111,873	$ 125,328	$ 140,971

OPEB Expense
Under GASB Statement No. 75, as it applies to plans that qualify for the Alternative Measurement Method, changes in the total OPEB liability are not permitted to be included in deferred outflows of resources or deferred inflows of resources related to OPEB. These changes will be immediately recognized through OPEB expense. For the year ended September 30, 2025, the District recognized OPEB expense of $(28,723).

Note 13 - Subsequent Events

In connection with the preparation of the financial statements, the District has evaluated subsequent events through March 9, 2026, the date of the financial statements. No subsequent events occurred which would have a material impact on the District's financial statements.

REQUIRED SUPPLEMENTARY INFORMATION

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
SCHEDULE OF REVENUES, EXPENDITURES, AND
CHANGES IN FUND BALANCE - BUDGET AND ACTUAL - GENERAL FUND
FOR THE YEAR ENDED SEPTEMBER 30, 2025

	Original Budget	Final Budget	Actual	Variance Favorable (Unfavorable)
Revenues				
Non-Ad Valorem Assessments	$ 7,521,113	$ 7,521,113	$ 7,626,577	$ 105,464
Miscellaneous Revenue	20,600	20,600	41,654	21,054
Interest Income	100,000	100,000	241,344	141,344
Total Revenues	7,641,713	7,641,713	7,909,575	267,862
Expenditures				
Current:				
Physical Environment:				
Personnel Services	4,546,244	4,546,244	4,114,469	431,775
Operating Expenditures	1,650,789	1,650,789	1,680,128	(29,339)
Debt Service:				
Principal	-	-	12,351	(12,351)
Interest and Fiscal Charges	-	-	1,653	(1,653)
Total Expenditures	6,197,033	6,197,033	5,808,601	388,432
Excess of Revenues Over Expenditures	1,444,680	1,444,680	2,100,974	656,294
Other Financing Uses				
Transfers Out	(1,444,298)	(1,987,819)	(1,787,819)	200,000
Total Other Financing Uses	(1,444,298)	(1,987,819)	(1,787,819)	200,000
Net Change in Fund Balance	382	(543,139)	313,155	856,294
Fund Balance, Beginning of Year	4,195,709	4,195,709	5,535,431	1,339,722
Fund Balance, End of Year	$ 4,196,091	$ 3,652,570	$ 5,848,586	$ 2,196,016

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
SCHEDULE OF CHANGES IN THE DISTRICT'S TOTAL
OTHER POSTEMPLOYMENT BENEFITS LIABILITY AND RELATED RATIOS
LAST 10 FISCAL YEARS*

Total OPEB Liability	**2025**	**2024**	**2023**	**2022**	**2021**	**2020**	**2019**	**2018**
Service Cost	$ 7,798	$ 7,727	$ 20,222	$ 20,745	$ 14,226	$ 12,514	$ 11,875	$ 11,585
Interest	8,306	8,066	6,461	5,377	7,363	7,456	6,099	5,175
Difference Between Expected and Actual Experience	(27,674)	-	26,021	-	(11,266)	11,589	(6,266)	-
Changes of Assumptions or Other Inputs	(19,253)	(1,811)	(129,451)	(7,900)	33,282	-	2,781	(13,781)
Benefit Payments	(13,049)	(12,167)	(3,119)	(2,915)	(6,186)	(5,728)	(2,906)	(6,819)
Net Change in Total OPEB Liability	(43,872)	1,815	(79,866)	15,307	37,419	25,831	11,583	(3,840)
Total OPEB Liability - Beginning	169,200	167,385	247,251	231,944	194,525	168,694	157,111	160,951
Total OPEB Liability - Ending	$ 125,328	$ 169,200	$ 167,385	$ 247,251	$ 231,944	$ 194,525	$ 168,694	157,111
Covered-Employee Payroll	$ 3,036,192	$ 1,719,636	$ 1,677,694	$ 1,888,510	$ 1,842,449	$ 1,826,218	$ 1,823,219	1,583,812
Total OPEB Liability as a Percentage of Covered Employee Payroll	4.13%	9.84%	9.98%	13.09%	12.59%	10.65%	9.25%	9.92%
Discount Rate	4.06%	4.87%	4.77%	2.43%	2.14%	3.58%	4.18%	3.64%

*The District implemented Governmental Accounting Standards Board Statement No. 75 for the fiscal year ended September 30, 2019.
As a result, this schedule will present 10 years as information is available.

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
SCHEDULE OF THE DISTRICT'S PROPORTIONATE SHARE
OF THE NET PENSION LIABILITY
FLORIDA RETIREMENT SYSTEM AND HEALTH INSURANCE SUBSIDY PENSION PLANS
LAST 10 FISCAL YEARS

FLORIDA RETIREMENT SYSTEM PENSION PLAN

Year Ended June 30,	District's Proportion of the FRS Net Pension Plan	District's Proportion Share of the FRS Net Pension Plan Liability	District's Covered-Employee Payroll (FYE June 30)	District's Proportionate Share of the FRS Net Pension Liability as a Percentage of its Covered-Employee Payroll	FRS Plan Fiduciary Net Position as a Percentage of the Total Pension Liability	Discount Rate for FRS Pension Plan
2016	0.003917377%	$ 989,141	$ 1,550,321	63.80%	84.88%	7.60%
2017	0.003896250%	1,152,485	1,607,812	71.68%	83.89%	7.10%
2018	0.004126695%	1,242,982	1,776,565	69.97%	84.26%	7.00%
2019	0.004213234%	1,450,979	1,888,358	76.84%	82.61%	6.90%
2020	0.003798690%	1,646,408	1,821,586	90.38%	78.85%	6.80%
2021	0.003926601%	296,610	1,864,621	15.91%	96.40%	6.80%
2022	0.004110491%	1,529,433	2,005,433	76.26%	82.89%	6.70%
2023	0.003718039%	1,481,521	2,048,472	72.32%	82.38%	6.70%
2024	0.003903138%	1,509,935	2,396,169	63.01%	83.70%	6.70%
2025	0.004000415%	1,241,534	2,646,237	46.92%	87.26%	6.70%

HEALTH INSURANCE SUBSIDY PENSION PLAN

Year Ended June 30,	District's Proportion of the HIS Net Pension Plan	District's Proportion Share of the HIS Net Pension Liability	District's Covered-Employee Payroll (FYE June 30)	District's Proportionate Share of the HIS Net Pension Liability as a Percentage of its Covered-Employee Payroll	HIS Plan Fiduciary Net Position as a Percentage of the Total Pension Liability	Discount Rate for HIS Pension Plan
2016	0.004969450%	$ 579,169	$ 1,550,321	37.36%	0.97%	2.85%
2017	0.004969945%	539,343	1,607,812	33.55%	1.64%	3.58%
2018	0.005439247%	575,696	1,776,565	32.41%	2.15%	3.87%
2019	0.005646287%	631,763	1,888,358	33.46%	2.63%	3.50%
2020	0.005248454%	640,828	1,821,586	35.18%	3.00%	2.21%
2021	0.005334476%	654,354	1,864,621	35.09%	3.56%	2.16%
2022	0.005502947%	582,850	2,005,433	29.06%	4.81%	3.54%
2023	0.005170624%	821,164	2,048,472	40.09%	4.12%	3.65%
2024	0.005661841%	849,331	2,396,169	35.45%	4.80%	3.93%
2025	0.005922359%	759,095	2,646,237	28.69%	6.36%	5.20%

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
SCHEDULE OF DISTRICT'S CONTRIBUTIONS
FLORIDA RETIREMENT SYSTEM AND HEALTH INSURANCE SUBSIDY PENSION PLANS
LAST 10 FISCAL YEARS

FLORIDA RETIREMENT SYSTEM PENSION PLAN

Year Ended September 30,	Contractually Required Contribution	FRS Contribution in Relation to the Contractually Required Contribution	FRS Contribution Deficiency (Excess)	District's Covered-Employee Payroll (FYE September 30)	FRS Contributions as a Percentage of Covered-Employee Payroll
2016	$ 101,827	$ (101,827)	$ -	$ 1,584,071	6.43%
2017	104,759	(104,759)	-	1,693,445	6.19%
2018	119,538	(119,538)	-	1,790,432	6.68%
2019	131,304	(131,304)	-	1,873,644	7.01%
2020	130,752	(130,752)	-	1,828,977	7.15%
2021	153,109	(153,109)	-	1,892,344	8.09%
2022	170,678	(170,678)	-	1,943,102	8.78%
2023	189,349	(189,349)	-	2,140,223	8.85%
2024	226,754	(226,754)	-	2,453,738	9.24%
2025	239,665	(239,665)	-	2,681,835	8.94%

HEALTH INSURANCE SUBSIDY PENSION PLAN

Year Ended September 30,	Contractually Required Contribution	HIS Contribution in Relation to the Contractually Required Contribution	HIS Contribution Deficiency (Excess)	District's Covered-Employee Payroll (FYE September 30)	HIS Contributions as a Percentage of Covered-Employee Payroll
2016	$ 26,033	$ (26,033)	$ -	$ 1,584,071	1.64%
2017	28,117	(28,117)	-	1,693,445	1.66%
2018	29,727	(29,727)	-	1,790,432	1.66%
2019	31,109	(31,109)	-	1,873,644	1.66%
2020	30,367	(30,367)	-	1,828,977	1.66%
2021	31,420	(31,420)	-	1,892,344	1.66%
2022	32,262	(32,262)	-	1,943,102	1.66%
2023	37,216	(37,216)	-	2,140,223	1.74%
2024	49,194	(49,194)	-	2,453,738	2.00%
2025	53,206	(53,206)	-	2,681,835	1.98%

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
NOTES TO THE REQUIRED SUPPLEMENTARY INFORMATION

Changes in Benefit Terms and Assumptions

Changes in Benefit Terms:

No significant changes.

Changes in Assumptions:

- **FRS—** The municipal rate used to determine total pension liability remained the same as in the prior year at 6.70%.
- **HIS—**The municipal rate used to determine total pension liability increased from 3.93% to 5.20%.
- **OPEB—**The municipal rate used to determine the OPEB liability decreased from 4.87% to 4.06%.

SUPPLEMENTARY INFORMATION

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
BALANCE SHEET - NON-MAJOR GOVERNMENTAL FUNDS
SEPTEMBER 30, 2025

ASSETS

	Debt Service	Total Non-Major Governmental Funds
Assets		
Cash and Cash Equivalents	$ 856,684	$ 856,684
Total Assets	$ 856,684	$ 856,684

LIABILITIES AND FUND BALANCES

Total Liabilities	-	-
Fund Balances		
Assigned for:		
Debt Service	856,684	856,684
Total Fund Balances	856,684	856,684
Total Liabilities and Fund Balances	$ 856,684	$ 856,684

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
SCHEDULE OF REVENUES, EXPENDITURES, AND
CHANGES IN FUND BALANCES - NON-MAJOR GOVERNMENTAL FUNDS
FOR THE YEAR ENDED SEPTEMBER 30, 2025

	Debt Service	Formerly Non-Major Fund Bridge Renewal and Replacement	Total Non-Major Governmental Funds
Revenues			
Interest Income	$ 20,547	$ -	$ 20,547
Total Revenues	20,547	-	20,547
Expenditures			
Debt Service:			
Principal	782,653	-	782,653
Interest and Fiscal Charges	66,572	-	66,572
Total Expenditures	849,225	-	849,225
Excess (Deficiency) of Revenues Over (Under) Expenditures	(828,678)	-	(828,678)
Other Financing Sources			
Transfers In	849,225	-	849,225
Total Other Financing Sources	849,225	-	849,225
Net Changes in Fund Balances	20,547	-	20,547
Fund Balances, Beginning of Year, As Previously Presented	836,137	1,015,260	1,851,397
Change Within Financial Reporting Entity (Non-Major to Major)	-	(1,015,260)	(1,015,260)
Fund Balances, Beginning of Year, As Adjusted or Restated	836,137	-	836,137
Fund Balances, End of Year	$ 856,684	$ -	$ 856,684

COMPLIANCE SECTION

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
SCHEDULE OF EXPENDITURES OF STATE FINANCIAL ASSISTANCE
FOR THE YEAR ENDED SEPTEMBER 30, 2025

State Agency/Pass-Through Grantor State Project	CSFA	Award Amount	Contract/ Grant Number	Grant Expenditures
Florida Department of Environmental Protection				
Statewide Surface Water Restoration and Wastewater Projects	37.039	$ 4,060,000	LPA0423	$ 674,075
Statewide Surface Water Restoration and Wastewater Projects	37.039	4,805,000	L0063	112,515
Passed through the South Florida Water Management District:				
Statewide Surface Water Restoration and Wastewater Projects	37.039	15,500,000	OWP12	1,708
Total Department of Environmental Protection				788,298
Total Expenditures of State Financial Assistance				$ 788,298

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
NOTES TO THE SCHEDULE OF EXPENDITURES OF STATE FINANCIAL ASSISTANCE
FOR THE FISCAL YEAR ENDED SEPTEMBER 30, 2025

Note 1 - Basis of Presentation

The accompanying schedule of expenditures of state financial assistance includes the state grant activity of Lehigh Acres Municipal Services Improvement District (the District) and is presented using the same basis of accounting as the fund in which the grant is recorded, generally the accrual or modified accrual basis, as described in Note 1 to the District's financial statements. The information in this schedule is presented in accordance with the requirements of the *Florida Single Audit Act* and Chapter 10.550, *Rules of the Auditor General.*

Note 2 - Reporting Entity

The District's reporting entity is defined in Note 1 to the District's basic financial statements for the year ended September 30, 2025. All state financial assistance received from state agencies, as well as passed through other governmental agencies, is included in this schedule.

Note 3 - Contingencies

Grant monies received and disbursed by the District are for specific purposes and are subject to review by the grantor agencies. Such audits may result in requests for reimbursement due to the disallowance of expenditures. Based upon prior experience, the District does not believe that such disallowances, if any, would have a material effect on the financial position of the District.

INDEPENDENT AUDITOR'S REPORT ON INTERNAL CONTROL OVER FINANCIAL REPORTING AND ON COMPLIANCE AND OTHER MATTERS BASED ON AN AUDIT OF FINANCIAL STATEMENTS PERFORMED IN ACCORDANCE WITH *GOVERNMENT AUDITING STANDARDS*

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

We have audited, in accordance with the auditing standards generally accepted in the United States of America and the standards applicable to financial audits contained in *Government Auditing Standards,* issued by the Comptroller General of the United States, the financial statements of the governmental activities, each major fund, and the aggregate remaining fund information of Lehigh Acres Municipal Services Improvement District (the District) as of and for the year ended September 30, 2025, and the related notes to the financial statements, which collectively comprise the District's basic financial statements, and have issued our report thereon dated March 9, 2026.

Report on Internal Control Over Financial Reporting
In planning and performing our audit of the financial statements, we considered the District's internal control over financial reporting (internal control) as a basis for designing audit procedures that are appropriate in the circumstances for the purpose of expressing our opinions on the financial statements, but not for the purpose of expressing an opinion on the effectiveness of the District's internal control. Accordingly, we do not express an opinion on the effectiveness of the District's internal control.

A *deficiency in internal control* exists when the design or operation of a control does not allow management or employees, in the normal course of performing their assigned functions, to prevent, or detect and correct, misstatements on a timely basis. A *material weakness* is a deficiency, or a combination of deficiencies, in internal control, such that there is a reasonable possibility that a material misstatement of the entity's financial statements will not be prevented, or detected and corrected, on a timely basis. A *significant deficiency* is a deficiency, or a combination of deficiencies, in internal control that is less severe than a material weakness, yet important enough to merit attention by those charged with governance.

Our consideration of internal control was for the limited purpose described in the first paragraph of this section and was not designed to identify all deficiencies in internal control that might be material weaknesses or significant deficiencies. Given these limitations, during our audit we did not identify any deficiencies in internal control that we consider to be material weaknesses. However, material weaknesses or significant deficiencies may exist that were not identified.

Report On Compliance and Other Matters
As part of obtaining reasonable assurance about whether the District's financial statements are free from material misstatement, we performed tests of its compliance with certain provisions of laws, regulations, contracts and grant agreements, non-compliance with which could have a direct and material effect on

INDEPENDENT AUDITOR'S REPORT ON INTERNAL CONTROL OVER FINANCIAL REPORTING AND ON COMPLIANCE AND OTHER MATTERS BASED ON AN AUDIT OF FINANCIAL STATEMENTS PERFORMED IN ACCORDANCE WITH *GOVERNMENT AUDITING STANDARDS*

the financial statements. However, providing an opinion on compliance with those provisions was not an objective of our audit, and accordingly, we do not express such an opinion. The results of our tests disclosed no instances of non-compliance or other matters that are required to be reported under *Government Auditing Standards*.

Purpose of this Report

The purpose of this report is solely to describe the scope of our testing of internal control and compliance and the results of that testing, and not to provide an opinion on the effectiveness of the District's internal control or on compliance. This report is an integral part of an audit performed in accordance with *Government Auditing Standards* in considering the District's internal control and compliance. Accordingly, this communication is not suitable for any other purpose.

Purvis Gray

March 9, 2026
Sarasota, Florida

INDEPENDENT AUDITOR'S REPORT ON COMPLIANCE FOR EACH MAJOR STATE PROJECT AND ON INTERNAL CONTROL OVER COMPLIANCE REQUIRED BY CHAPTER 10.550, *RULES OF THE AUDITOR GENERAL*

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

Report on Compliance for Each Major State Project

Opinion on Each Major State Project
We have audited the Lehigh Acres Municipal Services Improvement District's (the District) compliance with the types of compliance requirements identified as subject to audit in the *Florida Department of Financial Services' State Project Compliance Supplement* that could have a direct and material effect on the District's major state project for the year ended September 30, 2025. The District's major state project is identified in the summary of auditor's results section of the accompanying Schedule of Findings and Questioned Costs.

In our opinion, the District complied, in all material respects, with the types of compliance requirements referred to above that could have a direct and material effect on its major state project for the year ended September 30, 2025.

Basis for Opinion on Each Major State Project
We conducted our audit of compliance in accordance with auditing standards generally accepted in the United States of America; the standard applicable to financial audits contained in *Government Auditing Standards*, issued by the Comptroller General of the United States, and the audit requirements of Chapter 10.550, *Rules of the Auditor General* (Chapter 10.550). Our responsibilities under those standards and Chapter 10.550 are further described in the Auditor's Responsibilities for the Audit of Compliance section of our report.

We are required to be independent of the District and to meet our other ethical responsibilities, in accordance with relevant ethical requirements relating to our audit. We believe that the audit evidence we have obtained is sufficient and appropriate to provide a basis for our opinion on compliance for each major state project. Our audit does not provide a legal determination of the District's compliance with the compliance requirements referred to above.

Responsibilities of Management for Compliance
Management is responsible for compliance with the requirements referred to above and for the design, implementation, and maintenance of effective internal control over compliance with the requirements of laws, statutes, regulations, rules, and provisions of contracts or grant agreements applicable the District's state projects.

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

INDEPENDENT AUDITOR'S REPORT ON COMPLIANCE FOR EACH MAJOR STATE PROJECT AND ON INTERNAL CONTROL OVER COMPLIANCE REQUIRED BY CHAPTER 10.550, *RULES OF THE AUDITOR GENERAL*

Auditor's Responsibilities for the Audit of Compliance

Our objectives are to obtain reasonable assurance about whether material non-compliance with the compliance requirements referred to above occurred, whether due to fraud or error, and express an opinion on the District's compliance based on our audit. Reasonable assurance is a high level of assurance but is not absolute assurance and, therefore, is not a guarantee that an audit conducted in accordance with generally accepted auditing standards, *Government Auditing Standards* and Chapter 10.550 will always detect material non-compliance when it exists. The risk of not detecting material non-compliance resulting from fraud is higher than for that resulting from error, as fraud may involve collusion, forgery, intentional omissions, misrepresentations, or the override of internal control. Non-compliance with the compliance requirements referred to above is considered material if there is a substantial likelihood that, individually or in the aggregate, it would influence the judgment made by a reasonable user of the report on compliance about the District's compliance with the requirements of each major state project as a whole.

In performing an audit in accordance with generally accepted auditing standards, *Government Auditing Standards*, and Chapter 10.550, we:

- Exercise professional judgment and maintain professional skepticism throughout the audit.

- Identify and assess the risks of material non-compliance, whether due to fraud or error, and design and perform audit procedures responsive to those risks. Such procedures include examining, on a test basis, evidence regarding the District's compliance with the compliance requirements referred to above and performing such other procedures as we considered necessary in the circumstances.

- Obtain an understanding of the District's internal control over compliance relevant to the audit in order to design audit procedures that are appropriate in the circumstances and to test and report on internal control over compliance in accordance with Chapter 10.550, but not for the purpose of expressing an opinion on the effectiveness of the District's internal control over compliance. Accordingly, no such opinion is expressed.

We are required to communicate with those charged with governance regarding, among other matters, the planned scope and timing of the audit and any significant deficiencies and material weaknesses in internal control over compliance that we identified during the audit.

Report on Internal Control Over Compliance

A *deficiency in internal control over compliance* exists when the design or operation of a control over compliance does not allow management or employees, in the normal course of performing their assigned functions, to prevent, or detect and correct, non-compliance with a type of compliance requirement of a state project on a timely basis. A *material weakness in internal control over compliance* is a deficiency, or a combination of deficiencies, in internal control over compliance, such that there is a reasonable possibility that material non-compliance with a type of compliance requirement of a state project will not be prevented, or detected and corrected, on a timely basis. A *significant deficiency in internal control over*

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

INDEPENDENT AUDITOR'S REPORT ON COMPLIANCE FOR EACH MAJOR STATE PROJECT AND ON INTERNAL CONTROL OVER COMPLIANCE REQUIRED BY CHAPTER 10.550, *RULES OF THE AUDITOR GENERAL*

compliance is a deficiency, or a combination of deficiencies, in internal control over compliance with a type of compliance requirement of a state project that is less severe than a material weakness in internal control over compliance, yet important enough to merit attention by those charged with governance.

Our consideration of internal control over compliance was for the limited purpose described in the Auditor's Responsibilities for the Audit of Compliance section above and was not designed to identify all deficiencies in internal control over compliance that might be material weaknesses or significant deficiencies in internal control over compliance. Given these limitations, during our audit we did not identify any deficiencies in internal control over compliance that we consider to be material weaknesses, as defined above. However, material weaknesses or significant deficiencies in internal control over compliance may exist that were not identified.

Our audit was not designed for the purpose of expressing an opinion on the effectiveness of internal control over compliance. Accordingly, no such opinion is expressed.

The purpose of this report on internal control over compliance is solely to describe the scope of our testing of internal control over compliance and the results of that testing based on the requirements of Chapter 10.550. Accordingly, this report is not suitable for any other purpose.

Purvis Gray

March 9, 2026
Sarasota, Florida

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
SCHEDULE OF FINDINGS AND QUESTIONED COSTS
STATE FINANCIAL ASSISTANCE PROJECTS
FOR THE FISCAL YEAR ENDED SEPTEMBER 30, 2025

Part A - SUMMARY OF AUDITOR'S RESULTS

Financial Statements

Type of Auditor's Report Issued:	Unmodified
Internal Control Over Financial Reporting:	
Material weakness(es) identified?	No
Significant deficiency(ies) identified?	No
Non-compliance material to financial statements noted?	No

State Financial Assistance Projects

Internal Control Over Major Programs:	
Material weakness(es) identified?	No
Significant deficiency(ies) identified?	None reported
Type of Auditor's Report Issued on Compliance for Major Projects:	Unmodified
Any audit findings disclosed that are required to be reported in accordance with Chapter 10.557 for local government entities?	No

Identification of Major Projects:

State Financial Assistance Projects

Florida Department of Environmental Protection	
Statewide Surface Water Restoration and Wastewater Projects	CSFA No. 37.039
Dollar Threshold Used to Distinguish Between Type A and Type B Programs - State Projects:	$300,000

PART B - FINDINGS - FINANCIAL STATEMENT AUDIT

None

PART C - FINDINGS AND QUESTIONED COSTS - MAJOR STATE FINANCIAL ASSISTANCE PROJECTS

None

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
SCHEDULE OF FINDINGS AND QUESTIONED COSTS
STATE FINANCIAL ASSISTANCE PROJECTS
FOR THE FISCAL YEAR ENDED SEPTEMBER 30, 2025

PART D - OTHER ISSUES

No summary schedule of prior audit findings is required because there were no prior audit findings related to the state project.

No corrective action plan is required because there were no current year findings required to be reported under the *Florida Single Audit Act*.

PURVIS GRAY

INDEPENDENT ACCOUNTANT'S REPORT ON COMPLIANCE WITH FLORIDA STATUTES, SECTION 218.415 – INVESTMENTS OF PUBLIC FUNDS

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

We have examined the Lehigh Acres Municipal Services Improvement District's (the District) compliance with Section 218.415, Florida Statutes, during the fiscal year ended September 30, 2025. Management is responsible for the District's compliance with those requirements. Our responsibility is to express an opinion on the District's compliance based on our examination.

Our examination was conducted in accordance with attestation standards established by the American Institute of Certified Public Accountants. Those standards require that we plan and perform the examination to obtain reasonable assurance about whether the District complied, in all material respects, with the requirements referenced above. An examination involves performing procedures to obtain evidence about whether the District complied with the specified requirements. The nature, timing, and extent of the procedures selected depend on our judgment, including an assessment of the risks of material non-compliance, whether due to fraud or error. We believe that the evidence we obtained is sufficient and appropriate to provide a reasonable basis for our opinion.

We are required to be independent and to meet our other ethical responsibilities in accordance with relevant ethical requirements relating to the engagement.

Our examination does not provide a legal determination on the District's compliance with specified requirements.

In our opinion, the District complied, in all material respects, with the aforementioned requirements during the fiscal year ended September 30, 2025.

This report is intended solely for the information and use of the Florida Auditor General, the Board of Commissioners of the District, and applicable management, and is not intended to be, and should not be, used by anyone other than these specified parties.

Purvis Gray

March 9, 2026
Sarasota, Florida

PURVIS GRAY

MANAGEMENT LETTER

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

Report on the Financial Statements
We have audited the financial statements of the Lehigh Acres Municipal Services Improvement District (the District) as of and for the fiscal year ended September 30, 2025, and have issued our report thereon dated March 9, 2026.

Auditor's Responsibility
We conducted our audit in accordance with auditing standards generally accepted in the United States of America; the standards applicable to financial audits contained in *Government Auditing Standards*, issued by the Comptroller General of the United States; and Chapter 10.550, *Rules of the Auditor General*.

Other Reporting Requirements
We have issued our Independent Auditor's Report on Internal Control Over Financial Reporting and on Compliance and Other Matters Based on an Audit of the Financial Statements Performed in Accordance with *Government Auditing Standards;* Independent Auditor's Report on Compliance for Each Major State Project and on Internal Control Over Compliance Required by Chapter 10.550, *Rules of the Auditor General,* Schedule of Findings and Questioned Costs; and Independent Accountant's Report on an examination conducted in accordance with American Institute of Certified Public Accountants *Professional Standards,* AT-C Section 315, regarding compliance requirements in accordance with Chapter 10.550, *Rules of the Auditor General*. Disclosures in those reports and schedule, which are dated March 9, 2026, should be considered in conjunction with this management letter.

Prior Audit Findings
Section 10.554(1)(i)1., *Rules of the Auditor General*, requires that we determine whether or not corrective actions have been taken to address findings and recommendations made in the preceding financial audit report. There were no findings or recommendations made in the preceding financial audit report.

Official Title and Legal Authority
Section 10.554(1)(i)4., *Rules of the Auditor General*, requires that the name or official title and legal authority for the primary government and each component unit of the reporting entity be disclosed in this management letter, unless disclosed in the notes to the financial statements. See Note 1 of the District's financial statements for this information. There are no component units related to the District.

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

MANAGEMENT LETTER

Financial Condition and Management
Sections 10.554(1)(i)5.a. and 10.556(7*), Rules of the Auditor General*, require us to apply appropriate procedures and communicate the results of our determination as to whether or not the District met one or more of the conditions described in Section 218.503(1), Florida Statutes, and to identify the specific condition(s) met. In connection with our audit, we determined that the District did not meet any of the conditions described in Section 218.503(1), Florida Statutes.

Pursuant to Sections 10.554(1)(i)5.b. and 10.556(8), *Rules of the Auditor General*, we applied financial condition assessment procedures for the District. It is management's responsibility to monitor the District's financial condition, and our financial condition assessment was based in part on representations made by management and the review of financial information provided by same.

Section 10.554(1)(i)2., *Rules of the Auditor General*, requires that we communicate any recommendations to improve financial management. In connection with our audit, we did not have any such recommendations.

Property Assessed Clean Energy (PACE) Programs
As required by Section 10.554(1)(i)6.a., *Rules of the Auditor General*, the District must state whether a PACE program authorized pursuant to Section 163.081 or Section 163.082, Florida Statutes, did/did not operate within the District's geographical boundaries during the fiscal year under audit. The District did not have a PACE program operating within their geographical boundaries during the fiscal year under audit.

Specific Information
The specific information below has not been subjected to the auditing procedures applied in the audit of the basic financial statements and, accordingly, we do not express an opinion or provide any assurance on it.

As required by Section 218.39(3)(c), Florida Statutes, and Section 10.554(1)(i)7, *Rules of the Auditor General*, the District reported:

a. The total number of District employees compensated in the last pay period of the District's fiscal year as 38.
b. The total number of independent contractors to whom non-employee compensation was paid in the last month of the District's fiscal year as 2.
c. All compensation earned by or awarded to employees, whether paid or accrued, regardless of contingency as $2,713,253.
d. All compensation earned by or awarded to non-employee independent contractors, whether paid or accrued, regardless of contingency as $227,869.
e. No construction projects with a total cost of at least $65,000 approved by the District that is scheduled to begin on or after October 1 of the fiscal year being reported.
f. A budget variance report based on the budget adopted under Section 189.016(4), Florida Statutes, before the beginning of the fiscal year being reported if the District amends a final adopted budget under Section 189.016(6), Florida Statutes. This information is included as a budget to actual schedule in the required supplementary information on page 45.

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
Lehigh Acres, Florida

MANAGEMENT LETTER

As required by Section 218.39(3)(c), Florida Statutes, and Section 10.554(1)(i)9, *Rules of the Auditor General*, the District reported:

a. The rate of non-ad valorem special assessments imposed by the District as of September 30, 2025, as an average rate of $172.48/acre.
b. The total amount of special assessments collected by or on behalf of the District as of September 30, 2025, as $7,911,589.
c. The District has no outstanding bonds as of September 30, 2025.

Additional Matters
Section 10.554(1)(i)3., *Rules of the Auditor General*, requires us to communicate non-compliance with provisions of contracts or grant agreements, or fraud, waste, or abuse, that have occurred, or are likely to have occurred, that have an effect on the financial statements that is less than material but warrants the attention of those charged with governance. In connection with our audit, we did not have any such findings.

Purpose of this Letter
Our management letter is intended solely for the information and use of the Legislative Auditing Committee, members of the Florida Senate and Florida House of Representatives, the Florida Auditor General, federal and other granting agencies, the Board of Commissioners of the District, and applicable management, and is not intended to be, and should not be, used by anyone other than these specified parties.

We wish to take this opportunity to thank you and your staff for the cooperation and courtesies extended to us during the course of our audit. Please let us know if you have any questions or comments concerning this letter, our accompanying reports, or other matters.

Purvis Gray

March 9, 2026
Sarasota, Florida

PURVIS GRAY

CERTIFIED PUBLIC ACCOUNTANTS

Gainesville | Ocala | Tallahassee | Sarasota | Orlando | Tampa

purvisgray.com

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: Joaquin Garcia, Right of Way Issue

Meeting Date: March 16, 2026

Agenda Item # 3C

☐ **CONSENT** ☒ **ACTION** ☐ **OLD BUSINESS** ☐ **NEW BUSINESS**

Request Initiated By: ☐ **COMMISSIONER** _____ ☐ **ATTORNEY**

☐ **BOARD** ☒ **MANAGER** ☐ **ENGINEER** ☐ **AUDITOR**

1 Recommended motion:

2 What the action accomplishes: Provides for the preservation of the original intent of the Right of Way Policy. Prevents the creation of a precedent to have many encroachments on LA-MSID right of ways.

3 Fiscal Impact: $0

4 Staff comments:

5 Staff review and recommended approval:

☒ **MANAGER** ☒ **ASSISTANT DISTRICT MGR**

☐ **ATTORNEY** ☒ **FIELD DIRECTOR**

☒ **ENGINEER** ☒ **FINANCE DIRECTOR**

6 Board Action:

☐ **APPROVED** ☐ **DENIED** ☐ **DEFERRED**

0523

Lehigh Acres Municipal Services Improvement District
601 East County Lane
Lehigh Acres, FL 33936
(239) 368-0044

1st NOTICE OF RIGHT-OF-WAY VIOLATION

Name: Joaquin Garcia & Dana Boyce **Date:** 1/27/2026

Address: 1005 Anderson St E., Lehigh Acres, FL. 33974

RE: Shed & sprinklers encroaching on canal right of way

Dear Joaquin & Dana :

As the owner of the above referenced property this **Notice** is to advise you that during a routine inspection of Lehigh Acres Municipal Services Improvement District's right-of-way the following condition(s) existed and require(s) your immediate attention.

Please remove your shed & sprinkers that are inside the canal right of way, on to your property! Your rear property corners have been marked. Will reinspect on 2/26/2026.

Please note that in order to avoid further enforcement actions, you are required to correct any violations within 30 Days of the receipt of this **Notice**. Be advised that failure to correct any of the aforementioned violation(s) may result in legal action.

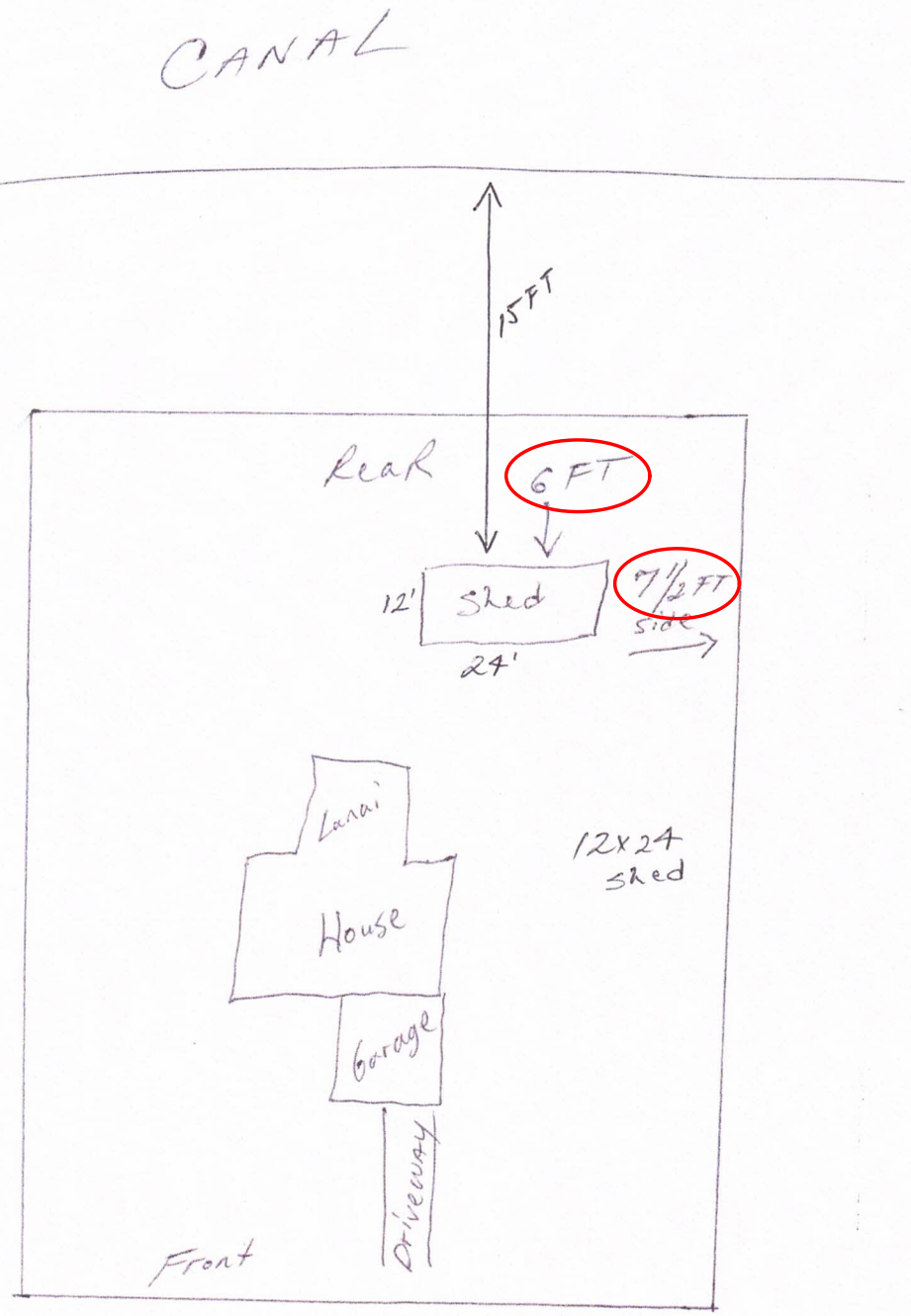
If you have questions, please contact LA-MSID at 239-368-0044 option 5.

Left at front door 2:20pm
Property Owner

Inspector

(Please Print)

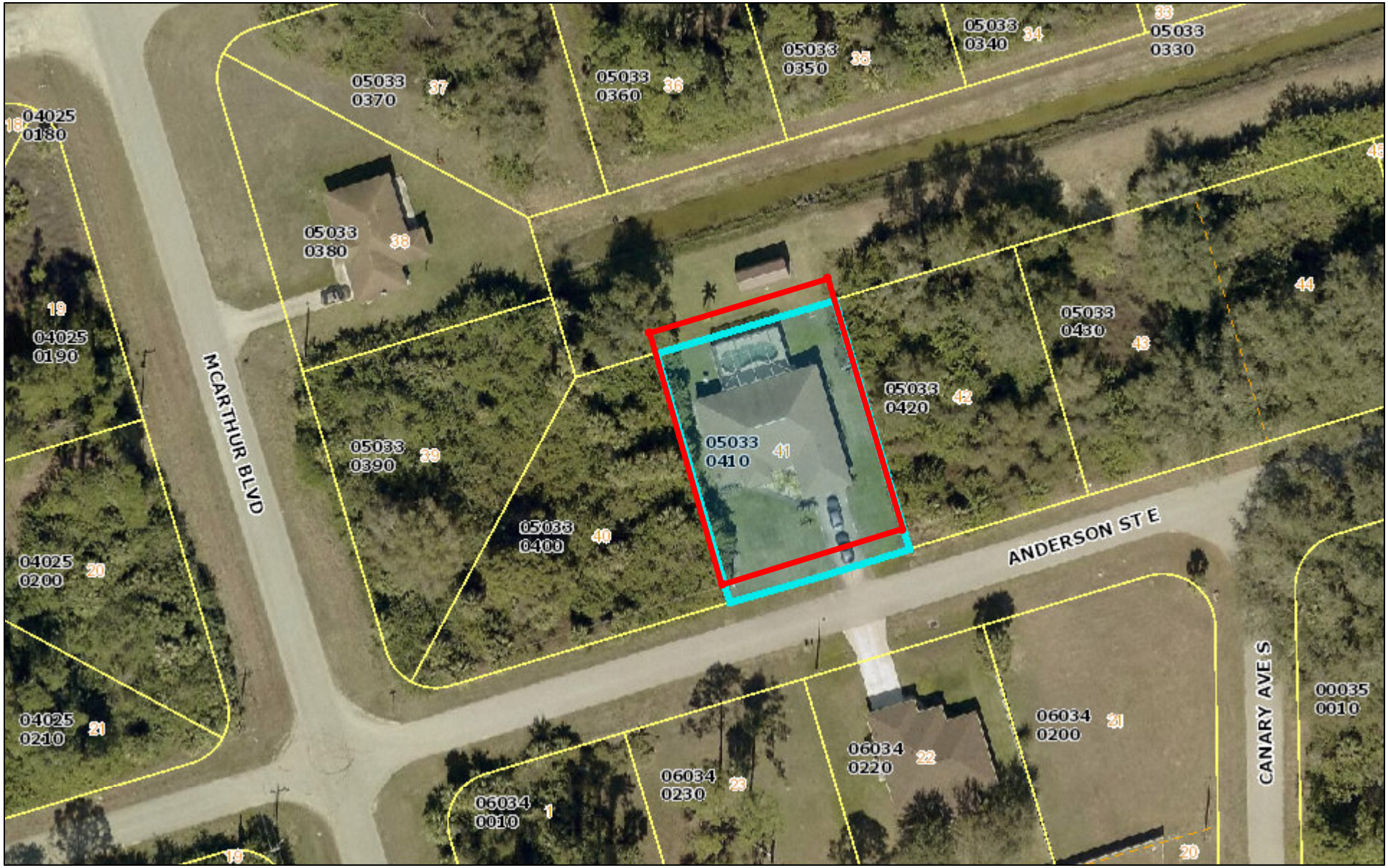
W. Weatherford
(Print)



Lee County Codes and Building Services
Approved
RES2022-03120
By: Sara Sheen Date:4/13/2022
For: 12 X 24 SHED

*Approval based on site information supplied by applicant.

GeoView Map



February 18, 2026 Air Photos: 2025 Hi-Res (3 inch) 1:797

0 35 70 140 ft

0 10 20 40 m

HospitalLocations

Library Locations

School Locations

School Locations

CCC_Parks

County Boundary

Major Roads Medium

I - 75

US 41

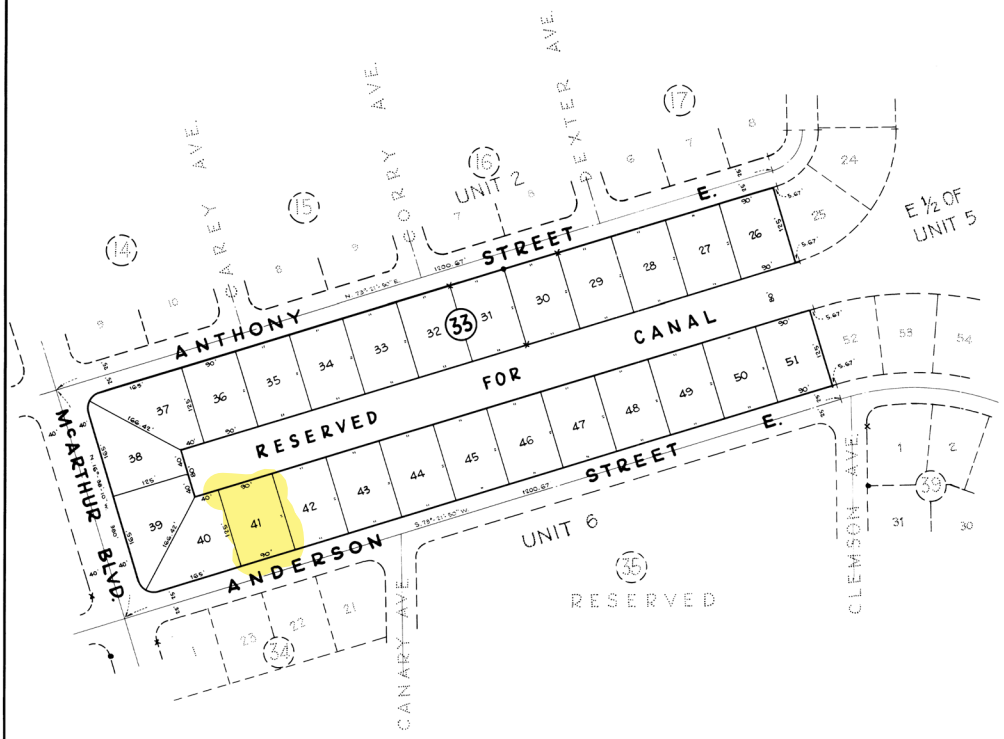
Other Highways

Other Roads

Parcels Near

This map is NOT a legal land survey and should not be used or relied upon as such.
No warranties, express or implied, are provided with the data, use, accuracy or interpretation.

PLAT BOOK 15 PAGE 193



PART OF PLAT OF UNIT 5
SECTION 2, TWP. 45 S., RGE. 27 E.
A SUBDIVISION OF

LEHIGH ACRES

LEE COUNTY, FLORIDA

100 50 0 100 200
SCALE IN FEET

All lots subject to a 6 foot Utility Easement on both sides and back, and a 6 foot Drainage Easement on the front and sides of lot on 50 foot R/W.

Interior P.R.M.s are 4" x 4" x 24" concrete.

• Denotes P.R.M.s

All corner lot radii are 25 feet except as shown and the lot dimensions are to the straight line intersections, except where arc distances are shown.

All Canals are hereby dedicated to East County Water Control District.

x Witness Monument.







1500 Monroe St, Fort Myers, FL 33901

office: (239) 533-8878

cell: (239) 839-0729

email: wdiaz@leegov.com

web: www.leegov.com/dcd

From: Wade Weatherford <WWeatherford@la-msid.com>
Sent: Wednesday, February 18, 2026 2:07:06 PM
To: Diaz, William <WDiaz@leegov.com>
Subject: 1005 Anderson St E, Lehigh Acres

Caution: This email originated from an external source. Be cautious of attachments and links, and do not provide login information. Report suspicious activity to the Service Desk: servicedesk@leegov.com or 533-HELP.

Good afternoon,

I am formally requesting a review of the permitted shed/structure located at 1005 Anderson St E, Lehigh Acres

It has come to our attention that the approved site plan associated with this permit does not appear to accurately reflect the actual placement of the structure. Based on field observation, the shed has been constructed within our canal right-of-way

As you are aware, canal rights-of-way must remain clear to preserve drainage function, maintenance access, and public safety. Construction within these areas presents operational and liability concerns.

We respectfully request that Lee County review the permit file, compare the approved documents with current site conditions, and determine whether the permit was issued based on inaccurate or misrepresented information. If so, we request that appropriate enforcement action be taken, including consideration of permit revocation to help us facilitate removal of the structure from the right-of-way.

Please advise if additional documentation or if a joint site inspection is needed. (Attached Document may need to be zoomed out)

Thank you for your attention to this matter.

Wade Weatherford
N.P.D.E.S / Right of Way Technician
Lehigh Acres Municipal Services And Improvement District
Hours of Operation: Monday-Thursday 7:00a.m-5:30p.m
601 East County Lane
Lehigh Acres, Fl. 33936
Phone: 239-368-0044 Ext. 25
Fax: 239-368-3412
Cell: 239-361-3698
Email: Wweatherford@la-msid.com

Disclaimer: Please do not send personal e-mails to this address. According to Florida Public Records Law, email correspondence to and from LA-MSID, including email addresses, is public record and must be made available to the public/media upon request, unless otherwise exempt by the Public Records Law. If you are not the intended recipient, please notify the sender immediately.

Receive updates from Lee County Government by subscribing to our newsletter

Please note: Florida has a very broad public records law. Most written communications to or from County Employees and officials regarding County business are public records available to the public and media upon request. Your email communication may be subject to public disclosure.

Under Florida law, email addresses are public records. If you do not want your email address released in response to a public records request, do not send electronic mail to this entity. Instead, contact this office by phone or in writing.

From: McStravic, Jack <JMcStravic@leegov.com>
Sent: Wednesday, February 18, 2026 3:47:01 PM
To: Diaz, William <WDiaz@leegov.com>; Delnay, Thomas <TDelnay@leegov.com>
Subject: RE: 1005 Anderson St E, Lehigh Acres

Good afternoon,

Based upon the site plan from the original house, I would agree with the water district. It appears the shed is not on the owners property. A new survey may be required.

Jack McStravic - Deputy Building Official
Department of Community Development
1500 Monroe St, Fort Myers, FL 33901
office: (239) 533-8369
cell: (239) 823-2805
email: jmcstravic@leegov.com
web: www.leegov.com/dcd
Connect With Us On Social Media

From: Diaz, William <WDiaz@leegov.com>
Sent: Wednesday, February 18, 2026 3:06 PM
To: Delnay, Thomas <TDelnay@leegov.com>
Cc: McStravic, Jack <JMcStravic@leegov.com>
Subject: Fw: 1005 Anderson St E, Lehigh Acres

Please see below request for review of revocation.

Thanks.

William Diaz | Code Enforcement Manager

Department of Community Development

email: mcalvi@leegov.com

web: www.leegov.com/dcd

Connect With Us On Social Media

From: Diaz, William <WDiaz@leegov.com>
Sent: Monday, March 2, 2026 6:21 PM
To: Calvi, Maria <MCalvi@leegov.com>
Subject: Fw: 1005 Anderson St E, Lehigh Acres (res2022-03120)

Hi Maria,

Is this your area?

William Diaz | Code Enforcement Manager

Department of Community Development

1500 Monroe St, Fort Myers, FL 33901

office: (239) 533-8878

cell: (239) 839-0729

email: wdiaz@leegov.com

web: www.leegov.com/dcd

From: Delnay, Thomas <TDelnay@leegov.com>
Sent: Monday, March 2, 2026 3:20:23 PM
To: Diaz, William <WDiaz@leegov.com>; McStravic, Jack <JMcStravic@leegov.com>
Cc: Sweigert, Rebecca <RSweigert@leegov.com>
Subject: RE: 1005 Anderson St E, Lehigh Acres (res2022-03120)

Hello William,

Permit was passed based on inaccurate site plan provided by owner. Code enforcement

should create a violation for the shed location based on inaccurate site plan provided by applicant for res2022-03120. Owner will need to apply for a permit to relocate the shed within the limits of the property with observed setbacks.

Thomas (Tad) DelNay, CBO
Building Official

Department of Community Development

1500 Monroe St, Fort Myers, FL 33901
office: (239) 533-8322
email: tdelnay@leegov.com
web: www.leegov.com/dcd
Connect With Us On Social Media

From: Diaz, William <WDiaz@leegov.com>
Sent: Friday, February 27, 2026 5:01 PM
To: McStravic, Jack <JMcStravic@leegov.com>; Delnay, Thomas <TDelnay@leegov.com>
Subject: Re: 1005 Anderson St E, Lehigh Acres

Good afternoon Gents,

Following up. Will the permit be revoked?

Thanks.

William Diaz | Code Enforcement Manager

Department of Community Development

1500 Monroe St, Fort Myers, FL 33901

office: (239) 533-8878

cell: (239) 839-0729

email: wdiaz@leegov.com

web: www.leegov.com/dcd

William Diaz | Code Enforcement Manager

Department of Community Development

1500 Monroe St, Fort Myers, FL 33901

office: (239) 533-8878

cell: (239) 839-0729

email: wdiaz@leegov.com

web: www.leegov.com/dcd

From: Calvi, Maria <MCalvi@leegov.com>
Sent: Tuesday, March 3, 2026 11:52:57 AM
To: Diaz, William <WDiaz@leegov.com>
Subject: Re: 1005 Anderson St E, Lehigh Acres (res2022-03120)

Good morning,

Yes, it is. I will look into it.

Thank you

Respectfully,



Maria Calvi | Code Enforcement Specialist

Department of Community Development

1500 Monroe St, Fort Myers, FL 33901

office: (239) 533-8895

cell: (239) 841-7158

Caution: This email originated from an external source. Be cautious of attachments and links, and do not provide login information. Report suspicious activity to the Service Desk: servicedesk@leegov.com or 533-HELP.

Good morning,

Trying to send conformation to our board members that a pending investigation is under way, or that a violation has been issued. Thanks

Wade Weatherford
N.P.D.E.S / Right of Way Technician
Lehigh Acres Municipal Services And Improvement District
Hours of Operation: Monday-Thursday 7:00a.m-5:30p.m
601 East County Lane
Lehigh Acres, Fl. 33936
Phone: 239-368-0044 Ext. 25
Fax: 239-368-3412
Cell: 239-361-3698
Email: Wweatherford@la-msid.com

Disclaimer: Please do not send personal e-mails to this address. According to Florida Public Records Law, email correspondence to and from LA-MSID, including email addresses, is public record and must be made available to the public/media upon request, unless otherwise exempt by the Public Records Law. If you are not the intended recipient, please notify the sender immediately.

From: Diaz, William <WDiaz@leegov.com>
Sent: Tuesday, March 3, 2026 12:21 PM
To: Wade Weatherford <WWeatherford@la-msid.com>
Subject: Fw: 1005 Anderson St E, Lehigh Acres (res2022-03120)

We are working on it.

I'll call you back in a couple minutes.

N.P.D.E.S / Right of Way Technician
Lehigh Acres Municipal Services And Improvement District
Hours of Operation: Monday-Thursday 7:00a.m-5:30p.m
601 East County Lane
Lehigh Acres, Fl. 33936
Phone: 239-368-0044 Ext. 25
Fax: 239-368-3412
Cell: 239-361-3698
Email: Wweatherford@la-msid.com

Disclaimer: Please do not send personal e-mails to this address. According to Florida Public Records Law, email correspondence to and from LA-MSID, including email addresses, is public record and must be made available to the public/media upon request, unless otherwise exempt by the Public Records Law. If you are not the intended recipient, please notify the sender immediately.

From: Diaz, William <WDiaz@leegov.com>
Sent: Thursday, March 5, 2026 4:49 PM
To: Wade Weatherford <WWeatherford@la-msid.com>
Subject: RE: 1005 Anderson St E, Lehigh Acres (res2022-03120)

Hi Wade,

I sent the attached email on Tuesday. Did you need more info?

Thanks.

Lee County
Southwest Florida

William Diaz | Code Enforcement Manager

Department of Community Development

1500 Monroe St, Fort Myers, FL 33901
office: (239) 533-8895
email: wdiaz@leegov.com
web: www.leegov.com/dcd
Connect With Us On Social Media

From: Wade Weatherford <WWeatherford@la-msid.com>
Sent: Thursday, March 5, 2026 8:17 AM
To: Diaz, William <WDiaz@leegov.com>
Subject: RE: 1005 Anderson St E, Lehigh Acres (res2022-03120)

Please see attached the notice of violation that was mailed to the property owner today.

Let me know if we can be of further assistance.

Thank you sir.

Lee County
Southwest Florida

William Diaz | Code Enforcement Manager

Department of Community Development

1500 Monroe St, Fort Myers, FL 33901
office: (239) 533-8895
email: wdiaz@leegov.com
web: www.leegov.com/dcd
Connect With Us On Social Media

From: Wade Weatherford <WWeatherford@la-msid.com>
Sent: Thursday, March 5, 2026 4:56 PM
To: Diaz, William <WDiaz@leegov.com>
Subject: RE: 1005 Anderson St E, Lehigh Acres (res2022-03120)

Caution: This email originated from an external source. Be cautious of attachments and links, and do not provide login information. Report suspicious activity to the Service Desk: servicedesk@leegov.com or 533-HELP.

Yes unfortunately, I don't but the board does. Possibly a copy of the violation or a link to one. It's on the agenda for the next board meeting coming up. They want it on hand incase the resident shows up to plead his case.
Thanks, I'd appreciate whatever you can get me.

LEHIGH ACRES MSID

Wade Weatherford

From: Wade Weatherford
To: "Diaz, William"
Subject: RE: 1005 Anderson St E, Lehigh Acres (res2022-03120)
Date: Monday, March 9, 2026 4:32:00 PM
Attachments: image002.png
image018.png
image019.png
image020.png
image021.png
image022.png
image023.png
image024.png
image025.png
image026.png
image027.png
image028.png
image029.png
image030.png
image031.png

Thank you, sir! Much appreciated

Wade Weatherford
N.P.D.E.S / Right of Way Technician
Lehigh Acres Municipal Services And Improvement District
Hours of Operation: Monday-Thursday 7:00a.m-5:30p.m
601 East County Lane
Lehigh Acres, Fl. 33936
Phone: 239-368-0044 Ext. 25
Fax: 239-368-3412
Cell: 239-361-3698
Email: Wweatherford@la-msid.com

Disclaimer: Please do not send personal e-mails to this address. According to Florida Public Records Law, email correspondence to and from LA-MSID, including email addresses, is public record and must be made available to the public/media upon request, unless otherwise exempt by the Public Records Law. If you are not the intended recipient, please notify the sender immediately.

From: Diaz, William <WDiaz@leegov.com>
Sent: Monday, March 9, 2026 4:27 PM
To: Wade Weatherford <WWeatherford@la-msid.com>
Subject: RE: 1005 Anderson St E, Lehigh Acres (res2022-03120)

Hi Wade,

NOTICE OF CODE VIOLATION

No Permit

Case # VIO2026-01149 **Date** 03/09/2026

GARCIA JOAQUIN C JR + BOYCE DANA L
1005 ANDERSON ST E
LEHIGH ACRES, FL 33974

DEPT. OF COMMUNITY DEVELOPMENT
CODE ENFORCEMENT OFFICE
PO BOX 398
FORT MYERS, FL 33902-0398
239-533-8895

Re: **Code Violation: No permit- shed (RES2022-03120 requires setbacks) without a permit.**

at 1005 ANDERSON ST E, LEHIGH ACRES, FL 33974
Parcel Number: 02-45-27-L1-05033.0410
(Located in Lee County, Florida)

Dear Property Owner/Tenant:

An inspection of the above referenced property on 3/6/2026 revealed a violation of:

Lee County Land Development Code Section 6-111, adopting the Florida Building Code, as amended, Section 105.1 for failing to obtain a required permit.

As the property owner(s), you must correct this violation within 30 day(s) of receipt of this notice by:

obtaining required permits and receiving certificates of compliance/occupancy OR by removing the improvements made without permits.

To apply for a permit, go to our eConnect website at https://accelaaca.leegov.com/aca or come to the Permitting Lobby located at 1500 Monroe St. in downtown Fort Myers between 9am-4pm. For additional information on acquiring a permit, please contact Lee County Building Department at 239-533-8329 M-F.

If the violation is not corrected within the time frame specified above -

- You may be issued a citation to appear in County Court, **OR**
- You may be scheduled to appear at a hearing before a Hearing Examiner and subject to **a fine up to the limits set forth in Lee County Land Development Code Section 2-427 for each day** the violation continues, plus prosecution costs.

When you have corrected the violation, arrange for an inspection by calling the Code Enforcement Officer at the number indicated below. To avoid the hearing, fines, and costs, Lee County Code Enforcement must verify the violation is corrected through an on-site inspection.

NOTE: If the violation is corrected after the deadline, but before the scheduled hearing, the case may still be presented to the Hearing Examiner for a finding that a violation did exist. Once there has been a Finding of Violation, the next time is cited for violating the same Lee County regulation it may be treated as a Repeat Violation. Repeat Violations may result in fines up to $5,000.00 per day from the date the repeat violation is discovered by a code officer.

Please contact Lee County Code Enforcement at 239-533-8895 or email MCalvi@leegov.com with respect to any question you may have regarding this notice.

Maria Calvi

PRINTED NAME of Accepting Party

Code Enforcement Officer

SIGNATURE of Accepting Party DATE

Acceptance of this document does not constitute an admission of responsibility, but does fulfill the statutory notice requirement of Florida Statute 162.12.

****

I swear or affirm the above statements are true and correct to the best of my knowledge and belief and, if accepted, the person accepting the document was informed of the contents at the time of delivery.

9214890101258200054111136 6

Certified Mail

INSPECTOR/AFFIANT DATE

NOTICE OF CODE VIOLATION

No Permit

Case # VIO2026-01149 **Date** 03/09/2026

GARCIA JOAQUIN C JR + BOYCE DANA L
1005 ANDERSON ST E
LEHIGH ACRES, FL 33974

DEPT. OF COMMUNITY DEVELOPMENT
CODE ENFORCEMENT OFFICE
PO BOX 398
FORT MYERS, FL 33902-0398
239-533-8895

Re: **Code Violation: No permit- shed (RES2022-03120 requires setbacks) without a permit.**

at 1005 ANDERSON ST E, LEHIGH ACRES, FL 33974
Parcel Number: 02-45-27-L1-05033.0410
(Located in Lee County, Florida)

Dear Property Owner/Tenant:

An inspection of the above referenced property on 3/6/2026 revealed a violation of:

Lee County Land Development Code Section 6-111, adopting the Florida Building Code, as amended, Section 105.1 for failing to obtain a required permit.

As the property owner(s), you must correct this violation within 30 day(s) of receipt of this notice by:

obtaining required permits and receiving certificates of compliance/occupancy OR by removing the improvements made without permits.

To apply for a permit, go to our eConnect website at https://accelaaca.leegov.com/aca or come to the Permitting Lobby located at 1500 Monroe St. in downtown Fort Myers between 9am-4pm. For additional information on acquiring a permit, please contact Lee County Building Department at 239-533-8329 M-F.

If the violation is not corrected within the time frame specified above -

- You may be issued a citation to appear in County Court, **OR**
- You may be scheduled to appear at a hearing before a Hearing Examiner and subject to **a fine up to the limits set forth in Lee County Land Development Code Section 2-427 for each day** the violation continues, plus prosecution costs.

When you have corrected the violation, arrange for an inspection by calling the Code Enforcement Officer at the number indicated below. To avoid the hearing, fines, and costs, Lee County Code Enforcement must verify the violation is corrected through an on-site inspection.

NOTE: If the violation is corrected after the deadline, but before the scheduled hearing, the case may still be presented to the Hearing Examiner for a finding that a violation did exist. Once there has been a Finding of Violation, the next time is cited for violating the same Lee County regulation it may be treated as a Repeat Violation. Repeat Violations may result in fines up to $5,000.00 per day from the date the repeat violation is discovered by a code officer.

Please contact Lee County Code Enforcement at 239-533-8895 or email MCalvi@leegov.com with respect to any question you may have regarding this notice.

Maria Calvi

PRINTED NAME of Accepting Party

Code Enforcement Officer

SIGNATURE of Accepting Party DATE

Acceptance of this document does not constitute an admission of responsibility, but does fulfill the statutory notice requirement of Florida Statute 162.12.

****

I swear or affirm the above statements are true and correct to the best of my knowledge and belief and, if accepted, the person accepting the document was informed of the contents at the time of delivery.

9214890101258200054111136 6

Certified Mail

INSPECTOR/AFFIANT DATE

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: RES 26-05, Change to Investment Policy

Meeting Date: March 16, 2026

Agenda Item # 3D

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve Resolution 26-05, change to Investment Policy.

What the action accomplishes: Provides for a change to the investment policy for the investment option of purchasing tax certificates, in the pursuit of acquiring the asset, and also provides additional language to meet the requirements for an investment policy as set forth in FS 218.415.

Fiscal Impact: Approximately 3 hrs. of legal time.

Staff comments:

Staff review and recommended approval:

[x] **MANAGER** [x] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

RESOLUTION 2026-05

A RESOLUTION OF THE BOARD OF COMMISSIONERS OF LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT TO AMEND SECTION 6-THE FINANCE POLICIES

WHEREAS, the Board of Commissioners (hereafter referred to as the "Board") of Lehigh Acres Municipal Services Improvement District (hereafter referred to as the "District") is authorized and empowered by Chapters 189, 197, and 298, F.S., Chapter 2015-202, 2017-216 and Chapter 2024-285 Laws of Florida (hereafter referred to as the "Special Acts"), to exercise the General and Special Powers authorized by the Special Act; and

WHEREAS, the Board may choose to create or amend policies and procedures from time to time; and

WHEREAS, the District has not updated its Finance Policies since July 16, 2018 and is choosing to do so, and

WHEREAS, the District has determined that it would like to add the option of an investment in tax certificates to Section 6.3 - Investment Policy, of the Finance Policy, so long as our eventual intent is to purchase the tax deed.

NOW, THEREFORE, be it resolved by the Board of Commissioners of Lehigh Acres Municipal Services Improvement District as follows:

Policy and Procedures, as attached, is amended to add the option of an investment in tax certificates to Section 6.3 - Investment Policy, of the Finance Policy, so long as our intent is to purchase the tax deed. Additional minor revisions are also included.

THIS RESOLUTION INTRODUCED and ADOPTED by the BOARD OF COMMISSIONERS on March 16, 2026.

LEHIGH ACRES MUNICIPAL SERVICE IMPROVEMENT DISTRICT

By: _____
Mike Bonacolta, Chair

(DISTRICT SEAL)

Attest: _____
David Deetscreek, Secretary

6.3 CASH MANAGEMENT/INVESTMENT POLICIES

1. PURPOSE

The purpose of this statement is to set forth the policy and objectives governing the cash management and investment of the Lehigh Acres Municipal Services Improvement District's cash and cash reserves. The District recognizes that liquidity needs must be met and has established cash flow projections for the debt service fund, capital project funds, and general operating funds.

2. SCOPE

In accordance with Section 218.415, Florida Statues, his cash management and investment policy applies to all all cash and investments held or controlled by the District; provided, however, this policy does not apply to the District's pension funds and funds related to the issuance of debt where there are other existing policies or indentures in effect for such funds. Additionally, any future revenues, which have statutory investment requirements conflicting with this Policy and funds held by state agencies (e.g., Department of Revenue), are not subject to the provisions of this Policy.

3. OBJECTIVES

The cash management and investment objectives are to determine adequate cash flow needs to meet the District's financial obligations and to invest idle funds in a manner that ensures the safety and preservation of investment principal, provides for liquidity, and maximizes the return on investment (yield) while controlling risk through diversification.

The objectives of this policy are, in priority order, (a) safety of principal, (b) liquidity, and (c) investment income.

a) Safety of Principal

The foremost objective of this Policy is the safety of the principal of those funds within the portfolios. Investment transactions shall seek to keep capital losses at a minimum, whether they are from securities defaults or erosion of market value. To attain this objective, diversification is required in order that potential losses on individual securities do not exceed the income generated from the remainder of the portfolio.

b) Maintenance of Liquidity

The portfolios shall be managed in such a manner that funds are available to meet reasonably anticipated cash flow requirements in an orderly manner. Periodical cash flow analyses will be completed in order to ensure that the portfolios are positioned to provide sufficient liquidity.

c) Investment Income/Return on Investment

Investment portfolios shall be designed with the objective of attaining a market rate of return throughout budgetary and economic cycles, taking into account the investment risk constraints and liquidity needs. Return on investment is of least importance compared to the safety and liquidity objectives described above. The core of investments is limited to relatively low risk securities in anticipation of earning a fair return relative to the risk being assumed.

4. DELEGATION OF AUTHORITY

The Finance Director is designated as the investment officer for the District and is responsible for investment decisions and activities under the direction of the District Manager. The day-to-day administration of the cash management program is handled by the Finance Director or the Finance Director's designee.

No person may engage in an investment transaction except as provided under the terms of this policy and the procedures established. Positions authorized as investment signatories are the District Manager, Assistant District Manager, Finance Director, and Board Members.

The Finance Director shall maintain an Investment Procedures and Internal Controls Manual based on this Policy. The Finance Director shall be responsible for monitoring internal controls, administrative controls and to regulate the activities of the District's staff involved with the investment program.

The District Manager, the Finance Director and the District's named fiduciaries shall comply with the applicable requirements Section 218.415(24), Florida Statutes. Accordingly, the District Manager, Finance Director, and its named fiduciaries shall make decisions based solely on pecuniary factors and shall not subordinate the interests of the people of this state to other objectives, including sacrificing investment return or undertaking additional investment risk to promote any nonpecuniary factor. The weight given to any pecuniary factor must appropriately reflect a prudent assessment of its impact on risk or returns. [F.S. 218.415(24)(b)].

The term "pecuniary factor" is defined as a factor that the District and/or its Investment Manager "prudently determines is expected to have a material effect on the risk or returns of an investment based on appropriate investment horizons consistent with applicable investment objectives and funding policy. The term does not include the consideration of the furtherance of any social, political, or ideological interests." [F.S. 218.415(24)(a)].

5. PRUDENCE

The standard of prudence to be applied by the investment officer shall be the "Prudent Person" Rule which states:

"Investments shall be made with judgment and care, under circumstances then prevailing, which persons of prudence, discretion, and intelligence exercise in the management of their own affairs,

not for speculation, but for investment, considering the probable safety of their capital as well as the probable income to be derived."

The Prudent Person Rule shall be applied in the context of managing the overall portfolio. Investment decisions shall be made solely on pecuniary factors, including risk, return, liquidity, and cash flow needs, and not for political or social purposes.

The investment officer and staff, acting in accordance with written procedures and exercising due diligence, shall not be held personally responsible for a specific security's credit risk or market price changes, provided that these deviations are reported as soon as they are identified and that appropriate action is taken to control adverse developments.

6. CONTINUING EDUCATION

The Finance Director and appropriate staff shall annually complete eight (8) hours of continuing education in subjects or courses of study related to investment practices and products as required by Section 218.415, Florida Statutes.

7. ETHICS AND CONFLICTS OF INTEREST

Employees involved in the investment process shall refrain from personal business activity that could conflict with proper execution of the investment program, or which could impair their ability to make impartial investment decisions. Also, employees involved in the investment process shall disclose to the District Manager any material financial interests in financial institutions that conduct business with the District, and they shall further disclose any material personal financial/investment positions that could be related to the performance of the District's investment program.

8. INTERNAL CONTROLS

The District has established internal controls to prevent loss of funds by fraud, employee error, or imprudent actions by District employees. These internal controls include the following:

- **a)** Authority to conduct investment transactions is limited to specific individuals within the Finance/Administration Department.
- **b)** Wire transfer authority is restricted to specific individuals and subject to defined dollar limits within the Finance/Administration Department. All non-repeat wire transfers require confirmation authorization by a second individual specified in wire authority documents executed with the District's primary depository.
- **c)** In all cases, every investment transaction requires two authorizing signatures. However, the District Manager and the Assistant District Manager are not authorized to countersign each other's signature..
- **d)** Staff in the Finance Department reconcile the District's general depository account on a monthly basis by comparing the District's general ledger with the applicable bank

account statements. The reconciliation of the general depository account reveals any difference in investment transaction recording and the actual movement of funds.

e) Each month, the Accountant reconciles investments reflected in the custodial statements with the District's records. The reconciliation is reviewed monthly by the Finance Director.

f) Each year the District's external auditors review existing internal controls and investment transactions by examining data on a random basis and determine compliance with this policy and applicable law, including §218.415, Florida Statutes.

9. METHOD OF OPERATION – CASH MANAGEMENT

a) The Finance Director's designee will monitor, on a daily basis, all funds deposited into and withdrawn from the various accounts maintained by the District.

b) A daily record shall be kept of the following information for the District's primary bank account:

i. Deposits
ii. Withdrawals
iii. Balances
iv. Scheduled and estimated cash flows for the day
v. Wire transfers
vi. Payroll and other check clearing
vii. Targeted ending balances

c) An analysis shall be made of the monthly statements provided by the bank to ensure that the fees charged are in accordance with contractual agreements and that collected balances required to maintain services are maintained and/or fees paid.

d) As a policy to ensure the security of District funds, the wiring of funds shall be strictly controlled and monitored daily. Control shall be maintained through segregation of duties requiring at least two individuals to affect a wire via an authorized individual to originate the wire or transfer and another authorized individual to confirm the wire.

e) The wiring of funds into, out of, and among the District accounts will be limited to the Accountant, Finance Director, Assistant District Manager, and District Manager.

f) For each outgoing wire or account transfer, a transfer form shall be completed and signed by an authorized individual. The form shall be forwarded to the Finance Director, or District Manager in the absence of the Finance Director, confirming the wire by signing and dating the transfer form.

g) A copy of each wire transfer form shall be retained for recording and auditing purposes with appropriate notations regarding the wiring of funds.

10. MONITORING AND ADJUSTING THE PORTFOLIO

The investment officer will routinely monitor the contents of the portfolio, the available markets, and the relative values of competing instruments and will adjust the portfolio accordingly.

10A. PERFORMANCE MEASURES

The District shall evaluate the performance of its investment portfolio on a quarterly basis. Performance measurements shall include:

- **a)** Review of portfolio composition, diversification, and liquidity;
- **b)** Assessment of adherence to the investment policy and statutory requirements;
- **c)** Documentation of any deviations from target objectives and corrective actions taken.

11. AUTHORIZED INSTITUTIONS

The Finance Director shall maintain a list of financial institutions approved for investment purposes by the Board of Commissioners.

The District shall purchase certificates of deposit only through qualified public depositories of the State of Florida as identified by the Chief Financial Officer, in accordance with Chapter 280, Florida Statutes.

12. AUTHORIZED INVESTMENTS

Funds of the District may be invested in the following:

a) The Local Government Surplus Funds Trust Fund or any intergovernmental investment pool authorized pursuant to the Florida Inter-local Cooperation Act of 1969, as provided in s.163.01. Investments in this category shall not exceed seventy-five percent (75%) of the District's total investment portfolio at any one time.

b) Securities and Exchange Commission registered money market funds with the highest credit quality rating from a nationally recognized rating agency.

c) Insured or fully collateralized certificates of deposit of banks:
   i. Must be registered public funds depository in the State of Florida as defined in s.280.02, *Florida Statutes*.
   ii. Maturities of three years or less
   iii. Maximum of 25 percent of available funds invested in any one bank at the time of purchase

d) Direct obligations of the U.S. Treasury with maturity dates within five years or less.

e) **Tax Certificate Investments:**
   The District is authorized to purchase, hold, and redeem tax certificates sold or held by the Lee County Tax Collector and Hendry County Tax Collector in accordance with Chapter 197.

   Tax certificate investments shall be subject to the following conditions:

   i. Only county-held tax certificates or tax certificates acquired directly through the county tax collector may be purchased;
   ii. The purchase of a tax certificate must be authorized by a Board Resolution following a determination that the proposed purchase meets the investment objectives and requirements as set forth in this Investment Policy;

iii. **The District shall only purchase Tax certificates for properties the District has identified as potentially needed for a specific governmental or public purpose. If a certificate is not redeemed, the District may, when it determines it is in its best interests, timely pursue tax deed proceedings and seek to acquire title to the property to support that governmental purpose;**
iv. **Tax certificates acquired by the District for governmental purposes are not considered surplus funds investments under this Policy. Any funds used for such purposes shall be evaluated separately to ensure the District maintains sufficient liquidity to meet operating and debt obligations.**
v. **The Investment Officer shall establish prudent concentration limits for tax certificate holdings;**
vi. **Detailed records shall be maintained for each tax certificate investment, including purchase amount, interest rate, redemption status, and outstanding balance;**
vii. **Tax certificate activity shall be included in the District's periodic investment reporting to the Board.**

f) Other investments authorized by law or by resolution.

13. DEBT SERVICE REQUIREMENT/BOND COVENANT

Any debt service requirements or bond covenants will supersede this policy.

14. DIVERSIFICATION

The investment officer shall diversify use of investment instruments to avoid incurring unreasonable risks inherent in over-investing in specific instruments, individual financial institutions, or maturities. Diversification strategies shall be determined and revised periodically in consultation with the District Manager.

a) LIQUIDITY – shall be assured through practices ensuring that investment maturities match known cash needs and anticipated cash flow requirements.
b) CREDIT RISK – Minimizing credit risk will be accomplished by limiting the maximum percentage that may be invested in any one entity or instrument at one time, as outlined in this policy.
c) MARKET RISK – Risks of market price volatility shall be controlled through maturity diversification limitations established by this policy.

15. ACCOUNTING METHOD

Investments will be carried at their cost or market value. Gains or losses from investments will be credited or charged to investment income at the time of sale. The District shall comply with Government Accounting Standards Board (GASB) requirements.

16. REPORTING

The Finance Director shall provide a written report to the Board of Commissioners at least quarterly. The report shall include:

a) Types of investments held;
b) Value of each investment;
c) Maturity schedules;
d) Investment income earned;
e) Compliance with this policy and §218.415, Florida Statutes;

Any other information required by law.

17. THIRD-PARTY CUSTODIAL AGREEMENTS

Securities should be held with a third party; and all securities purchased by, and all collateral obtained by, the unit of local government should be properly designated as an asset of the unit of local government. No withdrawal of securities, in whole or in part, shall be made from safekeeping, except by an authorized staff member of the unit of local government. Securities transactions between a broker-dealer and the custodian involving purchase or sale of securities by transfer of money or securities must be made on a "delivery vs. payment" basis, if applicable, to ensure that the custodian will have the security or money, as appropriate, in hand at the conclusion of the transaction.

18. INVESTMENT POLICY ADOPTION

The Investment Policy shall be adopted by the District Boad of Commissioners. The Finance Director shall review the Policy annually and submit recommendations to the District Manager for review and Board approval.

Approved by the District Board of Commissioners

By:_____

Title: President

Date:

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: ROV Purchase

Meeting Date: March 16, 2026

Agenda Item # 3E

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve the purchase of a Remotely Operated Underwater Vehicle (ROV) from for $15,295.97, from Deep Trekker.

What the action accomplishes: Allows staff to better inspect culverts, borings and canal conditions. Due to the increased incedence of fiberoptic companies operating in Lehigh and the increases in permit applications, we need to make sure that the companies didn't bore through our culverts. Also, will be helpful at the sinkhole.

Fiscal Impact: $15,295.97.

Staff comments: Staff was only able to secure two quotes, but we have included information from the two companies that would not respond.

Staff review and recommended approval:

[x] **MANAGER** [x] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

Lehigh Acres Municipal Services Improvement District

Bid Tabulation Sheet

Remotely Operated Underwater Vehicle

Company	Meets Specifications	Warranty	Country of Origin	Price
Deep Trekker	Yes	One Year	Canada	$15,295.97
Deep Ocean	Yes	One Year	U.S.A.	$145,000.00
Video Ray	N/A	N/A	N/A	Did not Respond
OceanBotics	N/A	N/A	N/A	Did not Respond

DEEP TREKKER

Lehigh Acres Municipal Services Improvement District - DTG3 & Grabber

Lehigh Acres Municipal Services Improvement District
601 East County Lane
Lehigh Acres, Florida 33936
United States

Reference: 20260303-150312576
Quote created: March 3, 2026
Quote expires: April 2, 2026
Quote created by: Tanya Mendonca

tmendonca@deeptrekker.com

Tim Browning
tbrowning@la-msid.com
+12393680044 ext 23

Comments from Tanya Mendonca

Products & Services

Item & Description	SKU	Quantity	Unit Price	Total
DTG3 BASE PACKAGE WITH 4K CAMERA DTG3 ROV BASE 7" LCD Handheld Controller With 64GB SD Card BRIDGE Software Enhanced 4K Camera 75M (246') Tether Tether Reel Pelican Case Sensors (Heading, Depth, Temp, Pitch, Roll, Auto Heading/Depth) Auxiliary Floodlights Toolless Ballast 3 Magnetically Coupled Thrusters (Precision Thruster + 2 Main Thrusters) Hybrid Power Batteries and Chargers Included 200M Depth Rating Fully Assembled and Tested 1-Year Manufacturer's Warranty	I7340-BASE-4K	1	$12,061.25	$12,061.25
2 FUNCTION GRABBER + INTEGRATION KIT - DTG3 Add a Grabber Arm to DTG3 Rotates 360 Degrees Continuously Opens to 5" (127mm) Closes with 70lbs (32kg) of Force ROV Pitch Enables Third Degree of Freedom Take Angle on Targets/Objects Comes With Standard Claws	I9726	1	$2,487.50	$2,487.50
Large Box Shipping to USA- 2 Way Shipping Costs Are Estimated And May Change Based On Location Or Size		1	$292.00	$292.00

Item & Description	SKU	Quantity	Unit Price	Total
Shipping Insurance Optional Coverage Against Damage/Loss During Shipment Price includes Insurance coverage for all shipments of this vehicle back and forth to Deep Trekker in case of Warranty Repair	Shipping Insurance	1	$455.22	$455.22
	One-time subtotal			$15,295.97
		Total		**$15,295.97**

Purchase terms

Currency:USD
Payment Terms:100% Prior to Shipment
Incoterm:CIP

Payment Methods Accepted

MasterCard, Visa, Bank Transfer (Please contact us if you have an alternative payment method you prefer).
Credit Cards are only accepted for Invoices of $10,000 or less in value.
Final invoices are the only time Deep Trekker bank wire transfer details (Bank of America - Canada Branch) will be provided.

Company Information

Sourcewell Contract Number: 011223-DTK
DUNS: 203115175
Federal Tax Code (HST): 834315202
CAGE Code: L09C8
NAICS: 334310
UEI: JDW2RG8E7S63
REVOLUTION NATO Stock Number (NSN): 583615U342399

By agreeing to this quotation, you agree to our Purchase Terms & Conditions.

Questions? Contact me

Tanya Mendonca
tmendonca@deeptrekker.com

Deep Trekker Inc.
155 Washburn Drive
Kitchener, ON N2R 1S1
Canada

DTG3 Mini ROV



Need a quick, reliable underwater inspection ROV? The DTG3 offers maneuverability, is fully equipped for immediate use, and has modular add-ons available to adapt to your specific surveys.

CONFIGURE YOURS NOW

COMPARE ROVS

DEEP OCEAN ENGINEERING



Tim Browning
Information Services Coordinator
LA-MSID
601 East County Lane
Lehigh Acres, FL 33936
(239) 368-0044 ext 23
(239) 368-5412 fax
tbrowning@LA-MSID.com

Quotation

Quote Number: 2026007
Date: 3/9/2026
Quote Expires on: 6/7/2026
Salesperson: Darrell Martin

VEHICLE TYPE: T5
ORIGIN: US

DELIVERY: 16-18 Weeks ARO
FOB: San Jose, CA
PAYMENT TERMS: 50% Down/50% before Shipment

Line #	Part #	Description	Price	Qty	Extend
1	A-T5	Phantom T5 Vectored ROV System includes: - Vehicle with 5X brushless thrusters, RDA camera tilt +/-90 degrees, electronic depth and heading, 2 X LED lights emitting 18,000K+ lumens, adjustable buoyancy (ballast weights) and leak detectors. 15lb payload - 300m depth rating - DOE IP Color Smart/Zoom Camera: auto/manual focus, 720P+ resolution - Laptop computer supporting real-time control and monitoring of ROV and sensors. Supports add-on external monitors for concurrent display of ROV Video and GUI - Continuous diagnostics/control of ROV functions via digital communications link - Rackmount Power Supply and Junction Box Module - Transport cases for rapid deployment of ROV	$145,000.00	1	$145,000.00
		All Copper Umbilical Options			
2	UM-05032-04 A	50m Copper umbilical for T series ROVs	$4,500.00	1	$4,500.00
3	UM-05032-03	100m Copper umbilical for T series ROVs	$6,625.00	1	$6,625.00
4	UM-05032-06	150m Copper umbilical for T series ROVs	$8,450.00	1	$8,450.00

Telephone: (408)-436-1102
Facsimile: (408)-436-1108
Website: www.deepocean.com
Email: sales@deepocean.com
Customersolutions@deepocean.com
darrellmartin@deepocean.com

Deep Ocean Engineering, Inc.
2261 Fortune Drive, Unit C
San Jose, CA 95131, USA

Line #	Part #	Description	Price	Qty	Extend
		Spares/ Additional Options			
5	KT-STD-SPARES-T	KIT, STANDARD SPARES, T-SERIES - THRUSTER ASSEMBLY - LED ASSEMBLY - CAMERA MODULE - CAMERA WHIP - THRUSTER WHIP - PILOT BOX CABLE - BULLET WEIGHT (4) - HAND TOOLS	$21,000.00	1	$21,000.00

Total	**$185,575.00**

Deep Ocean Engineering, Inc.
2261 Fortune Drive, Unit C
San Jose, CA 95131, USA

Telephone: (408)-436-1102
Facsimile: (408)-436-1108
Website: www.deepocean.com
Email: sales@deepocean.com

Deep Ocean Engineering, Inc.

TERMS & CONDITIONS OF SALE

1. **GENERAL**.Deep Ocean Engineering, Inc. a California corporation and the purchaser of products or services of ("Purchaser") agree that the within terms and conditions shall constitute a part of any contract for the sale of products or services by Deep Ocean Engineering,Inc. to Purchaser and shall take precedence over other terms and conditions. No contrary, additional or different provisions or conditions shall be binding on Deep Ocean Engineering, Inc. unless accepted in writing by the home office of Deep Ocean Engineering, Inc. Conditions not specifically stated herein shall be governed by established trade customs. These terms and conditions shall apply to and govern Purchaser's order, and in the event of any inconsistency between these terms and conditions and Purchaser's order, these terms and conditions shall prevail. These terms and conditions are subject to change without prior written notice at any time, in the sole discretion Deep Ocean Engineering,I Inc.

2. **PERIOD**. Unless otherwise stipulated, proposals and quotations of Deep Ocean Engineering, Inc. become void unless accepted by Purchaser, or extended by Deep Ocean Engineering, Inc. in writing, within sixty (60) days from the date of submission to Purchaser by Deep Ocean Engineering, Inc.

3. **PRICING**. All pricing is F.O.B. San Jose California, U.S.A. unless otherwise agreed to in writing by Deep Ocean Engineering, Inc. Prices are based on costs and conditions existing on the date of quotation and are subject to change by Deep Ocean Engineering, Inc. before final acceptance. All quotations and agreements are contingent upon and are subject to change because of strikes, accidents, fires, availability of materials and all other causes beyond the control of Deep Ocean Engineering, Inc... Deep Ocean Engineering, Inc. reserves the right to correct all typographical errors which may occur in its prices or specifications. Unless otherwise stated, all prices are in U.S. dollars.

4. **TAXES**. All prices are exclusive of federal, state, and local use, sales, property, excise or similar taxes however designated. Unless Purchaser provides Deep Ocean Engineering, Inc. with a valid and correct tax exemption certificate applicable to the product ship-to location prior to Deep Ocean Engineering, Inc. accepting the order, Purchaser is responsible for any of the foregoing taxes, or amounts in lieu thereof paid or payable by Deep Ocean Engineering, Inc.. In case of new taxes or increased rates or the repeal of taxes or the reduction of rates, the contract price shall be adjusted accordingly.

5. **SHIPPING**. All orders will be shipped freight collect. Separate charges for shipping and handling will be shown on Purchaser's invoice. Where it is deemed impractical by Purchaser or not possible to ship freight collect, if Deep Ocean Engineering, Inc. elects, Deep Ocean Engineering, Inc. may prepay and invoice all freight charges plus twenty-five percent (25%) for documentation and handling. All risk of loss shall pass to Purchaser when the goods are delivered to the carrier.

6. **DELIVERY**. Deep Ocean Engineering, Inc. will make a reasonable effort to meet the proposed delivery schedule, but shall not be liable for loss or damage resulting from delay and non-delivery or default in shipment, in whole or in part, delivery or other failure of performance due to events beyond the control of Deep Ocean Engineering, Inc. and without the fault or negligence ofDeep Ocean Engineering,Inc., including without limitation, contingencies of transportation, procurement of materials or parts, labor difficulties, governmental, action or acts of God.

7. **MINIMUM ORDER**. Orders for less than $100.00 must be prepaid, C.O.D. or credit card. A $15.00 handling charge will be added .

8. **INSPECTION**. Purchaser must examine the Product upon receipt. If any item is damaged or missing, Purchaser must notify Deep Ocean Engineering, Inc. within ten (10) days from the date of delivery. Purchaser's failure to present a written claim respecting any shipment, within ten (10) days after receipt thereof, shall constitute a waiver of all claims with respect thereto.

9. **PAYMENT**. For orders invoiced and shipped in the United States, the terms are thirty (30) days net from the date of invoice, subject to approval of credit by Deep Ocean Engineering, Inc.. For orders invoiced or shipped outside of the United States, prepayment is required. If, in the judgment of Deep Ocean Engineering, Inc., the financial condition of Purchaser at any time does not justify shipment or the continuation of production on the aforesaid terms of payment, Deep Ocean Engineering, Inc. may require payment in advance, alternative payment terms in form and substance satisfactory to Deep Ocean Engineering, Inc. or terminate the purchase order or any agreement related thereto without liability and without waiving any other remedies. Purchaser shall pay interest on all past due sums at the rate of 1.0% per month. Credit cards (Visa/MasterCard) and prepayment by cash or electronic funds transfer (EFT) are accepted.

10. **LIMITED WARRANTY**. Deep Ocean Engineering, Inc. warrants that the products sold hereunder shall be free from defects in materials and workmanship under normal use and service when correctly installed, used and maintained for a period of one (1) year from date of shipment from DOE Inc.. Purchaser's receipt of any product delivered hereunder shall be an unqualified acceptance of and a waiver by Purchaser of the right of Purchaser to make a claim with respect to such product unless Purchaser gives DOE Inc. notice of any claim within one (1) year after the receipt of such product. This warranty is limited to repair or replacement of the said product at Deep Ocean Engineering, Inc.'s option, F.O.B. the Deep Ocean Engineering, Inc. plant in San Jose, California, providing the product was not abused or operated other than in accordance with the DOE Inc. instruction manuals. With respect to the sale Deep Ocean Engineering, Inc. oceanographic instruments which are pressure tested to rated depth prior to shipment, Deep Ocean Engineering, Inc. does not assume responsibility for any damage due to leakage or implosion. Deep Ocean Engineering, Inc. reserves the right to modify its warranty at any time, in its sole discretion. THIS LIMITED WARRANTY IS NOT TRANSFERABLE.

11. **REPAIRS**. Prior to returning any product to Deep Ocean Engineering, Inc. for warranty repair, the Purchaser must first contact Deep Ocean Engineering, Inc. and obtain a return material authorization number (the "RMA Number"). Purchaser will clearly label each product returned with the appropriate RMA Number. All products requiring warranty repairs must be returned by Purchaser to Deep Ocean Engineering, Inc. in San Jose, California, or to such other location designated by Deep Ocean Engineering, Inc., with a letter stating the problem and probable cause, with a date and time of failure, together with freight, handling, customs duties (if any) and proper documentation. Return freight must also be prepaid.

12. **EXCHANGES**. From time to time Deep Ocean Engineering, Inc. may, in its sole discretion, exchange products or portions of a product. Any exchanges will be made in accordance with Deep Ocean Engineering, Inc.'s exchange policies in effect on the date of the exchange.

13. **RESTOCKS/RETURNS/CANCELLATIONS**. Unused Deep Ocean Engineering, Inc. products may only be returned with prior written authorization by Deep Ocean Engineering, Inc. and must be resalable as new. A 15% restock fee will be charged ($50.00 minimum). Products that may not be returned include: batteries, OEM sensors, specifically codified versions of standard products, or non-standard products obtained from manufacturers as a service to the Purchaser. Cancelled orders are subject to a 35% cancellation fee.

14. **U.S. GOVERNMENT CONTRACTS**. If the goods or services being purchased are to be used in the performance of a U.S. Government contract or subcontract and a U.S. Government contract number appears on Purchaser's purchase order, those clauses of the applicable U.S. Government procurement regulations which are mandatory under federal statute to be included in U.S. Government subcontracts shall be incorporated herein by reference.

15. **INTELLECTUAL PROPERTY RIGHTS**. No rights in intellectual property, including but not limited to, license or other rights under any patents, copyrights, trade secrets, or trademarks owned or controlled by Deep Ocean Engineering,Inc. or under whichDeep Ocean Engineering, Inc. is licensed, are granted to Purchaser or implied by the sale of products or services hereunder. Purchaser shall not identify as genuine products of Deep Ocean Engineering, Inc. any Deep Ocean Engineering, Inc. products purchased hereunder which Purchaser has treated, modified, or altered in any way. Purchaser shall not use the trademarks of Deep Ocean Engineering to identify such products provided, however, that Purchaser may identify such products as utilizing, containing, or having been manufactured from genuine products of DOE Inc. as treated, modified, or altered by Purchaser or a representative of Purchaser. If products or services sold hereunder are manufactured according to the specifications of Purchaser, Purchaser shall indemnify Deep Ocean Engineering, Inc. against any liability for patent, copyright, or trademark infringement on account of such manufacture or performance.

16. **LIMITATION OF LIABILITY**. Deep Ocean Engineering, Inc. makes no other warranty regarding its products or the products of others either express or implied, and, any implied warranty of merchantability or fitness for a particular purpose which exceeds the forgoing warranties is hereby disclaimed by Deep Ocean Engineering, Inc. and excluded from any agreement made by acceptance of any order Deep Ocean Engineering, Inc. does not accept liability beyond the remedies set forth herein including any liability for products not being available for use or for lost or corrupted data, loss of business, loss of profits, loss of use of the product or any associated equipment, cost of capital, cost of substitute or replacement product, facilities or services, down-time, charges for purchaser's time and effort, the claims of third parties, injury to property, or any other direct, indirect, special, reliance, incidental or consequential damages, regardless of the nature of the claim and whether or not foreseeable and whether or not based on breach of warranty, contract or tort (including negligence) or strict liability, even if Deep Ocean Engineering, Inc. has been advised of the possibility of such damages, or for any claim by any third party except as expressly provided herein. This limitation of liability applies both to products and services and support provided to purchaser under this agreement. No oral or written information or advice given by Deep Ocean Engineering, Inc., its agents or employees shall create a warranty or in any way increase the scope of the limited warranty provided in section 10 above. Any and all liability of Deep Ocean Engineering, Inc. is expressly limited to the price purchaser has paid for the products. Purchaser's sole remedy against Deep Ocean Engineering, Inc. in any dispute under this agreement shall be to seek recovery of the amounts purchaser paid, pursuant to section 9 above, upon the payment of which Deep Ocean Engineering, Inc., its agents and employees, and affiliates, will be released from and discharged of all further obligations and liability to purchaser. The limited warranty of Deep Ocean Engineering, Inc. gives purchaser specific legal rights, and purchaser may also have other rights that vary from state to state. Some states do not allow limitations on how long an implied warranty lasts or the exclusion or limitation of incidental or consequential damages, so the above limitations or exclusions may not apply to purchaser.

17. **APPLICABLE LAW**. Purchaser agrees to comply with all applicable laws and regulations of the various states and of the United States or any foreign country, as applicable.

18. **HEADINGS**. The section headings used herein are for convenience of reference only and do not form a part of these terms and conditions, and no construction or inference shall be derived there from.

19. **ASSIGNMENTS**. The rights and duties of Purchaser hereunder shall not be assignable by Purchaser without the prior written consent of DOE Inc., and any purported assignment or assumption without such consent shall be void.

20. **SEVERABILITY**. The terms and conditions contained herein are separate and severable and the invalidity or unenforceability of one or more of such terms and conditions shall not affect the validity or enforceability of any other term or condition.

21. **GOVERNING LAW**. The U.N. Convention on Contracts for the International Sale of Goods, 1980 will not apply to this transaction. This Agreement is to be construed under, and the respective rights of Deep Ocean Engineering, Inc. and Purchaser are to be determined according to the laws of the state of California.

22. **WAIVER**. The failure ofDeep Ocean Engineering, Inc. to enforce at any time or for any period of time any of the provisions hereof shall not be construed to be a waiver of such provisions nor the right ofDeep Ocean Engineering, Inc. thereafter to enforce each and every such provision.

23. **FACTORY AND ORDERING ADDRESS:**

Deep Ocean Engineering, Inc.
2261 Fortune Drive Unit C
San Jose, CA
95131

DEEP OCEAN ENGINEERING

PHANTOM® P-150

PHANTOM® P-150 — POWERFUL • EXPANDABLE • RUGGED

- Designed for video stability in confined/inaccessible spaces
- Access through 20" dia. manholes and into PWR recirculating piping
- Open-frame design allows easy integration of additional sensors; 2 functions manipulator for FME retrieval, UT gauge, LED lighting schemes, HD camera
- Unmatched video stability in Ultra-Compact ROVs
- Two horizontal and two "vertrans" thrusters, with additional thrusters/configurations available
- No electronics in the sub-unit ensures reliability in areas of high radiation
- Hardwired for fast and easy integration with accessories and easy fault detection
- Designed for nuclear industry with stable materials / FME concerns as a priority



NUCLEAR PWR APPLICATIONS OF THE PHANTOM® P-150

- Upper and lower internals, lifts and visual examination
- RWST visuals
- Small object identification and retrieval
- Recirculating piping
- Support of tri-nuke vacuuming operations, both flange and vessel floor
- Vessel inlet and outlet visual exams
- Vessel flange pre/post cleaning visual examination

CONVENTIONAL APPLICATIONS OF THE PHANTOM® P-150

- Potable Water Tank
- Wastewater Treatment
- Elevated/in-ground holding tanks

THE DEEP OCEAN ENGINEERING ADVANTAGE

Deep Ocean Engineering, Inc. is a USA based manufacturer of powerful, expandable, rugged underwater and surface drone vehicles, headquartered in the technology capital of the world, Silicon Valley, California. Its legendary Phantom® lines of ROVs and USVs, many of which have been in use around the world for decades, are integrated with the latest digital technology and the highest quality components available in the market today, including thrusters, sonar, cameras, lighting, navigation software (GPS) and power.

2403 Qume Drive
San Jose, CA 95131, USA

WWW.DEEPOCEAN.COM

P: 408-436-1102
F: 408-436-1108
sales@deepocean.com

TECHNICAL SPECIFICATIONS

PHANTOM® P-150

VEHICLE SPECIFICATIONS*	
Maximum working depth	53.3m (175 ft)
Length	64.25cm (25.30 in)
Width	35.24cm (13.875 in)
Height	29.21cm (11.5 in)
Weight	18kg (40 lbs)

TOPSIDE	
Console	NIS-301 Nuclear Inspection System. Software run by rack mounted computer. Includes rack mount Power Supply and 1 space, pull-out flip-up monitor
Pilot Box	2 Joysticks Pilot Box on 15' deck cable Auto depth, camera focus / zoom

CHASSIS
Hard anodized aluminum, non-corroding, and shock resistant
Foam buoyancy with replaceable skid cover plate
Safety wired stainless hardware
FME requirements met for PWR environments

THRUSTERS
4 Shaft driven 48VDC brushed motors
Individually controlled
Mineral oil filled compensator on shaft seal

UMBILICAL	
Diameter	13.9mm (0.55in)
Minimum dynamic bend radius	245mm (9.6in)
Weight in Air	7.0kg (15.4lb)
Weight in Water	3.2kg (7.0lb)
Length	53m (175ft) with optional 114m (375ft) deck cable

CAMERA
Sony HD Camera (1080p)
Horizontal field of view: 65°
30x optical zoom; 12x digital zoom with image stabilization
Depth Rating: 300m (984ft)
Front mounted on mechanical pan/tilt unit (+/- 90°)
1.4 Lux with white balance and image adjustments

LIGHTING
LED Sealite LSL-1000 (10,000 Lumens)
5,600 Lux @ 1m; 75° Beam angle 4,000m Depth rating
90-140 VAC 50/60 Hz

* Specifications subject to change
07112019

OPTIONS

- Radiation tolerant cameras
- Rear facing camera (Color, B/W, Low Light) and light combination
- Depth pressure transducer
- S-Video functionality on Sony HD camera
- Video Overlay System with options to display customer tooling
- UT thickness gauge
- 2 function manipulator
- Spot Cleaning Brush
- Technical Training
- Optional 500ft Umbilical

2403 Qume Drive
San Jose, CA 95131, USA

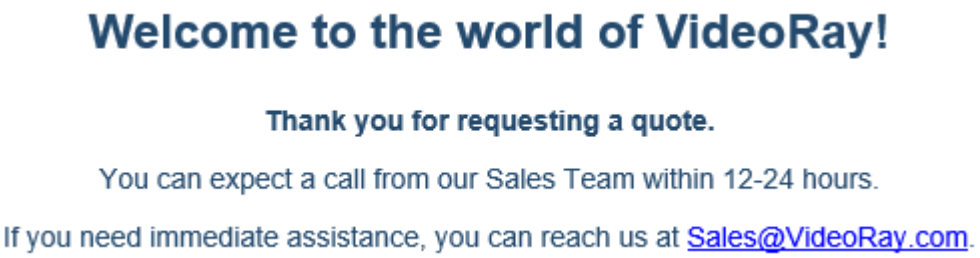
WWW.DEEPOCEAN.COM

P: 408-436-1102
F: 408-436-1108
sales@deepocean.com

Dave Lindsay

From: Tim Browning
Sent: Tuesday, March 10, 2026 10:28 AM
To: Dave Lindsay
Subject: Other ROV Quote request

Other companies I attempted to contact for a quote on a ROV.
VideoRay
12/17/25
3/3/2026
3/9/2026
This Email was received, but no phone call or quote emailed.



Oceanbotics
Left request for quote. No reply
3/3/2026
3/4/2026
3/9/2026

Thank You ,

Tim Browning
Information Services Coordinator
LA-MSID
601 East County Lane
Lehigh Acres, FL 33936
(239) 368-0044 ext 23
(239) 368-5412 fax
tbrowning@LA-MSID.com

Disclaimer: Please do not send personal e-mails to this address. According to Florida Public Records Law, email correspondence to and from the LA-MSID, including email addresses, is public record and must be made available to the public/media upon request, unless otherwise exempt by the Public Records Law. If you are not the intended recipient, please notify the sender immediately

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: Dump Truck Purchase

Meeting Date: March 16, 2026

Agenda Item # 3F

[x] **CONSENT** [ ] **ACTION** [ ] **OLD BUSINESS** [ ] **NEW BUSINESS**

Request Initiated By: [ ] **COMMISSIONER** _____ [ ] **ATTORNEY**

[ ] **BOARD** [x] **MANAGER** [ ] **ENGINEER** [ ] **AUDITOR**

Recommended motion: Move to approve the purchase of a dump truck from Boyce Equipment & Parts Co., Inc. for $57,765.00

What the action accomplishes: Allows staff to replace a current dump truck with a sturdier, less expensive option.

Fiscal Impact: $57,765.00

Staff comments:

Staff review and recommended approval:

[x] **MANAGER** [x] **ASSISTANT DISTRICT MGR**

[ ] **ATTORNEY** [ ] **FIELD SUPERINTENDENT**

[x] **ENGINEER** [x] **CONTROLLER**

Board Action:

[ ] **APPROVED** [ ] **DENIED** [ ] **DEFERRED**

Boyce Equipment & Parts Co., Inc.

2893 S. American Way
Ogden, UT 84401

Quote

Date	Quote #
3/10/2026	1481

Name / Address
Lehigh Acres Kail Silcox 601 E. County Ln. Lehigh Acres FL. 33936 239-368-0044

Rep	Project
DB	

Item	Description	Qty	Cost	Total
900 Series 6x6 Tr...	M929A2 900 Series 5 Ton 6x6 Truck (Serviced, 8.3 Cummins Turbo Engine, 5 Speed Allison Automatic Transmission, Air-Shift Transfer Case, Power Steering, Air Brakes, High Tread Tires, Recovered Seats, Paint-White Cab w/Black Undercarriage)	1	43,000.00	43,000.00
Option	DOT Lighting Package- Top/Bed/Headlight/License Plate- LED	1	1,500.00	1,500.00
Option	24/12 V 60 Amp Solid State Reducer- *Don't need the 20 amp if you are running this*	1	565.00	565.00
Option	Air Conditioning (12V/24V)- Above Head	1	4,000.00	4,000.00
Option	Rear Trailer Hitch Installed w/7 Pin Hook-Up	1	900.00	900.00
Option	Turn Key Ignition Switch- ON/OFF Switch	1	100.00	100.00
Option	80 Amp Alternator Upgrade	1	500.00	500.00
Option	14.00R20 *NEW* Tire Upgrade per Tire 49" x6 = $1200 ea.	6	1,200.00	7,200.00
Shipping/Freight	TBD		0.00	0.00

Requested Quote. Please let us know if you need any further assistance.

Total $57,765.00

900 Series 5 Ton 6x6 dump build sheet

Truck #: **Quote Date:** **Quote good for 30 Days**

Model: M929A1 OR M929A2 DUMP **VIN:** **Customer Name:**

Base Price- $35,000.00 (A1 Model) Includes:
Base Price- $37,500.00 (A2 Model) Includes:
Price w/Paint- $40,500.00 (A1 (NHC250)) Includes:
Price w/Paint- $43,000.00 (A2 (8.3L)) Includes:

New Fiberglass Top (Removable)
Recovered Seats (Driver Seat Spring Ride and Passenger Bench Seat)
Electrical- Checked out completely
NHC250 Cummins or 8.3 Turbo Charged Engine Checked out and Serviced
5 Speed Allison Automatic Transmission- Serviced
Air Shift Front Engage Transfer Case- Serviced
Air Brakes- Serviced
6 Military 5 Ton Wheels or 10 Split Rims (Dual Tire Setup)
6 High Tread 395/85R20 (46") Tires or 10 11.00R20 Dualed Tires
Cooling System Serviced w/ New Hoses and Belts
Stock Cargo Bed Length- 14'
Dump Version- 10' Dump Bed
Fully Serviced
Good Clear Window Glass
Operating Windshield Wipers
4 New Group 34 Batteries
West Coast Mirrors
Shortened or Lenghtened to Customer Specs
DOT Lighting Package
CTIS Removed with A2 Model
Power Steering
Heater
Upgraded Floor Padding Added

Options Include:

	DOT Lighting Package- Top/Bed/Headlight/License Plate- **LED**	**$1,500.00**
	Sliding Rear Window	**$210.00**
	24/12 V 20 Amp Solid State Reducer w/Cigerette Hook-Up	**$270.00**
	Camo Painting or 3 tone paint	**$2,600.00**
	Detroit Locker Installed (Per Axle)	**$1,550.00**
	Air Conditioning (12V/24V)- Above Head	**$4,000.00**
	Block Heater	**$185.00**
	24/12 V 60 Amp Solid State Reducer	**$565.00**
	Winch Installed (Hydraulic)	**$3,500.00**
	PTO Installed	**$1,200.00**
	Rear Trailer Hitch Installed w/7 Pin Hook-Up	**$900.00**
	Used Spare Tire w/ 5 Ton Wheel- New Tire w/Wheel-$1000	**$750.00**
	Locking Door Handles	**$150.00**
	Troop Seats w/Bows	**$800.00**
	Turn Key Ignition Switch- ON/OFF Switch	**$100.00**
	15,000 LB Electric Winch (Installed)- **Extended or Undermount**	**$2,000.00**
	140 Amp Alternator Upgrade	**$650.00**
	18,000 LB Electric Winch (Installed)- **Undermount or Extended**	**$2,300.00**
	80 Amp Alternator Upgrade	**$500.00**
	16.00R20 Tire Upgrade per Tire 53" **x6**= $1300 ea.	**$7,800.00**
	14.00R20 *NEW* Tire Ugrade per Tire 49" **x6** = $1200 ea.	**$7,200.00**
	LineX Bedliner Bed/Bumper	**$1,400.00**
	HEMMT Wheel (w/16.00R20 Tire Upgrade)- ONLY W/ A1 MODEL	**$400.00**
	Sub Total with options:	

Other Discussed Options:

	TOTAL- OPTIONS	
	TRUCK	
	TOTAL- TRUCK W/OPTIONS	
	HALF DOWN	**$ -**
	Freight Quote	
	Total with all options & Freight:	

All sales require 50% Down Payment
2 Months Mechanical Parts Warranty

Boyce Equipment & Parts Co., Inc.
2893 S American Way
Ogden, UT 84401
800-748-4269 Fax: 801-392-4746

THE PETE STORE

CONNECTICUT | DELAWARE | FLORIDA | GEORGIA | MARYLAND | MASSACHUSETTS
NORTH CAROLINA | RHODE ISLAND | SOUTH CAROLINA | TENNESSEE | VIRGINIA

Peterbilt

BUYER'S ORDER

PLEASE ENTER MY ORDER FOR THE FOLLOWING: ☑ NEW ☐ USED ☑ TRUCK ☐ TRACTOR ☐ TRAILER

YEAR	MAKE	MODEL	BODY TYPE
2025	Peterbilt	537	Dump

VEHICLE ID-NUMBER	MILEAGE	STOCK #
TBA	TBA	TBA

DEALER'S PRINTED NAME (SELLER): The Peterbilt Store

DEALER'S STREET ADDRESS: 5313 Dr. Martin Luther King Jr Blvd

CITY	STATE	ZIP
Fort Myers	FL	33905

CUSTOMER'S PRINTED NAME (BUYER)
1) Lehigh Acres Municipal Services Improvement District

CUSTOMER'S PRINTED NAME (BUYER)
2)

BUYER'S STREET ADDRESS: 601 E. County Ln

CITY	STATE	ZIP
Lehigh Acres	FL	33936

EMAIL	PHONE NUMBER
mcook@la-msid.com	

TRADE-IN

YEAR	MAKE	MODEL	BODY TYPE
N/A	N/A	N/A	N/A

VEHICLE ID-NUMBER	TRIM	TITLE NO.	MILEAGE
N/A	N/A	N/A	N/A

EXISTING LIEN HOLDER: N/A

ACCOUNT #	BALANCE DUE
N/A	N/A

LIEN HOLDER INFORMATION

LIEN HOLDERS NAME	PHONE NUMBER
N/A	
ADDRESS	ELT CODE

CITY	STATE	ZIP

1) BASE PRICE OF VEHICLE; 2) ACCESSORIES, MODIFICATIONS, OR EXTENDED WARRANTIES:	1) $ 159,505.00
12' Ox Body Dump	$ 29,843.00
Dealer Fee	$ 485.00
Battery Fee	$ 3.00
Tire Fee	$ 6.00
	2) $ 189,842.00

ANY ITEM NOTATED AS "ESTIMATE" OR "QUOTE" IS BASED ON THE BEST INFORMATION AVAILABLE TO THE DEALER AND IS SUBJECT TO CHANGE WHEN THE TRUE AMOUNT IS DETERMINED.

FOR "AS IS" SALE ONLY: I UNDERSTAND THAT THIS VEHICLE IS BEING SOLD "AS IS" WITH ALL FAULTS AND IS NOT COVERED BY ANY DEALER WARRANTY. I UNDERSTAND THAT THE DEALER IS NOT REQUIRED TO MAKE ANY REPAIRS AFTER I BUY THIS VEHICLE. I WILL HAVE TO PAY FOR ANY REPAIRS THIS VEHICLE WILL NEED. (SEE ADDITIONAL TERMS AND CONDITIONS)

06/26/2024
DATE

SIGNATURE

3) TOTAL VEHICLE PRICE		3) $ 189,842.00
4) TRADE IN ALLOWANCE		
A) GROSS ALLOWANCE		
B) LESS BALANCE OWING (ESTIMATED)		
C) NET TRADE-IN (A-B)	$ 0.00	4) $ 0.00
5) VEHICLE CASH PRICE (3-4)	$ 189,842.00	5) $ 189,842.00
6) OTHER CHARGES	$ 200.00	
LICENSE FEES:	TITLE FEE: $ 200.00	TITLE TAX:
UMVFEE	DEALER BUSINESS LICENSE TAX:	
OTHER:		
7) PROCESSING FEE*		6) $ 200.00
8) DOWNPAYMENT		7)
9) BALANCE DUE AT SETTLEMENT		8)
		9) $ 190,042.00

THIS ORDER IS NOT VALID UNLESS SIGNED BY AN AUTHORIZED REPRESENTATIVE OF THE DEALERSHIP

* A PROCESSING FEE IS NOT AN OFFICIAL FEE. A PROCESSING FEE IS NOT REQUIRED BY LAW, BUT MAY BE CHARGED TO BUYERS FOR HANDLING DOCUMENTS RELATING TO THE CLOSING OF A SALE.

NO LIABILITY INSURANCE INCLUDED

CUSTOMER, by the execution of this Buyers Order ("Order"), offers to purchase the vehicle(s) described herein, subject to the terms and conditions set forth in this Order and the Additional Terms and Conditions hereto.

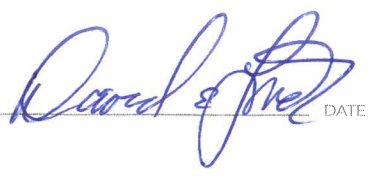
THIS ORDER, TOGETHER WITH THE ADDITIONAL TERMS AND CONDITIONS ATTACHED HERETO, AS OF THE DATE HEREOF COMPRISES THE ENTIRE AGREEMENT AFFECTING THIS PURCHASE AND CANCELS AND SUPERSEDES ANY PRIOR AGREEMENTS. CUSTOMER ACKNOWLEDGES THAT CUSTOMER HAS READ THE TERMS AND CONDITIONS OF THIS ORDER, INCLUDING THE ADDITIONAL TERMS AND CONDITIONS, AGREES TO THE SAME, AND HAS RECIEIVED A TRUE COPY OF THE SAME. THERE ARE NO UNWRITTEN AGREEMENTS BETWEEN THE PARTIES

THERE IS NO AGREEMENT, GUARANTEE, OR WARRANTY OF MERCHANTABILITY OR OTHERWISE, VERBAL OR WRITTEN, EXPRESSED OR IMPLIED, EXTENDING BEYOND THE DESCRIPTION OF THE GOODS ON THE PAGE OF THIS AGREEMENT.

TERMS ARE C.O.D. UNLESS PRIOR ARRANGEMENTS HAVE BEEN APPROVED.

VA Purchasers: CREDIT SALES- FOR SALES INVOLVING DEALER-ARRANGED FINANCING (§ 46.2-1530) SEE VA BUYER'S ORDER ADDENDUM.

RECEIVED BY: _____ DATE 06/26/2024
SALESPERSON

(1) PURCHASER SIGNATURE  DATE 06/26/2024

ACCEPTED BY: _____ DATE 06/26/2024
AUTHORIZED REPRESENTATIVE OF DEALERSH P

(2) PURCHASER SIGNATURE _____ DATE 06/26/2024

BUYER'S ORDER
ADDITIONAL TERMS AND CONDITIONS

1. PARTIES; DEFINITIONS. As used in this Buyer's Order ("Order"), the terms: (a) "Dealer" shall mean the Pete Store Dealer identified on the first page of this Order, to whom this Order is addressed, and who shall become a party hereto only by its acceptance hereof; (b) "Customer" shall mean Customer identified on the first page of this Order and executing this Order as such; (c) "Manufacturer(s)" shall mean the entity or entities that manufactured the Product(s), it being understood by Customer that Dealer is in no respect the agent of Manufacturer(s); and (d) "Product(s)" shall mean the new and/or used vehicle(s) or other products, accessories, or components, which are being purchased by Customer, as set forth in this Order; and (e) "Agreement" means this Buyer's Order and the Additional Terms and Conditions.

2. WARRANTY DISCLAIMERS AND LIMITATIONS. Any warranties on any Product(s) sold hereby are limited only to any printed Manufacturer's warranties delivered to the Customer with the Product(s). EXCEPT FOR ANY SUCH WARRANTIES MADE BY MANUFACTURERS, SAID PRODUCT(S) ARE ON AN "AS IS, WHERE IS" BASIS, AND SOLD WITHOUT ANY WARRANTIES BY DEALER, EXPRESS OR IMPLIED, INCLUDING ANY WARRANTY OF MERCHANTABILITY OR FITNESS FOR A PARTICULAR PURPOSE, EACH OF WHICH ARE EXPRESSLY DISCLAIMED.

NO OTHER WARRANTIES. EXCEPT AS SET FORTH ABOVE, DEALER EXPRESSLY DISCLAIMS ALL WARRANTIES, EXPRESS OR IMPLIED.

3. CHANGES BY MANUFACTURER; MANUFACTURER SURCHARGES; EFFECT ON ORDER. The Manufacturer has reserved the right to change the price to Dealer of any Product that is not currently in Dealer stock, without notice to Dealer. Accordingly, Dealer reserves the right to change the Product price to reflect any prince increases imposed by the Manufacturer. If such Product price is increased by Dealer, Customer may, if dissatisfied there with, cancel this Order in writing within 3-business days of such notice to Customer. In the event the Order is canceled in accordance with this Section 3, and a motor vehicle has been traded in ("Trade-in Vehicle") as a part of the consideration for the Product(s) ordered hereunder, such Trade-in Vehicle shall be either: 1) returned to Customer upon payment of a reasonable charge for storage and repairs (if any); or 2) if such Trade-in Vehicle has been previously sold by Dealer, the amount received therefore, less any expenses incurred and a selling commission of 15% shall be returned to the Customer. In the event Manufacturer shall change or modify the design of any Product after the Order has been accepted by Dealer, the Customer shall have no claim or right against Dealer should the Customer's Product(s) not contain such changes or modifications, nor shall the Dealer be required to effect such changes or modifications to the Product(s).

4. REAPPRAISAL OF TRADE-IN VEHICLE. If the Trade-in Vehicle is not to be delivered to Dealer until delivery to Customer of the Product(s), the Trade-in Vehicle shall be reappraised at that time and such reappraised value shall determine the allowance made for such Trade-in Vehicle. If such reappraised value is lower than the original allowance shown on the front of this Order, Customer may, if dissatisfied therewith, cancel this Order.

5. DELIVERY OF TRADE-IN VEHICLE BY CUSTOMER; CUSTOMER WARRANTY OF TITLE. Customer agrees to deliver to Dealer satisfactory evidence of title to any Trade-in Vehicle at the time of delivery of such Trade-in Vehicle to Dealer. Customer warrants any such Trade-in Vehicle to be Customer's property free and clear of all liens and encumbrances except as otherwise noted herein.

6. PAYMENT; RISK OF LOSS. Customer agrees to complete payment and accept delivery as soon as reasonably practicable, in any event not to exceed thirty (30) days, after notification that the Product(s) are ready. Upon full payment or delivery to the Customer the Product(s) shall be held and used at Customer's risk, including expenses, loss or damage, taxes, and liabilities of any kind.

7. NON-CANCELABLE; REFUSAL OF ACCEPTANCE OF MOTOR VEHICLE; RIGHTS OF DEALER. Once accepted by the Dealer, except as provided in paragraphs 3 and 4 above, this order is non-cancelable by Customer. When Dealer orders the Product(s) from the Manufacturer, Customer understands and acknowledges that the Product is being ordered from the Manufacturer specifically for the Customer and it is not readily available from any other source. Accordingly, Customer shall not cancel or terminate the Order or refuse to accept delivery of the Product(s) except as specifically provided herein. Any attempted cancellation, failure or refusal of Customer to accept del very of any product ordered hereunder and/or to comply with the terms of this Order, or the existence of circumstances, which provide Dealer with a reasonable basis for believing Customer intends to breach its obligations under this Order shall be considered a Breach of this Agreement by Customer and Dealer shall have the right to pursue remedies as outlined in Section 11.

8. DELAY OR FAILURE IN DELIVERY; LIMITATION OF DEALER LIABILITY. Dealer shall not be liable for failure to deliver or delay in delivering any Product(s) covered by this Order where such failure or delay is due, in whole or in part, to any cause beyond the reasonable control, or is without the gross negligence or intentional misconduct of Dealer.

9. LIABILITY FOR TAXES. The price for the Product(s) specified on the face of this Order includes reimbursement to Dealer for Federal Excise taxes paid but does not include sales or use taxes or occupational taxes based on sales volume, (Federal, State or Local) unless expressly so stated. Customer assumes and agrees to pay, unless prohibited by law, any such sales or use or occupational taxes imposed on or applicable to the transaction covered by this Order, regardless of which party may have primary tax liability thereof. Customer is responsible for federal excise tax on add-ons purchased after date of truck sale.

10. CUSTOMER DEPOSIT. Placement of any Customer's deposit into Dealer's account shall not mean that Dealer has accepted this Order. This Order shall not become binding upon Dealer, until accepted and executed by Dealer's authorized representative. A list of authorized representatives may be obtained, by request sent to legal@thepetestore.com or The Pete Store, Attention: Legal, 2441 S. State Road 7, Fort Lauderdale, FL 33317. Any Customer deposit, whether cash or Trade-in Vehicle value, shall not be refunded except due to Dealer's failure to deliver the Product(s).

11. BREACH; DEFAULT; REMEDIES. Upon any breach, threatened breach, or default by Customer, which includes, but is not limited to, Customer's: a) payment method being returned without payment, disputed, or otherwise charged-back to the Dealer; b) failure to make any payment when due; c) failure to deliver Trade-in Vehicle to Dealer; d) failure to deliver Trade-in Vehicle title unencumbered; e) failure to cooperate and sign documents; f) failure or refusal accept delivery of the Product(s), and/or g) failure to perform any of Customer's obligations hereunder, Dealer shall be entitled, at its discretion, to exercise any and/or all of the following remedies: a) demand specific performance; b) cancel this Order and recover its damages, including, but not limited to, incidental and consequential loss caused by such breach; c) retain any cash deposit made by Customer as initial liquidated damages for the breach; d) in the event of a Trade-in, sell such Trade-in and apply the proceeds of such sale to offset the expenses specified in Section 3 above and for such other expenses and losses as Dealer may incur or suffer as a result of the breach; e) repossess the Product(s) without notice; f) cancel or rescind the Order/transaction; g) seek collection for any amounts due plus any damages incurred; and/or h) or any other remedy available at law or in equity. Customer agrees to indemnify and hold Dealer harmless from any breach of any term of this Order, including, but not limited to prepossession or collection costs, attorney's fees and litigation costs. Customer agrees to pay all attorney's fees and litigation costs of Dealer in any litigation or arbitration procedure where Dealer substantially prevails.

12. ADDITIONAL DOCUMENTS. Customer agrees to cooperate and execute all documents required by Dealer to complete the sale/lease of a vehicle. In the event the Buyer's Order must be retyped or changed, Customer agrees to execute a new Buyer's Order so long as there is not a material change in the terms agreed upon.

14. SECURITY AGREEMENT. Customer hereby grants Dealer, its successors and assigns, a security interest in the motor vehicle, equipment and accessories to be purchased pursuant to this agreement and such security interest shall remain in effect until all sums due hereunder have been paid in full.

15. GOVERNING LAW; VENUE; TIME TO COMMENCE ACTION. The rights and obligations of the parties hereunder shall be governed by, and construed and interpreted in accordance with, the laws of the state in which Dealer is located, without regard to conflict of laws principles. The mandatory venue for any claim, litigation, civil action or any other legal or administrative proceeding ("Action") involving any controversy or claim between or among the parties to this Order, is the state in which the Dealer is located. Customer has one (1) year from the accrual of any cause of action arising from the purchase of the Product(s) to commence an action against Dealer.

16. LIMITATION OF DAMAGES. Customer agrees that in the event of any civil action by Customer against Dealer, Customer shall not be entitled to recover any consequential damages as defined in the Uniform Commercial Code, including but not limited to, indirect or special damages, loss of income or anticipated profits, or down-time, or punitive damages.

17. EXECUTION AND DELIVERY BY ELECTRONIC MEANS. Customer acknowledges and agrees that Dealer may require or permit this Order or related documents be signed electronically using electronic signatures. Customer agrees that Customer shall be bound by its electronic signature and by the terms, conditions, requirements contained in any electronic records of this Order or related documents. Such electronic signature shall be considered an original for all purposes.

18. WAIVER; SEVERABILITY. No waiver of any term of this Order shall be valid unless it is in writing and signed by Dealer's authorized representative. Any waiver shall not constitute a continuing waiver. If any provision of part of any provision of this Order shall be deemed to violate any applicable law or regulation, such invalid provision shall be severable and shall not invalidate any other provision or portion thereof, the remainder of this Order shall continue to be binding and enforceable.

19. USED VEHICLE DISCLOSURE. If applicable, the information on the window form for this vehicle(s) is part of this Agreement/Order. Information on the window form overrides any contrary provisions in the contract/order of sale.

THE PETE STORE

City of Lehigh Acers
601 E. County Ln
Lehigh Acers, FL 33936

Bid#FSA23-VEH21.0-HEAVY TRUCKS-ITEM 49,PETERBILT,537,537

2025 model Peterbilt 537 (specifications attached)

Base Chassis Price: $115,029.00

- Meritor RS23- 160 23K AXLE
- PACCAR PX9 Engine $12,116.00
- Allison 3000 Trans
- A/E Connections in EOF Crossmember
- Aluminum Battery Box Cover
- Aero Rearview motorized mirrors
- Cab Air Suspension
- Rear Locking axle
- Full steel inner liner
- Meritor mx-120 10k front drive axle $14,022.00
- Transfer case oil cooler
- Meritor MTC 4210 Transfer Case 2-speed $18,832.00
- Peterbilt Aero 109in BBC SMC Hood

Total Chassis Price:

OX Body -Per specifications attached: $29,843.00

Total Price: $189,842.00

We are pleased to quote Lehigh Acers and look forward to supplying equipment for the future.

Matthew R. Stander- Truck Sales Representative

The Peterbilt Store – Fort Myers (P752)
5313 State Road 82/Mlk
Ft. Myers, Florida 33905

CITY OF LEHIGH ACERS
601E. COUNTY LN
LEHIGH ACERS, Florida 33936
United States of America

Matthew Stander
Cell Phone:
Office Phone:
Email: mstander@thepetestore.com

Vehicle Summary

Unit	
Model:	Model 537
Type:	Full Truck
Description 1:	Lehigh Acers 4x4
Description 2:	Copy of Lehigh Acers
Application	
Intended Serv.:	Construction Dump-On/Off Highway
Commodity:	Construction Materials
Body	
Type:	End Dump
Length (ft):	24
Height (ft):	13.5
Max Laden Weight (lbs):	1000
Trailer	
No. of Trailer Axles:	0
Type:	
Length (ft):	0
Height (ft):	0
Kingpin Inset (in):	0
Corner Radius (in):	0
Restrictions	
Length (ft):	40
Width (in):	102
Height (ft):	13.5

Chassis	
Fr Axle Load (lbs):	10000
Rr Axle Load (lbs):	22700
G.C.W. (lbs):	33000
Road Conditions:	
Class A (Highway)	100
Class B (Hwy/Mtn)	0
Class C (Off-Hwy)	0
Class D (Off-Road)	0
Maximum Grade:	6
Wheelbase (in):	175
Overhang (in):	60
Fr Axle to BOC (in):	69.5
Cab to Axle (in):	105.5
Cab to EOF (in):	165.5
Overall Comb. Length (in):	275
Special Req.	

Approved by: _____ **Date:** _____

Note: All sales are F.O.B. designated plant of manufacture.

Std/ Opt	Description	Weight
Base Model		
S	**Model 537** The Peterbilt Model 537 is highly versatile to take on the big jobs. It is available as a Class 7 truck or tractor with a GVW up to 33,000 lbs. With a full range of suspensions and an available all-wheel-drive configuration, the 537 is the perfect match for applications such as wrecker, tanker, beverage delivery, and municipal utilities just about any job you can throw at it.	9,870
O	**Construction Materials**	0
O	**Construction Dump-On/Off Highway** Truck or tractor without liftable trailing axle which carries bulk materials (excavated earth, sand/gravel, asphalt, demolition debris, etc.) and unloads by dumping out the back, over the side of the body, or out the bottom of the trailer.	0
O	**End Dump**	0
O	**All Wheel Drive Vehicle** All Wheel Drive VehicleFor all vehicles with front drive axle.	0
S	**United States Registry** Canadian Registry PackageRequires Air Conditioning Excise Tax Canada, Speedometer to be KPH ipo MPH, Daytime Running Lights and Rubber Battery Pad in Bottom of Battery Box.	0
Configuration		
S	**Not Applicable** Secondary Manufacturer	0
Frame & Equipment		
S	**10-5/8" Steel Rails to 285"** 10.625 x 3.45 x .313 Dimension, 1,776,000 RBM; Yield Strength: 120,000 psi. Section Modulus: 14.8 cubic inches. Weight: 1.44 lbs/inch pair	74
O	**Full Steel Inner Liner**	489
O	**Heavy Duty 5-Piece Crossmember BOC IPO Standard** Class 5, 6, 7	20
O	**Aluminum Frame Rail Crossmembers** Excludes suspension	-7
O	**3-Piece Suspension Crossmembers**	0
S	**EOF Square without Crossmember** End-of-frame square without crossmember. For use with body builder installed crossmember.	0
S	**Omit Rear Mudflaps and Hangers**	0
Front Axle & Equipment		

Std/ Opt	Description	Weight
O	**Meritor MX-120 10,000 LBS. Front Drive**	1,045
O	**Taper Leaf Springs, Shocks 12,000 lb** Hendrickson taper leaf springs, shocks for 12,000 lbs.	169
S	**Power Steering Sheppard HD94** Sheppard HD94 power steering gear is a light weight version of heavy duty line-haul power steering gear. For use with 10,000 to 13,200 lb. axle ratings.	0
O	**Power Steering Reservoir Frame Mounted** The power steering reservoir is a steering system that eases drivability by applying hydraulic pressure to the steering gear.	0
O	**Hub/Wheel Ends Included With Front Drive Axles**	0
O	**Meritor Wide Track IPO Standard, Front Axle** Meritor wide track front axle offers greater turning radius and ease of service. MFS+ 3.5" Drop / MFS 3.74" Drop. 71in KPI IPO 69in with MFS and MFS+ axles.	15
S	**Gusseted Cam Brackets, Steer Axle**	0
O	**Drum Brakes Included with Front Drive Axle**	0

Rear Axle & Equipment

O	**Meritor RS23-160 23,000 lb**	181
S	**PHP10 Iron PreSet PLUS Hubs**	0
O	**Rear Brake Camshaft Reinforcement** Rear brake camshaft reinforcement helps guard against wear and corrosion.	9
O	**Gusseted Cam Brackets, Drive Axle(s)**	2
S	**SBM Valve** Full trucks require a spring brake modulation (SBM) system for emergency braking application. This system requires an SBM valve and a relay valve with spring brakes on the rear axles. The SBM valve allows the foot valve to operate the rear axle spring brakes if a failure exists in the rear air system.	0
O	**Meritor Q+ Air Cam Rear Drum Brakes 16.5x8.625**	5
O	**Diff Lock, Single Drive Axle with Speed Interlock**	30
S	**Stability System Not Selected Or Not Available**	0
S	**Anti-Lock Braking System (ABS) 4S4M** ABS-6. Includes air braking system.	0
S	**Synthetic Axle Lubricant All Axles** Peterbilt heavy duty models include Fuel Efficient Cognis EMGARD FE75W-90 which provides customers performance advantages over current synthetic lubricants with reduced gear wear and extended	0

Std/ Opt	Description	Weight
	maintenance intervals, resulting in increased uptime. In addition, the lubricant provides improved fluid flow to protect gears in extreme cold conditions and withstand the stress from high temperatures, extending component life.	
O	**Ratio 5.13 Rear Axle**	0
O	**Reyco 79KB Multi Leaf 23,000 Lb**	-35

Engine & Equipment

O	**PACCAR PX-9 350@2000 GOV@2200 1050@1200** Productivity (2024 Emissions) N21350 C121 64....Maximum Accelerator Pedal Ve N21370 C128 64....Maximum Cruise Speed (P059) N21460 C132 1400..Max PTO Speed (P046) N21520 C133 5.....Timer Setting (P030) N21610 C188 39....Low Ambient Temperature Thre N21620 C189 60....Intermediate Ambient Tempera N21630 C190 80....High Ambient Temperature Thr N21550 C206 35....Engine Load Threshold (P516) N21340 C209 120...Hard Maximum Speed Limit (P1 N21510 C225 YES...Enable Idle Shutdown Park Br N21450 C231 NO....Gear Down Protection (P026) N21570 C233 NO....Idle Shutdown Manual Overrul N21440 C234 NO....Engine Protection Shutdown (N21480 C238 NO....Auto Engine Brake In Cruise N21470 C239 NO....Cruise Control Auto Resume (N21430 C333 0.....Reserve Speed Limit Offset (N21410 C334 0.....Maximum Cycle Distance (N202 N21590 C382 YES...Enable Hot Ambient Automatic N21500 C395 0.....Expiration Distance (N209) N21530 C396 YES...Enable Impending Shutdown Wa N21540 C397 60....Timer For Impending Shutdown N21320 C399 120...Standard Maximum Speed Limit N21400 C400 252...Reserve Speed Function Reset N21420 C401 10....Maximum Active Distance (N20 N21330 C402 0.....Expiration Distance (N207)	475
S	**VMUX Electronics Architecture**	0
O	**Engine Idle Shutdown Timer Enabled**	0
O	**Enable EIST Ambient Temp Overrule**	0
	Eff EIST NA Expiration Miles	0
	Effective VSL Setting NA	0
O	**Typical Operating Speed 65 MPH**	0

Std/ Opt	Description	Weight
O	**Powertrain Optimized for Balance** Best analysis for vehicles where service includes frequent start and stop cycles.	0
S	**Remote PTO/Throttle, 12-Pin, 250K BOC/BOS** J1939, Remote Control Provision	0
O	**EPA Emission Warranty**	0
S	**EPA Engine Idling Compliance**	0
S	**PACCAR 160 Amp Alternator, Brushed** PACCAR 160 AMP alternator, brushed producing 160 Amps at road speed and 100 Amps at idle.	0
O	**PACCAR 12V Starter, N/A PACCAR MX Engines** PACCAR 12-volt electrical system. With centralized power distribution incorporating plug-in style relays. Circuit protection for serviceability, 12-volt light system w/circuit protection circuits number & color coded.	0
S	**2 PACCAR Premium 12V Starting Batteries 2000 CCA** Threaded stud type terminal. Stranded copper battery cables are double aught (00) or larger to reduce resistance.	0
O	**Low Voltage Disconnect System**	0
S	**HORTON ON/OFF Fan Clutch** Horton DM Advantage Two-Speed Fan Clutch On/Off for heavy-duty, medium-duty and a variety of off-highway equipment.	0
S	**18.7 CFM Air Compressor** N/A X15. Furnished on engine. Teflon lined stainless steel braided compressor discharge line.	0
S	**VGT Exhaust Brake** (Variable Geometry Turbo). Provides approximately 90-100 HP of retardation and is part of the turbocharger.	0
S	**PACCAR Fuel/Water Separator Standard Service** PACCAR Fuel/Water separator standard service intervals. High efficiency media protects critical engine components.	0
S	**No Fluid Heat Option for Fuel Filter**	0
S	**No Electric Heat Option for Fuel Filter**	0
S	**High Efficiency Cooling System** Cooling module is a combination of steel and aluminum components, with aluminum connections to maximize performance and cooling capability. Silicone radiator & heater hoses enhance value, durability, & reliability. Constant tension band clamps reduce leaks. Chevron Delo Extended Life Coolant (NOAT) extends maintenance intervals reducing maintenance costs. Anti-freeze effective to -30 degrees F helps protect the engine. Low coolant level sensor warns of low coolant condition to prevent engine damage. Radiator Size by Model: 520 1202 sq in, 579 1456.9 sq in, 535/536/537/548 949.3 sq in, 537/548 VOC 1000.3 sq in, 389 HH 1604 sq in, 567 1379 sq in.	0
S	**(1) Air Cleaner Engine Mounted**	0

Std/ Opt	Description	Weight
O	**Exhaust Single RH Horizontal** DPF/SCR RH Under Cab, Single Module	158
Transmission & Equipment		
O	**Allison 3000 RDS-P Transmission, Gen 6** Rugged Duty Series	459
O	**SPL170 HD-XL Driveline, 1 Midship Bearing**	95
O	**SPL170 Driveline Btw Main & Aux Trans or T Case** Please note: For NMD the aux trans will be available at a later date	67
O	**PTO F/O Chelsea**	0
O	**(1) Dash Mounted Single Acting EOH PTO Control** Electric-over-hydraulic, spec'ing PTO switch does not ensure the PTO will fit. Sends signal to Allison to engage PTO.	1
O	**Transfer Case Oil Cooler** Oil-To-Water	6
O	**Allison FuelSense Not Desired** Dynamic Shift Sensing	0
O	**Allison Neutral At Stop** Neutral at Stop features and benefits: Reduces or eliminates the load on the engine when vehicle is stopped, can help lower fuel consumption and C02 emissions, and is included in FuelSense 2.0 Plus and Max packages only.	0
O	**Allison 6-Speed Configuration, Close Ratio Gears** 3000 Series Transmissions.	0
O	**Dash Mounted Push Button Shifter** Available with Allison transmissions	3
O	**LH Mounted Trans PTO Provisions**	0
O	**Meritor MTC 4210 Transfer Case, 2-Speed** Use with Meritor MX-120 single drive	620
Air & Trailer Equipment		
O	**Bendix AD-HF EP Air Dryer, Heater** Coalescing filter, extended purge. Bendix AD-HF air filters protects the life of your engine system and components. Proven PuraGuard oil coalescing technonlgy in the the air dryer catridge. This oil coalescing filter ensures the removal of oil and oil aerosols before they can contaminate the moisture removing desiccant.	0
S	**Nylon Chassis Hose**	0
S	**Steel Painted Air Tanks** All air tanks are steel with painted finish except when Code 4543330 Polish Aluminum Air Tanks is also selected (then exposed air tanks outside the frame rails will be polished aluminum). Peterbilt will determine the optimal size and location of required air tanks. Narratives requesting a specific air tank size or location will not be accepted for	0

Std/ Opt	Description	Weight
	factory installation. See ECAT to determine number or location of air tanks installed.	
O	**Body Connections 5' BOC** Junction box contains light and power circuits for body connections located 5' from back-of-cab	4
O	**AE Connections In EOF Crossmember**	-10
Tires & Wheels		
O	**FF: MN 14ply 275/80R22.5 XZE2** Efficiency Rating: Poor A regional, all-position radial with extra-wide, extra-deep tread designed to help deliver exceptional wear in high scrub regional and urban applications. Diameter = 40.2 inches; SLR = 18.6 inches.	0
O	**RR: CN 14ply 295/75R22.5 HDL2 DL EP** Efficiency Rating: Fair	76
O	**Code-rear Tire Qty 04**	0
O	**FF: Accur Stl Armor 29001PK 22.5X7.5** PHP10-5 Hand Holes	16
S	**RR: Accur Stl Armor 51455PK 22.5X8.25** PHP10-5 hand holes.	0
O	**Code-rear Rim Qty 04**	0
Fuel Tanks		
S	**D-Shaped Aluminum 50 Gallon Fuel Tank RH U/C** Non-slip step RH under cab	0
O	**D-Shaped Aluminum 50 Gallon Fuel Tank LH BOC**	0
O	**Location LH BOC 50 Gallon**	0
S	**DEF Tank Mounted LH BOC** Models 220 and 520 mounted left hand back-of-cab.	0
O	**DEF To Fuel Ratio 2:1 Or Greater**	0
S	**DEF Tank Small, HD 14 Gal 2.1M MD 5.5 Gal**	0
Battery Box & Bumper		
S	**MD Battery Box, Non-Slip Step LH Under Cab**	0
O	**Aluminum Battery Box Cover**	4
O	**Aftertreatment Aluminum Non-Slip Cab Entry** Aftertreatment right-hand under cab step. DPF/SCR for diesel engines, catalyst for natural gas engines. On Models 579 specifying chassis fairings, the box is aerodynamic.	0
O	**Steel Bumper Tapered Painted** Two tow points, painted same color as frame. ***Warning*** Due to a new legal regulations, all New Medium Duty trucks require headlights to	40

Std/ Opt	Description	Weight
	be located in the bumper when the chassis height is GREATER than the following chassis height: Measurement to bottom of frame at front of frame: for the 107 Aero Hood 32.6, for the 109 Aero Hood 31.4, for the 109 Vocational Hood 26.7. Work with applications if your chassis height is unacceptable.	

Cab & Equipment

O	**Peterbilt Aero 109in BBC SMC Hood** With molded charcoal crown and aluminum cab.	15
O	**Rubber Fender Lips 4" Extra Wide**	8
S	**Sears Driver Seat**	0
S	**Sears (2) Person Passenger Seat**	0
S	**Toolbox Under Passenger Seat** Non-Suspension Seat	0
S	**Air Ride Driver**	0
O	**High Back Driver**	0
S	**Vinyl Driver**	0
S	**Non-Air Ride Passenger**	0
S	**Low Back Passenger**	0
S	**Vinyl Passenger**	0
S	**Adjustable Steering Column - Tilt/Telescope**	0
S	**Steering Wheel With Peterbilt Logo** Steering Wheel with embossed Peterbilt logo over horn button.	0
S	**Probilt Interior Charcoal Gray - Flat Roof**	0
S	**Day Cab Rear Window** Day cab rear window flush to back of cab.	0
S	**1-Piece Glass Rear Cab Window Fixed**	0
S	**1-Piece Curved Windshield**	0
S	**Power Door Locks and Power Window Lifts** Standard	0
S	**Combo Fresh Air Heater/Air Conditioner** With radiator mounted condenser, dedicated side window defrosters, Bi-Level Heater/Defroster Controls, 54,500 BTU/HR, and silicone heater hoses.	0

Std/ Opt	Description	Weight
S	**Aero Rear View Mirror Housing, Molded Black**	0
O	**Peterbilt Aero Rear View Mirror, Motorized** Includes top mirror with motorized, adjustable dual axis heated glass. Bottom mirror is an integrated convex surface. Includes black textured arms with breakaway feature.	3
S	**Look Down Mirror Over Passenger Door** with Black Housing	0
O	**Air Horn Mounted Under Cab**	8
O	**ConcertClass, AM/FM, Weatherband, 3.5 Aux**	11
O	**Standard Speaker Package For Cab** (2) Speakers	4
O	**SmartLINQ Remote Diagnostics** SmartLINQ is Peterbilts proprietary remote diagnostics service which monitors the engine and aftertreatment for diagnostic codes providing real-time code analysis maximizing vehicle uptime and strengthening the fleets partnership with their dealer. SmartLINQ provides fault coverage for over 800 codes, a customizable email notification for 116 codes plus a web portal to manage your entire fleet included at no additional charge. SmartLINQ is compatible with any telematics system and doesnt require a specific fleet management system. For those whose customers utilize PeopleNet, the pre-wire with remote diagnostics will provide a more integrated solution utilizing the existing SmartLINQ modem. For those whose customers utilize other fleet services products, the existing pre-wire option for the other fleet service devices will continue to be available. Standard on Class 8 engines and available on Models 348, 337, 330 and 325 with a PX-9, PX-7 or Cummins Westport natural gas engine.	0
S	**Peterbilt Electric Windshield Wipers** With Intermittent Feature.	0
O	**Cab Air Suspension**	15
O	**Triangle Reflector Kit, Ship Loose** Florescent triangle emergency road flares are designed to meet and exceed all DOT standards.	13
O	**Fire Extinguisher, Mounted Inboard Driver Seat** Hazmat approved UL listed/rated ABC	9
O	**Backup Alarm (87-112 DB)** Variable adjusting	6
O	**Transmission Oil Temperature (Main)** Located in Digital Cluster Display.	0
S	**Air Restriction Indicator** Mounted on air cleaner, intake piping, or firewall	0
O	**Dual Scale Speedometer MPH Over KPH**	0
S	**Main Instrumentation Panel** Digital Cluster 7" Display includes: Physical (Analog) - Speedometer, Tachometer, Oil Pressure, Coolant Temp and Display Gauges - Fuel	0

Std/ Opt	Description	Weight
	Level, DEF Level, DPF Filter Status, Fuel Economy, Volts Telltale, OAT and Primary Air Pressure, Secondary Air Pressure for air brake trucks.	
S	**Headlights Composite Fender Mounted** Integral DRL/Park, Turn, and Side Marker	0
S	**(5) Marker Lights, Aero LED** Standard Black Housing. Chrome Housing available with Sun Visor.	0
S	**Self-Canceling Turn Signal**	0
S	**LED Stop/Turn/Tail/Backup** Bracket mounted left-hand / right-hand end of frame	0
S	**Moveable EOF Crossmember For Mounting Tail Lights** Square end of frame with or without end of frame crossmember	0
Paint		
O	**Standard Paint Color Selection**	0
S	**(1) Color Axalta Two Stage - Cab/Hood** Base Coat/Clear Coat N85020 A - L0006EY WHITE N85700 BUMPER L0001EA BLACK N85500 CAB ROOF L0006EY WHITE N85300 FENDER L0006EY WHITE N85200 FRAME L0001EA BLACK N85400 HOOD TOP L0006EY WHITE	0
Shipping Destination		
Options Not Subject To Discount		
O	**Peterbilt Severe Service Coverage** 1 Year/50,000 Miles (80,000 km)	0
O	**SmartLINQ RD - 5YR Sub** PACCAR PX Engines	0
O	**Base Warr: Emissions 5YR/100K MI-EPA Engine**	0
Miscellaneous		
S	**Aero Hood (MODEL 537)**	0
O	**State Of Registry: Florida**	0
O	**2024 Series Emissions Engine**	0
O	**Unit Not Drivable w/o Modifications** There is an option on the order that makes the option is not legally allowed to be driven without modification. This does not prevent Active from making modifications (temporary marker lights) to have it is the lead truck.	0

Std/ Opt	Description	Weight

Promotions

Special Requirements

Special Requirement 1 0098170
Special Requirement 2 0098090
Special Requirement 3
Special Requirement 4

Order Comments

Total Weight (lbs)	13,973

Prices and Specifications Subject to Change Without Notice.

Unpublished options may require review/approval.
Dimensional and performance data for unpublished options may vary from that displayed in CRM.

PRICING DISCLAIMER

While we make every effort to maintain the web site to preserve pricing accuracy, prices are subject to change without notice. Although the information in this price list is presented in good faith and believed to be correct at the time of printing, we make no representations or warranties as to the completeness or accuracy of this information. We reserve the right to change, delete or otherwise modify the pricing information which is represented herein without any prior notice. We carefully check pricing specifications, but occasionally errors can occur, therefore we reserve the right to change such prices without notice. We disclaim all liability for any errors or omissions in the materials. In no event will we be responsible for any damages of any nature whatsoever from the reliance upon information from these materials. Please check your order prebills to confirm your pricing information

THE PETE STORE

CONNECTICUT | DELAWARE | FLORIDA | GEORGIA | MARYLAND | MASSACHUSETTS
NORTH CAROLINA | RHODE ISLAND | SOUTH CAROLINA | TENNESSEE | VIRGINIA

Peterbilt

BUYER'S ORDER

PLEASE ENTER MY ORDER FOR THE FOLLOWING: ☑ NEW ☐ USED ☑ TRUCK ☐ TRACTOR ☐ TRAILER

YEAR	MAKE	MODEL	BODY TYPE
2025	Peterbilt	537	Dump

VEHICLE ID-NUMBER	MILEAGE	STOCK #
TBA	TBA	TBA

DEALER'S PRINTED NAME (SELLER): The Peterbilt Store

DEALER'S STREET ADDRESS: 5313 Dr. Martin Luther King Jr Blvd

CITY	STATE	ZIP
Fort Myers	FL	33905

CUSTOMER'S PRINTED NAME (BUYER)
1) Lehigh Acres Municipal Services Improvement District

CUSTOMER'S PRINTED NAME (BUYER)
2)

BUYER'S STREET ADDRESS: 601 E. County Ln

CITY	STATE	ZIP
Lehigh Acres	FL	33936

EMAIL	PHONE NUMBER
mcook@la-msid.com	

TRADE-IN

YEAR	MAKE	MODEL	BODY TYPE
N/A	N/A	N/A	N/A

VEHICLE ID-NUMBER	TRIM	TITLE NO.	MILEAGE
N/A	N/A	N/A	N/A

EXISTING LIEN HOLDER: N/A

ACCOUNT #	BALANCE DUE
N/A	N/A

LIEN HOLDER INFORMATION

LIEN HOLDERS NAME	PHONE NUMBER
N/A	
ADDRESS	ELT CODE

CITY	STATE	ZIP

1) BASE PRICE OF VEHICLE; 2) ACCESSORIES, MODIFICATIONS, OR EXTENDED WARRANTIES:	1) $ 159,505.00
12' Ox Body Dump	$ 29,843.00
Dealer Fee	$ 485.00
Battery Fee	$ 3.00
Tire Fee	$ 6.00
	2) $ 189,842.00

ANY ITEM NOTATED AS "ESTIMATE" OR "QUOTE" IS BASED ON THE BEST INFORMATION AVAILABLE TO THE DEALER AND IS SUBJECT TO CHANGE WHEN THE TRUE AMOUNT IS DETERMINED.

FOR "AS IS" SALE ONLY: I UNDERSTAND THAT THIS VEHICLE IS BEING SOLD "AS IS" WITH ALL FAULTS AND IS NOT COVERED BY ANY DEALER WARRANTY. I UNDERSTAND THAT THE DEALER IS NOT REQUIRED TO MAKE ANY REPAIRS AFTER I BUY THIS VEHICLE. I WILL HAVE TO PAY FOR ANY REPAIRS THIS VEHICLE WILL NEED. (SEE ADDITIONAL TERMS AND CONDITIONS)

06/26/2024
DATE

SIGNATURE

3) TOTAL VEHICLE PRICE		3) $ 189,842.00
4) TRADE IN ALLOWANCE		
A) GROSS ALLOWANCE		
B) LESS BALANCE OWING (ESTIMATED)		
C) NET TRADE-IN (A-B)	$ 0.00	4) $ 0.00
5) VEHICLE CASH PRICE (3-4)	$ 189,842.00	5) $ 189,842.00
6) OTHER CHARGES	$ 200.00	
LICENSE FEES:	TITLE FEE: $ 200.00	TITLE TAX:
UMVFEE	DEALER BUSINESS LICENSE TAX:	
OTHER:		
7) PROCESSING FEE*		6) $ 200.00
8) DOWNPAYMENT		7)
9) BALANCE DUE AT SETTLEMENT		8)
		9) $ 190,042.00

THIS ORDER IS NOT VALID UNLESS SIGNED BY AN AUTHORIZED REPRESENTATIVE OF THE DEALERSHIP

* A PROCESSING FEE IS NOT AN OFFICIAL FEE. A PROCESSING FEE IS NOT REQUIRED BY LAW, BUT MAY BE CHARGED TO BUYERS FOR HANDLING DOCUMENTS RELATING TO THE CLOSING OF A SALE.

NO LIABILITY INSURANCE INCLUDED

CUSTOMER, by the execution of this Buyers Order ("Order"), offers to purchase the vehicle(s) described herein, subject to the terms and conditions set forth in this Order and the Additional Terms and Conditions hereto.

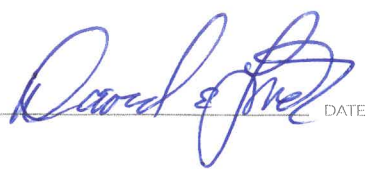
THIS ORDER, TOGETHER WITH THE ADDITIONAL TERMS AND CONDITIONS ATTACHED HERETO, AS OF THE DATE HEREOF COMPRISES THE ENTIRE AGREEMENT AFFECTING THIS PURCHASE AND CANCELS AND SUPERSEDES ANY PRIOR AGREEMENTS. CUSTOMER ACKNOWLEDGES THAT CUSTOMER HAS READ THE TERMS AND CONDITIONS OF THIS ORDER, INCLUDING THE ADDITIONAL TERMS AND CONDITIONS, AGREES TO THE SAME, AND HAS RECIEIVED A TRUE COPY OF THE SAME. THERE ARE NO UNWRITTEN AGREEMENTS BETWEEN THE PARTIES

THERE IS NO AGREEMENT, GUARANTEE, OR WARRANTY OF MERCHANTABILITY OR OTHERWISE, VERBAL OR WRITTEN, EXPRESSED OR IMPLIED, EXTENDING BEYOND THE DESCRIPTION OF THE GOODS ON THE PAGE OF THIS AGREEMENT.

TERMS ARE C.O.D. UNLESS PRIOR ARRANGEMENTS HAVE BEEN APPROVED.

VA Purchasers: CREDIT SALES- FOR SALES INVOLVING DEALER-ARRANGED FINANCING (§ 46.2-1530) SEE VA BUYER'S ORDER ADDENDUM.

RECEIVED BY: _____ DATE 06/26/2024
SALESPERSON

(1) PURCHASER SIGNATURE  DATE 06/26/2024

ACCEPTED BY: _____ DATE 06/26/2024
AUTHORIZED REPRESENTATIVE OF DEALERSH P

(2) PURCHASER SIGNATURE _____ DATE 06/26/2024

BUYER'S ORDER
ADDITIONAL TERMS AND CONDITIONS

1. PARTIES; DEFINITIONS. As used in this Buyer's Order ("Order"), the terms: (a) "Dealer" shall mean the Pete Store Dealer identified on the first page of this Order, to whom this Order is addressed, and who shall become a party hereto only by its acceptance hereof; (b) "Customer" shall mean Customer identified on the first page of this Order and executing this Order as such; (c) "Manufacturer(s)" shall mean the entity or entities that manufactured the Product(s), it being understood by Customer that Dealer is in no respect the agent of Manufacturer(s); and (d) "Product(s)" shall mean the new and/or used vehicle(s) or other products, accessories, or components, which are being purchased by Customer, as set forth in this Order; and (e) "Agreement" means this Buyer's Order and the Additional Terms and Conditions.

2. WARRANTY DISCLAIMERS AND LIMITATIONS. Any warranties on any Product(s) sold hereby are limited only to any printed Manufacturer's warranties delivered to the Customer with the Product(s). EXCEPT FOR ANY SUCH WARRANTIES MADE BY MANUFACTURERS, SAID PRODUCT(S) ARE ON AN "AS IS, WHERE IS" BASIS, AND SOLD WITHOUT ANY WARRANTIES BY DEALER, EXPRESS OR IMPLIED, INCLUDING ANY WARRANTY OF MERCHANTABILITY OR FITNESS FOR A PARTICULAR PURPOSE, EACH OF WHICH ARE EXPRESSLY DISCLAIMED.

NO OTHER WARRANTIES. EXCEPT AS SET FORTH ABOVE, DEALER EXPRESSLY DISCLAIMS ALL WARRANTIES, EXPRESS OR IMPLIED.

3. CHANGES BY MANUFACTURER; MANUFACTURER SURCHARGES; EFFECT ON ORDER. The Manufacturer has reserved the right to change the price to Dealer of any Product that is not currently in Dealer stock, without notice to Dealer. Accordingly, Dealer reserves the right to change the Product price to reflect any prince increases imposed by the Manufacturer. If such Product price is increased by Dealer, Customer may, if dissatisfied there with, cancel this Order in writing within 3-business days of such notice to Customer. In the event the Order is canceled in accordance with this Section 3, and a motor vehicle has been traded in ("Trade-in Vehicle") as a part of the consideration for the Product(s) ordered hereunder, such Trade-in Vehicle shall be either: 1) returned to Customer upon payment of a reasonable charge for storage and repairs (if any); or 2) if such Trade-in Vehicle has been previously sold by Dealer, the amount received therefore, less any expenses incurred and a selling commission of 15% shall be returned to the Customer. In the event Manufacturer shall change or modify the design of any Product after the Order has been accepted by Dealer, the Customer shall have no claim or right against Dealer should the Customer's Product(s) not contain such changes or modifications, nor shall the Dealer be required to effect such changes or modifications to the Product(s).

4. REAPPRAISAL OF TRADE-IN VEHICLE. If the Trade-in Vehicle is not to be delivered to Dealer until delivery to Customer of the Product(s), the Trade-in Vehicle shall be reappraised at that time and such reappraised value shall determine the allowance made for such Trade-in Vehicle. If such reappraised value is lower than the original allowance shown on the front of this Order, Customer may, if dissatisfied therewith, cancel this Order.

5. DELIVERY OF TRADE-IN VEHICLE BY CUSTOMER; CUSTOMER WARRANTY OF TITLE. Customer agrees to deliver to Dealer satisfactory evidence of title to any Trade-in Vehicle at the time of delivery of such Trade-in Vehicle to Dealer. Customer warrants any such Trade-in Vehicle to be Customer's property free and clear of all liens and encumbrances except as otherwise noted herein.

6. PAYMENT; RISK OF LOSS. Customer agrees to complete payment and accept delivery as soon as reasonably practicable, in any event not to exceed thirty (30) days, after notification that the Product(s) are ready. Upon full payment or delivery to the Customer the Product(s) shall be held and used at Customer's risk, including expenses, loss or damage, taxes, and liabilities of any kind.

7. NON-CANCELABLE; REFUSAL OF ACCEPTANCE OF MOTOR VEHICLE; RIGHTS OF DEALER. Once accepted by the Dealer, except as provided in paragraphs 3 and 4 above, this order is non-cancelable by Customer. When Dealer orders the Product(s) from the Manufacturer, Customer understands and acknowledges that the Product is being ordered from the Manufacturer specifically for the Customer and it is not readily available from any other source. Accordingly, Customer shall not cancel or terminate the Order or refuse to accept delivery of the Product(s) except as specifically provided herein. Any attempted cancellation, failure or refusal of Customer to accept del very of any product ordered hereunder and/or to comply with the terms of this Order, or the existence of circumstances, which provide Dealer with a reasonable basis for believing Customer intends to breach its obligations under this Order shall be considered a Breach of this Agreement by Customer and Dealer shall have the right to pursue remedies as outlined in Section 11.

8. DELAY OR FAILURE IN DELIVERY; LIMITATION OF DEALER LIABILITY. Dealer shall not be liable for failure to deliver or delay in delivering any Product(s) covered by this Order where such failure or delay is due, in whole or in part, to any cause beyond the reasonable control, or is without the gross negligence or intentional misconduct of Dealer.

9. LIABILITY FOR TAXES. The price for the Product(s) specified on the face of this Order includes reimbursement to Dealer for Federal Excise taxes paid but does not include sales or use taxes or occupational taxes based on sales volume, (Federal, State or Local) unless expressly so stated. Customer assumes and agrees to pay, unless prohibited by law, any such sales or use or occupational taxes imposed on or applicable to the transaction covered by this Order, regardless of which party may have primary tax liability thereof. Customer is responsible for federal excise tax on add-ons purchased after date of truck sale.

10. CUSTOMER DEPOSIT. Placement of any Customer's deposit into Dealer's account shall not mean that Dealer has accepted this Order. This Order shall not become binding upon Dealer, until accepted and executed by Dealer's authorized representative. A list of authorized representatives may be obtained, by request sent to legal@thepetestore.com or The Pete Store, Attention: Legal, 2441 S. State Road 7, Fort Lauderdale, FL 33317. Any Customer deposit, whether cash or Trade-in Vehicle value, shall not be refunded except due to Dealer's failure to deliver the Product(s).

11. BREACH; DEFAULT; REMEDIES. Upon any breach, threatened breach, or default by Customer, which includes, but is not limited to, Customer's: a) payment method being returned without payment, disputed, or otherwise charged-back to the Dealer; b) failure to make any payment when due; c) failure to deliver Trade-in Vehicle to Dealer; d) failure to deliver Trade-in Vehicle title unencumbered; e) failure to cooperate and sign documents; f) failure or refusal accept delivery of the Product(s), and/or g) failure to perform any of Customer's obligations hereunder, Dealer shall be entitled, at its discretion, to exercise any and/or all of the following remedies: a) demand specific performance; b) cancel this Order and recover its damages, including, but not limited to, incidental and consequential loss caused by such breach; c) retain any cash deposit made by Customer as initial liquidated damages for the breach; d) in the event of a Trade-in, sell such Trade-in and apply the proceeds of such sale to offset the expenses specified in Section 3 above and for such other expenses and losses as Dealer may incur or suffer as a result of the breach; e) repossess the Product(s) without notice; f) cancel or rescind the Order/transaction; g) seek collection for any amounts due plus any damages incurred; and/or h) or any other remedy available at law or in equity. Customer agrees to indemnify and hold Dealer harmless from any breach of any term of this Order, including, but not limited to prepossession or collection costs, attorney's fees and litigation costs. Customer agrees to pay all attorney's fees and litigation costs of Dealer in any litigation or arbitration procedure where Dealer substantially prevails.

12. ADDITIONAL DOCUMENTS. Customer agrees to cooperate and execute all documents required by Dealer to complete the sale/lease of a vehicle. In the event the Buyer's Order must be retyped or changed, Customer agrees to execute a new Buyer's Order so long as there is not a material change in the terms agreed upon.

14. SECURITY AGREEMENT. Customer hereby grants Dealer, its successors and assigns, a security interest in the motor vehicle, equipment and accessories to be purchased pursuant to this agreement and such security interest shall remain in effect until all sums due hereunder have been paid in full.

15. GOVERNING LAW; VENUE; TIME TO COMMENCE ACTION. The rights and obligations of the parties hereunder shall be governed by, and construed and interpreted in accordance with, the laws of the state in which Dealer is located, without regard to conflict of laws principles. The mandatory venue for any claim, litigation, civil action or any other legal or administrative proceeding ("Action") involving any controversy or claim between or among the parties to this Order, is the state in which the Dealer is located. Customer has one (1) year from the accrual of any cause of action arising from the purchase of the Product(s) to commence an action against Dealer.

16. LIMITATION OF DAMAGES. Customer agrees that in the event of any civil action by Customer against Dealer, Customer shall not be entitled to recover any consequential damages as defined in the Uniform Commercial Code, including but not limited to, indirect or special damages, loss of income or anticipated profits, or down-time, or punitive damages.

17. EXECUTION AND DELIVERY BY ELECTRONIC MEANS. Customer acknowledges and agrees that Dealer may require or permit this Order or related documents be signed electronically using electronic signatures. Customer agrees that Customer shall be bound by its electronic signature and by the terms, conditions, requirements contained in any electronic records of this Order or related documents. Such electronic signature shall be considered an original for all purposes.

18. WAIVER; SEVERABILITY. No waiver of any term of this Order shall be valid unless it is in writing and signed by Dealer's authorized representative. Any waiver shall not constitute a continuing waiver. If any provision of part of any provision of this Order shall be deemed to violate any applicable law or regulation, such invalid provision shall be severable and shall not invalidate any other provision or portion thereof, the remainder of this Order shall continue to be binding and enforceable.

19. USED VEHICLE DISCLOSURE. If applicable, the information on the window form for this vehicle(s) is part of this Agreement/Order. Information on the window form overrides any contrary provisions in the contract/order of sale.

THE PETE STORE

City of Lehigh Acers
601 E. County Ln
Lehigh Acers, FL 33936

Bid#FSA23-VEH21.0-HEAVY TRUCKS-ITEM 49,PETERBILT,537,537

2025 model Peterbilt 537 (specifications attached)

Base Chassis Price: $115,029.00

- Meritor RS23- 160 23K AXLE
- PACCAR PX9 Engine $12,116.00
- Allison 3000 Trans
- A/E Connections in EOF Crossmember
- Aluminum Battery Box Cover
- Aero Rearview motorized mirrors
- Cab Air Suspension
- Rear Locking axle
- Full steel inner liner
- Meritor mx-120 10k front drive axle $14,022.00
- Transfer case oil cooler
- Meritor MTC 4210 Transfer Case 2-speed $18,832.00
- Peterbilt Aero 109in BBC SMC Hood

Total Chassis Price:

OX Body -Per specifications attached: $29,843.00

Total Price: $189,842.00

We are pleased to quote Lehigh Acers and look forward to supplying equipment for the future.

Matthew R. Stander- Truck Sales Representative

The Peterbilt Store – Fort Myers (P752)
5313 State Road 82/Mlk
Ft. Myers, Florida 33905

CITY OF LEHIGH ACERS
601E. COUNTY LN
LEHIGH ACERS, Florida 33936
United States of America

Matthew Stander
Cell Phone:
Office Phone:
Email: mstander@thepetestore.com

Vehicle Summary

Unit	
Model:	Model 537
Type:	Full Truck
Description 1:	Lehigh Acers 4x4
Description 2:	Copy of Lehigh Acers
Application	
Intended Serv.:	Construction Dump-On/Off Highway
Commodity:	Construction Materials
Body	
Type:	End Dump
Length (ft):	24
Height (ft):	13.5
Max Laden Weight (lbs):	1000
Trailer	
No. of Trailer Axles:	0
Type:	
Length (ft):	0
Height (ft):	0
Kingpin Inset (in):	0
Corner Radius (in):	0
Restrictions	
Length (ft):	40
Width (in):	102
Height (ft):	13.5

Chassis	
Fr Axle Load (lbs):	10000
Rr Axle Load (lbs):	22700
G.C.W. (lbs):	33000
Road Conditions:	
Class A (Highway)	100
Class B (Hwy/Mtn)	0
Class C (Off-Hwy)	0
Class D (Off-Road)	0
Maximum Grade:	6
Wheelbase (in):	175
Overhang (in):	60
Fr Axle to BOC (in):	69.5
Cab to Axle (in):	105.5
Cab to EOF (in):	165.5
Overall Comb. Length (in):	275
Special Req.	

Approved by: _____ **Date:** _____

Note: All sales are F.O.B. designated plant of manufacture.

Std/ Opt	Description	Weight
Base Model		
S	**Model 537** The Peterbilt Model 537 is highly versatile to take on the big jobs. It is available as a Class 7 truck or tractor with a GVW up to 33,000 lbs. With a full range of suspensions and an available all-wheel-drive configuration, the 537 is the perfect match for applications such as wrecker, tanker, beverage delivery, and municipal utilities just about any job you can throw at it.	9,870
O	**Construction Materials**	0
O	**Construction Dump-On/Off Highway** Truck or tractor without liftable trailing axle which carries bulk materials (excavated earth, sand/gravel, asphalt, demolition debris, etc.) and unloads by dumping out the back, over the side of the body, or out the bottom of the trailer.	0
O	**End Dump**	0
O	**All Wheel Drive Vehicle** All Wheel Drive VehicleFor all vehicles with front drive axle.	0
S	**United States Registry** Canadian Registry PackageRequires Air Conditioning Excise Tax Canada, Speedometer to be KPH ipo MPH, Daytime Running Lights and Rubber Battery Pad in Bottom of Battery Box.	0
Configuration		
S	**Not Applicable** Secondary Manufacturer	0
Frame & Equipment		
S	**10-5/8" Steel Rails to 285"** 10.625 x 3.45 x .313 Dimension, 1,776,000 RBM; Yield Strength: 120,000 psi. Section Modulus: 14.8 cubic inches. Weight: 1.44 lbs/inch pair	74
O	**Full Steel Inner Liner**	489
O	**Heavy Duty 5-Piece Crossmember BOC IPO Standard** Class 5, 6, 7	20
O	**Aluminum Frame Rail Crossmembers** Excludes suspension	-7
O	**3-Piece Suspension Crossmembers**	0
S	**EOF Square without Crossmember** End-of-frame square without crossmember. For use with body builder installed crossmember.	0
S	**Omit Rear Mudflaps and Hangers**	0
Front Axle & Equipment		

Std/ Opt	Description	Weight
O	**Meritor MX-120 10,000 LBS. Front Drive**	1,045
O	**Taper Leaf Springs, Shocks 12,000 lb** Hendrickson taper leaf springs, shocks for 12,000 lbs.	169
S	**Power Steering Sheppard HD94** Sheppard HD94 power steering gear is a light weight version of heavy duty line-haul power steering gear. For use with 10,000 to 13,200 lb. axle ratings.	0
O	**Power Steering Reservoir Frame Mounted** The power steering reservoir is a steering system that eases drivability by applying hydraulic pressure to the steering gear.	0
O	**Hub/Wheel Ends Included With Front Drive Axles**	0
O	**Meritor Wide Track IPO Standard, Front Axle** Meritor wide track front axle offers greater turning radius and ease of service. MFS+ 3.5" Drop / MFS 3.74" Drop. 71in KPI IPO 69in with MFS and MFS+ axles.	15
S	**Gusseted Cam Brackets, Steer Axle**	0
O	**Drum Brakes Included with Front Drive Axle**	0

Rear Axle & Equipment

O	**Meritor RS23-160 23,000 lb**	181
S	**PHP10 Iron PreSet PLUS Hubs**	0
O	**Rear Brake Camshaft Reinforcement** Rear brake camshaft reinforcement helps guard against wear and corrosion.	9
O	**Gusseted Cam Brackets, Drive Axle(s)**	2
S	**SBM Valve** Full trucks require a spring brake modulation (SBM) system for emergency braking application. This system requires an SBM valve and a relay valve with spring brakes on the rear axles. The SBM valve allows the foot valve to operate the rear axle spring brakes if a failure exists in the rear air system.	0
O	**Meritor Q+ Air Cam Rear Drum Brakes 16.5x8.625**	5
O	**Diff Lock, Single Drive Axle with Speed Interlock**	30
S	**Stability System Not Selected Or Not Available**	0
S	**Anti-Lock Braking System (ABS) 4S4M** ABS-6. Includes air braking system.	0
S	**Synthetic Axle Lubricant All Axles** Peterbilt heavy duty models include Fuel Efficient Cognis EMGARD FE75W-90 which provides customers performance advantages over current synthetic lubricants with reduced gear wear and extended	0

Std/ Opt	Description	Weight
	maintenance intervals, resulting in increased uptime. In addition, the lubricant provides improved fluid flow to protect gears in extreme cold conditions and withstand the stress from high temperatures, extending component life.	
O	**Ratio 5.13 Rear Axle**	0
O	**Reyco 79KB Multi Leaf 23,000 Lb**	-35

Engine & Equipment

O	**PACCAR PX-9 350@2000 GOV@2200 1050@1200** Productivity (2024 Emissions) N21350 C121 64....Maximum Accelerator Pedal Ve N21370 C128 64....Maximum Cruise Speed (P059) N21460 C132 1400..Max PTO Speed (P046) N21520 C133 5.....Timer Setting (P030) N21610 C188 39....Low Ambient Temperature Thre N21620 C189 60....Intermediate Ambient Tempera N21630 C190 80....High Ambient Temperature Thr N21550 C206 35....Engine Load Threshold (P516) N21340 C209 120...Hard Maximum Speed Limit (P1 N21510 C225 YES...Enable Idle Shutdown Park Br N21450 C231 NO....Gear Down Protection (P026) N21570 C233 NO....Idle Shutdown Manual Overrul N21440 C234 NO....Engine Protection Shutdown (N21480 C238 NO....Auto Engine Brake In Cruise N21470 C239 NO....Cruise Control Auto Resume (N21430 C333 0.....Reserve Speed Limit Offset (N21410 C334 0.....Maximum Cycle Distance (N202 N21590 C382 YES...Enable Hot Ambient Automatic N21500 C395 0.....Expiration Distance (N209) N21530 C396 YES...Enable Impending Shutdown Wa N21540 C397 60....Timer For Impending Shutdown N21320 C399 120...Standard Maximum Speed Limit N21400 C400 252...Reserve Speed Function Reset N21420 C401 10....Maximum Active Distance (N20 N21330 C402 0.....Expiration Distance (N207)	475
S	**VMUX Electronics Architecture**	0
O	**Engine Idle Shutdown Timer Enabled**	0
O	**Enable EIST Ambient Temp Overrule**	0
	Eff EIST NA Expiration Miles	0
	Effective VSL Setting NA	0
O	**Typical Operating Speed 65 MPH**	0

Std/ Opt	Description	Weight
O	**Powertrain Optimized for Balance** Best analysis for vehicles where service includes frequent start and stop cycles.	0
S	**Remote PTO/Throttle, 12-Pin, 250K BOC/BOS** J1939, Remote Control Provision	0
O	**EPA Emission Warranty**	0
S	**EPA Engine Idling Compliance**	0
S	**PACCAR 160 Amp Alternator, Brushed** PACCAR 160 AMP alternator, brushed producing 160 Amps at road speed and 100 Amps at idle.	0
O	**PACCAR 12V Starter, N/A PACCAR MX Engines** PACCAR 12-volt electrical system. With centralized power distribution incorporating plug-in style relays. Circuit protection for serviceability, 12-volt light system w/circuit protection circuits number & color coded.	0
S	**2 PACCAR Premium 12V Starting Batteries 2000 CCA** Threaded stud type terminal. Stranded copper battery cables are double aught (00) or larger to reduce resistance.	0
O	**Low Voltage Disconnect System**	0
S	**HORTON ON/OFF Fan Clutch** Horton DM Advantage Two-Speed Fan Clutch On/Off for heavy-duty, medium-duty and a variety of off-highway equipment.	0
S	**18.7 CFM Air Compressor** N/A X15. Furnished on engine. Teflon lined stainless steel braided compressor discharge line.	0
S	**VGT Exhaust Brake** (Variable Geometry Turbo). Provides approximately 90-100 HP of retardation and is part of the turbocharger.	0
S	**PACCAR Fuel/Water Separator Standard Service** PACCAR Fuel/Water separator standard service intervals. High efficiency media protects critical engine components.	0
S	**No Fluid Heat Option for Fuel Filter**	0
S	**No Electric Heat Option for Fuel Filter**	0
S	**High Efficiency Cooling System** Cooling module is a combination of steel and aluminum components, with aluminum connections to maximize performance and cooling capability. Silicone radiator & heater hoses enhance value, durability, & reliability. Constant tension band clamps reduce leaks. Chevron Delo Extended Life Coolant (NOAT) extends maintenance intervals reducing maintenance costs. Anti-freeze effective to -30 degrees F helps protect the engine. Low coolant level sensor warns of low coolant condition to prevent engine damage. Radiator Size by Model: 520 1202 sq in, 579 1456.9 sq in, 535/536/537/548 949.3 sq in, 537/548 VOC 1000.3 sq in, 389 HH 1604 sq in, 567 1379 sq in.	0
S	**(1) Air Cleaner Engine Mounted**	0

Std/ Opt	Description	Weight
O	**Exhaust Single RH Horizontal** DPF/SCR RH Under Cab, Single Module	158
Transmission & Equipment		
O	**Allison 3000 RDS-P Transmission, Gen 6** Rugged Duty Series	459
O	**SPL170 HD-XL Driveline, 1 Midship Bearing**	95
O	**SPL170 Driveline Btw Main & Aux Trans or T Case** Please note: For NMD the aux trans will be available at a later date	67
O	**PTO F/O Chelsea**	0
O	**(1) Dash Mounted Single Acting EOH PTO Control** Electric-over-hydraulic, spec'ing PTO switch does not ensure the PTO will fit. Sends signal to Allison to engage PTO.	1
O	**Transfer Case Oil Cooler** Oil-To-Water	6
O	**Allison FuelSense Not Desired** Dynamic Shift Sensing	0
O	**Allison Neutral At Stop** Neutral at Stop features and benefits: Reduces or eliminates the load on the engine when vehicle is stopped, can help lower fuel consumption and C02 emissions, and is included in FuelSense 2.0 Plus and Max packages only.	0
O	**Allison 6-Speed Configuration, Close Ratio Gears** 3000 Series Transmissions.	0
O	**Dash Mounted Push Button Shifter** Available with Allison transmissions	3
O	**LH Mounted Trans PTO Provisions**	0
O	**Meritor MTC 4210 Transfer Case, 2-Speed** Use with Meritor MX-120 single drive	620
Air & Trailer Equipment		
O	**Bendix AD-HF EP Air Dryer, Heater** Coalescing filter, extended purge. Bendix AD-HF air filters protects the life of your engine system and components. Proven PuraGuard oil coalescing technonlgy in the the air dryer catridge. This oil coalescing filter ensures the removal of oil and oil aerosols before they can contaminate the moisture removing desiccant.	0
S	**Nylon Chassis Hose**	0
S	**Steel Painted Air Tanks** All air tanks are steel with painted finish except when Code 4543330 Polish Aluminum Air Tanks is also selected (then exposed air tanks outside the frame rails will be polished aluminum). Peterbilt will determine the optimal size and location of required air tanks. Narratives requesting a specific air tank size or location will not be accepted for	0

Std/ Opt	Description	Weight
	factory installation. See ECAT to determine number or location of air tanks installed.	
O	**Body Connections 5' BOC** Junction box contains light and power circuits for body connections located 5' from back-of-cab	4
O	**AE Connections In EOF Crossmember**	-10
Tires & Wheels		
O	**FF: MN 14ply 275/80R22.5 XZE2** Efficiency Rating: Poor A regional, all-position radial with extra-wide, extra-deep tread designed to help deliver exceptional wear in high scrub regional and urban applications. Diameter = 40.2 inches; SLR = 18.6 inches.	0
O	**RR: CN 14ply 295/75R22.5 HDL2 DL EP** Efficiency Rating: Fair	76
O	**Code-rear Tire Qty 04**	0
O	**FF: Accur Stl Armor 29001PK 22.5X7.5** PHP10-5 Hand Holes	16
S	**RR: Accur Stl Armor 51455PK 22.5X8.25** PHP10-5 hand holes.	0
O	**Code-rear Rim Qty 04**	0
Fuel Tanks		
S	**D-Shaped Aluminum 50 Gallon Fuel Tank RH U/C** Non-slip step RH under cab	0
O	**D-Shaped Aluminum 50 Gallon Fuel Tank LH BOC**	0
O	**Location LH BOC 50 Gallon**	0
S	**DEF Tank Mounted LH BOC** Models 220 and 520 mounted left hand back-of-cab.	0
O	**DEF To Fuel Ratio 2:1 Or Greater**	0
S	**DEF Tank Small, HD 14 Gal 2.1M MD 5.5 Gal**	0
Battery Box & Bumper		
S	**MD Battery Box, Non-Slip Step LH Under Cab**	0
O	**Aluminum Battery Box Cover**	4
O	**Aftertreatment Aluminum Non-Slip Cab Entry** Aftertreatment right-hand under cab step. DPF/SCR for diesel engines, catalyst for natural gas engines. On Models 579 specifying chassis fairings, the box is aerodynamic.	0
O	**Steel Bumper Tapered Painted** Two tow points, painted same color as frame. ***Warning*** Due to a new legal regulations, all New Medium Duty trucks require headlights to	40

Std/ Opt	Description	Weight
	be located in the bumper when the chassis height is GREATER than the following chassis height: Measurement to bottom of frame at front of frame: for the 107 Aero Hood 32.6, for the 109 Aero Hood 31.4, for the 109 Vocational Hood 26.7. Work with applications if your chassis height is unacceptable.	

Cab & Equipment

O	**Peterbilt Aero 109in BBC SMC Hood** With molded charcoal crown and aluminum cab.	15
O	**Rubber Fender Lips 4" Extra Wide**	8
S	**Sears Driver Seat**	0
S	**Sears (2) Person Passenger Seat**	0
S	**Toolbox Under Passenger Seat** Non-Suspension Seat	0
S	**Air Ride Driver**	0
O	**High Back Driver**	0
S	**Vinyl Driver**	0
S	**Non-Air Ride Passenger**	0
S	**Low Back Passenger**	0
S	**Vinyl Passenger**	0
S	**Adjustable Steering Column - Tilt/Telescope**	0
S	**Steering Wheel With Peterbilt Logo** Steering Wheel with embossed Peterbilt logo over horn button.	0
S	**Probilt Interior Charcoal Gray - Flat Roof**	0
S	**Day Cab Rear Window** Day cab rear window flush to back of cab.	0
S	**1-Piece Glass Rear Cab Window Fixed**	0
S	**1-Piece Curved Windshield**	0
S	**Power Door Locks and Power Window Lifts** Standard	0
S	**Combo Fresh Air Heater/Air Conditioner** With radiator mounted condenser, dedicated side window defrosters, Bi-Level Heater/Defroster Controls, 54,500 BTU/HR, and silicone heater hoses.	0

Std/ Opt	Description	Weight
S	**Aero Rear View Mirror Housing, Molded Black**	0
O	**Peterbilt Aero Rear View Mirror, Motorized** Includes top mirror with motorized, adjustable dual axis heated glass. Bottom mirror is an integrated convex surface. Includes black textured arms with breakaway feature.	3
S	**Look Down Mirror Over Passenger Door** with Black Housing	0
O	**Air Horn Mounted Under Cab**	8
O	**ConcertClass, AM/FM, Weatherband, 3.5 Aux**	11
O	**Standard Speaker Package For Cab** (2) Speakers	4
O	**SmartLINQ Remote Diagnostics** SmartLINQ is Peterbilts proprietary remote diagnostics service which monitors the engine and aftertreatment for diagnostic codes providing real-time code analysis maximizing vehicle uptime and strengthening the fleets partnership with their dealer. SmartLINQ provides fault coverage for over 800 codes, a customizable email notification for 116 codes plus a web portal to manage your entire fleet included at no additional charge. SmartLINQ is compatible with any telematics system and doesnt require a specific fleet management system. For those whose customers utilize PeopleNet, the pre-wire with remote diagnostics will provide a more integrated solution utilizing the existing SmartLINQ modem. For those whose customers utilize other fleet services products, the existing pre-wire option for the other fleet service devices will continue to be available. Standard on Class 8 engines and available on Models 348, 337, 330 and 325 with a PX-9, PX-7 or Cummins Westport natural gas engine.	0
S	**Peterbilt Electric Windshield Wipers** With Intermittent Feature.	0
O	**Cab Air Suspension**	15
O	**Triangle Reflector Kit, Ship Loose** Florescent triangle emergency road flares are designed to meet and exceed all DOT standards.	13
O	**Fire Extinguisher, Mounted Inboard Driver Seat** Hazmat approved UL listed/rated ABC	9
O	**Backup Alarm (87-112 DB)** Variable adjusting	6
O	**Transmission Oil Temperature (Main)** Located in Digital Cluster Display.	0
S	**Air Restriction Indicator** Mounted on air cleaner, intake piping, or firewall	0
O	**Dual Scale Speedometer MPH Over KPH**	0
S	**Main Instrumentation Panel** Digital Cluster 7" Display includes: Physical (Analog) - Speedometer, Tachometer, Oil Pressure, Coolant Temp and Display Gauges - Fuel	0

Std/ Opt	Description	Weight
	Level, DEF Level, DPF Filter Status, Fuel Economy, Volts Telltale, OAT and Primary Air Pressure, Secondary Air Pressure for air brake trucks.	
S	**Headlights Composite Fender Mounted** Integral DRL/Park, Turn, and Side Marker	0
S	**(5) Marker Lights, Aero LED** Standard Black Housing. Chrome Housing available with Sun Visor.	0
S	**Self-Canceling Turn Signal**	0
S	**LED Stop/Turn/Tail/Backup** Bracket mounted left-hand / right-hand end of frame	0
S	**Moveable EOF Crossmember For Mounting Tail Lights** Square end of frame with or without end of frame crossmember	0
Paint		
O	**Standard Paint Color Selection**	0
S	**(1) Color Axalta Two Stage - Cab/Hood** Base Coat/Clear Coat N85020 A - L0006EY WHITE N85700 BUMPER L0001EA BLACK N85500 CAB ROOF L0006EY WHITE N85300 FENDER L0006EY WHITE N85200 FRAME L0001EA BLACK N85400 HOOD TOP L0006EY WHITE	0
Shipping Destination		
Options Not Subject To Discount		
O	**Peterbilt Severe Service Coverage** 1 Year/50,000 Miles (80,000 km)	0
O	**SmartLINQ RD - 5YR Sub** PACCAR PX Engines	0
O	**Base Warr: Emissions 5YR/100K MI-EPA Engine**	0
Miscellaneous		
S	**Aero Hood (MODEL 537)**	0
O	**State Of Registry: Florida**	0
O	**2024 Series Emissions Engine**	0
O	**Unit Not Drivable w/o Modifications** There is an option on the order that makes the option is not legally allowed to be driven without modification. This does not prevent Active from making modifications (temporary marker lights) to have it is the lead truck.	0

Std/ Opt	Description	Weight

Promotions

Special Requirements

Special Requirement 1 0098170
Special Requirement 2 0098090
Special Requirement 3
Special Requirement 4

Order Comments

Total Weight (lbs)	13,973

Prices and Specifications Subject to Change Without Notice.

Unpublished options may require review/approval.
Dimensional and performance data for unpublished options may vary from that displayed in CRM.

PRICING DISCLAIMER

While we make every effort to maintain the web site to preserve pricing accuracy, prices are subject to change without notice. Although the information in this price list is presented in good faith and believed to be correct at the time of printing, we make no representations or warranties as to the completeness or accuracy of this information. We reserve the right to change, delete or otherwise modify the pricing information which is represented herein without any prior notice. We carefully check pricing specifications, but occasionally errors can occur, therefore we reserve the right to change such prices without notice. We disclaim all liability for any errors or omissions in the materials. In no event will we be responsible for any damages of any nature whatsoever from the reliance upon information from these materials. Please check your order prebills to confirm your pricing information





Video

Trusted Seller
1/3 of all sales are from returning customers

In-House Experts
Ready to assist

Competitive Shipping
Same day or next day shippir

2004 Oshkosh MK29 MTVR 7 Ton 6x6 Dump Truck

(No reviews) Write A Review

~~$95,999.00~~ $89,999.00

Buy now and pay later with PayPal. Learn more

SKU: 10TDMWE364S081619

Quantity:

1

ADD TO CART

PayPal

Description

2004 Oshkosh MK29 MTVR 7 Ton 6x6 Dump Truck

- 3,971 Miles

- 708 Engine Hours

READ MORE

Related Products



Video

Trusted Seller
1/3 of all sales are from returning customers

In-House Experts
Ready to assist

Competitive Shipping
Same day or next day shippir

2011 Oshkosh MK29A1 MTVR 7 Ton 6x6 Dump Truck With A/C

(No reviews) Write A Review

~~$99,999.00~~ $93,999.00

Pay monthly for purchases of $49-$10,000 with PayPal. Learn more

SKU: 10tdmwe3x4S082319

Quantity:



Video

Trusted Seller
1/3 of all sales are from returning customers

In-House Experts
Ready to assist

Competitive Shipping
Same day or next day shippir

2013 Oshkosh MK29A1 MTVR 7 Ton 6x6 Dump Truck With A/C .

(No reviews) Write A Review

~~$99,999.00~~ $95,999.00

Pay monthly for purchases of $49-$10,000 with PayPal. Learn more

SKU: 10TDMWE377S596828

Quantity:

1

ADD TO CART

PayPal

Description

2013 Oshkosh MK29A1 MTVR 7 Ton 6x6 Dump Truck

- 1,524 Miles

- 378 Engine Hours

READ MORE

Related Products

2004 OSHKOSH MTVR MK30A1 6×6 DUMP TRUCK















(https://p45equipment.com/wp-content/uploads/2025/09/IMG_7512-scaled.jpeg) (https://p45equipment.com/wp-content/uploads/2025/09/IMG_7513-scaled.jpeg) (https://p45equipment.com/wp-content/uploads/2025/09/IMG_7514-scaled.jpeg) (https://p45equipment.com/wp-content/uploads/2025/09/IMG_7515-scaled.jpeg) (https://p45equipment.com/wp-content/uploads/2025/09/IMG_7516-scaled.jpeg) (https://p45equipment.com/wp-content/uploads/2025/09/IMG_7517-scaled.jpeg)

2004 Oshkosh MTVR MK30A1 6×6 Dump Truck

$65,000

For sale is a 2004 Oshkosh MTVR MK30A1 6×6 Dump Truck, a military-grade workhorse built to handle the toughest jobs. Powered by a 450 HP Caterpillar diesel engine and rated at 62,200 GVW, this truck delivers exceptional hauling capacity and strength. With full off-road capability, high ground clearance, and rugged durability, the MK30A1 is ready to move heavy loads on highways, jobsites, or challenging terrain.

2004 Oshkosh MK30A1 MTVR 7 Ton 6×6 Dump Truck

– 10,000 Original Miles

– C-12 Turbo Diesel Engine

– 3 stage Engine Brake

– Allison 7 Speed Automatic Transmission

– 20,000lbs Winch

– ABS

– Air Brakes

– (CTIS) Central Tire Inflation System

– 16.00R20 Tires

– OEM Hard Top Cab and Doors

– 62,200 lb. GVWR

– 14′ Bed Length with standard 34″ Frame Rail Width

– Clean Title

CONTACT US (HTTPS://P45EQUIPMENT.COM/CONTACT/)

PEOPLE ALSO VIEWED





Oshkosh M1070 8×8 with M1000 Heavy Haul Trailer

$150,000

(https://p45equipment.com/inventory/oshkosh-m1070-8x8-with-m1000-heavy-haul-trailer/)



2004 Oshkosh MTVR MK30A1 6×6 Dump Truck

$65,000

(https://p45equipment.com/inventory/2004-oshkosh-mtvr-mk30a1-6x6-dump-truck/)

1

ADD TO CART

PayPal

Description

Oshkosh MK29A1 MTVR 7 Ton 6x6 Dump Truck

- 7,268 Miles

- 262 Engine Hours

READ MORE

Related Products

RESOLUTION 2025-12

A RESOLUTION OF THE BOARD OF COMMISSIONERS OF LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT AUTHORIZING THE DISPOSITION OF SURPLUS PROPERTY;

WHEREAS, the Board of Commissioners (hereafter referred to as the "Board") of Lehigh Acres Municipal Services Improvement District (hereafter referred to as the "District") is authorized and empowered by Chapters 189, 197, and 298, F.S., Chapter 2015-202, Laws of Florida and Chapter 2017-216, Laws of Florida (hereafter referred to as the "Special Acts"), to exercise the certain enumerated general and special powers authorized by the Special Acts and general law; and

WHEREAS, the Board is authorized and empowered in accordance with Section 274.05, Florida Statutes, to declare as surplus any of its property, which is not otherwise disposed of, that is obsolete or the continued use of which is no longer economical or efficient, or no longer serves a useful function; and

WHEREAS, the Board of Commissioners of Lehigh Acres Municipal Services Improvement District shall from time to time find it necessary to declare as surplus its various assets and capital items; and

WHEREAS, the following property set forth herein no longer serves a useful function for District purposes and it is in the best interests of the District to dispose of the property; and

THEREFORE, be it resolved by the Board of Commissioners of Lehigh Acres Municipal Services Improvement District as follows:

The following property no longer serves a useful function for District purposes and is hereby declared surplus property by the District:

ITEMS BOOK VALUE

(See List Attached)

Introduced, considered favorably, and adopted this 17 day of November 2025.

LEHIGH ACRES MUNICIPAL SERVICES

IMPROVEMENT DISTRICT

By: _____

Katyna Hoover, Chair

(DISTRICT SEAL)



Attest: _____

Rebecca Thompson, Secretary

Attachment: Surplus List

EXHIBIT A

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
FIXED ASSET/INVENTORY SURPLUS
SURPLUS RESOLUTION 2025-12

FIXED ASSETS

Asset #	Asset Name	Aquistion Date	Original Costs	Accumulated Depreciation	Book Value	Useful Life	Remaining Life	Disposal Method	Minimum Value
HEAVY EQUIPMENT									
FA 01476 Tag 232	2002 Terex Ta27 Off Road Dump Truck	02/27/18	$ 39,098.17	$ 39,098.17	$ -	60	0	GovDeals	$ 15,000.00
FA 01076 Tag 125	2005 Dodge Ram 2500 4x4	02/23/05	$ 18,869.00	$ 18,869.00	$ -	60	0	GovDeals	$ 3,500.00
FA 01036 Tag 117	2002 Ford F5504x4	09/02/02	$ 116,035.00	$ 116,035.00	$ -	60	0	GovDeals	$ 4,500.00
	Total Heavy Equipment		**$ 174,002.17**	**$ 174,002.17**	**$ -**				**$ 23,000.00**

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: 2026 Legislative Appropriation

Meeting Date: March 16, 2026

Agenda Item # 4A1

☐ **CONSENT** ☐ **ACTION** ☒ **OLD BUSINESS** ☐ **NEW BUSINESS**

Request Initiated By: ☐ **COMMISSIONER** _____ ☐ **ATTORNEY**

☐ **BOARD** ☒ **MANAGER** ☐ **ENGINEER** ☐ **AUDITOR**

1 Recommended motion: N/A

2 What the action accomplishes: Provides a forum to discuss the RISE appropriation.

3 Fiscal Impact:

4 Staff comments:

5 Staff review and recommended approval:

☒ **MANAGER** ☒ **ASSISTANT DISTRICT MGR**

☒ **ATTORNEY** ☐ **FIELD DIRECTOR**

☐ **ENGINEER** ☐ **FINANCE DIRECTOR**

6 Board Action:

☐ **APPROVED** ☐ **DENIED** ☐ **DEFERRED**

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

AGENDA ITEM SUMMARY

Subject: Leeland Bottomless Lake Building

Meeting Date: March 16, 2026

Agenda Item # 4A1

☐ **CONSENT** ☐ **ACTION** ☒ **OLD BUSINESS** ☐ **NEW BUSINESS**

Request Initiated By: ☐ **COMMISSIONER** _____ ☐ **ATTORNEY**

☐ **BOARD** ☒ **MANAGER** ☐ **ENGINEER** ☐ **AUDITOR**

1 Recommended motion: N/A

2 What the action accomplishes: Provides for a forum to discuss the ongoing discussion surrounding the Leeland Bottomless Lake.

3 Fiscal Impact: None at this time.

4 Staff comments:

5 Staff review and recommended approval:

☒ **MANAGER** ☒ **ASSISTANT DISTRICT MGR**

☒ **ATTORNEY** ☐ **FIELD DIRECTOR**

☐ **ENGINEER** ☐ **FINANCE DIRECTOR**

6 Board Action:

☐ **APPROVED** ☐ **DENIED** ☐ **DEFERRED**

AIM Engineering & Surveying, Inc.

March 9, 2026

Board of Commissioners
Lehigh Acres Municipal Services Improvement District
601 East County Lane
Lehigh Acres, FL 33936

Re: Engineer's Report for the March 2026 Board Meeting

Dear Board Members:

The status of current projects and tasks involving the District Engineer is outlined as follows:

General Consulting

- Review the board agenda and prepare the monthly engineering report.
- Review and recommendation for permits within the report timeframe.
- Coordination and planning related to the Sinkhole parcel.
- Lee County Alico/Sunshine Road extension coordination.
- Continued nutrient removal analysis for Rim, Champion & Hendry Canals.
- Permitting polity update development.
- Welcome sign coordination.

Recently Completed Projects

- Southwest Lehigh Groundwater Recharge and Aquifer Restoration (February 2018)
- Hendry Canal Extension Widening (February 2021)
- Halfway Pond – SR 82 Segment 3 (April 2019)
- JUSWMSA Agreement – FDOT (March 2021)
- West Marsh Design (May 2021)
- Weir S-VL-1 Replacement (August 2023)
- CREST Phase 2 (February 2024)

Current Projects Progress

Frank Mann Preserve (Formerly GS-10): The Frank Mann Preserve property was purchased by Lee County Conservation 20/20. A March 2019 Memorandum of Understanding (MOU) between Lee County and LA-MSID allows LAMSID to construct and maintain a stormwater storage and water quality system within the parcel. LA-MSID issued a Purchase Order to AIM in March 2020 to prepare a Land Management Plan as one of the first step requirements of the MOU. Following a presentation to Lee County's Conservation Land Acquisition and Stewardship Advisory Committee (CLASAC) made in December 2020, the committee recommended approval. The LA-MSID Board approved the Land Management Plan in January 2021 for submission to the Lee County Board of County Commissioners. The Lee County Board approved the County Commissioners' plan on their March 2021 agenda. The LA-MSID Board approved a purchase order to AIM for an initial design scope at the August 2021 Board meeting. Lee County cleared a small park access area.

As a result of critical Hurricane Ian recovery efforts, progress on the Frank Mann Preserve project was delayed. LAMSID staff continued to complete exotic vegetation removal & maintenance berm/access work, which aided the design team in understanding the effort involved in re-establishing the berm in the NW corner of the project. The design team

recommended additional survey data to be collected along the northwest berm to aid in the completion of the 90% plans and permit application. Additional topographic survey data was completed in January 2024.

The project was selected to receive $15M in grant funding. LAMSID signed the agreement with SFWMD/FDEP to receive the funds. The purchase order for Phase 1 Post-Design & Construction-Phase services and Phase 2 Surveying, Modeling, Design, Permitting, Post-Design, and Construction Phase services was approved during the June board meeting.

LA-MSID staff have finalized exotic vegetation removal activities, and the final existing conditions were surveyed in late December 2025, as well as new drone aerials. Phase 1 Design is anticipated to be submitted for permit approvals in early 2026, which incorporates the new survey and drone aerials.

AIM and LA-MSID staff are working to amend the agreement to adjust the schedule and potentially incorporate the filter marsh envisioned for Phase 2 to be permitted and constructed in Phase 1. Conceptual filter marsh designs, including a maintenance path, have been developed to be incorporated in Phase 1. Design efforts continue using the new survey data from late December 2025.

Caloosahatchee River and Estuary Storage and Treatment (CREST): LA-MSID purchased approximately 105 acres for the CREST preserve and stormwater treatment area. The LA-MSID Board authorized the design of this project in November 2019. AIM staff had a meeting with LA-MSID staff in April 2020 to review concepts for developing final plans. The design elements include an interconnecting channel between Dog Canal and Hendry Canal to balance flows between the Bedman Creek and Carlos Waterway Drainage Basins. A retention reservoir also provides year-round treatment of suspended solids through increased flow way resident time and storage to attenuate peak stages during storm events. Additionally, wetland plantings provide water quality treatment through nutrient uptake within the circuitous wetland flow pathway.

Due to grant funding, the design plans and construction activities required a phased approach. AIM developed the scope of work for design phasing, post-design, and construction phase services and provided it to LA-MSID staff for review, which partially addressed the required Caracara monitoring. The LA-MSID Board approved a contract with AIM for this effort on 6/21/2021.

CREST Phase 1
Phase 1 of the CREST project is complete, which includes maintenance dredging of the Dog to Hendry canal and replacement and relocation of weir S-H-2.

CREST Phase 2
LA-MSID received a $2 million grant award to complete Phase 2. Bids for Phase 2 construction were received on December 15, 2022. The groundbreaking ceremony was held on February 15, 2023. Turnbull Environmental completed CREST Phase 2 construction in February 2024.

CREST Phase 3
LA-MSID received an additional $4.06 million in grant funds to allow for the construction of the full build-out of the CREST project (Phase 3). AIM worked to complete the additional design and permitting tasks required to revise the plans and permitting approach to include the full project build-out. AIM's Environmental Scientist coordinated with USFWS to clarify construction

restrictions related to the Caracara nesting site. The design team completed the 100% plan set for Phase 3, and the SFWMD permit was issued on May 22, 2023. Phase 3 bids were received on April 18, 2024, with Turnbull Environmental awarded the project at the April 2024 LAMSID board meeting.

The Notice to Proceed with construction was given on May 1, 2024. Phase 3 is due to be substantially completed by August 31, 2025, and finally completed by September 30, 2025. The SFWMD permit modification was submitted on 9/18/2024 to allow for the excavation of the Caracara nest buffer area. On August 20, 2025, LA-MSID received permit approval to excavate the previously protected Caracara nesting site and 300' buffer zone. A time-extension change order was approved to allow the contractor additional time to excavate the Caracara buffer area for additional stormwater storage within CREST. As part of approved change order 01, the contractor will plant a tighter density of plants and trees for Phase 3 while remaining under the awarded grant funding amount. Project/grant funds will also be utilized to repave portions of the roadway surrounding the CREST project that were disturbed by the construction and truck hauling process.

The project team is currently coordinating the design of an enhanced temporary pumping site, to be funded through grant funds, that will allow additional stormwater volume from Dog Canal to be pumped into the CREST project during severe storm events.

Rehydration of Bedman (Creek) Utilizing Storage & Treatment (ROBUST)
LA-MSID gained legislative funds to purchase and improve two parcels within Lehigh Acres: A Sinkhole Parcel (34-44-27-00-00001.0140) and the ROBUST Parcels (36-43-27-01-00000.A000 & 36-43-27-00-00004.0020). AIM conducted Phase I Environmental Reports for the Sinkhole and Robust parcels. The purpose of these assessments was to characterize the environmental conditions based on readily available information & site observations and to identify recognized environmental conditions. To assist with water quality grant funding applications, AIM prepared a conceptual ROBUST nutrient removal analysis.

ROBUST Phase 1
Grant funding for Phase 1 was achieved to purchase the triangular parcel (36-43-27-01-00000.A000), which is a 15-acre site envisioned to provide additional stormwater storage volume, flood mitigation, and water quality benefits to the Bedman Creek Drainage Basin.

ROBUST Phase 2
LA-MSID also received grant funding for Phase 2 of the ROBUST project, which includes the design and construction of the triangular stormwater storage and water quality facility to store flows from Dog Canal, as well as an emergency overflow connection to the Carlow Waterway during extreme storm events. With the grant agreement finalized, LAMSID approved a design scope of services during the January board meeting. The Notice to Proceed is March 10, 2025. The design team and LA-MSID conducted a kick-off meeting to strategize a project approach that best suits LA-MSID.

ROBUST Phase 3
Subsequently, a $1.705M grant was awarded for the ROBUST Phase 3 project, which expands the stormwater storage capacity and bypass benefits by utilizing the floodplain within Bedman Creek (storage) and adding a water control structure with berming to better direct flows from Bedman Creek to the Carlos Waterway (bypass). A grant amendment to the ROBUST project combines the phases into a single ROBUST project. LAMSID approved a scope of work that

includes the additional design, bidding, and construction-phase services related to the now combined Phase 2 & 3 ROBUST project in the form of a change order to Robust Phase 2.

ROBUST Phase 4
Another $1.6M grant was awarded for ROBUST Phase 4, which will allow for the design and construction of a more sophisticated structure for the Bedman to Carlos bypass that includes electrically actuated gates. LA-MSID staff are working to formalize the grant agreement with FDEP and will likely complete another amendment to have Phase 4 funds combined with a singular ROBUST project.

ROBUST (Combined project into a single phase)
AIM's environmental team has completed the environmental survey required for the permit application. Survey data collection is completed. Modeling efforts are nearing completion, although a new S-CW-6 structure is currently being evaluated. ROBUST Design is anticipated to be submitted for permit approvals in early 2026, which will incorporate the S-CW-6 modeling findings, once finalized.

Modeling efforts continue nearing completion, with S-CW-6 still under evaluation. ROBUST Design is ongoing, with permit approvals still on track for submittal in early 2026.

Bottomless Lake (Sinkhole Parcel)
AIM provided LAMSID staff with a conceptual scope, fee, and construction cost to be utilized toward the grant application submittal. AIM completed a high-level Due Diligence report for the parcel to identify current and/or needed permits and buffering requirements. LAMSID gained a Limited Development Order (LDO) permit to construct security fencing along the parcel's frontage adjacent to the roadway.

LA-MSID was awarded $2.2M for the Sinkhole project to move forward. AIM will work with LA-MSID staff on a Grant Work Plan and provide a scope of services for authorization so that the project's design can commence as soon as the grant agreement is finalized. A workshop was held on September 30 with LA-MSID board members and the public.

An architectural review (including their mechanical, electrical, plumbing, and general contractor team members) was conducted on October 2nd to review the building conditions and advise on expected renovation costs.

AIM is currently coordinating efforts to extend the fence around the entire site. The planning team is exploring a turnkey new building solution, such as Romtec, which could provide a cost-effective new construction alternative to an existing building retrofit. Clarification is required from the grant administrators as to whether grant funds can be used for new construction or only for renovation of the existing building. Trail route concepts are being developed.

AIM trail route concepts have been developed and were reviewed by the LA-MSID board at the January board meeting. LA-MSID staff continues to explore options for utilizing grant funds to construct a new building.

AIM and LA-MSID staff continue efforts to create a budgeting estimate to provide funds to construct a new building.

Alico Road Extension Analysis for Sunshine Boulevard

LAMSID was contacted by the Lee County Department of Transportation (Lee DOT) to determine water management requirements for a proposed new road connecting Alico Road to SR 80. The proposed road will connect to Sunshine Blvd at its junction with SR 82. The interchange with SR 82 will be enhanced to facilitate improved flow patterns, and the impervious area of that interchange will be significantly greater than the existing interchange. Sunshine Boulevard will be widened up to and through the junction with Able Canal north of Lee Blvd. The widened road will generate additional runoff, which will need to be managed so that peak water elevations do not increase in Able Canal as it passes under Sunshine Blvd. During a meeting on July 27, 2024, with Lee County DOT and Johnson Engineering, Inc., it was explained that the section of the Sunshine Blvd widening between SR 82 and 23rd Street SW is on a critical path schedule.

During said meeting, Water Science Associates (WSA) was requested to provide an accelerated analysis of the impact of the road widening on water levels and flows in Sailfish Canal, as well as impacts on canal water levels at the junction of Sailfish Canal and Able Canal. A more comprehensive analysis of the impacts of the proposed roadway changes is being negotiated between LA-MSID and Lee County DOT. WSA & AIM completed a draft report of the findings and provided draft recommendations for LAMSID staff and the board to consider to mitigate impacts to the LAMSID drainage system.

A follow-up meeting was conducted in September 2025 to further discuss the ROW capacity topic. The roadway improvements require LC to acquire canal ROW from LAMSID. AIM and WSA worked with LC DOT and Johnson Engineering to agree upon the volume of lost stormwater storage impact to mitigate due to LAMSID's lost canal ROW. A monthly meeting has been established between the stakeholders to keep coordination efforts moving forward. However, the October meeting was postponed to a date to be determined due to Hurricane Milton. During the November meeting, WSA, AIM, and LAMSID staff worked to clarify the acceptable lower and upper limits to calculate the lost ROW storage volume. However, LCDOT expressed a desire to develop an agreement for the entire stretch of the project from SR 82 to SR 80. LAMSID staff met with key LCDOT staff to discuss large-scale improvements that work for both LAMSID and LCDOT in February 2025.

LA-MSID and LCDOT are currently working on finalizing an agreement and means of fund transfer to allow initial modeling and other projects to commence, which will include required timelines for the projects to be completed that correspond to the given roadway phases. Funding for the District-wide modeling effort was on the Lee County Commissioner Board Meeting agenda for January 2026.

Funding for the District-wide modeling effort has been approved, and APEX (formerly known as WSA) has started initial modeling. Potential conflicts of interest between internal APEX companies have been resolved. Lee County has advertised the competitive negotiation for the design and construction of three weirs within Spratt, Moonfish, and Trout Canals, as well as the replacement of S-SF-1.

Centennial Flood Mitigation

LA-MSID coordinated with Lee County and SFWMD regarding the recent road flooding in the Centennial area south of Buckingham Road during the wet season 2024. The land elevation falls to the northwest, whereas the LA-MSID canals direct flow to the southeast. Therefore, flood mitigation concept projects were developed during the Southern Lee County Flood Mitigation Plan, including a stormwater storage area and a new outfall route to the north of Buckham Road. Initial coordination with Lee County has resulted in LA-MISD learning that the project(s)

are low on the County's flood mitigation prioritization list. However, this project is now being considered as one of the projects to analyze further in correlation with the Lee County Sunshine Widening project.

LA-MSID CIP Grant Appropriation Request for Weir Replacement & Retrofitting
AIM prepared cost estimates for the water control structures S-O-1, S-BP-1, S-BL-2, S-R-1, and S-SF-3 to be presented for a grant funding appropriation request for the next legislative session, which were considered at the October LA-MSID board meeting.

S-BP-1 involves full replacement, S-O-1 is being replaced and relocated, whereas S-R-2 and S-SF-3 are slated for modernization upgrades, including electric actuation and remote-control integration. S-BL-2 has been observed to float/tilt, so this structure's improvement includes repairs and modifications to mitigate the potential for floating/tilting, allowing the gates to be safely operated again.

If you have any questions or inquiries, please call my cell at (239) 823-8171.

Sincerely,
AIM Engineering & Surveying, Inc.
Daniel Schroeder, PE, MSE, PMP
Vice President & Civil Group Manager

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT

MANAGER'S REPORT

March 16, 2026

NEW BUSINESS

OLD BUSINESS

- **Interpretive Center –** FDEP has responded with a two-phase plan for LA-MSID to be eligible to add the option of building a new building on the sinkhole parcel. First, we have a signed buy-off from Senator Passidomo and Representative Giallombardo that they are amending their approval. Second, we would submit the plan to FDEP attorneys to see if there is a legal way to proceed. Robert Williams from Lewis, Longman and Walker is approaching the two legislators as we speak.

- **Frank Mann Preserve-Greenbriar Connector Parcels**– Staff has bid, won and purchased tax certificates for five larger parcels that lie between and enroute from Frank Mann Preserve to Greenbriar Swamp. Our long-term goal is to call them to sale and purchase them. The parcels either contain District facilities or are deep storage areas that we can use, or both. We should be able to call them to sale after April 1, 2026.

- **Buckingham Trails Preserve Reservoir Project –** Since the rainfall event in June 2024, the possibility of this project has gained steam. It was discussed at the Lee County/SFWMD meeting and was discussed at the Lehigh Watershed Initiative meeting in July '24. I will be discussing this with LDOT, in relation to the Sunshine widening. We waiting for the County's first draft of a comprehensive agreement The latest discussions have centered on adding this project to CDBG federal funding.

- **CREST –** Phase II is complete**.** The third phase – construction has begun on May 6th, 2024. We have begun the process of relocating Gopher Tortoises to an approved site, so that we can excavate the entire reservoir. We have won a Sustainable Project Award from the American Society of Civil Engineers, SW FL Branch for CREST, Phase III! The project is up for Sustainable project for the SE United States! We can expect that we will have a ribbon-cutting in the spring or summer of '26.

- **2025 Culvert Replacement** – Staff has completed the third culvert crossing for 2025. We should be finished with the first, second, third working on the fourth.

- **Parking at West Marsh** – Lee County does have a construction schedule for the West Marsh. Design and permitting has $262,500 allotted in 2024-25 and construction has been allotted $1,750,000 in 2025-26. The design of the project has been made public.

- **ROBUST -** The 2024 request of $2.885-million was partially funded at $1.5-million and that appropriation amount was signed by the Governor and FDEP will administer the grant. We are

awaiting the Grant Agreement from FDEP. The grant agreement has been signed and we signed a design agreement at the January 2025 Board Meeting. We have been awarded $1,705 Million from FDEP to finish the ROBUST Project, Phase III. The Phase II and Phase III project grants and tasks have been combined for ease of implementation. With the award of $1.6-million appropriation, the project now is funded at a total of $4.805-million. The project is in early design.

- **Alico Connector Road and Sunshine Widening–** Lee County is currently drawing up plans to connect Alico Road to Sunshine Blvd. In order to complete this project, they have to determine what they are going to do with in additional stormwater that the project generates. Lee County also wants to widen Sunshine to the north and ultimately connect it to State Road 80. They will need up to 50-acres of District land to accomplish these tasks. LA-MSID performed a modelling study to clarify stormwater discharge issues from the project. The additional run-off is approximately 12 acre-feet and we will lose 304 acre-feet of storage due to the loss of land to the project. We have substantial leverage to guarantee that it all be replaced. A second modelling project has begun to verify the lost amount of storage, based on the Lee County road land acquisition of LA-MSID canal right of way on Sailfish Canal. Staff has submitted a LA-MSID needs assessment to the County in February. The County and LA-MSID have tentatively on our modelling, build three new structures in the southwest, rebuild structure S-SF-1 and add on to Structure S-P-2. The schedule to begin construction on Phase III Sunshine widening from SR 82 to 23rd St is set for 2029. As of early, September 2025, we have asked the county to release funding in phases so that the modeling can begin that will answer everyone's questions. The County has funded the modeling; the study is underway. Mike Cook has been selected to be a non-voting member on the design engineering selection committees for the three new LA-MSID weirs and the rebuild of our Sailfish 1 structure.

- **Potential Lawsuits –**A June 2023 accident at Able Canal and Magnolia St. has been presented. I am not aware of any currently active lawsuits.

- **District Manager Enters Drop –** As of November 1st 2019, I officially entered into the drop program. The Legislature and the Governor have passed a law that will allow all participants to add 3 full years to their DROP for a total of eight. I have a new contract that will give me the opportunity to continue my employment up to October 31, 2027.

- **GS-10, Frank Mann Preserve** –The MOU with Lee County is fully executed. It explains the separation of duties between the two agencies LA-MSID must complete the Land Management Plan before we can move on to design. Our commissioners approved the Land Management Plan on January 25th, 2021. AIM Engineering has started the final design of the project as of September 1st, 2021. Our staff is mobilized and are doing a fantastic clearing job in GS 10. In 2024, Governor DeSantis awarded LA-MSID a $15.5 million grant, thanks to recommendations from Lee County and SFWMD. The grant will be administered through SFWMD. We are currently developing a contract that will govern how the money will be spent. The permits should be applied for within a few months. The final documents with SFWMD for funding have been signed, as of July 31, 2025, let's get to work! The additional design for Phase I is under way. We have recently heard that federal permitting can take from 10-15 months to complete. It appears that there will be some non-consequential changes in our agreement with SFWMD.

- **Greenway Agreement –**The agreement has been signed by all parties. The project is currently under design with Lee County Parks. We have sent staff and AIM design

comments to the County. They are at 90% design as of May 2022. Construction is slated to start in FY 25-26. The construction contract was signed in June 2025. Construction has begun.

- **Contract Renewals –** Renewal dates for the Board's consultants:

  AIM Engineering – March 1st of each year

  Hultman and Joshi Law Firm – February 24th of each year.

  David E. Lindsay – October 31, 2027

  Lewis, Longman & Walker – Potential Increase in December

COMMUNITY OUTREACH

Community Outreach Education LA-MSID is continually researching community outreach, NPDES and other relevant grants that could benefit the District. Staff continues to build relationships with other governmental, media and community groups in order to enhance LA-MSID's image and raise awareness.

The District has been getting great media coverage this year. Staff is continually working with staff to identify newsworthy items and has secured great coverage.

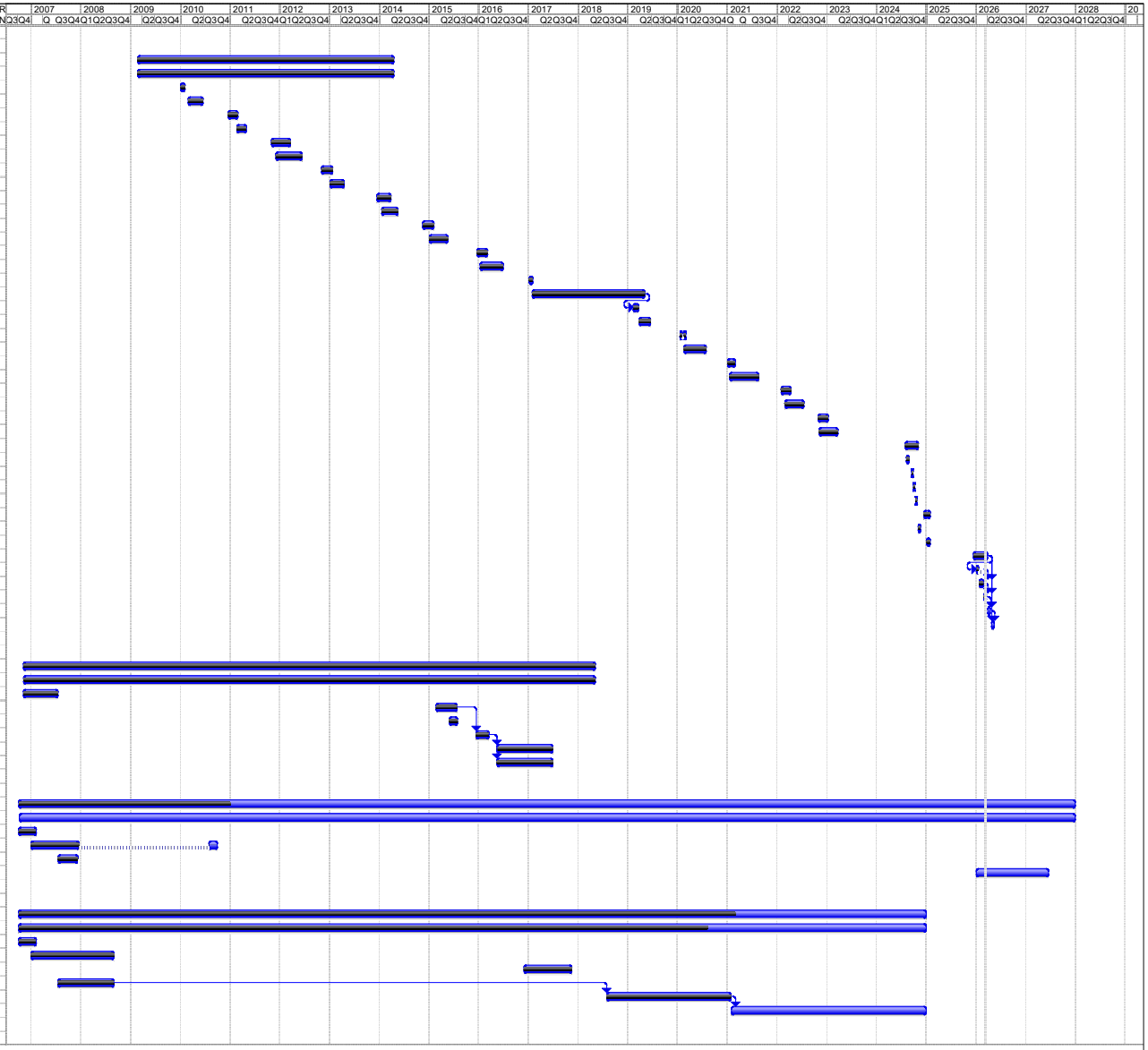
- Social Media Page is up and active.
- Staff is utilizing social media for hurricane preparedness education and to update residents on services like our mowing schedule, etc. Staff completed a hurricane and staff training day on June 25th to review hurricane procedures and provide safety training.
- Staff is reviewing policies to stay on top of our goals and keep the public up to date on our projects.
- We are continuing with our consistent social media posts.
- Staff participated in the fire department's Community Safety & Fire Prevention Day on October 18th.
- The Wings Over Water Festival was January 24, 2026! We have a fantastic setup of vendors for every area including our farmer's market, which has really grown this year.
- Staff executed Harns Marsh birding tour on 11/1 and 3/7. The tours were full and very well received by members of the community and the Audubon of the Western Everglades.

EMPLOYEES

- **Employee of the Month for March '26 is Phillip Butler**. Philip is our new Mechanic's Helper. Since he has been hired on, he has acquired his Hazmat Certification for his CDL License. He has been praised by several employees for his willingness and ability to learn. Philip has made many employee's jobs easier by his efficiency. Philip is a pleasant guy of few words.

Upcoming Events: **FASD** June 7-11, 2026 – Annual Conference - Orlando
FSA June 17-19, 2026 – Annual Conference - Charlote Harbor

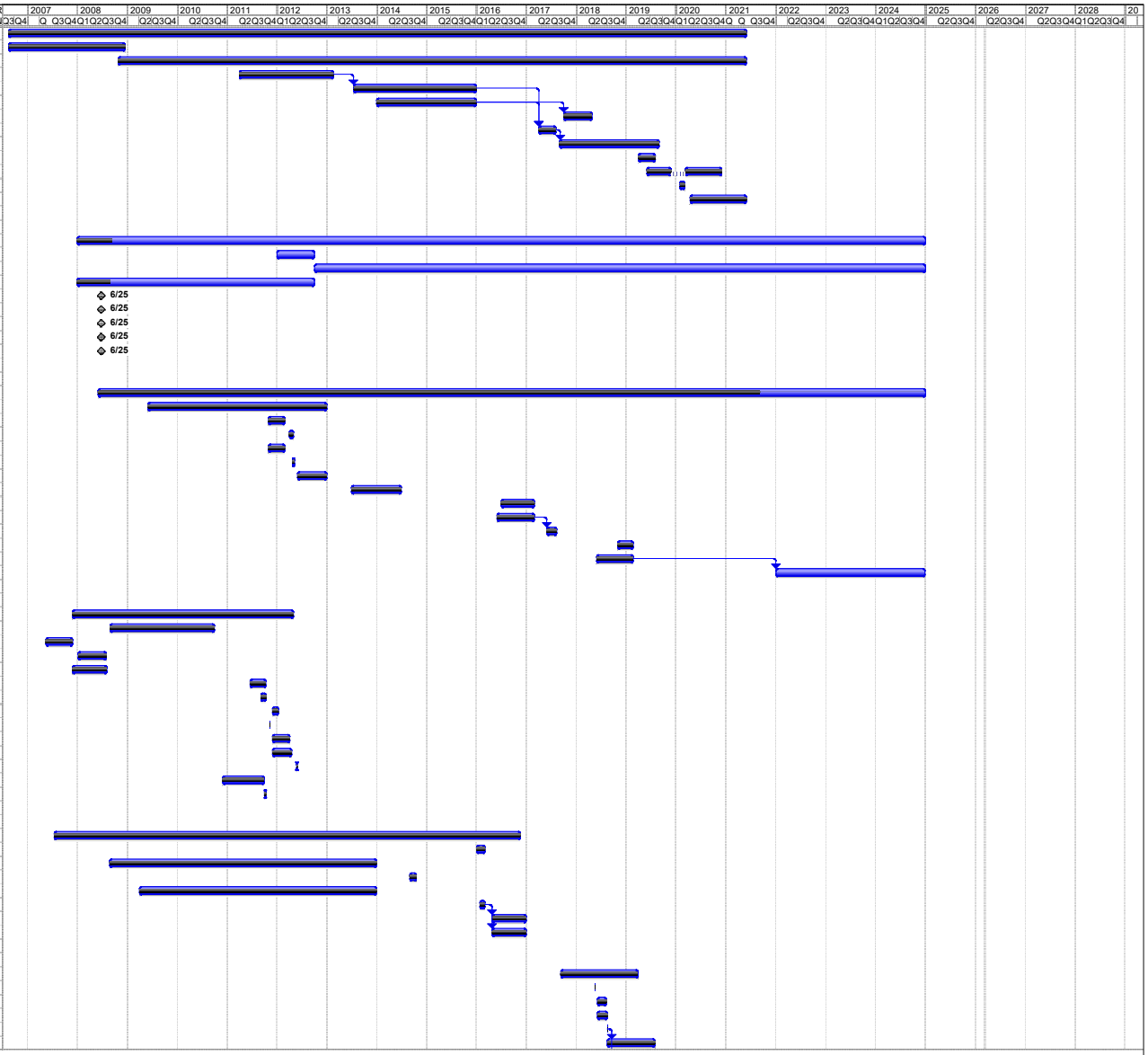
ID	ID	ID	Priori	Task Name	Duration	Start	Finish	P	R
1	1	1							
2	2	2							
3	3	3	0	2007 Facility Review Critical Culverts (25 Culverts)	1345 days?	Tue 2/24/09	Fri 4/18/14		
4	4	4	29	Funding (25 Culverts)	1348 days?	Tue 2/24/09	Fri 4/18/14		
5	5	5	30	Design (5 Culverts In-House - 2010)	25 days	Mon 1/4/10	Fri 2/5/10		
6	6	6	31	Construction (5 Culverts In-House - 2010)	79 days	Mon 3/1/10	Thu 6/17/10		
7	7	7	32	Design (5 Culverts In-House - 2011)	51 days?	Mon 12/20/10	Mon 2/28/11		
8	8	8	33	Construction (5 Culverts In-House 2011)	50 days	Tue 2/22/11	Mon 5/2/11		
9	9	9	34	Design (5 Culverts In-House 2012)	100 days	Thu 11/3/11	Tue 3/20/12		
10	10	10	35	Construction (5 Culverts In-House 2012)	139 days	Mon 12/5/11	Thu 6/14/12		
11	11	11	36	Design (5 Culverts In-House 2013)	60 days	Mon 11/5/12	Thu 1/24/13		
12	12	12	37	Construction (5 Culverts In-House 2013)	75 days	Mon 1/7/13	Fri 4/19/13		
13	13	13	38	Design (6 Culverts In-House 2014)	75 days	Mon 12/16/13	Fri 3/28/14		
14	14	14	39	Construction (6 Culverts In-House 2014)	85 days	Tue 1/21/14	Mon 5/19/14		
15	15	15	40	Design (4 Culverts In-House 2015)	60 days	Mon 11/17/14	Fri 2/6/15		
16	16	16	41	Construction (4 Culverts In-House 2015)	99 days	Mon 1/5/15	Thu 5/21/15		
17	17	17	42	Design (5 Culverts In-House 2016)	56 days?	Mon 12/21/15	Mon 3/7/16		
18	18	18	43	Construction (5 Culverts In-House 2016)	122 days	Wed 1/13/16	Thu 6/30/16		
19	19	19	100	Design (5 Culverts In-House 2017)	19 days?	Mon 1/9/17	Thu 2/2/17		
20	20	20	101	Construcion (5 Culverts In-House 2017)	591 days?	Mon 1/30/17	Mon 5/6/19		
21	21	21	200	Design (6 Culverts In-House 2019)	30 days?	Mon 2/11/19	Fri 3/22/19	2	
22	22	22	201	Construction (6 Culverts In-House 2019)	60 days	Tue 3/26/19	Mon 6/17/19		
23	23	23	202	Design (6 Culverts In-House 2020)	21 days	Tue 1/21/20	Mon 3/2/20		
24	24	24	203	Construcion (6 Culverts In-House 2020)	118 days	Tue 2/18/20	Thu 7/30/20		
25	25	25	500	Design (6 Culverts In-House 2021)	41 days?	Mon 1/4/21	Mon 3/1/21		
26	26	26	500	Construcion (6 Culverts In-House 2021)	153 days?	Tue 1/19/21	Thu 8/19/21		
27	27	27	500	Design (5 Culverts In-House 2022)	53 days	Tue 2/1/22	Thu 4/14/22		
28	28	28	500	Construction (5 Culverts In-House 2022)	100 days	Tue 3/1/22	Mon 7/18/22		
29	29	29	500	Design (1 Culverts In-House 2023)	54 days?	Mon 10/31/22	Thu 1/12/23		
30	30	30	500	Construction (1 Culverts In-House 2023)	100 days	Mon 11/7/22	Fri 3/24/23		
31	31	31	500	Design (4 Culverts In-House 2024)	68 days	Thu 8/1/24	Mon 11/4/24		
32	32	32	500	Construction (59th St. W. Culvert In-House 2024)	15 days?	Fri 8/9/24	Thu 8/29/24		
33	33	33	500	Construction (51st St. W. Culvert In-House 2024)	9 days?	Mon 9/16/24	Thu 9/26/24		
34	34	34	500	Construction (Centennial Blvd. @ Champion Canal In-House 2024)	9 days?	Mon 9/30/24	Thu 10/10/24		
35	35	35	500	Construction (Billings St. Outfall @ Adaline Canal In-House 2024)	8 days?	Tue 10/15/24	Thu 10/24/24		
36	36	36	500	Design (2 Culverts In House 2025)	34 days	Mon 12/16/24	Thu 1/30/25		
37	37	37	500	Construction (Ridgemont Drive @ Webworm Canal)	14 days?	Mon 11/4/24	Thu 11/21/24		
38	38	38	500	Construction (Magenta Ave @ Longwing Canal (47-4-2)	19 days?	Mon 1/6/25	Thu 1/30/25		
39	39	39	500	Design (5 Culverts In-House 2026)	75 days	Mon 12/15/25	Fri 3/27/26		
40	40	40	500	Construction (Culvert #1) Greenbriar Blvd. (W. of Richmond Ave.)	12.35 days	Mon 1/5/26	Thu 2/26/26	3	
41	41	41	500	Construction (Culvert #2) Roderick Ln. (N. of Windermere Dr)	24 days	Mon 1/26/26	Thu 2/26/26	3	
42	42	42	500	Construction (Culvert #3) Butler St. E. (E. of McArthur)	14 days	Mon 3/2/26	Thu 3/19/26	3	
43	43	43	500	Construction (Culvert #4) Butler St. E. (W. of McArthur)	14 days?	Mon 3/30/26	Thu 4/16/26	3	
44	44	44	500	Construction (Culvert #5) Butler Ave. S. (N. of Grant)	14 days	Mon 4/27/26	Thu 5/14/26	3	
45	45	45							
46	46	46							
47	47	47	0	SW Lehigh New Structures (SW Structures)	3001 days	Tue 11/7/06	Mon 5/7/18		
48	48	48	46	Funding (SW Structures)	3001 days	Thu 11/9/06	Mon 5/7/18		
49	49	49	47	Pre Engineering (SW Structures)	181 days?	Tue 11/7/06	Tue 7/17/07		
50	50	50	48	Revise Design (SW Structures)	110 days	Tue 2/24/15	Mon 7/27/15		
51	51	51	49	Revise SFWMD Permit (SW Structures)	45 days	Mon 6/1/15	Fri 7/31/15		
52	52	52	50	Bidding (SW Structures)	66 days	Thu 12/17/15	Thu 3/17/16	5	
53	53	53	51	Electric (SW Structures)	295 days	Mon 5/16/16	Fri 6/30/17	5	
54	54	54	51	Construction (SW Structures)	295 days	Mon 5/16/16	Fri 6/30/17	5	
55	55	55							
56	56	56							
57	57	57	0	Hickey's Creek Canal Widening (H.C. Widening)	5544 days?	Wed 10/4/06	Fri 12/31/27		
58	58	58	55	Funding (H.C. Widening)	5544 days?	Mon 10/9/06	Fri 12/31/27		
59	59	59	56	Pre Engineering (H.C. Widening)	93 days?	Wed 10/4/06	Fri 2/9/07		
60	60	60	57	Design (H.C. Widening)	297 days?	Tue 1/2/07	Thu 9/30/10		
61	61	61	58	Permit (H.C. Widening)	104 days	Fri 7/20/07	Wed 12/19/07		
62	62	62	59	Construction (H.C. Widening)	380 days	Mon 1/5/26	Fri 6/18/27		
63	63	63							
64	64	64							
65	65	65	0	Hendry Extension Canal Widening (H. Ext. Widening Phase I & II)	4764 days?	Wed 10/4/06	Tue 12/31/24		
66	66	66	62	Funding (H. Ext. Widening - Phase I)	4764 days?	Wed 10/4/06	Tue 12/31/24		
67	67	67	63	Pre Engineering (H. Ext. Widening - Phase I & II)	93 days?	Wed 10/4/06	Fri 2/9/07		
68	68	68	64	Design (H. Ext. Widening - Phase I & II)	435 days?	Tue 1/2/07	Mon 9/1/08		
69	69	69	65	Revised Design to FDOT Standard - Phase I	250 days	Thu 12/1/16	Wed 11/15/17		
70	70	70	66	Permit (H. Ext. Widening Phase I & II)	292 days?	Fri 7/20/07	Mon 9/1/08		
71	71	71	67	Construction (H. Ext. Widening - Phase I)	653 days	Wed 8/1/18	Fri 1/29/21	7	
72	72	72	500	Construction (Phase II) North of Milwaukee Blvd.	1022 days	Mon 2/1/21	Tue 12/31/24	7	
73	73	73							
74	74	74							



Project: Construction Project Schedule
Date: Mon 3/9/26

Task | Split | Progress | Milestone | Summary | Project Summary | External Tasks | External Milestone | Deadline

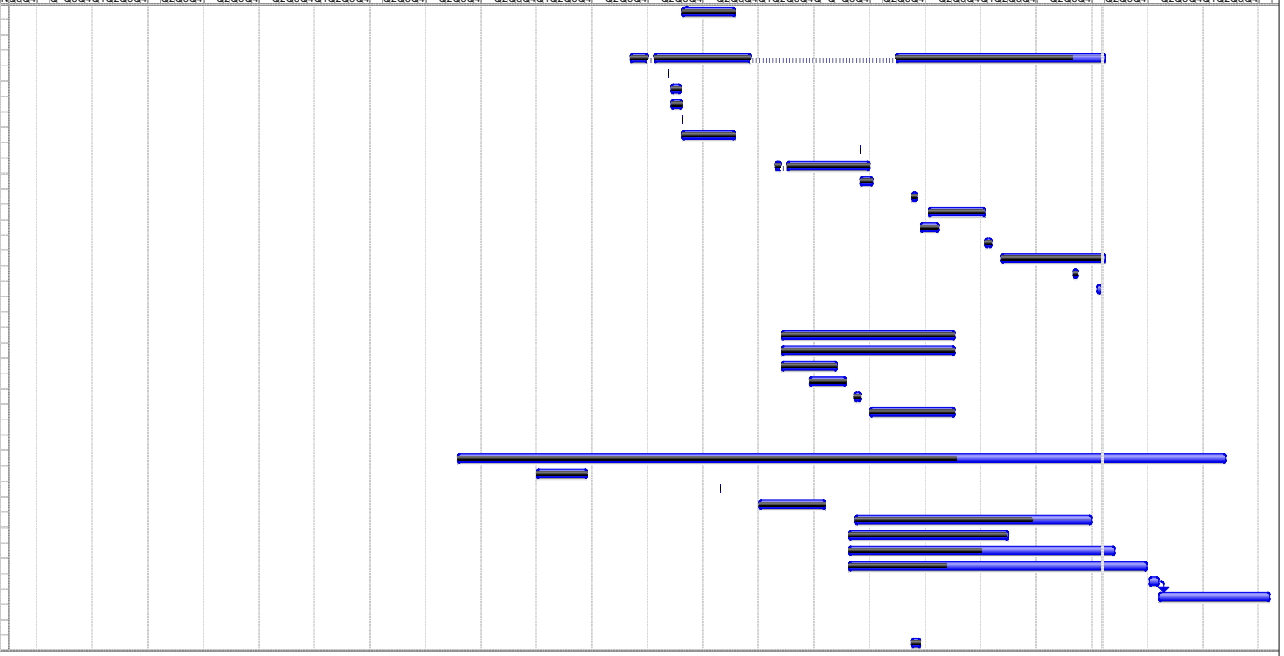
ID	ID	ID	Priori	Task Name	Duration	Start	Finish	P	R
75	75	75	0	West Marsh (W. Marsh) Phase I, II, & III	3859 days?	Thu 8/17/06	Mon 5/31/21		
76	76	76	70	Land Purchase (W. Marsh)	609 days	Thu 8/17/06	Mon 12/15/08		
77	77	77	71	Funding (W. Marsh)	3287 days	Mon 10/27/08	Mon 5/31/21		
78	78	78	72	Pre Engineering (W. Marsh Land Stewardship Plan)	496 days	Fri 4/1/11	Tue 2/19/13		
79	79	79	73	Design (W. Marsh)	643 days	Tue 7/16/13	Thu 12/31/15	7	
80	80	80	74	Permit (W. Marsh)	522 days	Wed 1/1/14	Thu 12/31/15		
81	81	81	79	Electric (W. Marsh)	150 days	Mon 10/2/17	Fri 4/27/18	8	
82	82	82	77	Bidding (W. Marsh Phase I) FDOT Seg. #1	90 days	Mon 4/3/17	Fri 8/4/17	7	
83	83	83	79	Construction (W. Marsh Phase I) FDOT Seg. #1	521 days	Fri 9/1/17	Fri 8/30/19	8	
84	84	84	82	Bidding (W. Marsh Phase II) FDOT Seg. #2	90 days	Mon 4/1/19	Fri 8/2/19		
85	85	85	83	Construction (W. Marsh Phase II) FDOT Seg. #2	317 days	Mon 6/3/19	Tue 12/1/20		
86	86	86	82	Bidding (W. Marsh Phase III) LA-MSID Structures	26 days	Thu 1/30/20	Thu 3/5/20		
87	87	87	85	Construction (W. Marsh Phase III) LA-MSID Structures	294 days	Wed 4/15/20	Mon 5/31/21		
88	88	88							
89	89	89							
90	90	90	0	Hendry Storage Reservoir (H.S. Reservoir)	4436 days?	Tue 1/1/08	Tue 12/31/24		
91	91	91	80	Land Purchase (H.S. Reservoir)	197 days?	Mon 1/2/12	Tue 10/2/12		
92	92	92	81	Funding (H.S. Reservoir)	3196 days?	Wed 10/3/12	Tue 12/31/24		
93	93	93	82	Pre Engineering (H.S. Reservoir)	1241 days	Tue 1/1/08	Tue 10/2/12		
94	94	94	83	Design (H.S. Reservoir)	0 days	Wed 6/25/08	Wed 6/25/08		
95	95	95	84	Permit (H.S. Reservoir)	0 days	Wed 6/25/08	Wed 6/25/08		
96	96	96	87	Electric (H.S. Reservoir)	0 days	Wed 6/25/08	Wed 6/25/08		
97	97	97	85	Bidding (H.S. Reservoir)	0 days	Wed 6/25/08	Wed 6/25/08		
98	98	98	87	Constriction (H.S. Reservoir)	0 days	Wed 6/25/08	Wed 6/25/08		
99	99	99							
100	100	100							
101	101	101	0	Moving Water South (M. Water S.) Phase I, II, & III	4328 days	Mon 6/2/08	Tue 12/31/24		
102	102	102	90	Funding (Halfway Pond Phase I)	940 days	Mon 6/1/09	Mon 12/31/12		
103	103	103	91	Pre Engineering (Halfway Pond Phase I)	87 days	Tue 11/1/11	Tue 2/28/12		
104	104	104	92	Design (Halfway Pond Phase I)	22 days?	Mon 4/2/12	Tue 5/1/12		
105	105	105	93	Permit (Halfway Pond Phase I)	87 days?	Tue 11/1/11	Tue 2/28/12		
106	106	106	94	Bidding (Halfway Pond Phase I)	11 days?	Wed 4/25/12	Wed 5/9/12		
107	107	107	95	Construction (Halfway Pond Phase I)	153 days	Fri 6/1/12	Mon 12/31/12		
108	108	108	96	Pre Engineering (Phase II) "Blackstone Preserve"	261 days	Mon 7/1/13	Mon 6/30/14		
109	109	109	96	Design (Halfway Pond Phase II) "Blackstone Prserve"	173 days	Fri 7/1/16	Tue 2/28/17		
110	110	110	98	Permit (Halfway Pond Phase II) "Blackstone Preserve"	195 days	Wed 6/1/16	Tue 2/28/17		
111	111	111	99	Bidding (Halfway Pond Phase II) "Blackstone Preserve"	52 days	Wed 5/31/17	Thu 8/10/17	1	
112	112	112	500	Construction of Pump Station - (JP Murphy Inc.) Phase II	82 days?	Thu 11/1/18	Fri 2/22/19		
113	113	113	100	Construction of Berm - (LA-MSID) Phase II	191 days	Thu 5/31/18	Thu 2/21/19		
114	114	114	500	Pre Engineering (Phase III) S. of the District	780 days?	Mon 1/3/22	Fri 12/27/24	1	
115	115	115							
116	116	116							
117	117	117	0	New District Headquarters (H.Q.)	1157 days?	Tue 11/27/07	Tue 5/1/12		
118	118	118	110	Funding (H.Q.)	548 days	Thu 8/28/08	Fri 10/1/10		
119	119	119	111	Pre Engineering Site (H.Q.)	139 days?	Thu 5/17/07	Tue 11/27/07		
120	120	120	112	Rezoning Site (H.Q.)	148 days?	Mon 1/7/08	Wed 7/30/08		
121	121	121	113	Design Site (H.Q.)	180 days?	Tue 11/27/07	Mon 8/4/08		
122	122	122	114	Design Building (Office Bldg.)	85 days	Mon 6/20/11	Thu 10/13/11		
123	123	123	115	Permit Phase II Site (Office Bldg.)	30 days	Tue 9/6/11	Fri 10/14/11		
124	124	124	116	Permit Building (Office Bldg.)	30 days	Thu 12/1/11	Thu 1/12/12		
125	125	125	117	Establish Phase II Project GMP (Office Bldg.)	1 day?	Mon 11/14/11	Mon 11/14/11		
126	126	126	118	Phase II Electric Service (Office Bldg.)	90 days	Thu 12/1/11	Wed 4/4/12		
127	127	127	118	Phase II Construction (Office Bldg.)	100 days	Thu 12/1/11	Wed 4/18/12		
128	128	128	120	Phase II Office Building Move-In (Office Bldg.)	6 days	Thu 5/24/12	Thu 5/31/12		
129	129	129	114	Maintenance Bldg. Construction (Maint. Bldg.)	218 days	Wed 12/1/10	Fri 9/30/11		
130	130	130	122	Maintenance Building Move-In (Maint. Bldg.)	10 days	Mon 10/3/11	Fri 10/14/11		
131	131	131							
132	132	132							
133	133	133	0	Homestead Road Widening Weir (S-57-4-6)	2440 days?	Tue 7/17/07	Fri 11/18/16		
134	134	134	125	Funding (Weir) 100% Lee Co. DOT	44 days	Mon 1/4/16	Thu 3/3/16		
135	135	135	126	Pre Engineering (Weir)	1401 days?	Mon 8/25/08	Tue 12/31/13		
136	136	136	127	Revise Design (H.D.R. Plans)	35 days	Mon 9/1/14	Fri 10/17/14		
137	137	137	128	Permit (H.D.R. Plans)	1244 days?	Wed 4/1/09	Tue 12/31/13		
138	138	138	129	Bidding (Weir)	26 days	Thu 1/28/16	Thu 3/3/16		
139	139	139	130	Electric (Weir)	180 days	Mon 4/25/16	Fri 12/30/16	1	
140	140	140	130	Construction (Weir 57-4-6)	180 days	Mon 4/25/16	Fri 12/30/16	1	
141	141	141							
142	142	142							
143	143	143	500	Hendry 2 Weir Replacement (S-H-2) Hurricane Irma Damage	405 days?	Mon 9/11/17	Fri 3/29/19		
144	144	144	134	Funding (90% NRCS) (10% LA-MSID)	1 day?	Tue 5/22/18	Tue 5/22/18		
145	145	145	135	Design (AIM Engineering)	48 days?	Tue 6/5/18	Thu 8/9/18		
146	146	146	136	Permit (SFWMD)	54 days?	Tue 6/5/18	Fri 8/17/18		
147	147	147	137	Bidding (Emergency Declaration)	1 day?	Thu 8/16/18	Thu 8/16/18		
148	148	148	137	Electric Service (Weir)	250 days?	Thu 8/16/18	Wed 7/31/19	1	



Project: Construction Project Schedule
Date: Mon 3/9/26

Task
Split
Progress
Milestone
Summary
Project Summary
External Tasks
External Milestone
Deadline

ID	ID	ID	Priori	Task Name	Duration	Start	Finish	P	R
149	149	149	137	Constrcution (J.P. Murphy, Inc.)	250 days	Thu 8/16/18	Wed 7/31/19	1	
150	150	150							
151	151	151							
152	152	152	500	CREST Storage (Dog/Hendry Connection)	1522 days?	Mon 9/11/17	Tue 3/31/26		
153	153	153	500	Funding (Phase I Construction) S-H-2 Weir	1 day?	Tue 5/22/18	Tue 5/22/18		
154	154	154	500	Design (Phase I)	48 days?	Tue 6/5/18	Thu 8/9/18		
155	155	155	500	Permit (Phase I) SFWMD	54 days?	Tue 6/5/18	Fri 8/17/18		
156	156	156	500	Bidding (Phase I)	1 day?	Thu 8/16/18	Thu 8/16/18		
157	157	157	500	Construction (Phase I) S-H-2 Weir	250 days?	Thu 8/16/18	Wed 7/31/19		
158	158	158	500	Funding (Phase II Construction) Structures/Filter Marsh	1 day	Mon 11/1/21	Mon 11/1/21		
159	159	159	500	Design (Phase II)	415 days?	Tue 4/21/20	Fri 12/31/21		
160	160	160	500	Permit (Phase II) SFWMD	60 days	Mon 11/1/21	Fri 1/21/22		
161	161	161	500	Bidding (Phase II)	25 days	Thu 10/6/22	Wed 11/9/22		
162	162	162	500	Construction (Phase II) Structures/Filter Marsh	267 days	Tue 1/24/23	Wed 1/31/24		
163	163	163	500	Permit (Phase III) SFWMD	87 days?	Thu 12/1/22	Fri 3/31/23		
164	164	164	160	Bidding (Phase III)	32 days	Wed 1/31/24	Thu 3/14/24		
165	165	165	161	Construction (Phase III - Lake Excavation)	490 days	Wed 5/15/24	Tue 3/31/26		
166	166	166	500	Plantings (South Storage Cell)	21 days	Tue 9/2/25	Tue 9/30/25		
167	167	167	500	Plantings (North Storage Cell)	21 days	Fri 2/6/26	Fri 3/6/26		
168	168	168							
169	169	169							
170	170	170	500	S-VL-1 Weir Replacement	815 days?	Mon 6/1/20	Sat 7/15/23		
171	171	171	500	Funding	815 days?	Mon 6/1/20	Sat 7/15/23		
172	172	172	500	Design	262 days?	Mon 6/1/20	Tue 6/1/21		
173	173	173	500	Permit (SFWMD)	174 days	Tue 12/1/20	Sat 7/31/21		
174	174	174	500	Bidding	31 days	Thu 9/23/21	Thu 11/4/21		
175	175	175	500	Construction	400 days	Mon 1/3/22	Sat 7/15/23		
176	176	176							
177	177	177							
178	178	178	500	Frank Mann Preserve "GS-10"	3609 days?	Fri 8/1/14	Wed 5/31/28		
179	179	179	500	Concept Plan	238 days?	Mon 1/4/16	Wed 11/30/16		
180	180	180	500	Land Purchase	1 day?	Thu 4/25/19	Thu 4/25/19		
181	181	181	500	Land Stewardship Plan	312 days?	Mon 1/6/20	Tue 3/16/21		
182	182	182	500	Start Exotic Vegetation Removal	1113 days?	Mon 9/27/21	Wed 12/31/25		
183	183	183	500	60% Design Plans	750 days?	Tue 8/17/21	Mon 7/1/24		
184	184	184	500	100% Design Plans	1250 days	Tue 8/17/21	Mon 6/1/26		
185	185	185	500	Permitting (SFWMD)	1403 days	Tue 8/17/21	Thu 12/31/26		
186	186	186	500	Bidding (Phase I)	45 days	Thu 1/14/27	Wed 3/17/27		
187	187	187	500	Construction (Phase I)	522 days	Thu 3/18/27	Fri 3/16/29	1	
188	188	188							
189	189	189							
190	190	190	500	South Marsh Berm Repairs (Hurricane Ian)	44 days	Mon 10/3/22	Thu 12/1/22		



Project: Construction Project Schedule
Date: Mon 3/9/26

Task | Progress | Summary | External Tasks | Deadline
Split | Milestone | Project Summary | External Milestone

Lehigh Acres - Municipal Services Improvement District
Capital Improvement Tracking Report

Discussion | Pre Design | Design | Permitting | Construction Funding | Construction | Close Out | No Proj. - Tracking Only

Line #	Proj#	Project Description	Engineer	Contractor	Total Estimated Construction Costs	Total Construction Contract Cost (includes change orders)	Total Spent to Date (Engineering / Construction)	Comments
		Active Projects						
2	4030	C.R.E.S.T.	AIM	*Turnbull Environmental*	$ 3,715,593.97	$ -	$ 3,877,268.86	Phase II FDEP Grant for $2 Mil. Turnbull Environmental was low bidder at $1,929,500. Phase II Construction Started Feb. 2023 & is 100% complete. Received SFWMD Phase III Permit. Phase III Grant $4,060,000. Phase III construction started 5/6/24 (90% complete). Pump delivered to site.
30	4067	GS-10	AIM	TBD	$ 15,000,000.00		$ 756,215.25	AIM finished study, staff completed 20/20 app for land purchase - MOU approved by each Board. Lee Co. Closed on property. Land Stewardship Plan is done. AIM finishing up Phase I Design. LAMSID working on clearing exotic vegetation.
	4071	Sinkhole	AIM	TBD	$ 3,700,000.00		$ 1,609,328.96	Sinkhole land was purchased. Fencing along Joel Blvd, along condos and along car wash has been completed. AIM & Architect working on cost estimates for building purchase and renovations.
	4072	Robust	AIM	TBD	$ 4,805,000.00		$ 518,524.03	AIM is working on survey and Roger Copp is working on modeling.
	5043	Greenbriar Blvd @ Swallowtail Canal (47-5-5)	LA-MSID	LA-MSID	$ 70,000.00		$ 85,348.73	LAMSID Crews started 1-5-26 (Greebriar Blvd. W. of Richmond) 90%
	5044	Roderick Ln @ Swallowtail Canal (47-5-5)	LA-MSID	LA-MSID	$ 35,000.00		$ 24,460.68	LAMSID Crews started 1-26-26 (Roderick Ln) 90%
	5045	Butler St. E. (RC 11-4527-2)	LA-MSID	LA-MSID	$ 35,000.00		$ 1,504.00	MSC working on plans (Butler St. E.) E. of McArthur Blvd.
	5046	Butler St. E. (RC 11-4527-1)	LA-MSID	LA-MSID	$ 35,000.00			MSC working on plans (Butler St. E.) W. of McArthur Blvd.
	5047	Butler Ave. S	LA-MSID	LA-MSID	$ 35,000.00			MSC working on plans (Butler Ave. S.) N. of Grant Blvd.
		Projects Waiting for Funding						
7	4012	Hendry Storage	*TBD*	*TBD*	$ -	$ -	$ -	CREST Project may eliminate the need for this project.
		Projects Waiting for Other Reasons						
15	4038	Sailfish Canal Greenway	Lee Co	*TBD*	$ -	$ -	$ -	Waiting on Lee Co. Sunshine Blvd. Widening Project
16	4040	Hickey's Creek Widening	AIM/Lee Co.	*TBD*	$ -	$ -	$ 83,775.50	Waiting on Lee Co. Sunshine Blvd. Widening Project
		Completed Projects FY 2016 through FY 2025						
30	4069	Panther Canal	ADA	LA-MSID			$ -	LA-MSID crews widened ditch into a canal & Modeling is complete.
	4026	SR 82 Widening Segments 3 & 4	FDOT	FDOT			$ -	Sacyr Construction started on SR 82 in July 2018 and they started widening Hendry Canal in September 2018. Hendry Canal Widening done. Clean up 100% (1-14-21).
13a	4028	Halfway Pond/Moving Water South - Phase 2	Tetra Tech	JP Murphy	$ 1,034,600.00	$ 1,034,600.00	$ 1,026,693.66	LCEC extended power to site - Start Up completed. Pump is operational.
21	4065	Southwest Lehigh Build Several Structures	RHT	Coastal Concrete	$ 3,113,538.13	$ -	$ 3,408,992.28	Coastal Concrete Products, LLC $3,128,000.00.
14	4036	Homestead Widening: Replacing S-57-4-6	HDR /Lee Co	JP Murphy	$ 400,000.00	$ -	$ 352,124.06	100% Completed
	4039	S-VL-1	AIM	QE	$ 639,014.75	$ 621,359.00	$ 669,584.73	SFWMD Permit was issued August 20th, 2021 - Bid Opening was November 4th, 2021 Contract was awarded to QE for $621,359.00 QE finishing up punch list items. Weir is fully operational. Concrete touch-up repairs completed. Completed close-out paperwork & final payment
17	4055	Hendry Extension Widening	T.Y. Lin	SACYR	TBD	$ -	$ 481,378.71	TY Lin finished updating plans for FDOT project. Sacyr hauling dirt for roadway 300-350 trucks per day. Canal widening is done. Sod finished. Project 100%.
20	4061	West Marsh Improvements	AIM	AJAX	$ 1,000,000.00	$ -	$ 870,037.07	AJAX paved roads and started hauling dirt in Nov. 2017. All Phase I is 100%. Starting on Phase II dirt hauling June 2019. Phase II is 100% complete. AJAX completed as-builts.
20	4062	West Marsh Phase III	AIM	QE	$ 3,000,000.00	$ -	$ 2,333,655.00	Phase III Awarded to QE and they started July 2020. Phase III is 100% complete. All 5 structures are built and gates installed. The bridge is finished, fencing complete and plantings are finished. Programming and final paper work are complete..
27	5001	Culvert Replacement - Empire @ Cocoplum	ECWCD	ECWCD			$ 24,835.83	#27 Empire Ave at 100%
26	5002	Culvert Replacement - HarrisonDr @ Spur C	ECWCD	ECWCD			$ 25,663.28	#28 Harrison Ave at 100%
28	5003	Culvert Replacement - McArthur Blvd @ Item	ECWCD	ECWCD			$ 26,888.69	#29 McArthur Ave at 100%
29	5004	Culvert Replacement - Parkdale @ Bullfrog	ECWCD	ECWCD			$ 91,817.77	#30 Parkdale Blvd. at 100%
30	5005	Culvert Replacement - Champion Ave @ Beauty Canal	LA-MSID	LA-MSID			$ 65,803.50	Champion Ave. Culvert Replacement Finished.
31	5006	Culvert Replacement - Martin Ave. @ Hammerhead Canal	LA-MSID	LA-MSID			$ 14,737.75	Martin Ave. Culvert Replacement Finished
32	5007	Culvert Replacement - Aletha Ave. S. @ Royal Canal	LA-MSID	LA-MSID			$ 21,706.49	Aletha Ave. S. Culvert Replacement Finished
33	5008	Culvert Replacement - Plumosa Ave. @ Blue Marl Canal	LA-MSID	LA-MSID			$ 12,570.71	Plumosa Ave. Culvert Replacement Finished
34	5009	Culvert Replacement - Lakeridge Drive @ Greenbriar Swamp	LA-MSID	LA-MSID			$ 29,289.08	Lakeridge Culvert Replacement Finished
35	5010	Culvert Replacement - W Hook Canal @ S Boundary Canal	LA-MSID	LA-MSID			$ 19,075.67	100% Completed
36	5011	Culvert Replacement - Richmond Ave. @ Perimeter Canal	LA-MSID	LA-MSID			$ 52,541.64	100% Completed
37	5012	Culvert Replacement - Richmond Ave. @ Amberwood Canal	LA-MSID	LA-MSID			$ 28,053.33	100% Completed
38	5013	Culvert Replacement - Anaconda Ave. S. @ Scrub Oak Canal	LA-MSID	LA-MSID			$ 27,341.20	100% Completed
39	5014	Culvert Replacement - Jauguar Blvd at Bald Eagle Canal	LA-MSID	LA-MSID	$ 25,000.00		$ 24,559.72	Jaguar Blvd Culvert Started 4-15-19 and Finished 5-2-19
	5015	Culvert Replacement - Ashley Road at Limpkin Canal	LA-MSID	LA-MSID	$ 20,000.00		$ 18,052.24	Ashley Road Culvert Started 3-26-19 and Finished 4-11-19
	5016	Culvert Replacement - Hull Ave. S. at Vulture Canal	LA-MSID	LA-MSID	$ 20,000.00		$ 15,019.40	Hull Ave. S. Culvert Started 5-7-19 and Finished 5-20-19
	5017	Culvert Replacement - McArthur Blvd. at Wagtail Canal	LA-MSID	LA-MSID	$ 20,000.00		$ 16,092.54	McArthur Blvd Culvert Started 5-21-19 and Finished 6-6-19
	5018	Culvert Replacement - Casey St. at Gannet Canal	LA-MSID	LA-MSID	$ 20,000.00		$ 17,592.21	Casey St. Culvert Starting 6-11-19 and Finished 6-27-19
	5019	Culvert Replacement - 48th Street SW	LA-MSID	LA-MSID	$ 25,000.00		$ 28,079.10	Finished construction on March 5th, 2020
	5020	Culvert Replacement - 49th Street SW	LA-MSID	LA-MSID	$ 25,000.00		$ 16,275.29	Started construction on March 6th, 2020
	5021	Culvert Replacement - 28th Street SW	LA-MSID	LA-MSID	$ 25,000.00		$ 19,072.22	Started construction on June 15th, 2020
	5022	Culvert Replacement - 59th Street W	LA-MSID	LA-MSID	$ 25,000.00		$ 14,092.36	Started construction on May 26th, 2020
	5023	Culvert Replacement - Long Avenue S	LA-MSID	LA-MSID	$ 25,000.00		$ 16,984.21	Finished construction on July 29th, 2020
	5024	Culvert Replacement - Richmond Ave. N. at Longwing Canal	LA-MSID	LA-MSID	$ 17,500.00		$ 18,773.14	Richmond Ave. N. Culvert Starting 8-28-19 and Finished 9-18-19
19	4060	Phase 2 Replace S-OR-1, S-OR SE, New Emergency Spillway	TT	WORTH	$ 1,550,020.00	$ 1,550,000.00	$ 1,423,304.00	S-OR-1 & S-OR-1-SE are 100% complete.
	4037	Culvert Replacement - 25th St. W. (S-R-1 Structure)	LA-MSID	LA-MSID	$ 75,000.00		$ 125,051.49	100% - Completed in-house with 2020 culverts.
	5025	Culvert Replacement - Bell Blvd	LA-MSID	LA-MSID	$ 35,000.00		$ 74,794.69	100% - Finished construction on February 10th, 2021.
	5026	Culvert Replacement - 40th Street SW	LA-MSID	LA-MSID	$ 25,000.00		$ 23,559.95	100% - Finished construction on March 4th, 2021.
	5027	Culvert Replacement - Golfview Blvd	LA-MSID	LA-MSID	$ 25,000.00		$ 25,427.66	100% - Finished construction on March 23rd, 2021.
	5028	Culvert Replacement - Douglas Lane	LA-MSID	LA-MSID	$ 35,000.00		$ 34,903.58	100% - Started construction March 30th, 2021.
	5029	Culvert Replacement - 51st Street West	LA-MSID	LA-MSID	$ 25,000.00		$ 20,733.71	Started design April 1st, 2021. Starting construction 5/11/21. Finished construction 5/27/21.
	5030	Culvert Replacement - Acacia Ave	LA-MSID	LA-MSID	$ 50,000.00		$ 78,726.42	Starting design May 10th, 2021. Construction is 100% done.
	5031	(Weir) Alabama Rd @ Panther Canal	LA-MSID	LA-MSID	$ 40,000.00		$ 27,749.42	Riser pipes damaged by a car wreck. New riser heads were installed and concrete poured around it. Construction is 100% complete.
	5032	(Weir) Bedman Creek @ Hendry Canal	LA-MSID	LA-MSID	$ 40,000.00		$ 24,575.94	Riser pipe was rusted out. New pipe and riser head installed. Construction is 100% complete.
	5033	Richmond @ 8th St. (S-G-1 Structure)	LA-MSID	LA-MSID	$ 40,000.00		$ 6,904.24	Remaining parts of rusted gate removed and a new gate was installed. Construction is 100% complete.
	5034	Beauty Canal (46-21-1) W. Side of Gunnery Rd.	LA-MSID	LA-MSID	$ 40,000.00		$ 4,108.56	Old pipe was removed and replaced with new pipe and rip-rap. Construction is 100% complete.
	5035	Ringneck Canal @ Airview Ave.	LA-MSID	LA-MSID	$ 40,000.00		$ 423.40	Rusted out riser head was plugged with large sand bags to test elevations for 1 year. Construction is 100% complete.
	5036	Culvert Replacement - 59th St. W. @ Remora Canal	LA-MSID	LA-MSID	$ 40,000.00		$ 40,770.05	59th St. W. In-House culvert replacement was started on August 9th and Completed on August 27th, 2024.
	5037	Culvert Replacement - 51st St. W. @ Runner Canal	LA-MSID	LA-MSID	$ 40,000.00		$ 14,688.73	51st St. W. In-House culvert replacement was started on September 16th, 2024 and Completed on September 26th, 2024.
	5038	Culvert Replacement - Centenial Outfall @ Champion	LA-MSID	LA-MSID	$ 40,000.00		$ 8,922.87	Culvert Replacement - Centenial Outfall @ Champion Completed October 10th, 2024
	5039	Culvert Replacement - Billings St. Outfall	LA-MSID	LA-MSID	$ 40,000.00		$ 5,567.45	Culvert Replacement - Billings St. Outfall Completed October 24th, 2024
	5040	Culvert Replacement - Ridgemont Drive @ Webworm Canal	LA-MSID	LA-MSID	$ 40,000.00		$ 31,668.77	Culvert Replacement - Ridgemont Drive @ Webworm Canal started on November 4th & finished on November 21st, 2024
	5041	Culvert- Replacement - Moore Ave	LA-MSID	LA-MSID	$ 40,000.00		$ 40,765.09	Construction started after South Marsh Hurricane Ian repairs were completed. Construction is 100% complete. County repaved crossing.
	5042	Culvert Replacement - Magenta Ave. @ Longwing Canal	LA-MSID	LA-MSID	$ 40,000.00		$ 48,021.99	Culvert Replacement - Magenta Ave. @ Longwing Canal - Completed January 30th, 2025
	6002	Buckingham Park - NRCS (Slope Repair)	AIM	LA-MSID	$ 788,601.00	$ 788,601.00	$ 470,731.78	LA-MSID crews started placing Flexamat slope stabilization product in Septemebr. LAMSID finished project and closing out with NRCS for payment.
	6003	Buckingham Park - NRCS (Debris)	AIM	LA-MSID	$ 17,394.30	$ 17,394.30	$ 14,955.08	LA-MSID crews finished debris removal and clean-up in September.
	6004	S-H-2 - NRCS (Rebuild Weir)	AIM	JP Murphy	$ 809,485.00	$ 809,485.00	$ 824,457.16	Steel sheet piling has been delivered and installed. Concrete cap and conc. slope paving poured, rip-rap placed. Gates installed, catwalk completed, slopes restored and sod placed. Weir is finished and power hooked up by FP&L power.

Change from last report

Funded by:
- South Florida Water Management
- Not yet determined
- Multiple Org/Indiv
- Florida Dept of Environmental Protection
- Lehigh Acres Municipal Services Improvement District

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
COMBINED BALANCE SHEET
JANUARY 31, 2026 (UN-AUDITED)

			Capital Project Funds			
	General	Debt Service	Capital Projects	Capital Equipment	Bridge Renewal Replacement	Total
ASSETS						
Cash and Cash Equivalents	$ 4,375,425.94	$ 863,204.19	$ 0.00	$ 0.00	$ 0.00	$ 5,238,630.13
Due from Other Governments	38,027.91	0.00	836,978.61	0.00	0.00	875,006.52
Due from Other Funds	1,330,410.81	0.00	0.00	0.00	0.00	1,330,410.81
Accounts Receivable, net	664,348.22	0.00	0.00	0.00	0.00	664,348.22
Prepaid Items	9,009.60	0.00	0.00	0.00	0.00	9,009.60
Deposits	1,675.00	0.00	0.00	0.00	0.00	1,675.00
Restricted Investments	3,444,721.38	0.00	759,529.89	147,294.71	1,381,797.94	5,733,343.92
Total Assets	$ 9,863,618.86	$ 863,204.19	$ 1,596,508.50	$ 147,294.71	$ 1,381,797.94	$ 13,852,424.20
LIABILITIES, DEFERRED INFLOWS & FUND BALANCE						
LIABILITIES						
Accounts Payable	179,011.48	0.00	11,123.26	1,873.75	0.00	192,008.49
Retainage Payable	0.00	0.00	50,225.59	0.00	0.00	50,225.59
Due to Other Funds	0.00	0.00	885,854.94	434,027.61	10,528.26	1,330,410.81
Deposits	5,440.91	0.00	0.00	0.00	0.00	5,440.91
Accrued Liabilities	165,002.18	0.00	0.00	0.00	0.00	165,002.18
Total Liabilities	349,454.57	0.00	947,203.79	435,901.36	10,528.26	1,743,087.98
DEFERRED INFLOWS						
Unavailable Revenue - Grants	644,087.45	0.00	836,978.61	0.00	0.00	1,481,066.06
Total Deferred Inflows	644,087.45	0.00	836,978.61	0.00	0.00	1,481,066.06
FUND BALANCE						
Fund Balance - Beginning	5,535,430.70	836,137.15	1,367,199.66	(1,072,165.23)	1,015,259.93	7,681,862.21
Change in Fund Balance	3,021,491.07	6,520.02	(409,195.67)	(207,469.43)	18,468.52	2,429,814.51
Total Fund Balance	8,556,921.77	842,657.17	958,003.99	(1,279,634.66)	1,033,728.45	10,111,676.72
Total Liabilities, Deferred Inflows & Fund Balance	$ 9,550,463.79	$ 842,657.17	$ 2,742,186.39	$ (843,733.30)	$ 1,044,256.71	$ 13,335,830.76

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
COMBINED STATEMENT OF REVENUES, EXPENDITURES AND CHANGES IN FUND BALANCE
JANUARY 31, 2026 (UN-AUDITED)
YEAR-TO-DATE

			CAPITAL PROJECT FUNDS			
	General	Debt Service	Capital Projects	Capital Equipment	Bridge Renewal Replacement	Total
REVENUES						
Tax Assessments	$ 5,632,035.78	$ 0.00	$ 0.00	$ 0.00	$ 0.00	$ 5,632,035.78
Tax Assessments - PY/Penalties/Interest	0.00	0.00	0.00	425.30	0.00	425.30
Tax Assessments - Discounts	(211,776.78)	0.00	0.00	0.00	0.00	(211,776.78)
Permit Fees	0.00	0.00	0.00	94,860.35	0.00	94,860.35
Intergovernmental - Grants	0.00	0.00	52,834.96	0.00	0.00	52,834.96
Miscellaneous Revenue	78,908.73	0.00	15,177.97	0.00	0.00	94,086.70
Interest Income	58,586.77	6,520.02	10,151.52	1,968.67	18,468.52	95,695.50
Total Revenues	$ 5,557,754.50	$ 6,520.02	$ 78,164.45	$ 97,254.32	$ 18,468.52	$ 5,758,161.81
EXPENDITURES						
Current						
Physical Environment						
Administration						
Personal Services	523,640.91	0.00	0.00	0.00	0.00	523,640.91
Operating Expenses	495,646.95	0.00	0.00	0.00	0.00	495,646.95
Total Administration	1,019,287.86	0.00	0.00	0.00	0.00	1,019,287.86
Field Operations						
Personal Services	1,047,850.83	0.00	0.00	0.00	0.00	1,047,850.83
Operating Expenses	274,524.56	0.00	0.00	0.00	0.00	274,524.56
Total Field Operations	1,322,375.39	0.00	0.00	0.00	0.00	1,322,375.39
Capital Outlay	0.00	0.00	487,868.12	335,873.75	0.00	823,741.87
Debt Service						
Principal	0.00	186,181.21	0.00	0.00	0.00	186,181.21
Interest	0.00	8,238.79	0.00	0.00	0.00	8,238.79
Total Debt Service	0.00	194,420.00	0.00	0.00	0.00	194,420.00
Total Expenditures	$ 2,341,663.25	$ 194,420.00	$ 487,868.12	$ 335,873.75	$ 0.00	$ 3,359,825.12
Excess of Revenues over (under) expenditures	$ 3,216,091.25	$ (187,899.98)	$ (409,703.67)	$ (238,619.43)	$ 18,468.52	$ 2,398,336.69
OTHER FINANCING SOURCES (USES)						
Transfers In	0.00	194,420.00	0.00	0.00	0.00	194,420.00
Transfers Out	(194,420.00)	0.00	0.00	0.00	0.00	(194,420.00)
Proceeds from Sale of Fixed Assets	0.00	0.00	0.00	31,150.00	0.00	31,150.00
Total other financing sources and uses	(194,420.00)	194,420.00	0.00	31,150.00	0.00	31,150.00
Net change in fund balance	3,021,671.25	6,520.02	(409,703.67)	(207,469.43)	18,468.52	2,429,486.69
Fund Balance - Beginning	5,226,044.70	836,137.15	1,367,199.66	(1,072,165.23)	1,015,259.93	7,372,476.21
Fund Balance - Ending	$ 8,247,535.77	$ 842,657.17	$ 958,003.99	$ (1,279,634.66)	$ 1,033,728.45	$ 9,802,290.72

Lehigh Acres Municipal Services Improvement District - Report 340

STATEMENT OF REVENUES, EXPENDITURES AND CHANGES IN FUND BALANCE

BUDGET vs. ACTUAL YTD - GENERAL FUND

For The Month Ended January 31, 2026 (Un-Audited)

Benchmark 33.33 %

	BUDGET($)	YTD ACTUAL($)	VARIANCE FAVORABLE/ (UNFAVORABLE)($)	PERCENT OF ACTUAL TO BUDGET(%)
Fund Balance - Beginning	5,417,270.03	5,226,044.70	(191,225.33)	96.47
REVENUES				
Tax Assessments	7,730,535.22	5,632,035.78	(2,098,499.44)	72.85
Tax Assessments - Discounts	(231,916.06)	(211,776.78)	20,139.28	91.32
Permit Fees	0.00	0.00	0.00	0.00
Miscellaneous Revenue	38,529.76	78,908.73	40,378.97	204.80
Interest Income	125,000.00	58,586.77	(66,413.23)	46.87
Prior Year Surplus	0.00	0.00	0.00	0.00
Total Revenues	7,662,148.92	5,557,754.50	(2,104,394.42)	72.54
EXPENDITURES				
Current				
Physical Environment				
Administration				
Personal Services	1,743,975.00	523,640.91	1,220,334.09	30.03
Operating Expenses	774,475.00	495,646.95	278,828.05	64.00
Total Administration	2,518,450.00	1,019,287.86	1,499,162.14	40.47
Stormwater Operations				
Personal Services	3,250,783.00	1,047,850.83	2,202,932.17	32.23
Operating Expenses	978,506.30	274,524.56	703,981.74	28.06
Total Stormwater Operations	4,229,289.30	1,322,375.39	2,906,913.91	31.27
Capital Outlay	0.00	0.00	0.00	0.00
Debt Service				
Principal	0.00	0.00	0.00	0.00
Interest	0.00	0.00	0.00	0.00
Other	0.00	0.00	0.00	0.00
Total Debt Service	0.00	0.00	0.00	0.00
Total Expenditures	6,747,739.30	2,341,663.25	4,406,076.05	34.70
Excess of Revenues over (under) Expenditures	914,409.62	3,216,091.25	2,301,681.63	351.71
OTHER FINANCING SOURCES (USES)				
Transfers Out	(1,215,224.54)	(194,420.00)	1,020,804.54	16.00
Total Other Financing Sources and Uses	(1,215,224.54)	(194,420.00)	1,020,804.54	16.00
Net Change in Fund Balance	(300,814.92)	3,021,671.25	3,322,486.17	(1,004.50)
Fund Balance - Ending	5,116,455.11	8,247,715.95	3,131,260.84	161.20

Lehigh Acres Municipal Services Improvement District - Report 340

STATEMENT OF REVENUES, EXPENDITURES AND CHANGES IN FUND BALANCE
BUDGET vs. ACTUAL YTD - DEBT SERVICE FUND
For The Month Ended January 31, 2026 (Un-Audited)

Benchmark 33.33 %

	BUDGET($)	YTD ACTUAL($)	VARIANCE FAVORABLE/ (UNFAVORABLE)($)	PERCENT OF ACTUAL TO BUDGET(%)
Fund Balance - Beginning	836,137.15	836,137.15	0.00	100.00
REVENUES				
Permit Fees	0.00	0.00	0.00	0.00
Interest Income	7,000.00	6,520.02	(479.98)	93.14
Prior Year Surplus	0.00	0.00	0.00	0.00
Total Revenues	7,000.00	6,520.02	(479.98)	93.14
EXPENDITURES				
Current				
Physical Environment				
Administration				
Total Administration	0.00	0.00	0.00	0.00
Stormwater Operations				
Total Stormwater Operations	0.00	0.00	0.00	0.00
Capital Outlay	0.00	0.00	0.00	0.00
Debt Service				
Principal	800,349.69	186,181.21	614,168.48	23.26
Interest	48,874.85	8,238.79	40,636.06	16.86
Other	0.00	0.00	0.00	0.00
Total Debt Service	849,224.54	194,420.00	654,804.54	22.89
Total Expenditures	849,224.54	194,420.00	654,804.54	22.89
Excess of Revenues over (under) Expenditures	(842,224.54)	(187,899.98)	654,324.56	22.31
OTHER FINANCING SOURCES (USES)				
Transfers In	849,224.54	194,420.00	(654,804.54)	22.89
Total Other Financing Sources and Uses	849,224.54	194,420.00	(654,804.54)	22.89
Net Change in Fund Balance	7,000.00	6,520.02	(479.98)	93.14
Fund Balance - Ending	843,137.15	842,657.17	(479.98)	99.94

Lehigh Acres Municipal Services Improvement District - Report 340

STATEMENT OF REVENUES, EXPENDITURES AND CHANGES IN FUND BALANCE
BUDGET vs. ACTUAL YTD - CAPITAL PROJECTS FUND

Benchmark 33.33 %

	BUDGET($)	YTD ACTUAL($)	VARIANCE FAVORABLE/ (UNFAVORABLE)($)	PERCENT OF ACTUAL TO BUDGET(%)
Fund Balance - Beginning	3,045,603.18	1,367,199.66	(1,678,403.52)	44.89
REVENUES				
Permit Fees	0.00	0.00	0.00	0.00
Intergovernmental - Revenue	3,115,000.00	52,834.96	(3,062,165.04)	1.70
Miscellaneous Revenue	0.00	15,177.97	15,177.97	0.00
Interest Income	20,000.00	10,151.52	(9,848.48)	50.76
Prior Year Surplus	0.00	0.00	0.00	0.00
Total Revenues	3,135,000.00	78,164.45	(3,056,835.55)	2.49
EXPENDITURES				
Current				
Physical Environment				
Administration				
Total Administration	0.00	0.00	0.00	0.00
Stormwater Operations				
Total Stormwater Operations	0.00	0.00	0.00	0.00
Capital Outlay	10,925,000.00	487,868.12	10,437,131.88	4.47
Debt Service				
Principal	0.00	0.00	0.00	0.00
Interest	0.00	0.00	0.00	0.00
Other	0.00	0.00	0.00	0.00
Total Debt Service	0.00	0.00	0.00	0.00
Total Expenditures	10,925,000.00	487,868.12	10,437,131.88	4.47
Excess of Revenues over (under) Expenditures	(7,790,000.00)	(409,703.67)	7,380,296.33	5.26
OTHER FINANCING SOURCES (USES)				
Total Other Financing Sources and Uses	0.00	0.00	0.00	0.00
Net Change in Fund Balance	(7,790,000.00)	(409,703.67)	7,380,296.33	5.26
Fund Balance - Ending	(4,744,396.82)	957,495.99	5,701,892.81	(20.18)

Lehigh Acres Municipal Services Improvement District - Report 340

STATEMENT OF REVENUES, EXPENDITURES AND CHANGES IN FUND BALANCE
BUDGET vs. ACTUAL YTD - EQUIPMENT RENEWAL & REPLACEMENT FUND

Benchmark 33.33 %

	BUDGET($)	YTD ACTUAL($)	VARIANCE FAVORABLE/ (UNFAVORABLE)($)	PERCENT OF ACTUAL TO BUDGET(%)
Fund Balance - Beginning	(1,186,879.03)	(1,072,165.23)	114,713.80	90.33
REVENUES				
Tax Assessments - PY/Penalties/Interest	0.00	425.30	425.30	0.00
Permit Fees	50,000.00	94,860.35	44,860.35	189.72
Interest Income	3,600.00	1,968.67	(1,631.33)	54.69
Prior Year Surplus	0.00	0.00	0.00	0.00
Total Revenues	53,600.00	97,254.32	43,654.32	181.44
EXPENDITURES				
Current				
Physical Environment				
Administration				
Total Administration	0.00	0.00	0.00	0.00
Stormwater Operations				
Total Stormwater Operations	0.00	0.00	0.00	0.00
Capital Outlay	410,000.00	335,873.75	74,126.25	81.92
Debt Service				
Principal	0.00	0.00	0.00	0.00
Interest	0.00	0.00	0.00	0.00
Other	0.00	0.00	0.00	0.00
Total Debt Service	0.00	0.00	0.00	0.00
Total Expenditures	410,000.00	335,873.75	74,126.25	81.92
Excess of Revenues over (under) Expenditures	(356,400.00)	(238,619.43)	117,780.57	66.95
OTHER FINANCING SOURCES (USES)				
Transfers In	366,000.00	0.00	(366,000.00)	0.00
Proceeds from Sale of Fixed Assets	0.00	31,150.00	31,150.00	0.00
Total Other Financing Sources and Uses	366,000.00	31,150.00	(334,850.00)	8.51
Net Change in Fund Balance	9,600.00	(207,469.43)	(217,069.43)	(2,161.14)
Fund Balance - Ending	(1,177,279.03)	(1,279,634.66)	(102,355.63)	108.69

Lehigh Acres Municipal Services Improvement District - Report 340

STATEMENT OF REVENUES, EXPENDITURES AND CHANGES IN FUND BALANCE
BUDGET vs. ACTUAL YTD - BRIDGE RENEWAL & REPLACEMENT FUND

Benchmark 33.33 %

	BUDGET($)	YTD ACTUAL($)	VARIANCE FAVORABLE/ (UNFAVORABLE)($)	PERCENT OF ACTUAL TO BUDGET(%)
Fund Balance - Beginning	1,297,132.74	1,015,259.93	(281,872.81)	78.27
REVENUES				
Tax Assessments	285,000.00	0.00	(285,000.00)	0.00
Permit Fees	0.00	0.00	0.00	0.00
Interest Income	15,000.00	18,468.52	3,468.52	123.12
Prior Year Surplus	0.00	0.00	0.00	0.00
Total Revenues	300,000.00	18,468.52	(281,531.48)	6.16
EXPENDITURES				
Current				
Physical Environment				
Administration				
Total Administration	0.00	0.00	0.00	0.00
Stormwater Operations				
Operating Expenses	50,000.00	0.00	50,000.00	0.00
Total Stormwater Operations	50,000.00	0.00	50,000.00	0.00
Capital Outlay	0.00	0.00	0.00	0.00
Debt Service				
Principal	0.00	0.00	0.00	0.00
Interest	0.00	0.00	0.00	0.00
Other	0.00	0.00	0.00	0.00
Total Debt Service	0.00	0.00	0.00	0.00
Total Expenditures	50,000.00	0.00	50,000.00	0.00
Excess of Revenues over (under) Expenditures	250,000.00	18,468.52	(231,531.48)	7.39
OTHER FINANCING SOURCES (USES)				
Total Other Financing Sources and Uses	0.00	0.00	0.00	0.00
Net Change in Fund Balance	250,000.00	18,468.52	(231,531.48)	7.39
Fund Balance - Ending	1,547,132.74	1,033,728.45	(513,404.29)	66.82

LEHIGH ACRES MSID

LEHIGH ACRES

MUNICIPAL SERVICES IMPROVEMENT DISTRICT

Memo

To: Board of Commissioners

From: Bill Walker

Date: 3/5/2026

Re: Field Report

The following Field Report details the field activities completed during the month of February2025. Staff is hereby requesting that the Board of Commissioners accept this report.

1) **Canals Sprayed:**
   Spray crews have been treating the Districts Canals and Right-of-ways. Preserve maintenance will be conducted on days when the weather is not conducive to treating canals.

2) **Canal Cleanout:**
   Canal clean out is on schedule for the year.

3) **Canal Mowing:**
   Mowing crews resumed the normal mowing schedule starting May 19, 2025. Slope mowers completed 35% of their third cycle. Crew stopped mowing first week of February and started special projects. The flat mowers completed the third cycle and have started working on special projects.

4) **Canal Grading and Clearing:**
   Crews are working on washouts and grading behind the canal cleanout crews.

5) **Right of Way Program:**
   Staff worked on clearing ROW obstructions ahead of the clean out crews, inspecting the District's road crossing culverts and the NDPES Annual Report.

6) **Preserves:**
   Spray crews worked on preserve maintenance and special projects when weather was not conducive for canal treatments.

7) **Water Quality:**
   Staff has not received the Water Quality sampling results from the City of Cape Coral at the time of this report.

8) **Facilities:**

No notable changes have occurred with the Districts facilities over the last month. Staff has continued to maintain the parks with mowing, mulching and trash removal.

1) **Fleet Maintenance:**

Normal fleet maintenance has occurred during the last month.

2) **Education/ Meetings:**

None to Report.

3) **Vandalism:**

None to Report.

Lehigh Acres Municipal Services Improvement District
Account Detail 10/01/2025 to 9/30/2026

Date	Post Type	Source	Reference	Description	Units	Debit	Credit	Balance
0001-0001-538-3100 Legal Fees								0.00
10/31/2025	Actual	A/P	SCB-166002	Lewis Longman & Walker P.A. General Counsel through 10/31/25		7,105.00		7,105.00
12/30/2025	Actual	A/P	SCB-166830	Lewis Longman & Walker P.A. General Counsel through 12/30/25		3,738.46		10,843.46
1/20/2026	Actual	A/P	20342	Hultman & Joshi, P.A. Matter: General through 08/28/25		925.00		11,768.46
1/20/2026	Actual	A/P	20448	Hultman & Joshi, P.A. Matter: General through 10/14/25		725.00		12,493.46
1/20/2026	Actual	A/P	20513	Hultman & Joshi, P.A. Matter: General for 10/13/25		75.00		12,568.46
2/03/2026	Actual	A/P	SCB-167415	Lewis Longman & Walker P.A. General Counsel through 01/26/26		1,155.00		13,723.46
2/25/2026	Actual	A/P	SCB-167632	Lewis Longman & Walker P.A. General Counseling through 02/25/26		3,515.00		17,238.46
0001-0001-538-3100 Legal Fees Total					0.00	17,238.46	0.00	17,238.46
0001-0001-538-3110 Legislative Fees								0.00
10/05/2025	Actual	A/P	MCL-165963	Lewis Longman & Walker P.A. Legislative Representation		4,286.90		4,286.90
10/09/2025	Actual	A/P	MCL-165596	Lewis Longman & Walker P.A. Legislative Representation		4,000.00		8,286.90
11/27/2025	Actual	A/P	SCB-166268	Lewis Longman & Walker P.A. General Counsel through 11/27/25		6,608.88		14,895.78
11/30/2025	Actual	A/P	MCL-166349	Lewis Longman & Walker P.A. Legislative Representation		4,169.46		19,065.24
12/31/2025	Actual	A/P	MCL-166752	Lewis Longman & Walker P.A. Legislative Represention		4,014.35		23,079.59
1/31/2026	Actual	A/P	MCL-167134	Lewis Longman & Walker P.A. Legislative Representation		4,148.28		27,227.87
3/10/2026	Actual	A/P	MCL-1676680	Lewis Longman & Walker P.A. Legislative Representation		4,383.01		31,610.88
0001-0001-538-3110 Legislative Fees Total					0.00	31,610.88	0.00	31,610.88
Report Total					**0.00**	**48,849.34**	**0.00**	**48,849.34**

Lehigh Acres Municipal Services Improvement District
Account Detail 10/01/2025 to 9/30/2026

Date	Post Type	Source	Reference	Description	Units	Debit	Credit	Balance
0001-0001-538-3130 Engineering								0.00
10/31/2025	Actual	A/P	252166-1	AIM Engineering & Surveying, Inc. General Engineering through 10/31/25		11,891.50		11,891.50
11/21/2025	Actual	A/P	252166-2	AIM Engineering & Surveying, Inc. General Engineering through 11/21/25		5,816.50		17,708.00
1/09/2026	Actual	A/P	252166-3	AIM Engineering & Surveying, Inc. General Engineering through 01/09/26		4,882.00		22,590.00
2/11/2026	Actual	A/P	252166-4	AIM Engineering & Surveying, Inc. General Engineering though 02/06/26		9,524.25		32,114.25
2/27/2026	Actual	A/P	252166-5	AIM Engineering & Surveying, Inc. General Engineering through 02/27/26		3,391.00		35,505.25
0001-0001-538-3130 Engineering Total					0.00	35,505.25	0.00	35,505.25
0306-4030-538-3130 C.R.E.S.T. Project - Engineering								0.00
11/21/2025	Actual	A/P	191026-67	AIM Engineering & Surveying, Inc. ADDITIONAL DESIGN & PERMITTING SERVICES for the CALOOSAHATCHEE RIVER ESTUARY STORAGE AND TREATMENT (CREST) PROJECT (ADDITIONAL PHASE 3		196.95		196.95
0306-4030-538-3130 C.R.E.S.T. Project - Engineering Total					0.00	196.95	0.00	196.95
0306-4067-538-3130 GS-10 - Engineering								0.00
10/31/2025	Actual	A/P	211364-46	AIM Engineering & Surveying, Inc. LA-MSID Section 10 Concept Plan - Conceptual Construction Budget Engineering Estimate through 10/31/25		730.00		730.00
10/31/2025	Actual	A/P	211364-47	AIM Engineering & Surveying, Inc. CHANGE ORDER NO. 01 – GS-10 Stormwater Storage and Treatment Project through 10/31/25		20,204.00		20,934.00

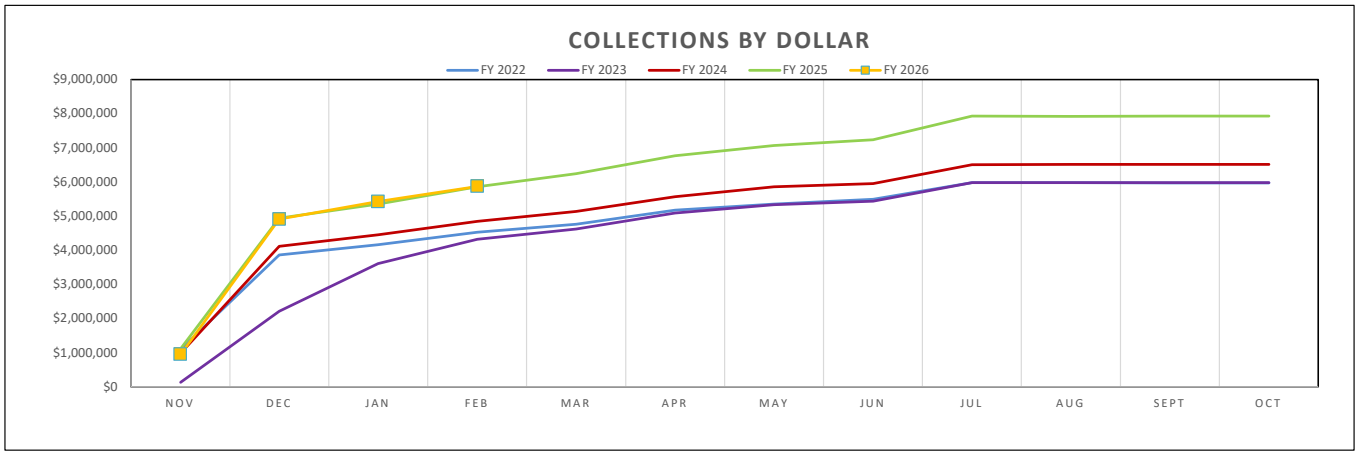
Date	Post Type	Source	Reference	Description	Units	Debit	Credit	Balance
11/21/2025	Actual	A/P	211364-48	AIM Engineering & Surveying, Inc. CHANGE ORDER NO. 01 – GS-10 Stormwater Storage and Treatment Project through 11/21/25		1,216.25		22,150.25
1/13/2026	Actual	A/P	211364-49	AIM Engineering & Surveying, Inc. CHANGE ORDER NO. 01 – GS-10 Stormwater Storage and Treatment Project through 01/09/26		11,457.50		33,607.75
2/11/2026	Actual	A/P	252112-1	AIM Engineering & Surveying, Inc. Frank Mann Preserve, Phase 2 Project though 02/06/26		4,487.50		38,095.25
2/11/2026	Actual	A/P	211364-50	AIM Engineering & Surveying, Inc. CHANGE ORDER NO. 01 – GS-10 Stormwater Storage and Treatment Project through 02/06/26		6,432.50		44,527.75
2/27/2026	Actual	A/P	211364-51	AIM Engineering & Surveying, Inc. CHANGE ORDER NO. 01 – GS-10 Stormwater Storage and Treatment Project through 02/27/26		8,165.00		52,692.75
2/27/2026	Actual	A/P	252112-2	AIM Engineering & Surveying, Inc. Frank Mann Preserve, Phase 2 Project through 02/27/26		3,488.75		56,181.50
0306-4067-538-3130 GS-10 - Engineering Total					0.00	56,181.50	0.00	56,181.50
0306-4071-538-3130 Sink Hole-Engineering								0.00
10/31/2025	Actual	A/P	252166-1	AIM Engineering & Surveying, Inc. General Engineering through 10/31/25		3,807.50		3,807.50
11/21/2025	Actual	A/P	252166-2	AIM Engineering & Surveying, Inc. General Engineering through 11/21/25		257.00		4,064.50
2/11/2026	Actual	A/P	252166-4	AIM Engineering & Surveying, Inc. General Engineering though 02/06/26		4,661.00		8,725.50
0306-4071-538-3130 Sink Hole-Engineering Total					0.00	8,725.50	0.00	8,725.50
0306-4072-538-3130 Robust Parcel- Engineering								0.00
10/31/2025	Actual	A/P	252161-2	AIM Engineering & Surveying, Inc. Change Order No. 02-Robust through 10/31/25		4,626.00		4,626.00
10/31/2025	Actual	A/P	252111-3	AIM Engineering & Surveying, Inc. Change Order No. 01-Robust through 10/31/25		4,112.00		8,738.00

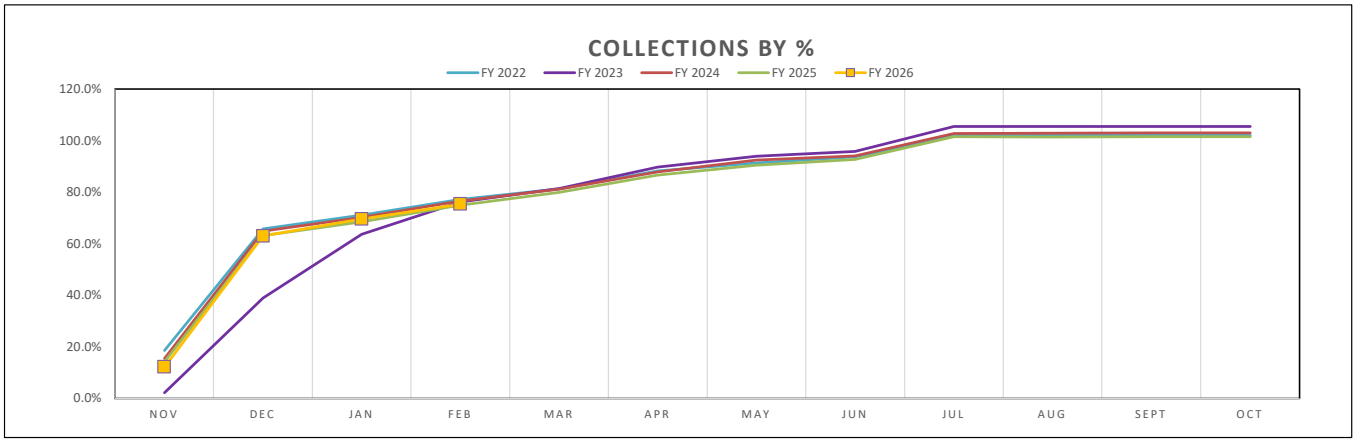
Date	Post Type	Source	Reference	Description	Units	Debit	Credit	Balance
10/31/2025	Actual	A/P	252051-7	AIM Engineering & Surveying, Inc. ROBUST PHII: This scope of work is for the design, post-design, and construction-phase services related to ROBUST Phase 2 through 10/31/25		20,847.50		29,585.50
11/21/2025	Actual	A/P	252051-8	AIM Engineering & Surveying, Inc. ROBUST PHII: This scope of work is for the design, post-design, and construction-phase services related to ROBUST Phase 2 through 11/21/25		17,017.00		46,602.50
11/21/2025	Actual	A/P	252161-3	AIM Engineering & Surveying, Inc. ROBUST PHII: This scope of work is for the design, post-design, and construction-phase services related to ROBUST Phase 2 through 11/21/25		3,598.00		50,200.50
11/21/2025	Actual	A/P	252111-4	AIM Engineering & Surveying, Inc. Change Order No. 01-Robust through 11/21/25		3,885.00		54,085.50
1/13/2026	Actual	A/P	252161-4	AIM Engineering & Surveying, Inc. Change Order No. 02-Robust through 01/09/26		6,939.00		61,024.50
1/13/2026	Actual	A/P	252051-9	AIM Engineering & Surveying, Inc. ROBUST PHII: This scope of work is for the design, post-design, and construction-phase services related to ROBUST Phase 2 through 01/09/26		24,346.50		85,371.00
1/13/2026	Actual	A/P	252111-5	AIM Engineering & Surveying, Inc. Change Order No. 01-Robust through 01/09/26		14,007.50		99,378.50
2/11/2026	Actual	A/P	252111-6	AIM Engineering & Surveying, Inc. Change Order No. 01-Robust through 02/6/26		3,545.00		102,923.50
2/11/2026	Actual	A/P	252051-10	AIM Engineering & Surveying, Inc. ROBUST PHII: This scope of work is for the design, post-design, and construction-phase services related to ROBUST Phase 2 through 02/06/26		3,780.00		106,703.50

Date	Post Type	Source	Reference	Description	Units	Debit	Credit	Balance
2/11/2026	Actual	A/P	252161-5	AIM Engineering & Surveying, Inc. Change Order No. 02-Robust through 02/06/26		2,762.75		109,466.25
2/27/2026	Actual	A/P	252111-7	AIM Engineering & Surveying, Inc. Change Order No. 01-Robust through 02/27/26		10,737.50		120,203.75
2/27/2026	Actual	A/P	252161-6	AIM Engineering & Surveying, Inc. Change Order No. 02-Robust through 02/27/26		3,341.00		123,544.75
0306-4072-538-3130 Robust Parcel- Engineering Total					0.00	123,544.75	0.00	123,544.75
Report Total					**0.00**	**224,153.95**	**0.00**	**224,153.95**

LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
Lee & Hendry County Tax Collections (Cash Basis)
Current Year
Cumulative Tax Revenues: % of Budget and Gross Dollars
FY 2022 - FY 2026

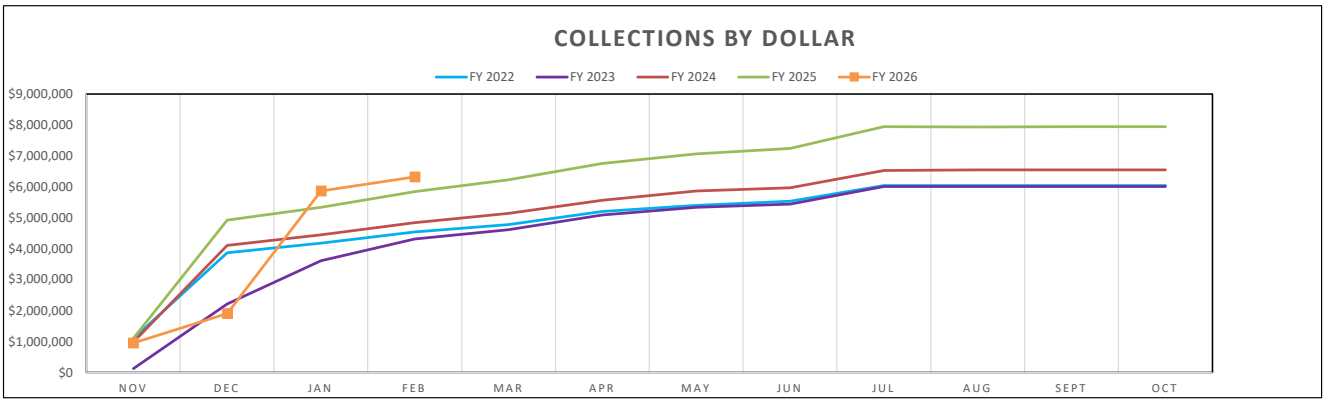
		Collections												Collections
FY	Budget	Nov	Dec	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sept	Oct	thru October
2022	5,854,755	1,094,645	2,759,047	305,803	359,295	237,789	408,249	181,345	139,853	487,719	43	25	-	5,973,813
		1,094,645	3,853,692	4,159,495	4,518,789	4,756,578	5,164,827	5,346,172	5,486,026	5,973,745	5,973,787	5,973,813	5,973,813	
		18.70%	65.82%	71.04%	77.18%	81.24%	88.22%	91.31%	93.70%	102.03%	102.03%	102.03%	102.03%	
2023	5,667,887	129,143	2,077,479	1,401,531	706,740	300,612	472,189	236,197	103,049	550,882	175	242	-	5,978,239
		129,143	2,206,622	3,608,153	4,314,893	4,615,505	5,087,695	5,323,891	5,426,940	5,977,822	5,977,996	5,978,239	5,978,239	
		2.28%	38.93%	63.66%	76.13%	81.43%	89.76%	93.93%	95.75%	105.47%	105.47%	105.48%	105.48%	
2024	6,327,476	987,239	3,120,390	338,282	396,529	292,731	425,667	290,528	99,949	549,841	11,710	4,297	-	6,517,164
		987,239	4,107,629	4,445,911	4,842,441	5,135,172	5,560,838	5,851,367	5,951,316	6,501,157	6,512,867	6,517,164	6,517,164	
		15.60%	64.92%	70.26%	76.53%	81.16%	87.88%	92.48%	94.06%	102.74%	102.93%	103.00%	103.00%	
2025	7,806,113	1,112,576	3,820,836	412,070	507,489	382,351	522,119	307,476	167,258	687,948	1,266	(352)	1	7,921,035
		1,112,576	4,933,412	5,345,481	5,852,970	6,235,321	6,757,440	7,064,915	7,232,173	7,920,121	7,921,387	7,921,034	7,921,035	
		14.25%	63.20%	68.48%	74.98%	79.88%	86.57%	90.50%	92.65%	101.46%	101.48%	101.47%	101.47%	
2026	7,783,619	951,417	3,957,059	511,783	449,849	-	-	-	-	-	-	-	-	5,870,108
		951,417	4,908,476	5,420,259	5,870,108	5,870,108	5,870,108	5,870,108	5,870,108	5,870,108	5,870,108	5,870,108	5,870,108	
		12.22%	63.06%	69.64%	75.42%	75.42%	75.42%	75.42%	75.42%	75.42%	75.42%	75.42%	75.42%	

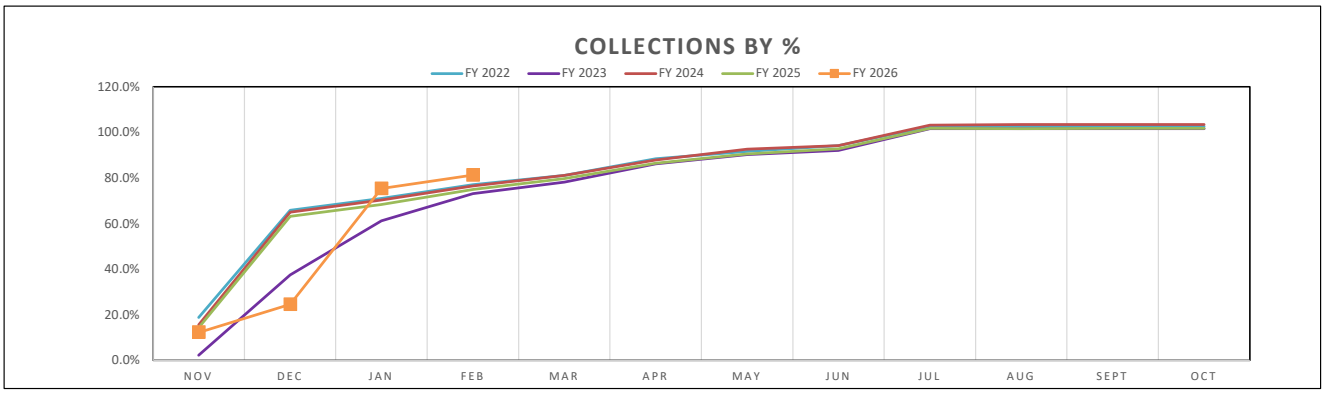




LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
Lee & Hendry County Tax Collections (Cash Basis)
Current Year, Prior Years, Interest and Penalties
Cumulative Tax Revenues: % of Budget and Gross Dollars
FY 2022 - FY 2026

FY	Budget	Collections												Collections thru October
		Nov	Dec	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sept	Oct	
2022	5,887,755	1,104,673	2,765,163	306,258	359,308	239,686	427,753	191,869	143,316	503,054	564	544		6,042,187
		1,104,673	3,869,836	4,176,094	4,535,402	4,775,088	5,202,841	5,394,710	5,538,025	6,041,079	6,041,643	6,042,187	6,042,187	
		18.76%	65.73%	70.93%	77.03%	81.10%	88.37%	91.63%	94.06%	102.60%	102.61%	102.62%	102.62%	
2023	5,912,503	131,731	2,077,393	1,401,576	706,743	300,374	472,453	242,861	105,645	568,046	175	3,543		6,010,541
		131,731	2,209,124	3,610,699	4,317,442	4,617,817	5,090,270	5,333,131	5,438,776	6,006,823	6,006,997	6,010,541	6,010,541	
		2.23%	37.36%	61.07%	73.02%	78.10%	86.09%	90.20%	91.99%	101.60%	101.60%	101.66%	101.66%	
2024	6,332,238	987,757	3,120,454	338,282	396,495	292,731	426,047	298,315	102,380	567,027	12,694	4,493	-	6,546,675
		987,757	4,108,211	4,446,494	4,842,989	5,135,720	5,561,767	5,860,082	5,962,462	6,529,489	6,542,182	6,546,675	6,546,675	
		15.60%	64.88%	70.22%	76.48%	81.10%	87.83%	92.54%	94.16%	103.12%	103.32%	103.39%	103.39%	
2025	7,809,313	1,112,594	3,811,371	412,137	507,415	382,684	522,119	315,593	171,259	703,033	1,342	(266)	106	7,939,388
		1,112,594	4,923,965	5,336,102	5,843,517	6,226,201	6,748,320	7,063,913	7,235,172	7,938,205	7,939,547	7,939,281	7,939,388	
		14.25%	63.05%	68.33%	74.83%	79.73%	86.41%	90.45%	92.65%	101.65%	101.67%	101.66%	101.67%	
2026	7,783,619	951,621	951,417	3,957,144	459,505		-	-	-	-	-	-	-	6,319,687
		951,621	1,903,038	5,860,182	6,319,687	6,319,687	6,319,687	6,319,687	6,319,687	6,319,687	6,319,687	6,319,687	6,319,687	
		12.23%	24.45%	75.29%	81.19%	81.19%	81.19%	81.19%	81.19%	81.19%	81.19%	81.19%	81.19%	





LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT
Total Revenue Collections (Cash Basis)
Cumulative Tax Revenues: % of Budget and Gross Dollars
FY 2022 - FY 2026

FY 2022 - FY 2026

FY	Budget	Collections												Collections thru October
		Nov	Dec	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sept	Oct	
2022	7,048,300	1,107,277	2,766,858	315,487	623,323	264,112	437,565	207,181	162,393	525,359	22,274	22,912	50,204	6,504,945
		1,107,277	3,874,135	4,189,622	4,812,945	5,077,057	5,514,622	5,721,803	5,884,196	6,409,554	6,431,829	6,454,741	6,504,945	
		15.7%	55.0%	59.4%	68.3%	72.0%	78.2%	81.2%	83.5%	90.9%	91.3%	91.6%	92.3%	
2023	6,884,353	163,433	2,106,138	1,467,362	743,902	352,979	537,627	287,879	156,899	607,497	104,918	15,458	59,785	6,603,878
		163,433	2,269,571	3,736,933	4,480,836	4,833,814	5,371,441	5,659,320	5,816,219	6,423,717	6,528,635	6,544,093	6,603,878	
		2.4%	33.0%	54.3%	65.1%	70.2%	78.0%	82.2%	84.5%	93.3%	94.8%	95.1%	95.9%	
2024	6,517,852	1,008,710	3,124,375	352,205	401,051	322,867	2,194,816	1,235,869	144,994	574,827	14,194	23,856	-	9,397,765
		1,008,710	4,133,085	4,485,290	4,886,341	5,209,208	7,404,025	8,639,894	8,784,888	9,359,714	9,373,908	9,397,765	9,397,765	
		15.5%	63.4%	68.8%	75.0%	79.9%	113.6%	132.6%	134.8%	143.6%	143.8%	144.2%	144.2%	
2025	7,956,913	1,140,414	3,924,840	412,137	578,421	460,078	550,767	324,306	180,832	731,561	54,008	3,394	53,247	8,414,003
		1,140,414	5,065,254	5,477,391	6,055,812	6,515,890	7,066,656	7,390,962	7,571,794	8,303,355	8,357,363	8,360,757	8,414,003	
		14.3%	63.7%	68.8%	76.1%	81.9%	88.8%	92.9%	95.2%	104.4%	105.0%	105.1%	105.7%	
2026	7,783,619	1,021,870	4,017,071	560,507	532,871	-	-	-	-	-	-	-	-	6,132,319
		1,021,870	5,038,940	5,599,448	6,132,319	6,132,319	6,132,319	6,132,319	6,132,319	6,132,319	6,132,319	6,132,319	6,132,319	
		13.1%	64.7%	71.9%	78.8%	78.8%	78.8%	78.8%	78.8%	78.8%	78.8%	78.8%	78.8%	

